

LANDSLIDES IN THE CHARLIE LAKE MAP SHEET,
FORT ST. JOHN

by

JORDAN M. SEVERIN

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JORDAN SEVERIN

Name of Author (please print)

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Abstract

Many large, deep-seated landslides have occurred along the Peace River Valley and its steep-sided tributary valleys. These failures mostly occur within the Lower Cretaceous Shaftesbury Formation, a horizontally-bedded marine shale and the interglacial silt and clay beds in the Quaternary sequence.

A detailed inventory of landslides was compiled in the Fort St. John area, using air photo interpretation and engineering-geological field mapping. The study area is on NTS Map Sheet 94A, comprising 14,000 km² of terrain. Practically all slope activity in the study area occurs on slopes of stream valleys. The observed landslides were separated into a series of failure types, involving bedrock, Quaternary deposits or both. The estimated areas of individual slide units range up to approximately 10,000 m² and volumes to 10,000,000 m³. The result of the inventory is a digital landslide map at a 1:20,000 scale and a corresponding database of landslide characteristics.

The main focus of the typological classification is the failure mechanism. The classification was reinforced by more detailed field study of selected type cases. There is a prevalence of multi-level failures, approximately 48%, utilizing weak surfaces at multiple levels. This type of failure occurs extensively both in the Quaternary sequence and the Cretaceous bedrock.

An activity classification was implemented based on field observations and dated air photographs. The classification comprises landslides that are very active, active, of low activity, ancient, and anthropogenically modified. Results indicate that approximately 90% of the total length of valley slopes in the study area are involved in mass movements. Approximately 15% are occupied by very active or active landslides and 61.0% are Inactive - Dormant.

Four areas were chosen to be studied in detail due to their accessibility from Fort St. John, diverse failure mechanisms, and diverse local stratigraphies. A generalized stratigraphic column and landslide model was produced for each area. These were generated from the detailed landslide investigations done.

TABLE OF CONTENTS

ABSTRACT.....	ii
LIST OF FIGURES	vi
LIST OF TABLES.....	ix
LIST OF APPENDICES	x
ACKNOWLEDGEMENTS.....	xi
1.0 Introduction.....	1
1.1 Objectives	2
1.2 Area Introduction.....	3
2.0 Literature Review	5
2.1 Landslides in Shale	5
2.1.1 Falls.....	5
2.1.2 Topples.....	6
2.1.3 Rotational slides (Slumps)	7
2.1.4 Translational Slides.....	9
2.1.5 Compound Slides.....	10
2.2 Landslides in Glacio-lacustrine Clay	12
2.2.1 Rotational Failures	12
2.2.2 Translational Failures.....	13
2.2.3 Compound Slides.....	15
2.2.4 Sudden Spreading Failures	15
2.2.5 EarthFlows	16
2.2.6 Mudflows	17
2.2.7 Flow Slides	17
2.3 Examples of Landslides from the Canadian Prairies	19
2.3.1 Shale.....	19
2.3.2 Glacio-lacustrine Clay	22
3.0 Regional Setting	25
3.1 Physiography	25
3.2 Climate.....	26
3.3 Quaternary History	27
3.4 Local and Regional Infrastructure	29
4.0 Bedrock Geology and Géotechnique.....	30
4.1 Outcrop Location	30
4.2 Description of Formations	32
4.4 Geotechnical Properties	37
4.4.1 Shaftesbury Shale	37
4.4.2 Dunvegan Sandstone.....	43
4.5 Comparison with Other Prairie Shales.....	44
5.0 Surficial Geology and Geotechnique.....	46

5.1 Location	46
5.2 Late-Quaternary Materials	47
5.2.1 Gravels	47
5.2.2 Pre-glacial Silt, Sand, and Clay	48
5.2.3 Late Wisconsinan Till	49
5.2.4 Glacial Lake Peace Clay	49
5.3 Geotechnical Properties	50
5.3.1 Bulk Properties	51
6.0 Landslide Inventory Mapping System	52
6.1 Slide Types in the Study Area	52
Slide Type 1: Raveling	53
Slide Type 2: Retrogressed Rotational Failure	54
Slide Type 3: Rotational Failure	55
Slide Type 4: Mobile Flow	56
Slide Type 5: Earth Flow	56
Slide Type 6: Multi-level Failures	57
Slide Type 7: Compound Failure	58
Slide Type 8: Shallow Retrogressive Failure	58
6.2 Activity Classifications	63
6.3 Morphological Symbols for the Inventory Map	70
7.0 Landslide Inventory	75
7.1 Objective	75
7.2 Referencing System	75
7.3 Methodology	80
7.4 Database	82
7.5 Results	85
7.5.1 Distribution	85
7.5.2 Activity	87
7.5.3 Failure Type	90
7.5.4 Material Involved	93
7.5.5 Slide Aspect	95
8.0 Detailed Investigation of Selected Landslides Areas	98
8.1 Slide Area 1	100
8.1.1 Location	100
8.1.2 Local Stratigraphy	100
8.1.3 Landslides	104
8.1.4 Area Model	117
8.2 Slide Area 2	119
8.2.1 Location	119
8.2.2 Local Stratigraphy	121
8.2.3 Landslides	122
8.2.4 Area Model	142
8.3 Slide Area 3	146

8.3.1 Location	146
8.3.2 Local Stratigraphy.....	148
8.3.3 Landslides	149
8.3.4 Area Model	179
8.4 Slide Area 4	186
8.4.1 Location	186
8.4.2 Local Stratigraphy.....	186
8.4.3 Landslides	189
9.0 Discussion	200
9.1 Landslide Mechanisms and Distribution	200
9.2 Generalized Geologic Model	203
10.0 Conclusions.....	205
References.....	209

LIST OF FIGURES

Figure 1.1	Site Location Map.....	4
Figure 2.1	Rotational Slide Failure Modes	8
Figure 2.2	Generalized cross-section, Brilliant Cut	9
Figure 2.3	Compound Slide Modes.....	11
Figure 2.4	Translational Failure Types	14
Figure 2.5	Stratigraphic profile of the Little Smoky landslide	21
Figure 2.6	Stratigraphy of the WID landslide	21
Figure 2.7	Cross section after 1939 Montagneuse landslide.....	24
Figure 2.8	Section through Rycroft landslide	24
Figure 2.9	Stratigraphy of the Attachie landslide.....	24
Figure 3.1	Quaternary Valley Fill Materials Location	28
Figure 4.1	Bedrock Geology Map.....	31
Figure 4.2.	Dunvegan Sandstone within the Beatton River Canyon.....	36
Figure 4.3	Valley Rebound with resultant deformation	38
Figure 4.4	Site C Damsite Schmettic with discontinuities	39
Figure 4.5	Typical sampling area of sheared shale and gouge.....	41
Figure 4.6	Tested Sample Location Map	42
Figure 5.1	Plasticity Chart (Unified System) – Peace River Samples	51
Figure 6.1	Slide Type 1: Raveling	59
Figure 6.2	Slide Type 2: Retrogressive Rotational Failure	59
Figure 6.3	Slide Type 3: Rotational Failure.....	60
Figure 6.4	Slide Type 4: Mobile Flow	60
Figure 6.5	Slide Type 5: Earth Flow	61
Figure 6.6	Slide Type 6: Multi-level Failure.....	61
Figure 6.7	Slide Type 7: Compound Failure.....	62
Figure 6.8	Slide Type 8: Shallow Retrogressive Failure.....	62
Figure 6.9	Very Active Landslide (Photo)	68
Figure 6.10	Active Landslide (photo)	68
Figure 6.11	Low Activity – Dormant Slide (Photo).....	69
Figure 6.12	Low Activity – Abandoned Slides (Photo).....	69
Figure 6.13	Modified Slide (Photo)	69
Figure 6.14	Morphological Classification.....	73
Figure 6.15	Landslide Type Examples using Morphological Classification	74
Figure 7.1	Reference Areas for Landslide Inventory	76
Figure 7.2	Landslide Outlines on Enlarged Photographs.....	84
Figure 7.3	Landslide Outlines on TRIM Map.....	84
Figure 7.4	Digitized Landslide Outline.....	84
Figure 8.1	Detailed Landslide Study Areas.....	99
Figure 8.2	Landslide Area 1	101
Figure 8.3	Area 1 – Stratigraphic Profile (not to scale)	103
Figure 8.4	Intact Pre-glacial Stratigraphy (Slide 1-1)	105
Figure 8.5	Former Big Bam Ski Hill Tower Line (Slide 1-2).....	106
Figure 8.6	Bison Vertebrae found in place (Slide 1-6)	110
Figure 8.7	Landslide Profile 1-1	111

Figure 8.8	Landslide Profile 1-2	112
Figure 8.9	Landslide Profile 1-3	113
Figure 8.10	Landslide Profile 1-4	114
Figure 8.11	Landslide Profile 1-5	115
Figure 8.12	Landslide Profile 1-6	116
Figure 8.13	Landslide Area 1 - Geologic Model.....	118
Figure 8.14	Landslide Area 2	120
Figure 8.15	Area 2 – Stratigraphic Profile (not to scale)	122
Figure 8.16	Shear Plane found and sampled (Slide 2-4).....	125
Figure 8.17	Landslide Profile 2-1	131
Figure 8.18	Landslide Profile 2-2	132
Figure 8.19	Landslide Profile 2-3	133
Figure 8.20	Landslide Profile 2-4	134
Figure 8.21	Landslide Profile 2-5	135
Figure 8.22	Landslide Profile 2-6	136
Figure 8.23	Landslide Profile 2-7	137
Figure 8.24	Landslide Profile 2-8	138
Figure 8.25	Landslide Profile 2-9	139
Figure 8.26	Landslide Profile 2-10	140
Figure 8.27	Landslide Profile 2-11	141
Figure 8.28	Landslide Area 2 – Geologic Model.....	145
Figure 8.29	Landslide Area 3	147
Figure 8.30	Area 3 – Stratigraphic Profile – North and East Valley Walls	150
Figure 8.31	Area 3 – Stratigraphic Profile – South Valley Wall	150
Figure 8.32	Area 3 – Stratigraphic Profile – Stoddart & St. John Creeks.....	151
Figure 8.33	Cross-Tilting of trees on Landslide Profile 3-4	154
Figure 8.34	Landslide Profile 3-1	165
Figure 8.35	Landslide Profile 3-2	166
Figure 8.36	Landslide Profile 3-3	167
Figure 8.37	Landslide Profile 3-4	168
Figure 8.38	Landslide Profile 3-5	169
Figure 8.39	Landslide Profile 3-6	170
Figure 8.40	Landslide Profile 3-7	171
Figure 8.41	Landslide Profile 3-8	172
Figure 8.42	Landslide Profile 3-10	173
Figure 8.43	Landslide Profile 3-11	174
Figure 8.44	Landslide Profile 3-12	175
Figure 8.45	Landslide Profile 3-13	176
Figure 8.46	Landslide Profile 3-14	177
Figure 8.47	Landslide Profile 3-15	178
Figure 8.48	Landslide Area 3 – Model 1.....	184
Figure 8.49	Landslide Area 3 – Model 2.....	185
Figure 8.51	Deltaic Sequence.....	188
Figure 8.52	Area 4 – Stratigraphic Profile	189
Figure 8.53	Intact Section Photo (Slide 4-1).....	190
Figure 8.54	Complex Stratigraphy (Slide 4-4).....	193

Figure 8.55	Landslide Profile 4-1	194
Figure 8.56	Landslide Profile 4-2	195
Figure 8.57	Landslide Profile 4-3	196
Figure 8.58	Landslide Profile 4-4	197
Figure 8.59	Landslide Area 4 – Geologic Model.....	199

LIST OF TABLES

Table 4.1.	Summary of classification test results, shale and gouge.....	43
Table 4.2.	Geotechnical Properties of Prairie Shales.....	46
Table 5.1.	Summary of classification test results for surficial materials	52
Table 7.1	Measured Distance within Slope Segment Districts.....	78
Table 7.2.	River Identifies within Charlie Lake Map Sheet	80

LIST OF APPENDICES

Appendix I	Geotechnical Lab Results – Shale and Clay
Appendix II	Air Photograph List
Appendix III	Landslide Maps – Overall and Each TRIM
Appendix IV	Landslide Inventory
Appendix V	Inventory Analysis Results
Appendix VI	Rosette Diagrams

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1.0 INTRODUCTION

The Peace River Area of northeastern British Columbia and northwestern Alberta is one of the most important oil and gas production districts in Canada. The vast amount of hydrocarbon extraction has created the need for an extensive system of pipelines, railways, and roads to be built that traverse the unstable riverbanks of the region. Many large, deep-seated landslides have occurred within the Peace River Valley and its steep-sided tributary valleys. A better understanding of landslides in the region, type, activity, and failure mechanisms will be beneficial to the management of current and future regional infrastructure. This thesis summarizes a research project devoted to the investigation, mapping, and classification of landslides in the Fort St. John area, which is the centre of the present petrochemical extraction activity. The project was a component in a wider study of landslides in the region conducted by the Geological Survey of Canada and headed by Dr. L. Dyke.

The landslides mostly occur within the Lower Cretaceous Shaftesbury Formation, a horizontally bedded marine shale and the pre-glacial silt and clay beds within the Quaternary sequence. Many landslides occur near critical pipeline crossings and several failures have resulted in pipeline rupture. In other cases, wellheads are threatened by head-ward retrogression of landslides.

Landslides in Cretaceous bedrock involve slow sliding along geological defects in horizontal strata. These defects are a result of primary deposition and valley rebound (Matheson and Thomson, 1973).

Landslides in the fine-grained Quaternary sediments may be rapid catastrophic flowslides, as at Attachie and Flatrock, or more commonly, slow moving slides. Failure is

related to the existence of highly plastic horizons, glaciotectonic shears, valley rebound, and syn-erosional slumping.

Pioneering observations on Peace River landslides were made by Mathews (1963, 1978) with further work being undertaken by Evans et al. (1996), Bobrowsky et al. (1991) and Fletcher et al. (1998, 2002). The relationship between pipeline performance and moving slopes in the area has been illustrated by McClarty and Cavers (1998).

Geotechnical aspects of the Cretaceous bedrock in the Peace River have been reported by B.C. Hydro in the geotechnical engineering literature (Hannah and Little, 1992; Imrie, 1991; Cornish and Moore, 1985; Sargent and Cornish, 1985; Maber and Stewart, 1976) and a number of unpublished reports. The failure of the Peace River Bridge at Taylor has been reported by Hardy (1963), Hardy et al. (1962), and Brooker and Peck (1993).

1.1 Objectives

The primary objective of this research was to establish the relations between landslides and geologic conditions in the northeastern B.C. region and to produce a landslide inventory map and database for the Charlie Lake Map sheet (NTS 94A). This was done using air photo interpretation; previously written geotechnical reports; and site-selected fieldwork. This map is to be produced in digital form with locations of landslides denoted by symbols distinguishing bedrock or surficial slides. Although landslides are very wide spread in the region, very little has been published on the relationship between the geology and geotechnics of landslides in the region and no comprehensive or systematic study of the distribution and behaviour of landslides in the Peace area has

been undertaken. The inventory will also be used to analyse landslide activity after 1948 and in prehistoric time for selected focus areas.

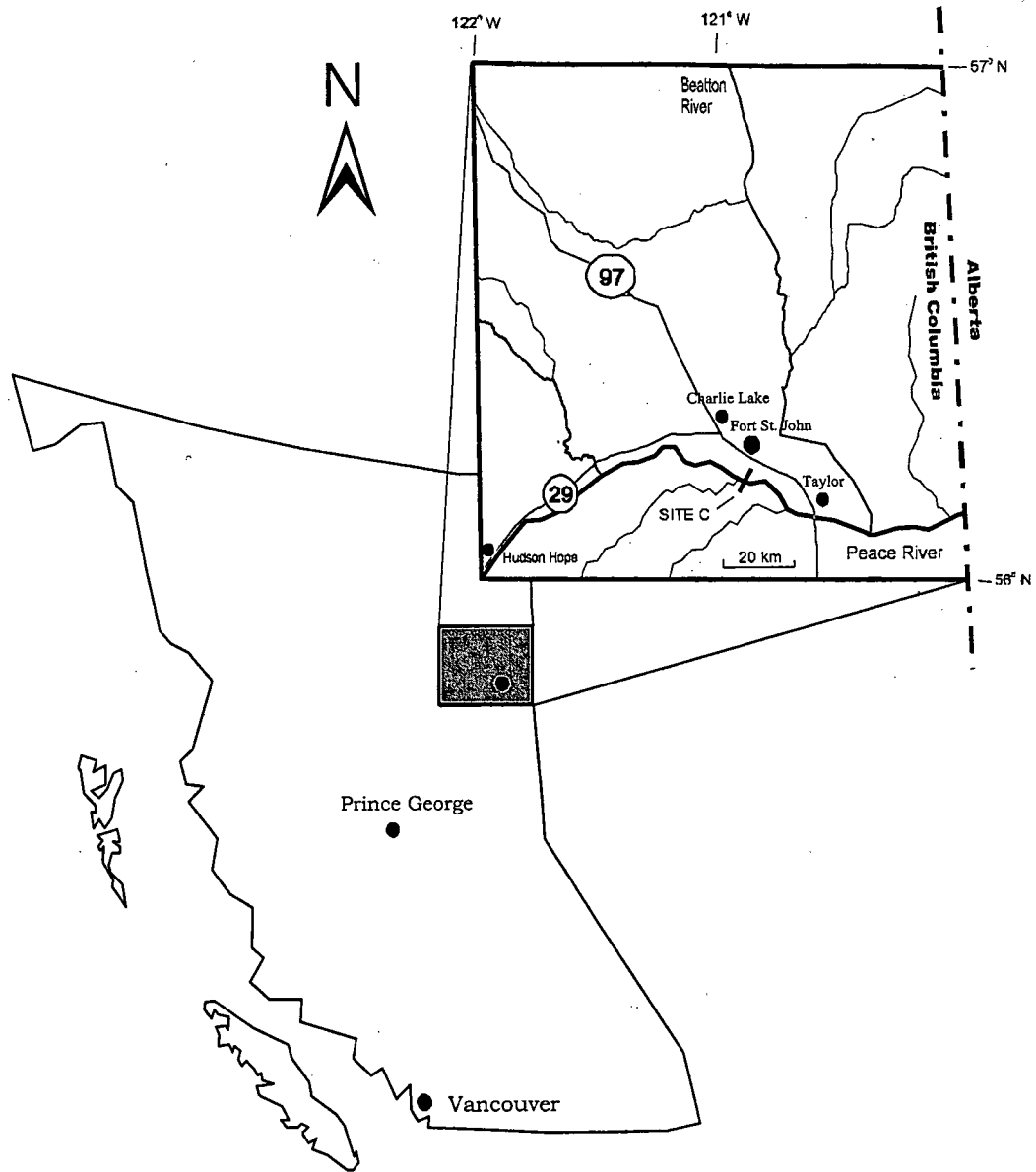
A further objective was to use selected fieldwork to characterize the materials and landslides in the area in order to determine typical failure mechanisms. Investigation of selected slides included detailed mapping, regional landslide models, and materials testing. Fieldwork was necessary to develop type landslide cases in the two main units, find marker horizons, and to form a better understanding of the area.

Using the landslide inventory and site descriptions, the final result is a synthesis of a geological-geotechnical model of slope behaviour. This will hopefully provide the oil and gas transmission industry with a geotechnical framework for understanding the behaviour of slopes in the study area.

1.2 Area Introduction

The study area is defined by the NTS map sheet 94A - Charlie Lake, which encompasses the city of Fort St. John as well as the towns of Hudson Hope, Taylor, and Charlie Lake (Figure 1.1). The study area boundaries are defined by the Alberta boundary to the east (120° W), the town of Hudson Hope to the west (122° W), and by the 56° N and 57° N parallels of latitude.

Figure 1.1 Site Location Map



2.0 LITERATURE REVIEW

2.1 Landslides in Shale

The body of literature available on landslides in shale is quite extensive, as they represent a unique problem in the field of engineering geology and geological engineering around the world. The Prairie shales of western North America have required a broad amount of investigation and research on the subject (Mollard, 1977; Thomson and Morgenstern, 1978, Locker, 1969; Scott and Brooker, 1968; Sauer et al., 1990; Cruden, 1996).

The following types of mass movements have been noted as important in shale failures within the Peace River Area; 1) Falls, 2) Topples, 3) Rotational Failures, 4) Translational Failures, 5) Compound Failures, and 6) Block Slides. A brief description of each and case histories are provided.

2.1.1 Falls

Rock falls are rapid to extremely rapid (Varnes, 1978) free descents of masses of rock. This type of failure occurs typically at a small scale in shale; however, a few large-scale cases are known (Gray et al, 1978). Few reports of shale falls are documented, as shales do not tend to support large cliffs, therefore, shale falls are generally thought of as insignificant owing to the small volumes involved and small hazards that they present. A shale fall originates from a steep slope; the mass then descends down the slope by a combination of free fall, rolling, and bouncing depending on the angle of the original slope.

Minor falls are commonly observed in natural slopes and excavations. Shale slopes are usually at a shallow enough angle to allow for detritus to accumulate instead of free falling. Vertical or near vertical slopes in shale do commonly occur along river valleys, especially in the Peace River area, however these generally allow the rock to fall directly into the river or creek. Slopes of shallower angles will accumulate colluvial aprons because of the low impact resistance of shale. Pieces of shale are usually tabular and break on contact with a harder surface. The disaggregation of shale releases much of the kinetic energy accumulated during the fall and the smaller pieces will not roll as far as a stronger material. The high plasticity of shale allows small clay flakes or blocks to break down quickly to the properties of clay. A clay talus cone will absorb much of the energy of falling rock instead of allowing the rock to bounce or roll down the slope.

Gray et al (1978) produce an example of shale failure near Ambridge, Pennsylvania. A 15m high nearly vertical cut was made of soft shale, claystone, and sandy silt shale. In this case the shale was compact enough to fall as a single 115-m³ block, which unfortunately killed 22 persons.

2.1.2 Topples

A shale topple typically involves detachment of a slab of material, defined by near-vertical fissures, followed by forward rotation out of the slope. This rotation takes place about a point or axis below the center of gravity of the displaced mass when an external force is applied, or when softening and ductile deformation of the base causes slight forward rotation of the mass. The motive force is usually supplied by the block's weight or by water or ice pressure exerted in a previously developed tension crack.

Shale can be susceptible to simple or flexural toppling if the bedding planes have been displaced. On a small scale, block toppling can occur although in horizontally bedded shale, however, is rare as the discontinuities are generally horizontal along the bedding plane, not vertical. If the shale has gone through advanced diagenesis the shale may contain fissures or vertical joints, however, a majority of horizontally bedded clay shales are poorly indurated.

2.1.3 Rotational slides (Slumps)

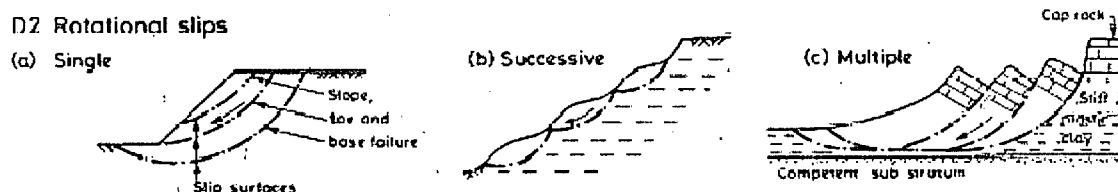
Rotational slides are those in which the movement of a mass is roughly rotational about an axis running parallel to the slope crest (Varnes, 1978). The term 'roughly' is used to allow for a condition where the basal shear surface is partly developed along a horizontal weak layer, which causes truncation of the normally curved or listric shear surface. The weak horizontal basal layer has to play a minor role so that the movement can still be considered rotational and little internal deformation of the sliding body occurs. A rotational movement will show downward displacement of the head of the mass, which commonly tilts backward toward the slope, and upward displacement at the toe. Rotational failures in shale occur on fresh steep scarps, as seen in highway road cuts or eroded riverbanks, or when there is a resistant cap rock. This is because the long-term equilibrium state of shale is at angles too shallow to form rotational failures.

Rotational failures occur mostly in deep, homogenous formations, i.e. London Clay (Hutchinson, 1988). In the Western Canadian Prairies where poorly indurated shales are predominant, rotational failures are rare. Poorly indurated shales, or clay shales, are highly anisotropic having weak-bedding planes that modify the rotational movement.

This type of failure may occur if it is small enough that it can take place between weak layers.

Hutchinson (1988) identified three modes of rotational slides: single, successive, and multiple rotational slides (Figure 2.1). Single rotational slides contain a single concave slip surface and the moving mass acts as a coherent unit. Successive rotational slides refer to many shallow rotational slips that are aligned head to toe up the failing slope. Often this type of failure will produce regular step-like terraces, which can easily be reactivated. Each of the failures can be described as a shallow to moderately deep-seated rotational slip. Multiple rotational slides occur from the retrogression of a single rotational slip with a curved, generally deep-seated failure surface. The retrogression of the slide forms two or more slipped blocks with slip surfaces tangential to a common slip base.

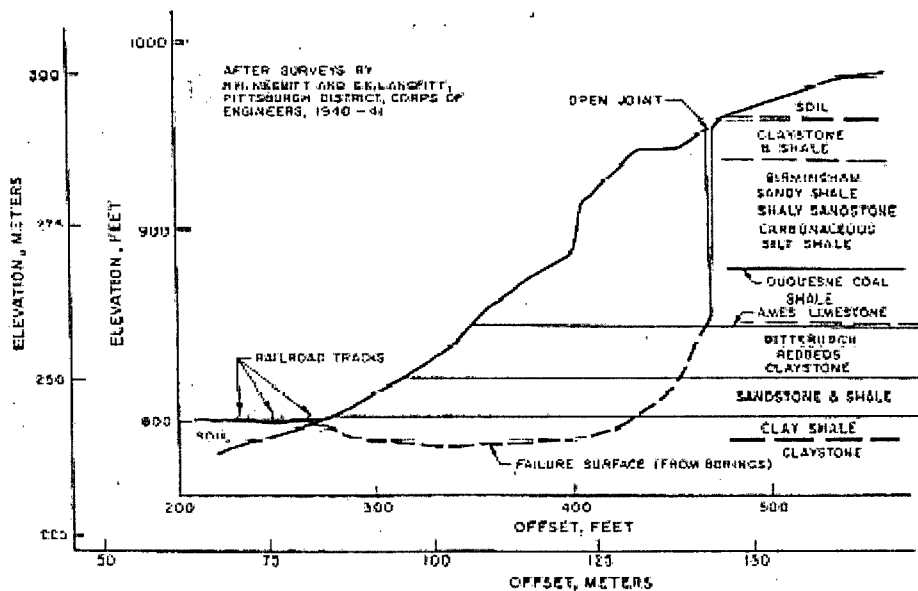
Figure 2.1 Rotational Slide Failure Modes (Hutchinson, 1988).



Gray et al (1978) presented a case history in which a rotational failure did occur within shale. The Brilliant Cut failure in the northern Appalachian Plateau occurred within a sequence of interbedded shale, sandstone, and limestone. A railway cut was made at 45° to a height of 50 metres in the early 1930's and within 10 years the slope was severely unstable. Over 70% of the failure surface was located within the soft clay shale

(Figure 2.2). The movement was marked with a rapid drop in the head of the slide, 5m within 90 seconds and was triggered by elevated pore pressures within the slope. The slide had a total volume of roughly $160,000 \text{ m}^3$.

Figure 2.2 Generalized cross-section, Brilliant Cut (Gray et al., 1978)



An open pit mine in Wyoming (Clough et al, 1978) displayed several rotational slides, which took place during excavation. Two of these rotational failures had volumes of 1.4M m^3 , and were largely confined to the shale strata.

2.1.4 Translational Slides

Translational slides are a common type of failure in bedded shales. The failed mass progresses along a more or less planar shear surface running sub-parallel to the bedding, with several well-defined blocks separated by oblique shears. Generally, the proximal segment of the rupture surface of the slide is a downward extension of the main

Extensional features and a low angle of movement dominate surface morphology. Horst and graben features within the debris mass may or may not be accompanied by back tilting. The upper blocks are the last to begin moving due to the removal of material near the toe. Other blocks may become detached, pushing material forward to a river or another erosional agent. If unimpeded, the displaced mass will run parallel to the slope, starting out as a coherent unit, but as sliding continues, the mass may become more distorted. The entire shear plane is active within a translational failure and the shear plane must be at a sufficient angle to horizontal to allow for movement in the downslope direction.

Within the Western Canadian Prairies, such failures can occur in Upper Cretaceous marine clay shales and develop retrogressively (Mollard, 1977) providing that the bedding is at a sufficient angle to allow movement. Failures may occur along bentonitic-rich layers within the shale, along weaker layers caused by softening, along interbed slip planes caused by the lateral release of valley rebound or glaciotectonic stresses, or release of residual tectonic forces caused by the mountain building processes to the west.

Examples of this type of failure are found in Section 2.3 Examples of Landslides from the Canadian Prairies.

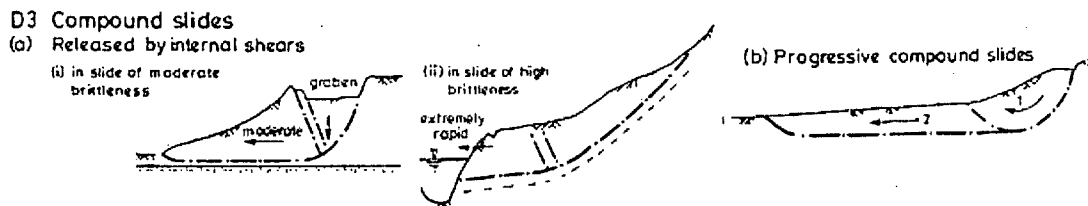
2.1.5 Compound Slides

Compound slides are a form of translational slide in which the head of the slide mass may rotate backward due to a curved proximal segment or subside to form a graben. They differ from translational slides in that sections along the shear plane are passive and

They differ from translational slides in that sections along the shear plane are passive and are simply pushed downslope by the upslope action. The failure plane typically occurs at a much lower angle than required for translational failures. These are a very common type of landslide in horizontally bedded shales within the Canadian Prairies. The failure plane is slightly deeper than the typical translational failure and the debris is normally extremely distorted. Surface morphology of this type of landslide will have both horst and graben type features, possibly combined with back tilting near the head scarp. These failures can also occur at very low angles, 5° to 11° .

Hutchinson (1988) classified compound slides into two categories: progressive compound slides and compound slides released by internal shearing (Figure 2.3). Progressive compound slides are those in which there is a rotational failure at the rear of the slide and the remaining basal slip surface is planar. Compound slides released by internal shearing are those that have a weak basal surface and have undergone internal shearing, to accommodate curvature at the intersection of the basal surface and the back scarp. Sliding will occur once the cohesion within the mass is removed.

Figure 2.3 Compound Slide Modes (Hutchinson, 1988)



Examples of this type of failure can also be found in section 2.3 Examples of Landslides from the Canadian Prairies.

2.2 Landslides in Glacio-lacustrine Clay

Landslides in clay and silt have been investigated more than any other material within the body of landslide literature (Hardy et al, 1962; Skempton, 1964; Skempton and Hutchinson, 1969; Varnes, 1978; Hutchinson, 1988; and Cruden and Varnes, 1996;).

Glacio-lacustrine clay has unique properties, which have to be considered in slope stability analysis (Townsend et al, 1965; Kenney, 1975; Evans, 1982; and Giraud et al, 1991). Evans (1982) suggests that within British Columbia, two distinct types of glacio-lacustrine clay are present. Type A (Southern Interior) has more silt than clay layers and is more likely to reflect the properties of the silt. Type B (Northern Interior) has a greater thickness of clay than silt and the properties are thought to reflect the properties of clay. Townsend et al. (1965) quantified the importance of material grain size within glacial varves with laboratory experiments.

The following types of mass movement have been noted as important within glacio-lacustrine materials in the Peace River area; 1) Rotational Slides, 2) Translational Slides, 3) Compound Slides, 4) Sudden Spreading Failures, 5) Earthflows, 6) Flowslides, 7) Mudflows, and 8) Complex Slope Movements. A brief description of each and case histories are provided.

2.2.1 Rotational Failures

Rotational failures in clay show the same mechanisms as rotational slides in shale, although they are more common in these deposits than in shale. Rupture takes place along an identifiable plane that approximates a listric or cylindrical surface. Again we see a degree of backward rotation to the slipped mass accompanied by sinking at the head of

the slide and heaving at the toe. Hutchinson (1988) identifies three modes of rotational slides; single, successive, and multiple rotational slides, as described earlier in Section 2.13 (Figure 2.1).

Glaciolacustrine clay is normally varved, which imparts an anisotropic nature to the material, rendering true rotational failures rare, though shearing across laminae can occur in some glaciolacustrine clays (Giraud, et al., 1991). Slides in varved clay deposits may also be approximately circular providing that the pore water pressure in the silt layers is low (Terzaghi and Peck, 1948).

Retrogression of a single rotational slip is more common in these deposits than in shale. An example of retrogression of a rotational failure has been documented near Quesnel, B.C. (Evans, 1982). An active slope movement is taking place in a 150 m thick sequence of varved Quaternary glacio-lacustrine clays and silt. Retrogression at the main scarp takes place by multiple rotational slumping and produces backward tilted slump blocks, which move down the slope and undergo progressive disintegration and remolding.

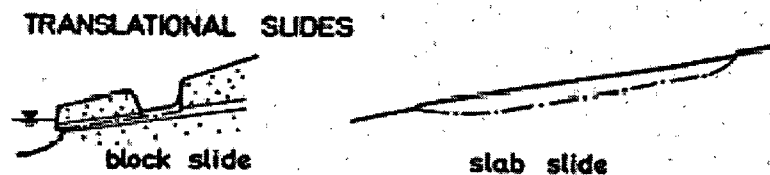
2.2.2 Translational Failures

This is a common type of failure in glacio-lacustrine clay. The clay body needs an exceptionally weak layer at sufficient angle to allow movement to fail along without disturbing most of the overlying debris. Within glaciolacustrine clay, highly plastic varves can serve as the weak layer. In some cases, translational failure can occur due to liquefaction of the basal slip surface. This is normally seen in quick clays in Eastern Canada and Norway, which are glacio-marine clays. Skempton and Hutchinson (1969)

Canada and Norway, which are glacio-marine clays. Skempton and Hutchinson (1969) note that a shallower depth to the weak layer increases the amount of translational movement that the slide will experience.

Two types of translational slides identified by Skempton and Hutchinson (1969) that arise in clays are block slides and slab slides (Figure 2.4). A block slide arises when a hard, jointed block of clay disconnects from the parent mass at a joint or weakness. Then the clay block slides along the plane of weakness with the clay mass. Slab slides take place on more weathered clay slopes when the mantle or crust of a residual soil or sensitive clay is disturbed. The broken crust or slab moves along the slope, with little distortion, as a single unit. This implies that the weak surface is sloping at a steep enough angle to allow for movement.

Figure 2.4 Translational Failure Types (Skempton and Hutchinson, 1969)



A translational slide in glacio-lacustrine sediments occurred in 1977 near the town of McBride, British Columbia (Thomson and Mekechuk, 1982). The failure plane was located within a thick bed of varved clay and silt deposited during the retreat of Cordilleran ice. The clay was medium to high plasticity and dipped slightly toward the river.

2.2.3 Compound Slides

Compound slides are a far more common failure type in glaciolacustrine clay and silt than are translational failures. This is due to the low strength of clay and relative ease with which shear can occur through varves and the horizontal bedding. This allows for a steep or circular head scarp to form in situations in which they could not occur in shale. For the most part, these failures are the same as in shale. Examples of compound failures in glacio-lacustrine are given in Section 2.3 Examples of Landslides from the Canadian Prairies.

2.2.4 Sudden Spreading Failures

Clay spreading involves extension of a cohesive mass combined with a general subsidence into a softer underlying material (Cruden and Varnes, 1996) making it a category of translational failure (Hutchinson, 1988). It commonly displays horst and graben structures where lateral extension has occurred accommodated by shear or tensile fractures. Movement is very rapid even over low gradients (Hutchinson, 1988). This type of failure is most described in soft, sensitive clay; however, Terzaghi and Peck (1948) note that this style of movement can occur in varved clays having high pore water pressures developed in silt or sand layers. Varnes (1978) identifies two types of spreading failure. The first is due to an overall extension of material without a recognizable basal shear surface or zone of plastic flow. This type is predominantly a bedrock feature so will not be discussed here. The second type of failure, called earth lateral spreads, involves fracturing and extension of a coherent material, in this case, a solid clay mass, due to a

plastic or liquefied basal layer. The upper unit blocks may move by any mechanism, such as subsidence, translation, or rotation.

Skempton and Hutchinson (1969) described these failures as a particular type of retrogressive compound slide. Gentle slopes, broad fronts, rapidity of movements, and the succession of horst and graben structures characterized them. The retrogressive nature of the slide comes from the subsidence, which drives the displaced mass forward. As the mass subsides, the lateral support acting on the new back scarp is insufficient to hold it in place; thus a new wedge will subside pushing the previous wedge forward.

Mitchell (1978) reports lateral spreading in fresh water varved clays at Wawbewawa and at Beattie Mine, Quebec.

2.2.5 EarthFlows

Earth flows are defined as rapid or slower, intermittent flows of plastic clayey earth (Hungr et al 2001). They involve spatially continuous movement in which surfaces of shear are short lived, closely spaced, and usually not preserved (Cruden and Varnes, 1996), with a distinct boundary between the moving mass and the intact material. This type of failure generally arises from a large influx of water either through an increase of precipitation or by the melting of ice or frozen ground. They are relatively shallow failures that resemble the flow of a viscous fluid and depend on the water content, movement, and mobility of the mass. The disturbed mass is thoroughly broken up and keeps little of its previous structural integrity. Over-consolidated and varved or laminated clays are most susceptible to earth flows.

“Earth flows are commonly tongue or teardrop-like in shape. They have rounded, bulging toes and sinusoidal longitudinal profiles, concave upward near the head of the earth flow and convex upward near the toe” Keefer and Johnson (1983). The Thistle slide in Utah (Duncan et al., 1986) is an example of an earth flow involving silty, sandy, and gravelly clays of medium plasticity (average LL = 40, PI = 18). The landslide mass is roughly 1800 m long and an elevation difference of 305m from the crest to the toe of the slide debris.

2.2.6 Mudflows

Mudflows are very rapid to extremely rapid flows of saturated plastic debris in a channel, involving significantly greater water content relative to that of the source material (Hung, 2001). Rainstorms can sometimes cause clayey colluvium to become diluted beyond its Liquid Limit in desiccated, especially dispersive clays. The triggering hillslope failures are usually shallow seated (< 5m thick). In areas of low relief, clay flows are generally restricted to oversteepened slopes, such as gully sidewalls and undercut river bluff slopes.

2.2.7 Flow Slides

Hung et al (2001) describe flow slides as very rapid to extremely rapid movements of sorted fine-grained sediment, involving liquefaction. Clay flow slides occur in association with quick clays and form due to a collapse of an internal structure and the build up of high water pressures. If significant disintegration of the failed mass has occurred it may be difficult to identify the failure mechanism.

The material liquefies when the metastable structure of the clay collapses. The loss of strength gives the failing clay a semi-fluid character, which allows the flow to develop. The result is a high velocity movement that travels a long distance.

Mitchell and Markell (1974) completed an investigation of flow sliding in the sensitive clays of eastern Canada. Their studies conclude that undrained failure in sensitive clays will not occur if the stability number for the clay is greater than 6. The stability number is defined as:

$$N_s = \gamma H / C_u$$

where γ is the bulk unit weight of the soil, H is the slope height, and C_u is the undrained strength of the soil. In this study, three proposed phases of the development of flow slides were introduced. First, an initial slide is caused by the long-term instability of the slope. After the initial slope failure, the slope undergoes a retrogressive failure caused by the undrained short-term instability of the successive slices. The third phase is an earth flow involving the extrusion and flow of the soft underlying materials.

Hutchinson (1988) refers to this type of failure as a flow slide, while others (Varnes, 1978) often group it in with the above category. Skempton and Hutchinson (1969) add a section in their category of multiple and complex landslides, quick clay slides. Here they present a failure peculiar to quick clays, the 'bottle-neck' type of retrogressive, multiple rotational failures. It initially begins with a rotational slip, then the disturbed mass is remolded to a liquid, which runs off, leaving the near scarp unsupported. This new scarp is then attacked; it fails, and flows out like the previous. This continues until a new back scarp is supported by other means.

The Thompson River experienced a flow slide in 1880 that involved approximately 15 Mm^3 of normally consolidated Early Holocene or Late Pleistocene glaciolacustrine sediments (Evans, 1984). The sediments were characterized by varves and laminations within the debris that dammed the Thompson River for approximately 44 hours. The cause of the landslide is speculated to be due to the irrigation of the bench in the area of the failure (Stanton, 1898).

2.3 Examples of Landslides from the Canadian Prairies

As the Peace River area is located within the Interior Plains, the geotechnical literature from Alberta is more relevant than that from British Columbia. Both shale and glacio-lacustrine examples are given.

2.3.1 Shale

The Little Smoky landslide is an excellent example of a retrogressive compound rotational failure in shale (Thomson and Hayley, 1975). The slide affects a bridge over the Little Smoky River near the town of Valleyview, Alberta. The basal failure plane is nearly horizontal and occurs within clay-shale of the Lower Smoky Formation (Figure 2.5). In the Peace River Area this formation is known as the Kaskapau Formation, and crops out to the east of the Kiskatinaw River. The slide occurs within a pre-glacial valley plugged with till and associated glacial deposits. The failure is reported to move at a slow creep toward the river, with each slice acting as a separate entity.

The Lesueur landslide (Thompson, 1971 & Cruden et al., 1998) is on the North Saskatchewan River on the northern outskirts of the city of Edmonton. It is an example of

a translational failure within the bentonite-rich Cretaceous shale. The landslide scarp was reported to be 50m wide, with a depth of rupture of 32m deep, and a volume 0.76 Mm^3 . The cause was river erosion, ice-blocked drainage, and weakening of the strata due to interbed slip from valley rebound of Upper Cretaceous Mudstone with bentonitic seams. It shows continued differential movement along the shale failure plane. This paper comments on Hutchinson's (1973) cycle of landsliding, which will be considered later in this thesis.

Dewar and Cruden (1998) report a landslide on the MacKay River that shows deep, translational movement (35m) along weak, sheared clay in Lower Cretaceous Clearwater Formation. The failure has a residual angle of 6° and the probable cause of shearing is valley rebound and swelling. It has a volume of 0.14 Mm^3 with 1 basal shear plane and a large graben at the head of the failure. The presence of bentonite was noted at this site.

A compound failure occurred in Cretaceous shale within the city of Calgary near the Western Irrigation District (WID) (Krahn and Weimer, 1984). The sliding surface was within the Paskapoo Fm, an interbedded mudstone, siltstone, and sandstone (Figure 2.6). The basal deep-seated horizontal movement occurred along a weak clay seam with a residual angle of the slope was 9° . The head scarp is interpreted as a rotational failure truncated by the same weak layer.

Figure 2.5 Little Smoky landslide (Thomson and Tweedie, 1975)

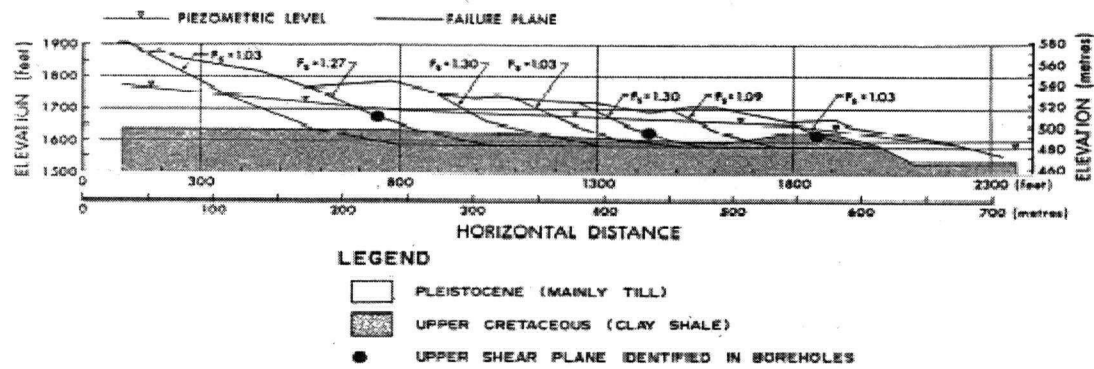
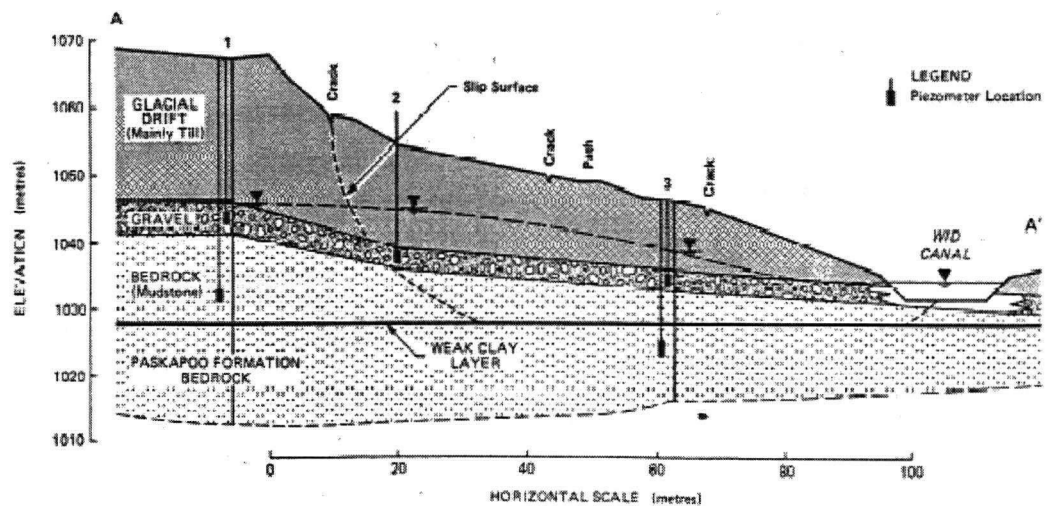


Figure 2.6 Western Irrigation District landslide (Krahn and Weimer, 1984).



2.3.2 Glacio-lacustrine Clay

The Montagneuse slide occurred in 1939 on west wall of the Montagneuse River Valley (Cruden et al 1997). It is a large compound failure noted to be the largest historic rapid landslide on the Interior Plains with a volume of 76 Mm^3 (Figure 2.7). The horizontal failure plane occurs within pre-glacial lacustrine sediments, with the mass subdivided into two main blocks, with the upper block breaking into many smaller blocks with oblique shears. There is little to no tilting of the debris masses and the rupture surface is relatively deep seated. This failure in this valley coincides with Shaftesbury pre-glacial channel of Peace River in common with many of the older, dormant landslides along this valley.

Nasmith (1964) describes 2 different stages of retrogressive rotational slides within the Meikle River Valley in northwestern Alberta. The basal failure plane occurs within pre-glacial lacustrine deposits beneath clay-rich till. The volumes of these are not given but assumed to be quite large as they were hard to view from the ground (Nasmith, 1964). The south facing slopes were noted to be stable at slightly steeper angles, giving rise to a slight asymmetry of the valley. Till is observed within the debris confirming the location of the failure plane below the glacial deposit.

A compound failure, 12 km northeast of the town of Rycroft, Alberta (Cruden et al, 1993), dammed the Saddle River. There is one deep-seated failure plane within a pre-glacial clay unit underneath a thick till unit on which the translational failure occurred (Figure 2.8). The trigger to this slide was lateral vertical and horizontal erosion as a result

of a severe rainstorm and it's resultant flood. The landslide was quite large with a volume of roughly 40 Mm³.

The Attachie slide (Figure 2.9) is a flow slide occurring 50 km west of Fort St. John along the Peace River near the town of Attachie (Evans et al, 1996; Fletcher et al, 2001). This was a catastrophic event involving the glacio-lacustrine unit seen through out the Charlie Lake map sheet. The total volume of the slide was 12.4 Mm³, similar size to the case above. The clay is pre-sheared parallel to the bedding caused by glaciotectionism and postglacial landsliding. This was a rapid event that dammed the Peace River for approximately 12 hours.

Figure 2.8. 1939 Montagneuse landslide (Cruden et al., 1997)

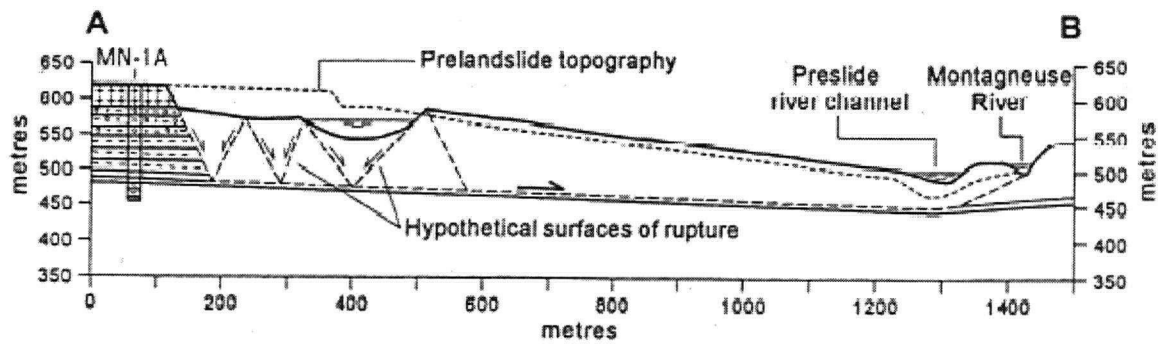


Figure 2.9. Rycroft landslide (Cruden et al., 1993)

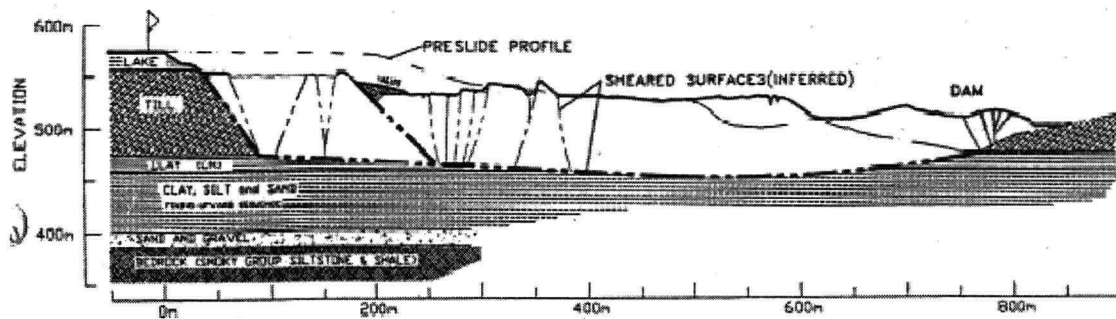
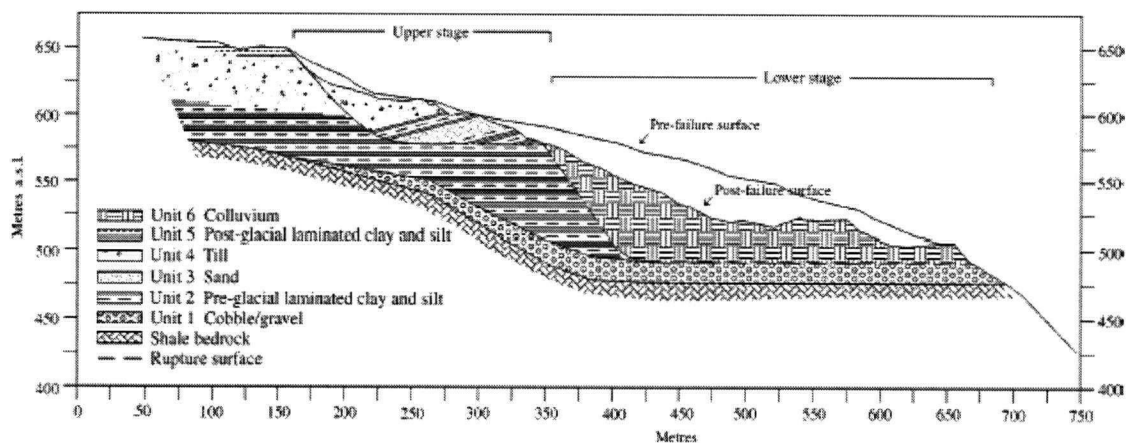


Figure 2.10. Attachie landslide (Fletcher et al., 2001)



3.0 REGIONAL SETTING

3.1 Physiography

The Charlie Lake Map Sheet lies on the western edge of the Alberta Plateau sub-province of the Great Plains (Bostock 1948, Holland 1964). The topography is subdued, reflecting the underlying Cretaceous bedrock, and includes northwesterly trending belts of rolling terrain with northeasterly directed drainage (Stott, 1982). The region is dissected by a deeply incised, integrated dendritic drainage system tributary to the Peace River, flowing through the area from west to east and draining the entire area. The Peace River flows within a broad, flat-floored trench that averages 3.2 km wide and 210 meters deep (Mathews, 1978). The walls of the Peace River and its tributaries rise sharply at angles close to 35° where the banks are cut into bedrock (Mathews, 1978), however the wall slope can be as low as 8° where failures have occurred. The upper platform above the river trench that connects the upland area to the river valley has a low continuous slope of about $1/4^{\circ}$ to 2° , although it bears a significant amount of micro-relief of mounds and swampy hollows (Mathews, 1978). The current Peace River Valley, cut in post-glacial time, mostly follows an interglacial valley of the Peace, which contains a dissected, complex Pleistocene fill (Mathews, 1978). Where the post-glacial channel follows the interglacial valley, the floor of the present trench is cut into the bedrock floor of the interglacial valley and is therefore lower (Evans, 1996). The significance of this is that the toe and basal shear plane of the landslides in the Pleistocene sediments lie above the present river valley (Evans, 1996). The downcutting also exposes steep sections of Cretaceous bedrock, allowing for failure along weak bedding planes. River elevations from Bobrowsky et al. (1991) at Hudson Hope (520 m) in the west and at the mouth of

the Kiskatinaw River (450 m) give an average gradient of the river over the 129 km section of 0.54m/km.

3.2 Climate

The present climate of the Fort St. John area is continental boreal, marked by cold winters (January mean temperature -14°C), warm, humid summers (July mean temperature 15°C), and moderate annual precipitation, 465mm (of which 185 mm as snow). The summer maximum of precipitation includes frequent, intense thunderstorms. These values are averages from the last 30 years and categorize the area as sub-humid (Drinkwater et al., 1969). Annual precipitation in the region is unlike much of British Columbia, with peak monthly precipitation in July and drier in the winter months, however, it mirrors the style seen in most of Alberta. The cold winters are due to rolling uplands and prairies allowing Arctic air to flow unobstructed. With westerly-dominated winds, the nearby Rocky Mountain Range may act as a barrier, placing Fort St. John in a rain shadow.

The Peace River region is within the Boreal White and Black Spruce biogeoclimatic zone (B.C. Ministry of Forests, 1988). On some south facing slopes, warmer plants such as the prickly pear cactus can be found locally in relative abundance. The south facing slopes are often covered with grass and scattered tree cover, while the north facing slopes are generally densely tree covered with few natural clearings.

3.3 Quaternary History

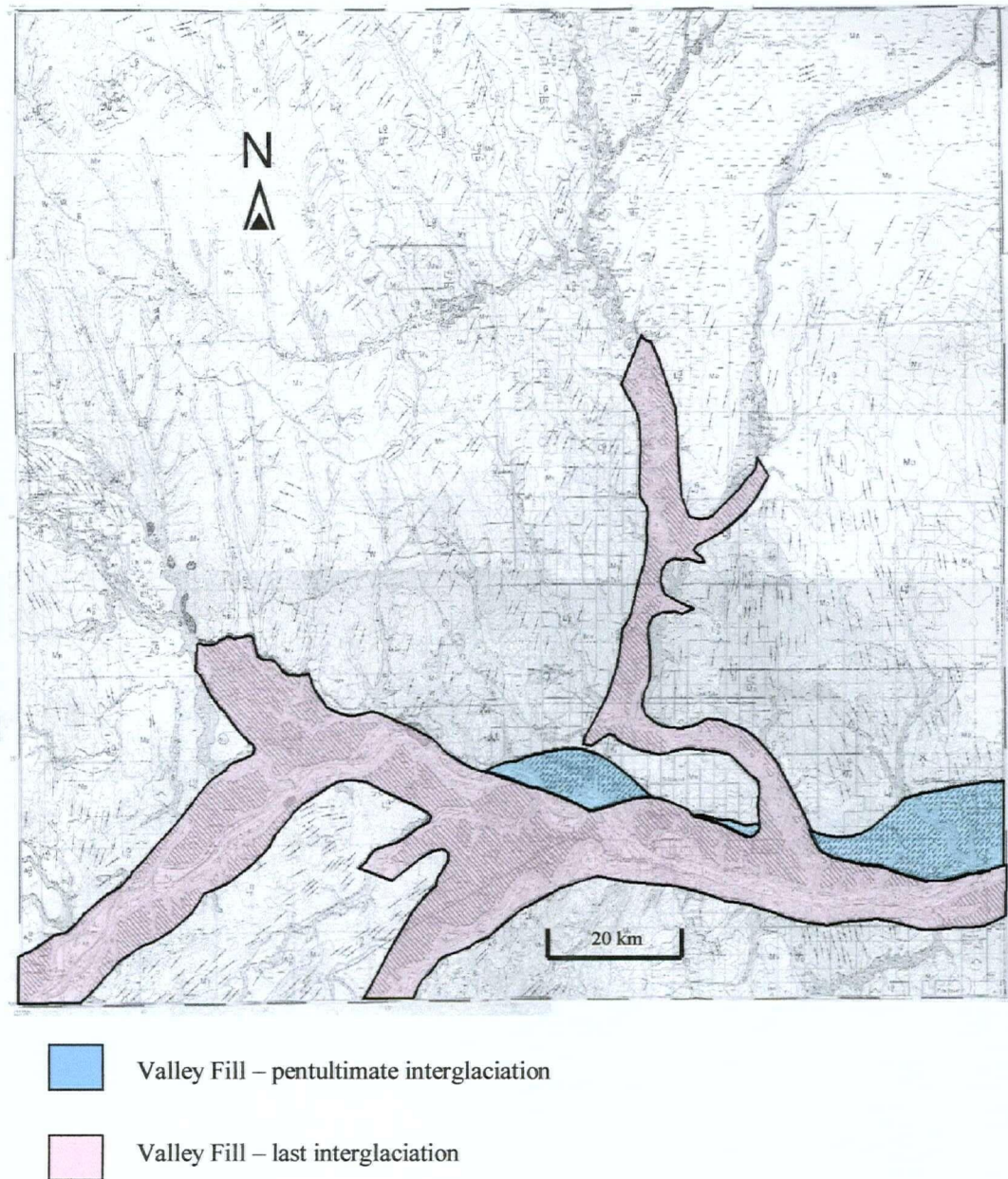
The Laurentide Ice Sheet advanced over the Peace River area on at least three separate occasions during the Quaternary (Mathews, 1978). Cordilleran ice from the Rocky Mountains also reached the area to an unknown extent, and is thought to be contemporaneous with the last Laurentide advance.

A bed of gravels represents the first glaciation with typical Canadian Shield gneiss within it. These gravels are found in one location within the map sheet, near Golata Creek. The second glaciation is represented by a small deposit of till above the aforementioned gravels and a thick bed of silt above the Peace River near Old Fort. These beds are localized and play little to no part in the stabilization of the river valley slopes. These deposits are represented by the light blue colour in Figure 3.1.

This glacial episode was followed by a well-defined erosional interval, widening the valley to 6.5 km – 8 km wide and 30 m above the present level. During this event well-graded gravel was being deposited under aggrading conditions while the valley widening process occurred.

The last, Late Pleistocene Laurentide advance from the east terminated this event. The ice-dammed Peace River likely began ponding early within the advance due to the size of the valley. This initiated a notable period of sedimentation of bedded sands, silts, and clays within a glacio-lacustrine environment. Because the lacustrine materials predate the maximum of the last glaciation, they are commonly referred to in the region as “Interglacial Clays and Silts”, or ICS. There are local occurrences of ice rafted pebbles within silt and clay suggesting a close proximity of glacial ice front. These deposits are represented by the light red colour in Figure 3.1.

Figure 3.1 Quaternary Valley Fill Materials Location (based on Mathews, 1978)



The last ice advance is marked by till and glacial grooves from the Laurentide sheet from the east and the Cordilleran from the west. It is thought that the two sheets met between Bear Flat and the town of Attachie (Mathews, 1978).

The ice retreat was slow, and at stages allowed the level of glacial lake Peace to drop, creating smaller ice-dammed lakes, ages 13,500 and 9,960 BP (Mathews, 1978). After the ice had retreated, the drainage of the lake occurred. This drainage caused rapid erosion of the present river valley, cutting through all of the sediments and into shale bedrock.

3.4 Local and Regional Infrastructure

Two major highways run through the Charlie Lake map sheet. B.C. Provincial Highway 97, known as the Alaska Highway, stretches from the southeast section of the region through Fort St. John and continues toward the northwest on its way to the Yukon boundary. The Alaska Highway only crosses the Peace River once, however, its significance lies in the fact that many Canadian and American motorists use this roadway on travels toward Alaska. B.C. Provincial Highway 29 runs from Charlie Lake to the town of Hudson Hope at the southwestern corner of the map sheet. This road follows the Peace River Valley for most of its length crossing the valley slope 3 times. There are several smaller roads created for access to homesteads, logging areas, and petroleum exploration sites. Due to the considerable amount of petroleum exploration and extraction activities, an extensive system of oil and gas pipelines runs through the area.

4.0 BEDROCK GEOLOGY AND GÉOTECHNIQUE

4.1 Outcrop Location

The flat lying succession of transgressive-regressive marine to near-marine formations of the late Cretaceous are generally covered by a thick succession of Quaternary sediments masking much of the bedrock topography, resulting in very few natural bedrock exposures. Most bedrock exposures occur along deeply dissected river and creek valleys within the Charlie Lake map sheet (NTS 94A) (Figure 4.1).

On the western border of the study area, near the town of Hudson Hope, the Peace River Valley exposes the Lower Cretaceous Moosebar, Gates, Hulcross, and Boulder Creek Formations, over a distance of approximately 17 kilometers. These formations are also well exposed along the entire length of Maurice Creek on the south bank of the Peace River; however, these locations are the only exposures in the map sheet.

Stratigraphically above the Hulcross Formation lies the Shaftesbury Shale, which crops out along the rest of the Peace River Valley walls to the Alberta border. The Beatton River Valley also displays Shaftesbury Shale outcrops along its walls for much of its length to the town of Rose Prairie located approximately at 56° 30'. The Shaftesbury also crops out along the valleys of the Pine, Moberly, and Halfway Rivers within the map area. Other steep tributary channels within the Shaftesbury Formation include the Farrell, Septimus, Red, and Cache Creeks along with the mouth of the Alces and Kiskatinaw Rivers.

The Sully group in the western section of the map sheet crops out only along the upper Halfway and the Cameron Rivers.

Figure 4.1 Bedrock Geology Map (based on Stott, 1982)

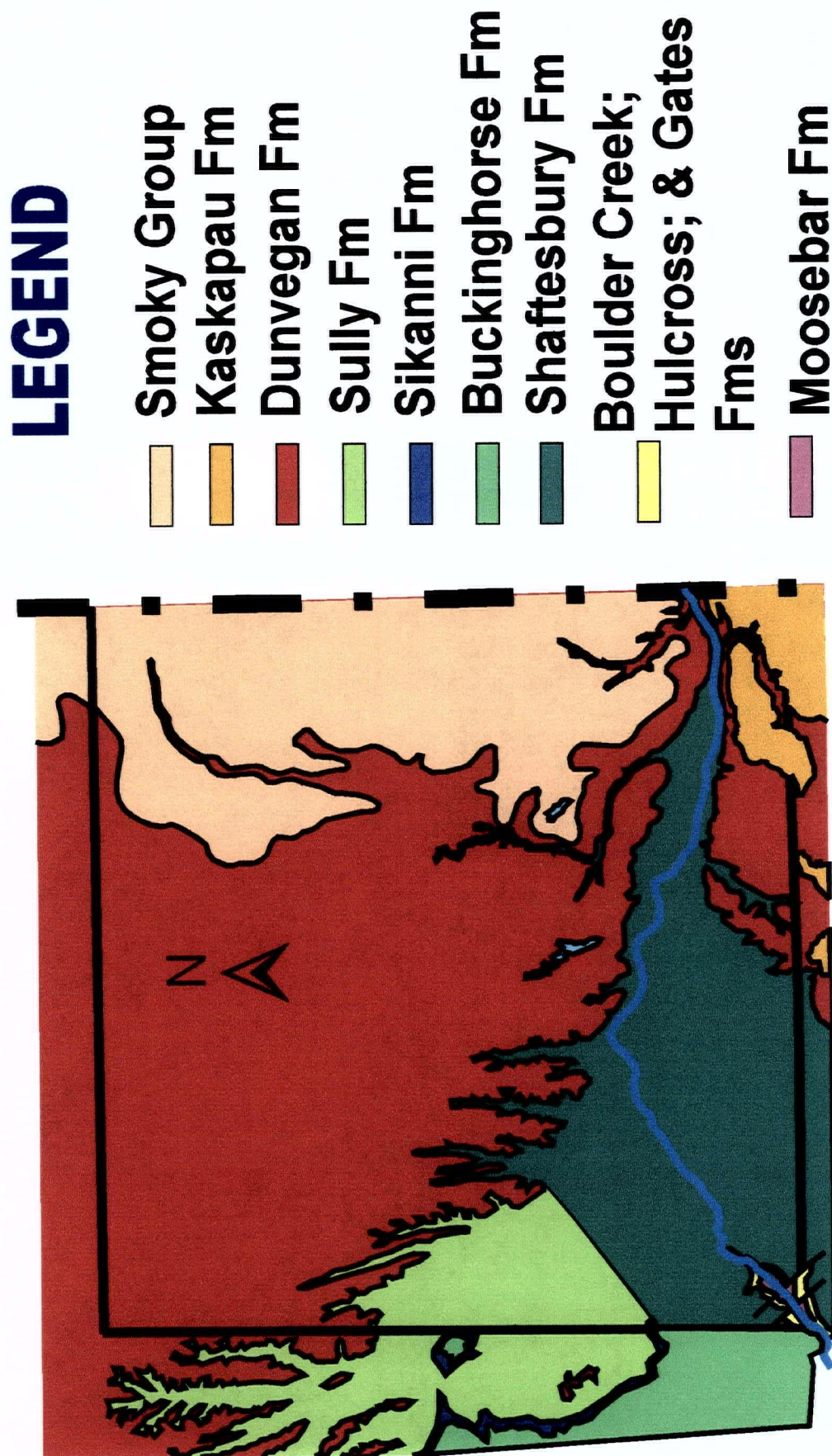


Figure 3a (Stott, 1982)

At the Peace River and Cache Creek confluence, outcropping of the Upper Cretaceous Dunvegan Sandstone appears above the Shaftesbury Shale on the north bank of the Peace River Valley. This outcropping continues for 5 km where it recedes off the river valley at Wilder Creek. The Dunvegan sandstone appears on the Peace River Valley again near Golata Creek and continues to the Alberta border. West of Cache Creek, the Dunvegan Sandstone trends to the west-northwest toward Cameron River making an escarpment throughout the map sheet. On the southern bank of the Peace River Valley, the Dunvegan Sandstone outcrops near the confluence of the Pine River at the head scarp of a large landslide and within the Kiskatinaw and Alces River Valleys.

The Smoky Group appears north-east of the Peace and Beatton River confluence to the Alberta border and extends to the top of the map sheet. It does not outcrop on either river (Figure 4.1). The Kaskapau shale, a formation within the Smoky Group, is differentiated along the south walls of the Peace, Alces, and Kiskatinaw River valley walls near the Alberta border.

4.2 Description of Formations

The Moosebar Formation, as mapped by Stott (1982), is part of the Fort St. John Group and consists of marine shales and siltstones. It has a maximum thickness of 290 metres in the Peace River Valley and lies conformably on the Gething Formation within the Bullhead Group and the Gates Formation. The shale of the Moosebar Formation is dark grey, rubbly to blocky, and contains reddish brown sideritic concretions. The basal shale grades upward into mudstone and siltstone until it comes in contact with a thick, continuous sandstone, which is the base of the Gates Formation. The shale beds have a

banded appearance with continuous and uniform layers from 1.5 to 10 cm. Stott (1967) reports thin bentonitic layers of up to 1 inch thick, however, it is believed that these may in fact be weathered gouge layers created along bedding plane shears caused by valley rebound (Matheson and Thomson, 1973). This unit extends for the first 12 of 17 kilometres covered by the lower Cretaceous unit along the Peace River at the western border of the map sheet.

The Gates Formation (Stott, 1967 and 1982) is a massive to thick bedded, fine-grained, well-sorted sandstone lying stratigraphically above the Moosebar shale and is marked by a thick, continuous sandstone bed. The unit is roughly 18 metres thick at the 'Gates' on the Peace River, a resistant island of rock, but over 61 metres at Steamboat Island. The Gates sandstone represents a complete marine succession with a sequence of sandstone, shale, and conglomerate layers with the on lap sandstones getting coarser upwards within the sequence. The sandstones of the Gates Formation are brownish-grey, slightly rusty, and fine-grained with fine laminations and uniform beds. The beds are dominantly quartz and chert with minor amounts of rock fragments, carbonate detritus, and feldspar. There are often thin beds of platy sandstone and shale, and the sandstone beds grade laterally into shale to the north. Flow rolls are fairly common within the Gates Formation on the Peace River as well as ripple marks and mud cracks within the bedding planes. The upper boundary with the Hulcross Formation shows a distinct change from sandstone to dark, marine shale

Stott (1982) mapped the Hulcross Formation as blocky to rubbly, silty, dark grey to black marine shale with sideritic concretions with no real bedding planes. The Hulcross shale thins to the south due to decreased sedimentation. The formation is stained

with limestone or a yellow powdery efflorescence caused by sulphur and iron weathering within the unit. The silt content of the unit increases upward within the unit and even becomes sandy to the top. The thickness and the number of sandstone beds increase upwards until the shale disappears. The shale is dense and structure-less with micro-granular carbonate with quartz, clay, and pyrite. Irish (1955) mapped the shale unit as being up to 113 metres thick east of the Gates Formation along the Peace River. The contact with the coarser Boulder Creek Formation above is also gradational and appears as a succession of siltstone and shale beds.

The Boulder Creek Formation is a succession of conglomerate beds (Stott, 1982), however, along the Peace River, the beds have graded into flaggy, silty marine siltstone with interbedded sandstone. It is almost indistinguishable from the Gates Formation, except for a lower percentage of carbonate detritus. The sandstones are mostly made up of quartz and chert with some rock fragments and matrix. Irish (1955) estimated the thickness of the Boulder Creek Formation as 22 metres. The sandstones are thickly bedded, laminated, and have abundant ripples on the bedding planes with abundant ironstone concretions. Within the map sheet, the Gates Formation, Hulcross Formation, and the Boulder Creek Formation are mapped as one indistinguishable unit.

The Shaftesbury Formation is a dark grey, flaky to fissile, rusty shale to mudstone, equivalent to the Hasler Formation and the Cruiser Formation to the south. The Goodrich sandstone, between the Hasler shale and Cruiser shale, grades into very silty shale within the Peace River Valley making the three formations undifferentiable. The unit contains abundant fish-scale layers, one of which is quite widespread and serves as a regional boundary unit between the Upper and Lower Cretaceous units. The lower

boundary of the Shaftesbury shale with the Boulder Creek Formation is distinct as the sandstone layers are directly overlain by dark marine shale, likely marking a hiatus. The upper boundary is gradational showing interbeds of shale and sandstone with the number of beds and their thickness increasing upwards. The beds of the Shaftesbury shale show a rhythmic development; flaky and fissile at the base to silty and blocky near the top. Stott (1982) was not able to give a thickness for the Shaftesbury Formation; however, it must be appreciable as the unit covers the entire Peace River from the Gates to at least the Alberta border. Stott (1982) maps several thin bentonitic layers, which may in fact be weathered clay within valley rebound bedding plane shears. In sections seen during the landslide investigations, there is appreciable degradation of the shale to a flaky texture with iron rich partings between the beds of shale. These partings are likely from alteration of bedding planes that have been disturbed during the valley rebound process. Along much of the Peace River Valley, the Shaftesbury shale represents the only bedrock unit exposed, as many of the Pleistocene sediments are deposited directly on the shale. This is due to the present river valley following the pre-glacial river valley, therefore, only exposing the lower units.

The Sully Formation, generally recognized in the Sikanni Chief region, is seen in the western most area of the Charlie Lake map sheet. It underlies the vicinity west of the Cameron River and is roughly equivalent to the Shaftesbury Shale. The Sully Shale are dark grey to black, silty, rubbly, and contains sideritic concretions as well as the same 'fish scale' marker seen in the Shaftesbury Formation. The formation is conformable to the older sandstones to the west and the Dunvegan Sandstone to the east.

The Upper Cretaceous Dunvegan Formation is a prominent sequence of fluvial and deltaic sandstones and conglomerates, with minor shale, which form distinct escarpments along many river valley walls throughout the map area. This formation is quite large, having been mapped from the Smoky River in Alberta to the Liard River in northern British Columbia. In the Fort St. John area it has been mapped by Crickmay (1944), Russel (1943), and Jones (1968) and is estimated to be of a thickness near 305 meters (Stott, 1982). Although the type section is near Fort Dunvegan, there are well-exposed sections near Bear Flat on the Peace River near Cache Creek and within the canyons on the Beatton River (Figure 4.2). In the Peace River area it is generally a fine-grained sandstone interbedded with shale. The sandstone is dominated by quartz and chert, with minor amounts of lithic fragments. Stott (1982) has done microscopic analyses of the unit, describing quartz grain boundaries and grain deformation. The Dunvegan Sandstone is conformable between the Shaftesbury Shale below marked by a well-bedded fine-grained sandstone that grades downward. South of the Peace River, the Kaskapau shale of the Smoky Group overlies the Dunvegan Sandstone conformably, with the contact being above a coaly horizon.

Figure 4.2. Dunvegan Sandstone within the Beatton River Canyon



The Kaskapau Formation is the youngest of the beds in the area. The beds are thick, over 700 meters, and consist of dark grey, marine shales, separated into members by calcareous or sideritic content and concretions. The basal unit, the Sunkay Member, seen in the Peace River area, contains much siltstone and sandstone. It is part of the Upper Cretaceous Smoky Group, seen undifferentiated north of the Peace River due to lack of outcrop exposure.

In terms of paleo-geography, the marine and near-marine sequence of sandstones, siltstones, and shales represents four transgressive – regressive cycles in the Western Interior Cretaceous Seaway during the Albian and Cenomanian periods (Plint, 1988).

4.4 Geotechnical Properties

4.4.1 Shaftesbury Shale

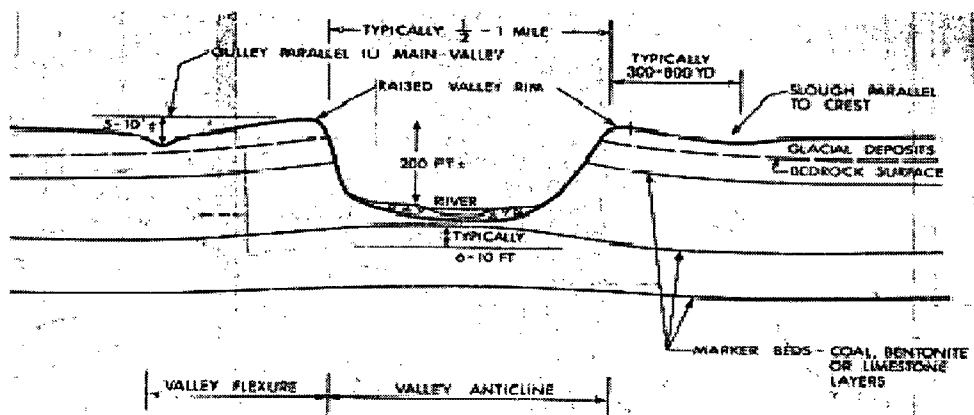
During the 1970's and 1980's, several investigations were conducted on behalf of British Columbia Hydro to determine the geotechnical properties of the Shaftesbury shale (Cornish and Moore 1985, Sargent and Moore 1985, Imrie 1991). A proposed dam site, Site C, was investigated in order to establish the feasibility of constructing an earthfill hydroelectric dam on the Peace River, 6 kilometres southwest of Fort St. John.

The Shaftesbury Shale is a compaction shale, heavily overconsolidated by Quaternary glaciations, that outcrops with slopes seldom steeper than 1H: 1V along most of the Peace River valley within the 94A map sheet. The regional dip is 1° to 5° to the northwest with small amounts of localized thrust faulting. The shale is poorly consolidated and therefore is prone to rapid weathering and breakdown as it is conducive to decomposition and swelling. The bedrock has been identified as a soft shale based on

the Morgenstern and Eigenbrod (1974) classification. According to Cornish and Moore (1985) the content of clay sized material ranges from 35 to 49%, with Atterberg limits of 40 – 50 % for Liquid Limit (L_L) and 21 – 25 % for the Plastic Limit (P_L). The Shaftesbury Shale was found to be mainly quartz (27 – 52 %), interlayered illite / smectite (34 – 52 %), illite (0 – 11 %), and kaolinite (3 – 10 %). The rock shows swelling pressures ranging from 20 to 50 KPa. The intact rock is moderately strong showing a range of compressive strength of 6 – 28 MPa (Imrie, 1991). The samples tested by Imrie (1991) show an average shear strength of $\phi_p = 45^\circ$ and $\phi_r = 20^\circ$.

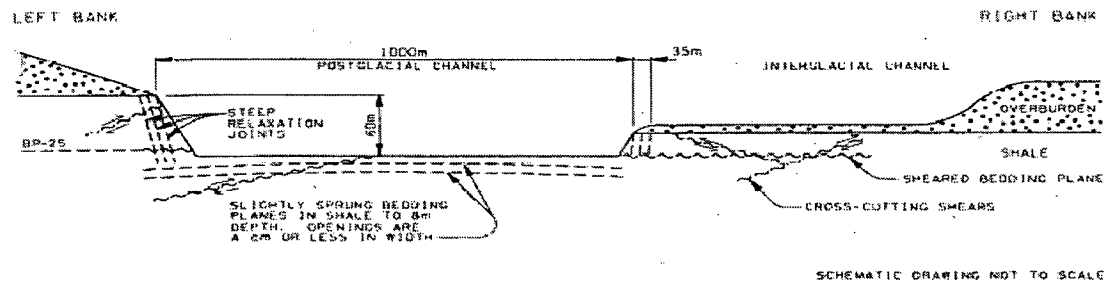
After the removal of late-Wisconsinan ice loads, post-glacial rebound and erosional unloading with associated river valley cutting have caused a bulging of valley floor materials and stress-relief deformation of valley side materials known as valley rebound (Matheson and Thoomson, 1973). This caused an inward relaxation of the valley walls and possible shearing displacements as well as local cross cutting shear zones (Figure 4.3).

Figure 4.3 Valley Rebound with deformation (Matheson and Thomson, 1973)



The outcrop examined in detail at Site C, was found to have three types of major discontinuities (Imrie, 1991); 1) bedding plane failures, 2) cross-cutting shears, and 3) relaxation joints (Figure 4.4). These discontinuities are thought to persist throughout much of the Peace River Valley.

Figure 4.4 Site C Damsite Schmatic with discontinuities (Imrie, 1991).



The changes in stress regimes, vertical unloading, caused arching within the central valley floor, which is accompanied by limited shearing and fracturing along the bedding planes in the river valley walls. These bedding plane shear zones often act as slide surfaces for deep-seated failures within the bedrock even though they appear as tightly interlocked hairline partings. These partings can range up to 1 centimetre thick, allowing for shale softening and the development of gouge filling. These fillings generally have mineralogy similar to the rock, indicating the rock as a parent material. They appear white or yellow and can be confused with seams of bentonite and are laterally discontinuous as they pinch out away from the valley wall. The weak bedding planes also control the foundation strength of the Shaftesbury shale within the Peace River valley (Imrie, 1991).

The cross-cutting shears occur within the bed of the Peace River mostly dipping south with the remainder dipping to the north, causing the right bank to be more intensely

sheared compared to the left bank. They are generally thrust faults in nature and have a maximum thickness of 5 cm, infilled with gouge, breccia, infilling, and distorted bedding. Imrie (1985) found vertical offsets of 0.2 m to 3 m with a dip movement of 7.6 m and vertical offset always less than 3 metres. The south-dipping shears form a complex pattern striking 80° to 150° and dipping 15° to 30° away from the river. The residual friction angle was found to vary from $\phi_r = 12^{\circ}$ to 20° with additional strength being provided by plane waviness at $\phi_i = 5^{\circ}$. Imrie (1991) discounts the remnant orogenic stresses as being a factor in their creation but rather suggests these features are valley rebound structures.

The final structures observed due to valley rebound are rock relaxation joints due to inward movement of the valley walls. These are near-vertical fractures that parallel the river, observed in adits during the investigation of site C. Imrie (1991) reports relaxation joints as far as 35m in from the ground surface. These are at least partially infilled by broken rock and clay.

Through rising and falling head tests, the permeability range was found to be 10^{-7} to 10^{-10} cm/sec with a downward head gradient. This is explained by the presence of the shear zones allowing water to saturate the upper beds of the shale while the lower beds remain out of equilibrium, therefore causing a downward gradient.

During the summer of 2003, samples were collected to determine the bulk properties of the materials found in association with landsliding. The locations of the tested samples can be found on Figure 4.6. The samples consisted of slickenensided, remolded shale (8) and a disturbed gouge material (4) which appears in conjunction with the rupture surface (Figure 4.5). The samples were collected by locating the shear plane

and removing the slicked material with a knife. The samples were tested for Atterberg Limits and Grain Size analysis, using both sieve and Hydrometer methods, at the Fort St. John field office of AMEC Earth & Environmental Ltd. The test sheets can be found in Appendix I. A summary of classification results for the samples collected is given in Table 4.1.

The sheared shale was found to have an average Liquid Limit of 49% and Plastic Limit of 20%. This indicates shale of moderate plasticity and not indicative of sheared material (Figure 5.1). The shale was dominated by silt with a mean of 70%, with minor amounts of clay, 20%, and sand, 10%. Hardy et al. (1962) suggest that the propensity of the shale to fail is determined more by bonding agents and pore water chemistry than by grain size alone. The test results of these failed shale samples would agree with their findings.

Figure 4.5 Typical sampling area of sheared shale and gouge.



Figure 4.5 Tested Sample Location Map

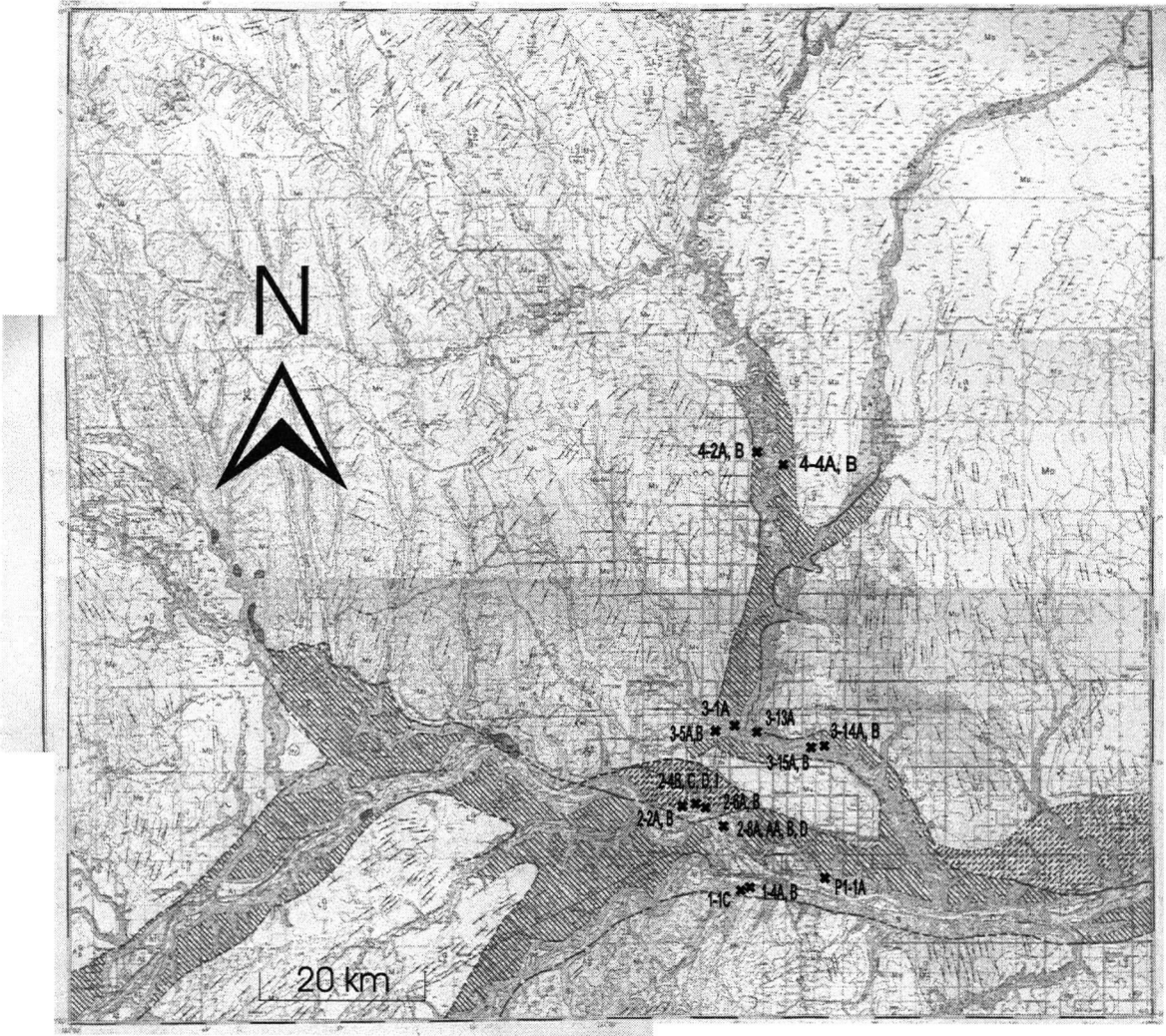


Table 4.1. Summary of classification test results, shale and gouge

Property	Sheared Shale n = 8	Gouge n = 4
Liquid Limit (%)	49.1 {40 – 80}	50.7 {45 – 56}
Plastic Limit (%)	19.9 {16 – 30}	26.4 {23 – 30}
Plasticity Index (%)	29.2 {20 – 51}	24.3 {20 – 29}
Clay Content (%)	20 {3 – 36}	11 {4 – 22}
Silt Content (%)	70 {50 – 85}	58 {40 – 68}
Sand Content (%)	10 {1 – 22}	31 {23 – 38}

n = number of samples tested

The gouge material found in relation with the sheared shale displayed a higher Liquid Limit, 51%, however, the Plasticity Index proved to be lower, suggesting a higher concentration of coarser material. From the grain size analysis done on the samples, the sand component, 31%, appears to be higher than the shale and both clay and silt were lower, 11% and 58%, respectively.

4.4.2 Dunvegan Sandstone

The Dunvegan Formation is very competent and acts as a cap rock for the weaker shale below. This allows the valley wall to achieve steeper slopes seen near Bear Flat on the Peace River, within the lower Beatton River, and at the Cameron and Halfway River confluence. In situations where the underlying shale is exposed, large bedrock failures occur as the static load of the Dunvegan Sandstone causes plastic deformation of the ductile shales below. Where the lower shale is not exposed in the upper Beatton,

Kiskatinaw, and Alces Rivers, deep canyons with stable walls are produced while the bedrock is exposed. The Dunvegan sandstone does produce large blocks that fall in the canyons, as there are discontinuities along bedding planes and minor amount of vertical jointing.

Saint Simon et al. (1979) had investigated the Peace River Valley near the town of Dunvegan, Alberta, which also shows the Shaftesbury, Dunvegan, and Kaskapau Formations. At this site, stress relief jointing and horizontal bedding joints of the Dunvegan were observed, indicating that the valley rebound process is likely wide spread throughout the prairie environment, not localized to the Peace River. The claystone investigated at this site was found to be moderate to highly plastic and the shear strength of both siltstone and claystone were found to decrease with increased moisture content. The clay stone also shows residual angles of 13° .

4.5 Comparison with Other Prairie Shales

The geotechnical properties of the Shaftesbury Shale were compared to those of other prairie shales located in Canada and the northwest of the United States. Data for this section was collected from published geotechnical papers (Cornish and Moore, 1985; Scott and Brooker, 1967; Haug et al., 1977; Peterson et al, 1960; Thompson and Hayley, 1975; Thompson and Tweedie, 1971; Thompson & Yacyschyn, (1977); and Beene (1967). Table 4.2 summarizes the Atterberg limits, peak and residual shear angle, and clay percentage are compared between the Shaftesbury Shale and the other shales.

The Shaftesbury Shale has a plastic limit roughly equal to that of other prairie shales allowing plastic deformation at similar moisture content. Deformation occurs

without cracking with the consistency of stiff putty to soft butter (Conduto, 1998). This low plastic limit also allows a pre-sheared surface to remain within a plastic state at natural moisture content. The liquid limit of the Shaftesbury Shale is lower than most of the other prairie shales, permitting it to be at a liquid state at lower moisture content. The Shaftesbury Shale has a low plasticity index (29.2%) suggesting that it is of moderate plasticity (Sowers, 1979). The Kaskapau and Pepper Formations also show a medium plasticity, however, the Bear Paw, Belly River, and Edmonton Formations all have high plasticity indexes making them highly plastic shales. The Shaftesbury Shale was sampled using a rotary drill during an investigation for diversion tunnels within the left bank. The rotary drill probably did not allow for the bentonitic-rich layers to be sampled alone. This contamination makes the samples appear to be less plastic than they really are. The Shaftesbury Shale also has lower clay content than many of the other materials sampled.

The residual friction angle of the Shaftesbury Shale at 20° appears to be much higher than the other materials listed below in Table 4.2. However, the listed value of 20° is thought to represent a bentonitic-poor zone within the formation, and therefore should not be used as a practical lower limit for strength within the formation as a whole. A lower value would be more practical to use for the shale of this area. The difference between the two values found for the Bear Paw Formation (Haug et al., 1977 and Peterson et al., 1960) was found to 14° . A similar difference should exist for the Shaftesbury Formation when testing bentonitic-rich and bentonitic-poor zones. Most of the prairie shales have residual angles within the range of 5° to 15° ; therefore, a practical value for the residual angle for the Shaftesbury Shale would lie within this range.

Table 4.2. Geotechnical Properties of Prairie Shales

Formation	Liquid Limit	Plastic Limit	Peak Shear ϕ_p	Residual Shear ϕ_r	Clay %	Source
Shaftesbury	40-50 %	21-25 %	45 ⁰	20 ⁰	35-49 %	Cornish & Moore (1985)
Bear Paw	65-100 %	20-25 %	-	8 ⁰ (Haug, 1977)	30-60 %	Scott & Brooker (1967)
Bear Paw	120 %	25 %	28 ⁰	22 ⁰		Peterson et al (1960)
Kaskapau	55 %	31 %	34 ⁰	14 ⁰	38-43 %	Thompson & Hayley (1975)
Belly River	102-149 %	20-30 %	-	8 ⁰	76 %	Thompson & Tweedie (1971)
Edmonton-Bentonitic Shale	125-220 %	30-45 %	14 ⁰	8 ⁰ -10 ⁰	-	Thompson & Yacyshyn (1977)
Edmonton - Bentonite	80-400 %	50-60 %	14 ⁰	5 ⁰ -10 ⁰	-	Thompson & Yacyshyn (1977)
Pepper	58 %	26 %	14 ⁰	9 ⁰	-	Beene (1967)

5.0 SURFICIAL GEOLOGY AND GEOTECHNIQUE

The surficial geology of the Peace River area has developed primarily from glacial episodes during the Quaternary (Mathews, 1978), as described briefly above. The last two of these episodes left deposits in the region while the evidence of any older glaciations have not been found. Geomorphic processes have altered the glaciated landscape slightly to produce the modern topography.

5.1 Location

Like the Cretaceous bedrock, the best exposures of Pleistocene sediments occur along the deeply dissected valley of the Peace River and its tributaries. Much of the valley exposures are so disturbed that a complete section is difficult to find, therefore, the stratigraphy has to be pieced together using head scarps and slump blocks of the landslides. The glacio-fluvial and glacio-lacustrine deposits are confined to pre-glacial valleys; however, till and post-glacial deposits are more can be observed widespread throughout the map area. The upland area is usually covered in blankets of till and glacial lake sediments with little to no exposure. The middle and north parts of the map sheet are

covered in an extensive bog land and muskeg. Much of the area that is not along the river valleys is covered in forest cover.

Another important source of Pleistocene stratigraphy are the many road cuts made throughout the area as the local infrastructure is well developed. Road cuts may also be highly disturbed due to small-scale slumping and disturbance from construction.

From oldest to youngest, the typical sequence of Late Quaternary materials within the Peace River Valley and environs consists of basal fluvial gravel, a thick sequence of interglacial silts, sands, and clays (ICS), a widespread blanket of basal till, and post-glacial lacustrine clay. Within the Beatton River Valley, the sequence is often like Peace to the south, however, around the area of North Pine, a deltaic sequence begins to occur. Sands with minor beds of silt and clay dominate this deltaic sequence. The Halfway River is mostly filled with glacio-fluvial sands and gravels except for in the south where the river enters the pre-glacial valley.

5.2 Late-Quaternary Materials

5.2.1 Gravels

The pre-glacial gravels are seen along the Peace River for most of its length, as well as some parts of the Beatton, Halfway, Pine, and Moberly Rivers. These are confined to the pre-glacial river valleys (Mathews, 1978), and are noticeably missing near Old Fort. The gravels have a rusted colour due to prolonged oxidation, well-rounded clasts up to 15 cm in size, and are partially cemented. The thickness of this unit varies from 1m to 30m. It may act as an aquifer in some slopes. The unit is clean, gap-graded, and contains a sand matrix with pebbles and cobbles. There are a few sand beds up to 25

to 30 cm thick. Quartzite clasts dominate the unit with well-cemented sandstones, gray granites, and volcanics. Shield rocks are present but rare. The gravel has sharp contacts, top and bottom, with the bedrock always underneath. The unit has a fairly constant elevation above river level. Evans et al. (1996) suggest that the gravel unit does not play a part in the instability of the slope.

5.2.2 Pre-glacial Silt, Sand, and Clay

The pre-glacial sand, silt, and clay lies above the gravels with sharp contact along the valley of the Peace River, much of the Beatton River, and the Pine, Moberly, and Halfway Rivers. It is roughly 100 to 150 m thick with silt or sand found at the base. For the most part, silt dominates sequence, except near the pre-glacial lake boundaries, where sand prevails. Sand can occur at any level (up to 35 m thick) and indicates aggrading fluvial condition. The beds are all well sorted, however, there can be extremely complex structures within this unit. Scattered pebbles are observed within the clay indicating the proximity to the glacial ice. Clay layers within the unit can be moderately to highly plastic, and can provide loci for landslides in the area (Mathews, 1978). This pre-glacial sedimentary package is not cemented but acts as a poor drainage element within most slopes on account of the interspersed, low conductivity clay beds.

The upper contact with the till is sharp, although some intact rip-up blocks of silt and clay can be found within the till. Several samples were taken of this unit during the investigation of several of the landslides in order to help characterize the beds.

5.2.3 Late Wisconsinan Till

The unit described in this section is from the final ice advance in this region as it is the only one that is preserved over the area. Mathews (1978) maps the till unit as two separate units from contemporaneous glacial advances from the Laurentian Ice Sheet in the East and Cordilleran Ice in the west. The two sheets coalesce near the town of Attachie. Even though the two tills are different, they are not distinct so will be considered one unit. Both tills rest on the interglacial river and lake deposits and act as an impermeable cap for surface recharge. Both are geotechnically more competent than the deposits below making them analogous to the sandstone above shale in the role that it plays in slope stability. The only difference seen between the two tills is that in the east the deposit has more gneissic clasts from the Canadian Shield and in the west, the deposit has more schist from the Rocky Mountain Trench.

The basal till is clay-rich, massive, and shows a gradation of drop stones near the top as the ice receded above it. The unit has an average thickness of 15 m and a maximum thickness of max 28 m, being slightly thicker in the eastern part of the map area.

5.2.4 Glacial Lake Peace Clay

The upper most unit within a typical section of the Late Pleistocene sequence is clay deposited from Glacial Lake Peace. It formed as water built up behind a glacier slowly retreating down the pre-glacial valley. The duration of deposition is a minimum of 220 years determined from counting varves within the deposit. The silt in the deposit forms distinct layers (fraction of an inch to feet thick) within clay, particularly in the

west. The unit acts as an impermeable cap controlling most of the surface morphology over all of the platform and upland areas. The clay has played role in initial drainage pattern in area after the retreat of the area.

The most abundant material within the unit is clay with few or no stones on platforms to east, however, there is clay with scattered pebbles that may look like the till below. Sand and gravel is found within this unit, although it is limited to the former lake limits. The clay is firm when dry, and slick and sticky when wet due to its high plasticity. There are shallow failures contained within this unit on the Beatton and Halfway Rivers, however, it has no importance for deep-seated failures.

The thickness of the clay unit shows greater isostatic rebound to west, suggesting that it has been ice-free for a longer period of time.

5.3 Geotechnical Properties

Several samples of sheared and intact pre-glacial clay were collected during the field investigation of the selected landslides. The location of the tested samples is found on Figure 4.6. These samples were then tested for Atterburg Limits and grain size analysis at AMEC Earth & Environmental Ltd in Fort St. John. Tests to obtain material properties were conducted on the principle units involved in the slide movement and materials resultant from the movement. A summary of classification test is given in Table 5.1. The test sheets can be found in Appendix I.

5.3.1 Bulk Properties

The pre-glacial lake clay deposits are generally classified as inorganic clay of low to medium plasticity (CL/CH) using the Modified Unified Classification System (Figure 5.1). Grain size analyses indicate that most of the samples originally described as clay are actually clayey silt with an average clay fraction of 30%. Few samples were relatively rich in clay, up to 57%. The clay fraction is not an indicator of failure potential as sheared clay beds had as few as 10% clay and intact beds had a high of 57%. Sand content is also a non-factor in stability as intact beds actually showed a lower average fraction of sand at 2%, than the sheared beds at 6% with a high of 25%. Many non-plastic silt beds were witnessed during the slide investigations beneath the sheared beds. These beds were rarely sampled and were not be analyzed.

Figure 5.1 Plasticity Chart (Unified System) – Peace River Samples

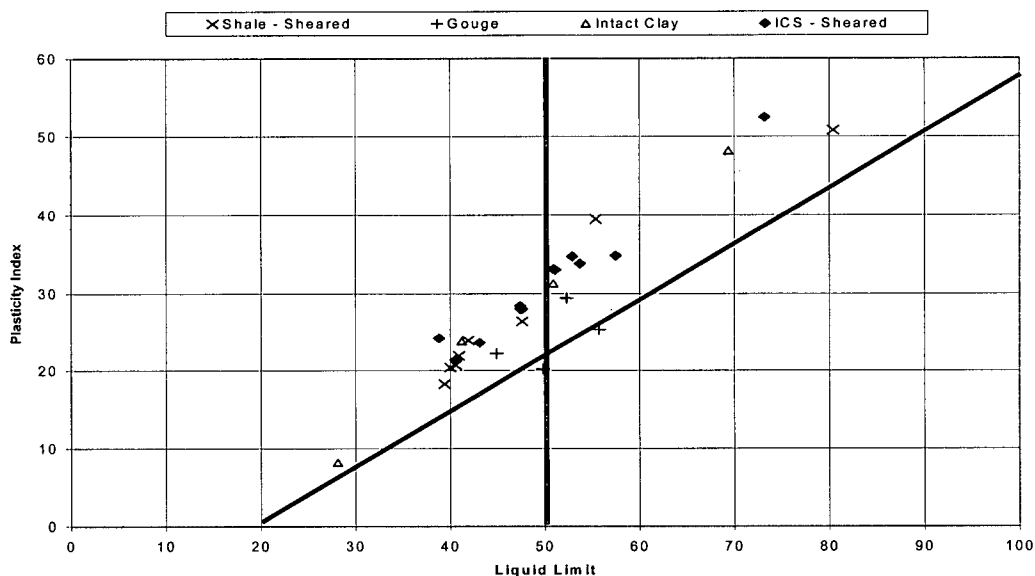


Table 5.1. Summary of classification test results for surficial materials

Property	Pre-glacial lake sediments	
	Intact n = 4	Sheared n = 12
Liquid Limit (%)	47 {28 – 69}	50 {39 – 73}
Plastic Limit (%)	20 {17 – 21}	20 {15 – 23}
Plasticity Index (%)	28 {8 – 48}	30 {18 – 53}
Clay Content (%)	29 {1 – 57}	30 {10 – 50}
Silt Content (%)	70 {43 – 97}	63 {49 – 86}
Sand Content (%)	2 {0 – 4}	6 {0 – 25}

n = number of samples tested

The Atterberg Limits of the clay indicate an average liquid limit is near 50 for both intact and sheared materials, bordering between high- and low-plasticity clay. The plasticity index for the sheared materials is only slightly higher at 30.3, making the clay highly plastic. The Atterberg Limits classify the material as medium to highly plastic clay while the grain size analysis suggests that the material is clayey silt.

The till is classified as well graded, medium plastic silty clay (CL) by Fletcher (2001).

6.0 LANDSLIDE INVENTORY MAPPING SYSTEM

6.1 Slide Types in the Study Area

To properly catalogue all of the landslides in the study area, two classification systems were created, based on activity and morphology of the landslides. Taking into

account previous classifications already in place (Hungar, Hutchinson, Skempton, etc), new classifications were developed for the activity and the morphology of the landslides. These classifications, as well as previously developed ones are discussed here. Slide types were defined in the order in which they were noted during the initial photo interpretation and desk study phases of the landslide inventory.

Slide Type 1: Raveling

The first type of slide identified in the Peace River region of British Columbia was simple raveling of the shale, sandstone, and cohesive surficial material (Figure 6.1). The scarps are generally clean as the process affecting them is active. The scarp height can vary although the debris apron will normally only accumulate on slopes with a height of at least 5m. There is no unique mechanism indicated here, as the process could be a simple fall, small scale slumping, or flaking of the material. The Shaftesbury Shale is easily slaked so the intact scarp tends to produce a weathered rind along the exposed face. Both the shale and the local clays and silts have very low impact strength so when the material strikes the intact ground or debris below it will accumulate producing an apron of undistinguishable debris. Often the scarp will be directly above the river so no accumulation can occur, however accumulations are common in the smaller creeks. The fissile nature of the shale and the steep relaxation joints and bedding plane discontinuities produced from the valley rebound process allows for small pieces to constantly break off. Often the shale breaks in paper-thin partings. This shale paper will flake off and slides down the shale face. The pre-glacial lacustrine deposits tend to break apart along silt interbeds and vertical jointing, also causing blocks of materials to fall.

Slide Type 2: Retrogressed Rotational Failure

The heterogeneity of both the Shaftesbury shale and the Pleistocene sediments generally does not favour single rotational failures. Instead the rupture surface will extend along a weak layer within the slope. Multiple rotational slides in the Peace River occur from upslope progression of a single rotational slip with a more or less planar basal shear surface, which terminates as a curved back scarp. Retrogression of the slide forms two or more slipped blocks with slip surfaces tangential to a common slip sole (Hutchinson, 1988). After an initial small failure, the intact scarp behind the failed mass becomes unstable, since toe support has been removed. This unstable scarp fails and now removes support for the next failure. In the Peace Region, the new failure can occur along the same plane or on a nearby weaker layer within the slope that can now be utilized. If the curved back scarp has a weak layer within it, a new failure will often occur along that discontinuity, as the toe has now been unloaded. These failures will retrogress further as successive slices become unstable from the removal of lateral support. The slope eventually self-stabilizes when the build up of debris downslope provides sufficient buttress support for materials further upslope (Figure 6.2). When the debris is removed, the intact section will become unstable again. These failures can progress a long distance into the slope as both the shale and the clay can fail at low angles and there are many weak layers within both that can be mobilized. During air photo interpretation, it can be very hard to distinguish these failures from compound failures. Often the difference is the lack of extensional features and the presence of back tilted blocks both near the head of the failure and within the debris mass. These features are often difficult to locate and the interpretation must be supplemented with a field examination. On older, dormant or

abandoned slopes, the ability to distinguish these features is reduced remarkably as the features are overgrown and weathered. A majority of these failures occur within the Pleistocene sequence, although they are not exclusively to it. When the failures occur within the upper portion of a section, removed from the erosive force of the river, they generally become stabilized and difficult to recognize.

Slide Type 3: Rotational Failure

The third type of failure recognized in the study area was a single rotational failure (Figure 6.3). These failures occur mostly within pre-glacial sediments, although they can also occur at a very small scale within Cretaceous shale. A majority of these failures occur on the south-facing slopes, although there are a few cases on north-facing slopes as well. As mentioned, single rotational failures rarely occur in earth materials that are heterogeneous and anisotropic, however, if the basal shear plane truncates the curvilinear failure plane deep within the failed mass, the movement will remain mostly rotational. The Peace River area has many rotational failures, even though the horizontally bedded materials are not a favorable material for such a movement. These rotational failures occur within the pre-glacial sediments at locations where the heterogeneity is at a minimum. The Pleistocene sequence is more favorable than the shale because of the weaker compressive strength of the material and a smaller discrepancy between the plastic and non-plastic materials. Many of the non-plastic clay layers actually act to hold the unconsolidated materials together, allowing for the curved back scarp to occur. As most of the failures within this region occur above river level, the debris of the rotational failure is often disturbed and does not appear as a slumped block but rather broken up debris covering the slope. Pre-sheared surfaces allow the initial

movement to occur and in most cases will allow the material behind to mobilize. These failures are important to recognize because many of the larger failures may actually begin as a rotational slide, which unloads the toe and lets a larger mass to fail. Generally, rotational failures in this area are of small size and occur either within Pleistocene materials or within the landslide debris of larger failures. Such failures may involve either intact or remobilized material. The small size of these failures makes their identification difficult using high-level photographs.

Slide Type 4: Mobile Flow

Several highly mobile flows, likely mudflows within the colluvium derived from the pre-glacial sand, silt, and clay or the post-glacial clay (Figure 6.4), were identified during the air photo investigation. These failures generally occur within gullies cut into river bluffs and show the typical fan shape at the exit point of the gully. Hungr et al (2001) classifies these failures as mud flows, which are extremely rapid or slower, fine-grained flows of liquid clayey soil. They are shallow failures occurring during heavy rainfall and snowmelt seasons. These are small, continuing processes that build up to form gentle fans near the mouth of the gullies. They occur in a range of sizes from 5,000 m³ to 500,000 m³; however, they are often very small and shallow. These failures were identified and noted within the air photos and database but not examined in detail in the field.

Slide Type 5: Earth Flow

A few slopes were identified as containing earth flows or earth flow complexes (Figure 6.5). These failures occur within the colluvium derived from the underlying

Pleistocene materials. They generally occur in the far western section of the study area around the town of Hudson Hope. Hungr et al. (2001) defines these movements as earth flows that are slow movements of earth near its plastic limit, sliding as a plug-like mass. They mostly occur within the gullies although they can also happen along the main valley walls as well. These failures occur exclusively within the pre-glacial valley boundaries. This type of failure was not examined in the field, only recognized during the air photo interpretation.

Slide Type 6: Multi-level Failures

The majority of the failures within the Peace River area utilize several weak shear planes at different levels. These are considered multi-level failures (Figure 6.6). The compound failures can involve any combination of the other slide types mentioned. The presence of two very weak materials, shale and pre-glacial clay, in which many shear planes can exist at different elevations, make this a dominant type of slide. There are two types of multi-level failures occurring within the study area: those in which the failures are independent of each other and those that appear to have failed together. If the failures are independent of each other, then there is an intact scarp between the two failures that is not covered by debris or colluvium. The debris from the upper failure may play a role in the stability of the lower failure(s) but the shear planes are not connected and distinct from each other and the failures occurred at different times. These failures are generally shallower and can occur in either material. Shear planes that have failed simultaneously, are generally deeper within the slope. The debris mass that has failed is large enough to cause the other failure too occur roughly at the same time. If the mass is not large enough to move a second mass, the disturbed material can either fail by itself or be buttressed and

supported by the stable material. When the stable material fails, debris from both shear planes will be mobilized. This also occurs in both materials.

Slide Type 7: Compound Failure

The Shaftesbury shale contains bentonitic-rich layers that are widespread, which allows a sheared surface to propagate deep into the slope leading to the development of large, low-angle compound failures (Figure 6.7). Many of these failures are dormant, so the original morphology of grabens and horsts is not recognizable on air photographs. These failures are prevalent on the north-facing slopes, although not exclusively. Most of these failures are older, making the features difficult to recognize both on the air photos and on the ground. Within the shale, the failures are thought to occur on pre-sheared surfaces at many different elevations above 420 meters a.s.l. Below this, no shear planes were witnessed or inferred. This type of failure also occurs within the pre-glacial sequence with the largest failures occurring near the base of the sequence. These failures can be large, up to 60 Mm³. Extensional features within the debris, such as horst and grabens, as well as the low angle at which debris can be found, distinguish these failures from all others. At very low angles of slip, a Compound failure is more likely than a rotational failure.

Slide Type 8: Shallow Retrogressive Failure

Shallow retrogressive failures (Figure 6.8) occur exclusively within the Glacial Lake Peace clay. These were only witnessed along the Halfway River (Bobrowsky, 1991) and near the canyons on the Beatton River. The till below acts as a stable base below, which the failure plane cannot penetrate. The basal shear plane normally coincides with

Figure 6.1 Slide Type 1: Raveling - Photo and Schematic Diagram (inset)

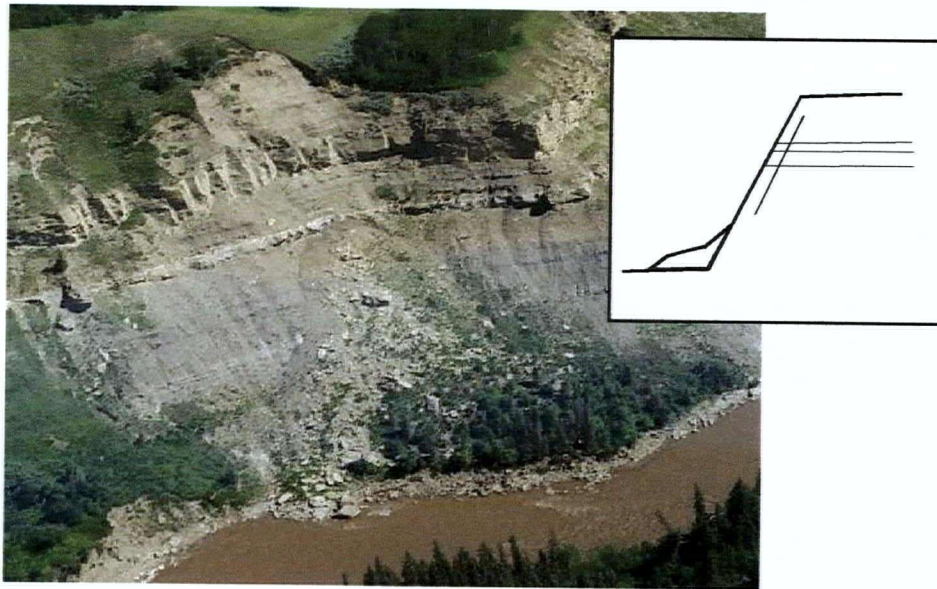


Figure 6.2 Slide Type 2: Retrogressive Rotational Failure - Photo and Schematic Diagram (inset)

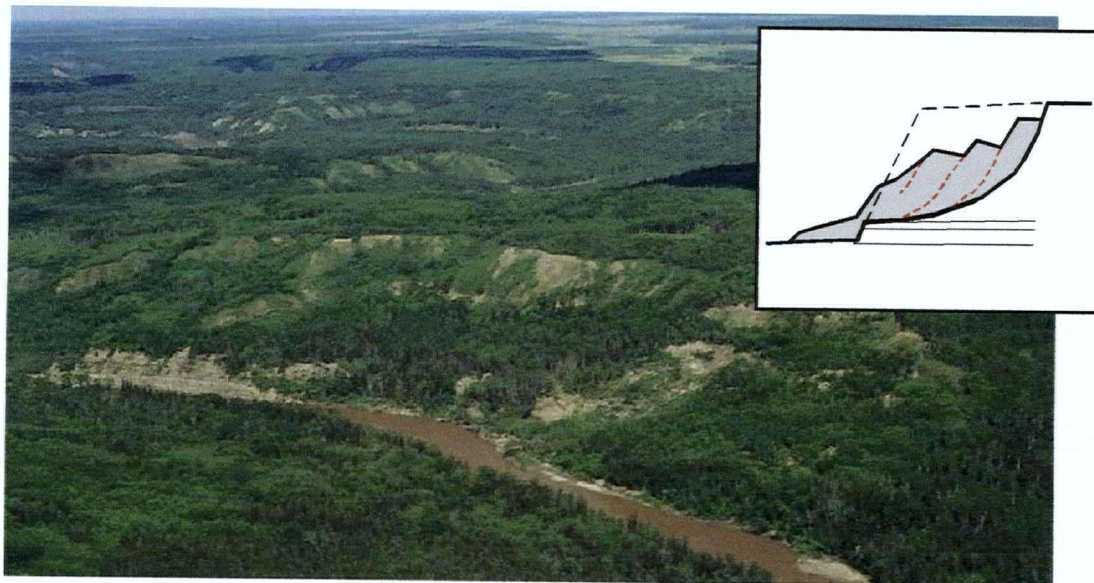


Figure 6.3 Slide Type 3: Rotational Failure - Photo and Schematic Diagram (inset)

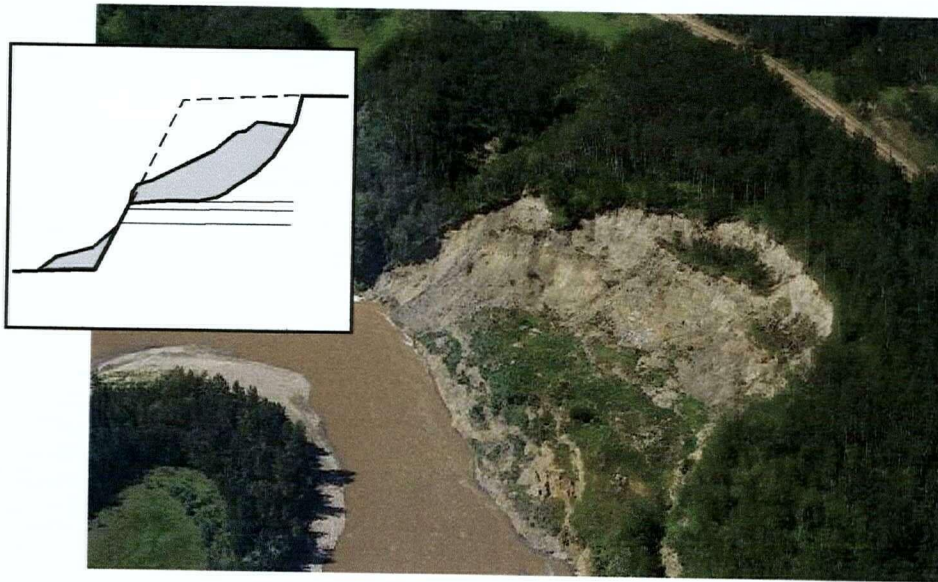


Figure 6.4 Slide Type 4: Mobile Flow - Photo and Schematic Diagram (inset)

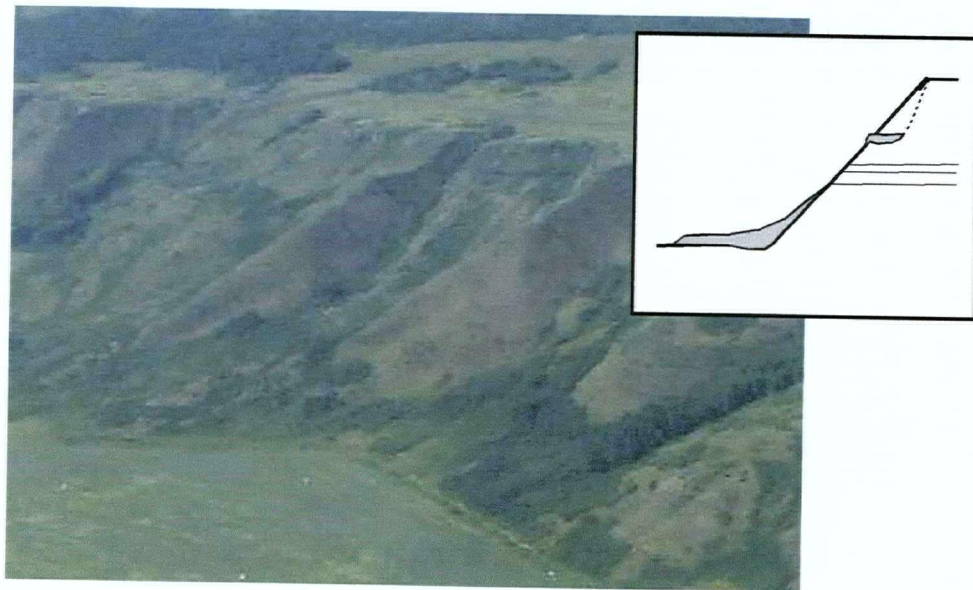


Figure 6.5 Slide Type 5: Earth Flow - Photo and Schematic Diagram (inset)

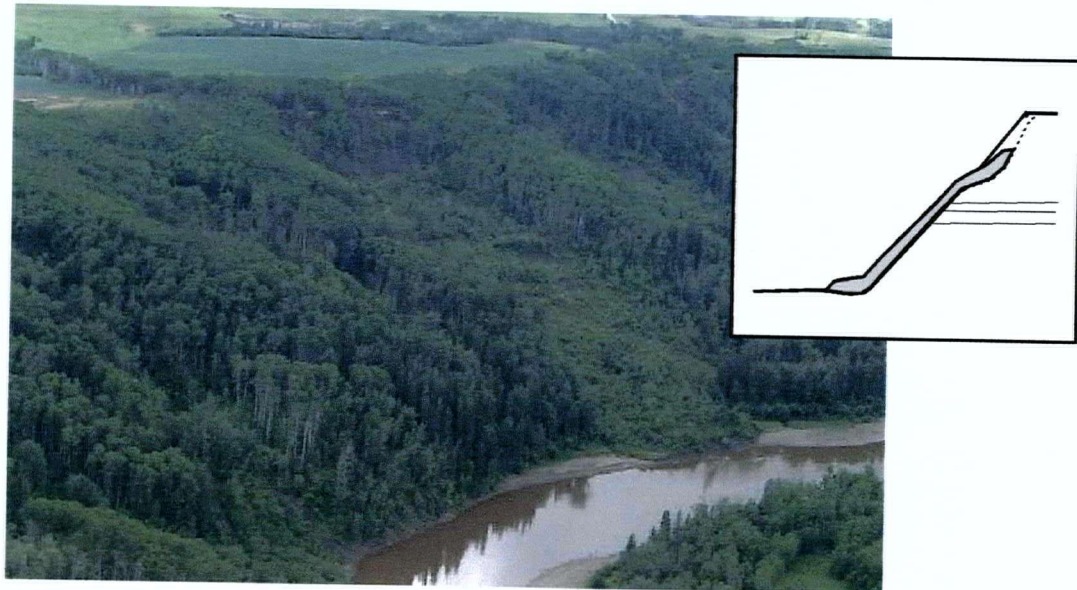


Figure 6.6 Slide Type 6: Multi-level Failure - Photo and Schematic Diagram (inset 1) and shear plane (inset 2)

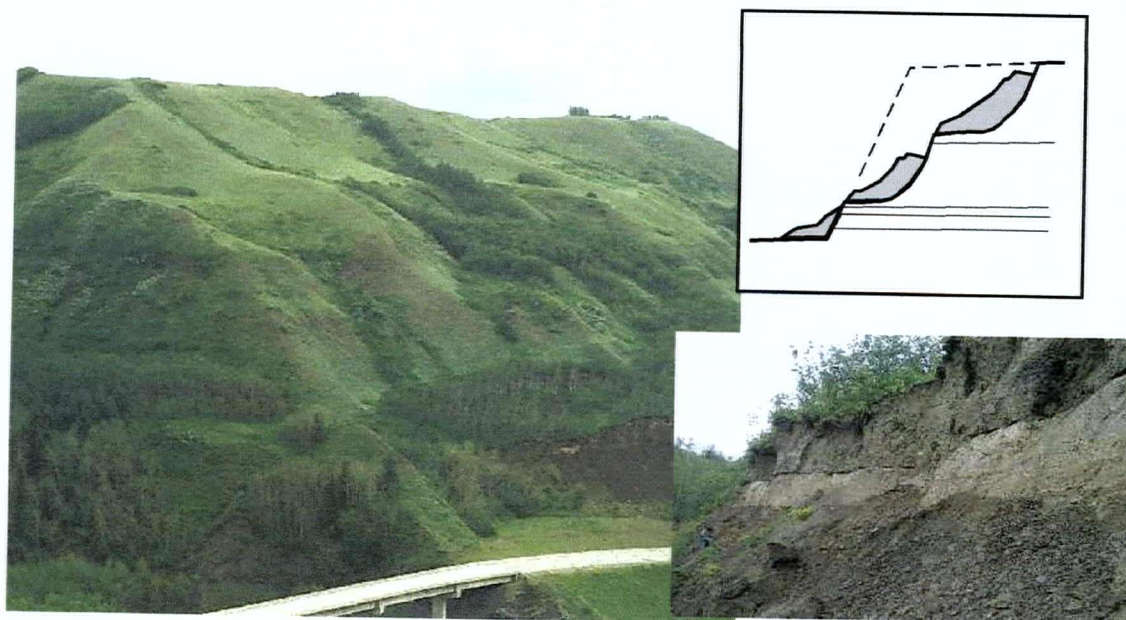


Figure 6.7 Slide Type 7: Compound Failure - Photo and Schematic Diagram (inset)

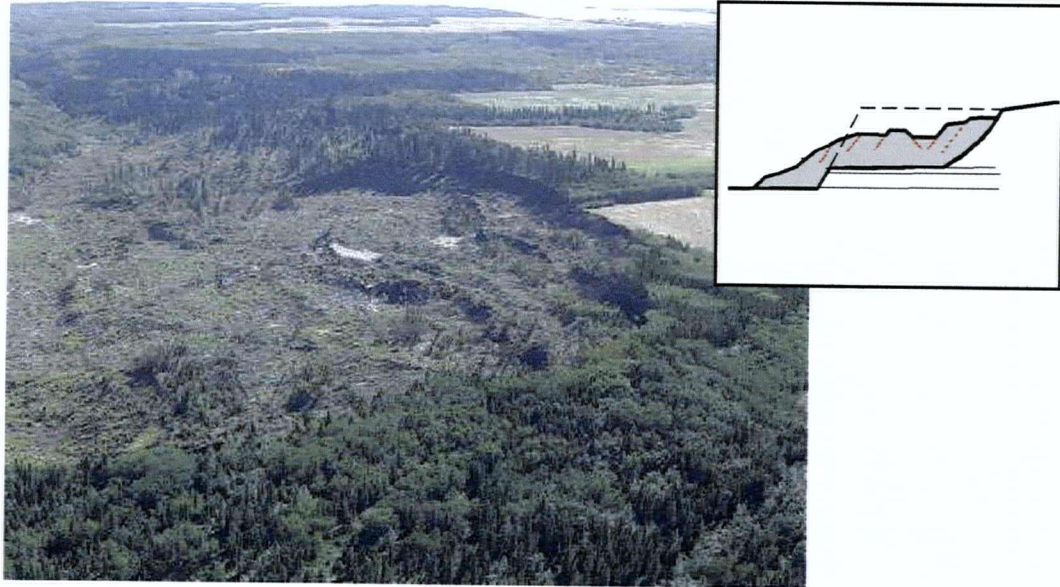
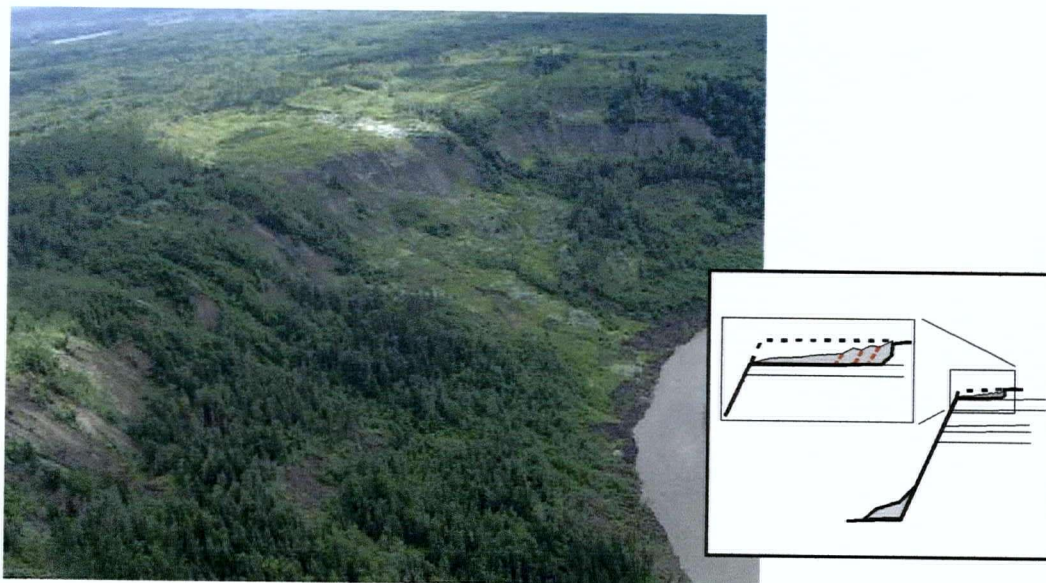


Figure 6.8 Slide Type 8: Shallow Retrogressive Failure - Photo and Schematic Diagram (inset)



the surface of the Wisconsin till. The sediment involved remobilizes and flows over a stable bench. The debris mass is thought to undergo quick disintegration and flow in a fluid-like manner. The retrogression occurs quickly and can extend for a slope angle of 3.5°. The mandate of this report was to focus on deep-seated failures, so this type of failure was not surveyed during fieldwork, only recognized during air photo interpretation.

6.2 Activity Classifications

Many attempts to create a system in which to map landslides in terms of activity (Cruden and Varnes, 1996), intensity (Hungr, 1997), and morphology (Sissakian et al., 1983 and Yanez, 1979) have been made, however, as of yet none have been fully recognized. This is partly due to the highly variable nature of slope movements around the world and the complexity of the mapping systems needed in order to map them. The landslide mapping system used for this project is a hybrid of previous classifications and unique features used for the Canadian Prairies.

Cruden and Varnes (1996) describe 6 states, Active, Suspended, Reactivated, Dormant, Stabilized, and Relict to describe the activity of a landslide. Active failures are those that are currently moving and reactivated failures are those which are currently active after being inactive. Suspended failures were defined as failures, which have been recently active, within the last annual cycle of seasons but not moving at present. Dormant movements include inactive landslides in which the cause of the movement remains apparent. Failures in which the toe has been protected against erosion are described as being stabilized, and those in which the failure occurred under different geomorphic or climatic conditions are described as Relict. This system is not practical for

use in the Peace River Area, as almost all of the slopes within the river valley are actively moving. The rate of the movement is the main difference. The failure planes on which the failures occur extend into the slope further than the current failure boundaries, therefore new movements can occur. This makes an otherwise "stabilized" slope still susceptible to failure, therefore, categorizing it as stable would be misleading. Another issue is that much of the debris of the initial failure remains on the slope due to the low angles of the failure planes. This debris moves very slowly, therefore, it is currently moving but to classify it as active would also be misleading. The contention is that there are very few stable slopes and that the classification has to reflect the relative activity of the failure.

Hungr (1998) describes a method of mapping landslides in terms of intensity. Hungr (1988) defines intensity as the set of quantitative or qualitative spatially distributed parameters, which determine the potential of a given landslide phenomenon to cause damage. If this inventory is to develop into a risk map, then the intensity of the failures should be considered. A map of this nature could not be undertaken at this time due to the lack of full knowledge of the elements of risk. Due to the preliminary and broad nature of this map, only the recognition of a hazard and the estimate of size could be undertaken. Hungr (1988) uses an incremental classification system based on the return period of a landslide to create a system of mapping failures relatively. This method was deemed unfeasible for this project, as the failures are not discrete movements in which a return period can be calculated, but rather constant slow moving failures, which seemingly randomly fail catastrophically.

A hazard map would be difficult to produce in this area as all of the slopes are failing. Generally (the Attachie Slide being an exception), once a slope is failed it is

known and can fail again, but it actually reduces the hazard as the slope is at a shallower angle and is closer to stabilizing itself. It is the slopes that have not failed catastrophically in recent times but have known failure planes that are of a high hazard.

An activity classification was developed to distinguish landslides and landslide areas, according to their degree of recent movement activity. This may allow for possible landslide triggers to be identified. As landslides are ubiquitous within in the area, the most active areas were identified to distinguish areas of higher hazard. In this way, a determination of the most active areas can be made for practical purposes. Although the active areas are distinguished, other slopes should not be thought as safe and stable, as dormant slopes have been known to fail catastrophically as in the Minias Creek Slide (Thurber, 1982). Another important reason for developing an activity classification was to help determine whether or not different types of slides had different activities. This may allow for a temporal analysis of mechanism or mode of failure. Cruden and Varnes (1996) discuss activity under three separate headings; State of Activity, Distribution of Activity, and Style of Activity. Within the classification used for this project, only the state of activity is considered, which is defined as what is known about the timing of the movements.

Activity levels were defined from dated air photographs taken between 1997 and 1945, and by site investigation during the summers of 2002 and 2003. Photos used for this purpose are noted in Appendix A. Due to the size of the area, not all river segments were visited in detail and therefore the index may show them as less active than they really are. Also, due to the small scale of the air photos, not all active features may have

been recognized. The activity levels created for this project are: 1) Very Active, 2) Active, 3) Low Activity - Dormant, 4) Low Activity - Abandoned, and 5) Modified.

Very Active slides are those that have failed within the last 5 years, in this case since 1997. They show up as a red colour within the inventory and had a very fresh appearance during the investigation (Figure 6.9). These slides were classed by field investigations done by the author and from reports from practicing engineers and geologists in the region. The river valleys were flown over during the summers of 2002 and 2003. The five year cut off for very active slide grouping was chosen because smaller features tend to begin to be muted after this time period and young trees begin to appear. It also allows practioners in the area enough time to add observed slides to the database.

Active slides are those that have failed within the last 50 years, in this case since 1953, excluding the above Very Active cases. In the inventory they appear with a light blue colour. In the field they can range from looking fairly fresh with no vegetation cover to grass covered with smaller trees (Figure 6.10). This activity can be hard to define in the field, as aspen grows at a very fast rate. Landslides with this activity were determined by studying air photos from 1985 and 1945 along with geological and geotechnical reports from practicing professionals within the area. The ground generally looks disturbed with stands of younger trees; however, after about 50 years the slide can become indistinguishable from the rest of the slope. This classification exists because the investigation of these landslides may still be useful for understanding slide mechanics within the area. With a separate classification, they can be selected and visited. After approximately 50 years, the features of the slide can become distorted and muted by weathering, leading to false interpretations. Also the work that has been done in the area

within the past 50 years can be added to the inventory in order not to lose a valuable resource.

Low Activity - Dormant landslides (Figure 6.11) are those, in which movements are probably older than 50 years, yet they still have active erosion near the toe (e.g., fluvial undercutting). Within the inventory they appear as a green colour. These failures are mainly determined through air photo interpretation with some selected field studies. This is an important classification because even though some of the river slopes appear to be stable, they still contain one or more pre-sheared layers within them. These pre-sheared planes allow the slope to fail easily with even a small trigger. The probability of failure is higher due to the active erosion as it can either undermine intact material at the toe of the landslide making it more susceptible to failure or transport debris of older sliding movements so that it can not act as support for the toe.

Low Activity - Abandoned landslides are those that are older than 50 years and do not currently have active erosion at the toe (Figure 6.12). They appear as the colour purple in the inventory. For this classification, an understanding of river morphology must be incorporated to deduce where past river erosion has taken place. These slopes are marked as they have failed since glacial times and therefore also likely contain sheared clay layers within the slope. For practical purposes, they are mapped due to the possibility of remobilization along the shear plane if the slopes are disturbed, either naturally or anthropogenically. As river erosion is a major trigger within the area, slides situated away from the river are less likely to fail and therefore of a lower hazard.

The final class of landslide defined within the area is Modified. These are landslides that have been stabilized by artificial earth works after failure (Figure 6.13). They are

Figure 6.9 Very Active Landslide (Photo)



Figure 6.10 Active Landslide (photo)



Figure 6.11 Low Activity – Dormant Slide (Photo)



Figure 6.12 Low Activity – Abandoned Slides (photo)



Figure 6.13 Modified Slide (Photo)



almost always along a part of the infrastructure, including roadways, railways, or pipelines. In order to be part of this grouping, the slide features have to be disturbed enough to be unrecognizable. These are important slides to note because in the years to come, the slope will be weathered and all traces of a failure will be erased. These slides must be noted so that the presence of the shear plane is not lost to other investigators.

One of the problems encountered using this classification is that some slides show different activities within different sections of the slide. A slide with an active toe and a dormant top must be defined as one or the other. In this case, the activity which best describes the majority of the slide will be used.

6.3 Morphological Symbols for the Inventory Map

A set of morphological symbols was designed for use on the landslides in the Peace River Area during this project. The features used for this inventory are seen in Figure 6.14. This is based upon Yanez (1979).

Figure 6.14a is the first symbol defined and is used to denote a large scarp within intact material. Intact material is defined as any large mass of material that has not previously failed, whether it is bedrock or Quaternary material. A large intact slump block is, therefore, mapped as debris rather than intact material. A large scarp is defined as any scarp that measures over 1.0 mm in plan width on the air photos used. The air photo sets (BC 86047, BC 86074, BC 86075, and BC 86099) are flown at a high altitude with a resultant scale of 1:60,000. The large scarp therefore corresponds to 60 meters in horizontal length. The same symbol is also used for large, nearly vertical cliff scarps whose length is not represented properly on air photos.

Figure 6.14*b* is used for large scarps within defined debris masses. This includes any materials that have incurred any movement including creep and intact blocks that have moved. Large scarps within the debris mass are rare but do occur on the larger failures. The symbol has the same shape as the intact material in order to allow the large slopes to be easily picked out. Often the determination of intact or debris material is difficult, therefore, when the material is in question, the conservative approach was taken and the material is assumed to be debris.

Figures 6.14*c* and 6.14 identify symbols used to denote small scarps within the study area, in intact material (rock or soil) and debris respectively. The latter symbol was created with a similar appearance to the small scarp in order that all small scarps could easily be identified. The definition of a small scarp includes any scarp with a down slope plan length less than 60 meters. This symbol is also used for a small vertical scarp. This is a very common symbol often used to denote a cliff. It was chosen to represent a small scarp due to the practical smaller size.

An arrow attached to any of the above symbols, indicates a movement in the direction of the arrow. The arrow by itself does not indicate a type of slide but rather the direction itself. A failure with this symbol is usually a rotational failure with no obvious back tilt or an older translational or compound failure. Other symbols within the failure can determine the type of slide. Figure 6.14*e* is a symbol that represents back tilting within the debris mass. The arrows from the head scarp and the arrows from the debris meet at the base of the head scarp. The symbol used for the head scarp depends on if the material is intact or in debris and the size.

Figure 6.14*f* represents the extensional feature of a graben and is usually used in conjunction with Figure 6.14*g*, which represents a horst. These symbols are not dependent on size and because these features only occur in debris, the intact version was not considered. In the case where a horst is at the base of a head scarp, the two features coalesce at the base of the head scarp. This symbol is used exclusively on compound and compound features.

The symbol for a debris apron is represented by Figure 6.14*h*, which can be used in conjunction with any of the other symbols to represent the debris of the slide. It can also be used by itself if a debris apron is observed at the base of a slope with no clear features. An example of this would be the base of a raveling slope.

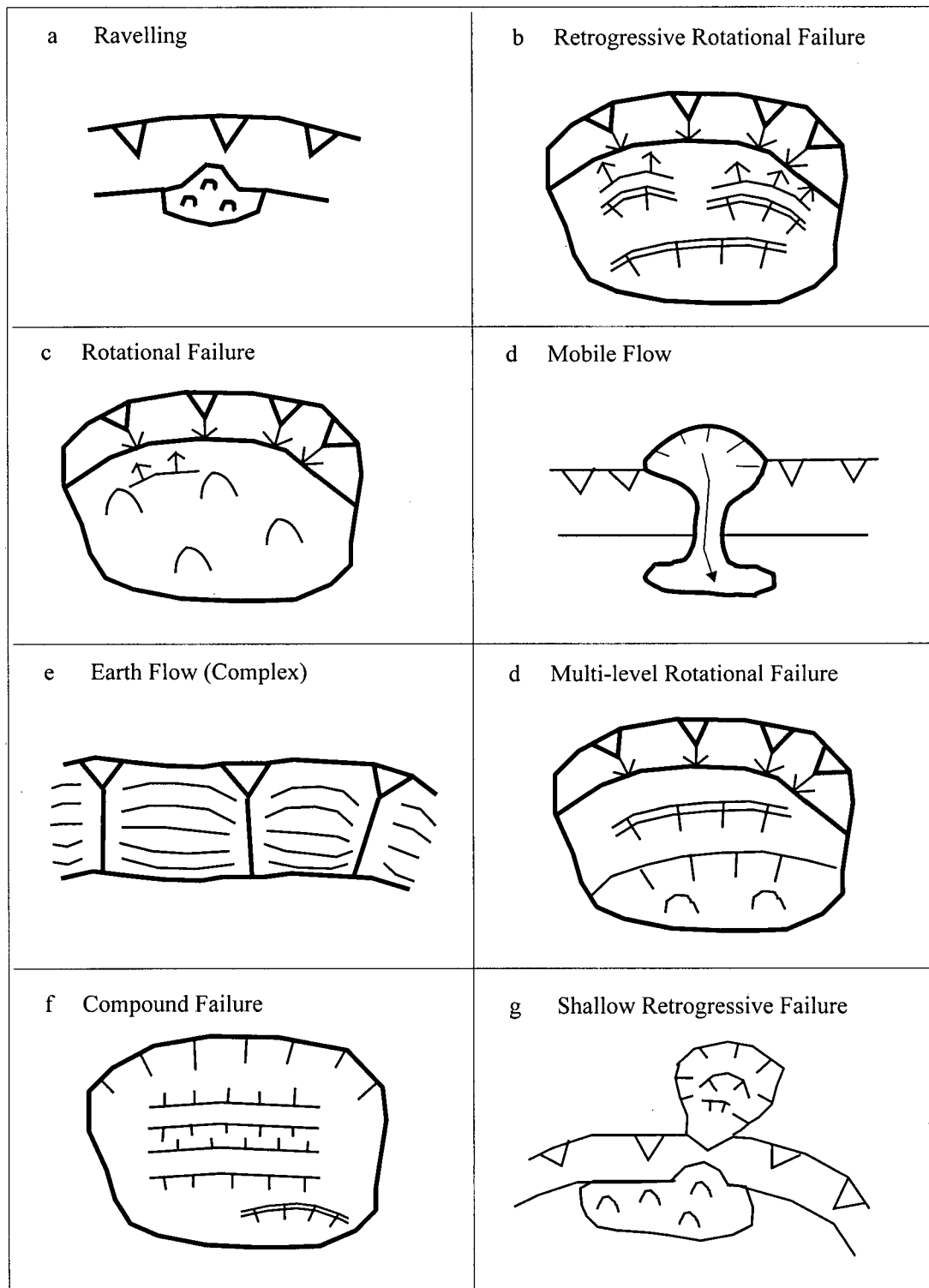
The above symbols can be combined in order to define most of the landslide types defined in Section 6, however, two unique symbols were created to describe specific conditions. A highly mobile flow is indicated by Figure 6.14*i*. The scarp at the head of the failure depends on the size and material. The body of the displaced material is outlined with an arrow outlining the suspected path of flow. A less mobile earth flow is represented by Figure 6.14*j* with the head scarp symbol defined by the size and material found. The body of the debris is outlined. The area in which debris has been removed is represented by a concave curve, the center of the mass is a straight line, and the accumulation area is represented by a convex curve. When an earth flow complex covers the whole slope, then the symbol represented by Figure 7.7*e* is used. The scarp symbol depends on the material and size.

The use of the morphological symbols in describing the eight landslide types described in Section 6.1 is shown in Figure 6.15.

Figure 6.14 Morphological Classification

Symbol	Description
a	<p>Large Scarp (Intact Material)</p> <p>A large scarp is defined as any scarp that measures over 1.0 mm in plan width on 1:60,000 scale air photos, corresponding to 60 meters in length. This symbol is also used for large, nearly vertical scarps whose length is not properly represented on air photos. Intact material is defined as any large mass of material that has not previously failed, whether it is bedrock or Quaternary material.</p>
b	<p>Large Scarp (in Debris)</p> <p>A large scarp, defined above to be over 60 meters in length in plan view, within a defined debris mass. This includes any materials that have incurred any movement including creep and intact blocks that have moved. The symbol has the same shape as the intact material in order to allow the large scarps to be easily picked out. The arrow indicates that movement in that direction has occurred.</p>
c	<p>Small Scarp (Intact Material)</p> <p>A small scarp is defined as any scarp measuring less than 1.0 mm in plan width on 1:60,000 scale photos, corresponding to less than 60 meters in length. The scarp may be within any intact material, Cretaceous bedrock or Quaternary sediments. It was chosen to represent a small scarp due to the practical smaller size.</p>
d	<p>Small Scarp (in Debris)</p> <p>A small scarp, defined above to be less than 60 meters in length in plan view, within a defined debris mass. This includes any materials that have incurred any movement including creep and intact blocks that have moved. The symbol has the same shape as the intact material in order to allow the small scarps to be easily picked out. The arrow indicates that movement in that direction has occurred.</p>
e	<p>Backtilt</p> <p>This symbol represents back tilting within the debris mass. The arrows from the head scarp and the arrows from the debris meet at the base of the head scarp. The symbol used for the head scarp depends on if the material is intact or in debris and the size.</p>
f	<p>Graben</p> <p>This symbol represents the extensional feature of a graben, usually used in conjunction with Figure 7.6g. This symbol is not dependent on size and only occurs in debris, therefore the intact version was not considered. This symbol is used exclusively on translational and rotational-translational features.</p>
g	<p>Horst</p> <p>This symbol represents the extensional feature of a horst, usually used in conjunction with Figure 7.6f. This symbol is not dependent on size and only occurs in debris, therefore the intact version was not considered. This symbol is used exclusively on translational and rotational-translational features.</p>
h	<p>Debris Apron</p> <p>A debris apron is an accumulation of debris. This symbol can be used in conjunction with any of the other symbols to represent the debris of the slide. It can also be used by itself if a debris apron is observed at the base of a slope with no clear features.</p>
i	<p>Mobile Flow (Mud Flow)</p> <p>Mobile Flow is a highly saturated mass of fines moving in a fluidized manner. The scarp at the head of the failure depends on the size and material. The body of the displaced material is outlined with an arrow outlining the suspected path of flow.</p>
j	<p>Earth Flow</p> <p>Earth Flow is an intermittent flow of plastic clayey earth moving in plug-like manner. The head scarp symbol defined by the size and material found. The body of the debris is outlined. The area in which debris has been removed is represented by a concave curve, the center of the mass is a straight line, and the accumulation area is represented by a convex curve.</p>

Figure 6.15 Landslide Type Examples using Morphological Classification



7.0 LANDSLIDE INVENTORY

7.1 Objective

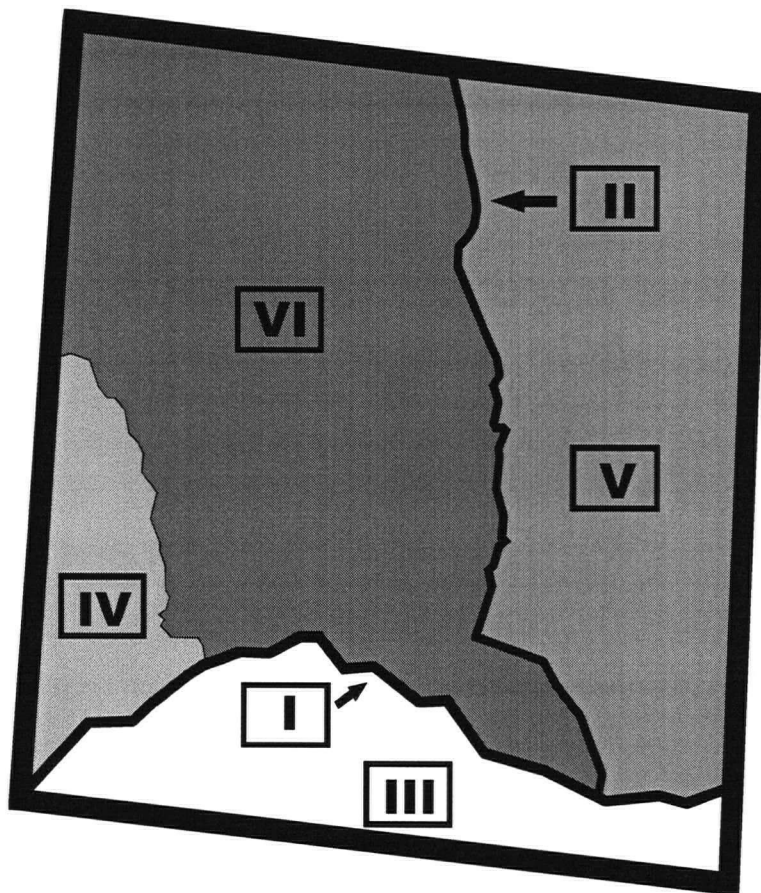
The primary focus of this thesis was to create a landslide inventory that could be used by practicing geologists and engineers working in the Peace River area. The hope is that this thesis will form the basis of the most detailed inventory of landslides in Canada, to be continuously added to and updated as more information is collected. The inventory will be implanted into a geographic information system (GIS) that is tied to a database that has been compiled during the inventory. This will hopefully become available through an open file from the Geological Survey of Canada – Terrain Sciences Division webpage. This inventory is not to be used for design purposes; however, it may give an insight to a prospective or practicing landslide investigator. Further detailed fieldwork is necessary in order for the inventory to be more reliable. The inventory is entirely based on air photo interpretation and the relatively limited field experience of two summers. The inventory and models are to provide a basis of ideas, to be added to and argued with. Due to the large size of the study area, small features on individual landslides may have been misinterpreted or missed. These features may help determine or completely change landslide type.

7.2 Referencing System

Before an inventory can be attempted, a referencing system must be in place in order to know where the slides have occurred. The landslide inventory is a linear inventory as almost all of the region's landslides occur within the current river valleys. The linear nature of the inventory requires a linear referencing system, in this case the river valleys had to be referenced. The size of the study area forced the referencing

system to be detailed and ordered in a way that is easy to reference. The system breaks down the entire area into 6 smaller, more manageable areas (Figure 7.1); 1) Peace River, 2) Beatton River, 3) Southern Tributaries, 4) Beatton River East, 5) Halfway River West, and 6) Halfway River to Beatton River. Each of these six areas was then broken down into individual rivers or large creeks. Smaller creeks are simply incorporated as tributaries to the larger creek or river numbering system. The total distance covered by this referencing system was 1292 kilometers (Table 7.1).

Figure 7.1 Reference Areas for Landslide Inventory



The Peace River division includes the Peace River and the Peace River Valley and covers 276 kilometers. The distinction is made because the current river level has created its own slope segments within the larger, Peace River Valley. No other rivers were added to this division because of the quantity of landslides and river segments within the Peace River valley. It is a large enough division without adding any other rivers as the river traverses the entire map sheet.

The Beaton River division is another that only has a single river within it. The Beaton River runs for almost the entire map sheet from the north to the south, incorporating 265 kilometers worth of river valleys. The number of tributaries to the Beaton River would make the division unmanageable.

The third division, Southern Tributaries, includes the Pine, Moberly, and Kiskatinaw Rivers. It also includes Maurice and Septimus Creeks. The bottom of the map sheet truncates all of the above-mentioned waterways except Septimus Creek. The rivers and creeks are tied together by the geographic boundary of the Peace River and were an easy target to group. The number of river segments was limited by the map sheet boundary limiting the total amount of distance covered to manageable size of 200 kilometers.

The fourth division established is Beaton River East, which incorporates the Doig, Alces, and Osborn Rivers as well as Milligan Creek. The Beaton River in the west and the Peace River to the south act as the boundaries of the area. The section encompasses 194 kilometers of river valleys, which is 15% of the whole area. Milligan and Osborn Rivers to the north, have only a few slides that are large enough to be

recognized, however, if a slide occurs, they will be able to be placed within the referencing system.

The area from the Halfway River west is the fifth area to be defined. It includes the Halfway and Cameron Rivers as well as Ground Birch and Farrell Creeks. There is 263 kilometers of River Valley within this section. This area is mostly outside of the pre-glacial valley and is underlain by the Sully Formation and not the Shaftesbury Shale (Stott, 1982). The Pleistocene sediments here are outwash sands and gravels (Mathews, 1978) so the number of failures and their types should vary in this division. It is also very easy to separate this western section as a separate unit.

The final division is made up of the rivers and creeks between the Halfway River in the west and the Beatton River in the east and north of the Peace River. The waterways include the Blueberry River and St. John, Red, and Cache Creeks. This area is upon the upper plateau and underlain by the competent Dunvegan Sandstone, so a majority of the activity in this section occurs within the pre-glacial valley boundary near the Peace and Beatton Rivers. There is only 94 kilometers (8%) of referenced river slope within this section.

Table 7.1 Measured Distance within Slope Segment Districts

Division	Peace River	Beatton River	S. Tributaries	Halfway West	Beatton East	Halfway to Beatton
Kilometers measured	276	265	200	263	194	94
% of Total	21.4	20.5	15.5	20.4	14.9	7.3

The individual rivers within the divisions were then broken up into separate river segments. This was done by examining the topographic map and breaking the slope up

into subjectively defined segments that appear homogeneous in terms of slope dimensions, stratigraphy, morphology, and activity. There were no maximum length criteria used as an entire valley wall could be classified as one slope segment as long as there were no large breaks in the wall (i.e. gullies) or distinct changes in direction or average slope angle. After both banks of each river were broken into these dissimilar slope segments, the segments were then numbered. The banks were numbered independently of each other, starting with the segments nearest to the mouth of that river. In the case of the Peace River, the numbering began in the east at the Alberta border and finished in the west at the Peace Canyon Dam. If a small creek, located along the valley wall, was made of more than two segments, it was labeled independently, with the number scheme matching that of the rivers. The left and right banks were numbered independently starting where that creek meets the larger river valley. In this manner, all slope segments within the study area were numbered independently of each other. For this exercise, the left bank is defined as the bank on the left hand side of the viewer while viewing in a downstream direction. Each river and creek was then given an identifier so that the river and the segment could easily be recognized (Table 7.2). For example, the eleventh segment on the left bank of the Beatton River is labeled as BL-11. Smaller tributaries that were large enough to be labeled separately but not large enough to have their own identifier were given a tributary identifier based on the larger river. The tributaries were also numbered starting at the mouth of the river with the left and right banks labeled separately. For example, the third segment of the right side of the sixth tributary found on the left hand side of the Peace River is labeled as PL-T6R 3.

Table 7.2. River Identifies within Charlie Lake Map Sheet

River	Identifier	River	Identifier	River	Identifier
Alces	A	Ground Birch	GB	Peace	P
Beatton	B	Halfway	H	Peace Valley	Pv
Blueberry	BB	Kiskatinaw	K	Pine	Pi
Cache Creek	Cc	Moberly	M	Red Creek	Rc
Cameron	C	Milligan Creek	Mc	Septimus Creek	Sc
Doig	D	Osborn	O	St. John Creek	SJc
Farrell Creek	Fc	North Cache Creek	NCc		

7.3 Methodology

Once a referencing system was in place, a database of landslides could be compiled. Before any landslides were identified and catalogued, a referencing system for the landslides themselves was created. This system is to be used in combination with the river slope segment referencing system, in order to locate a particular slide. The landslide format is set up similarly to the segment system and uses the same identifiers (Table 7.2). The main difference is that the numbering scheme is different. The landslides are numbered in order of their identification and not geographically oriented. This was done to allow for new slides to be placed within the inventory without using further qualifiers (e.g. BR1b). A new failure would appear as a higher number in the same slope segment. An example of this full labeling system is given for slide PiR-23 located on slope segment PiR-8

PiR-23 - PiR-8

Slide Identifier ↖ ↗ Slope Identifier

With a system of referencing the slides and their location in place, the identification of individual slides and slide complexes above a minimum size of 10,000 m² could be properly undertaken. Within the 6 previously defined divisions (Peace, Beatton, Southern Tributaries, Halfway West, East of Beatton, and Halfway to Beatton), the valley slope of each waterway was studied using high level air photographs with a scale of 1:60,000. These photos were used because they matched the 1986 Trim data, for which a complete set of maps is available at 1:20,000 scale. The photos were of good quality with only a small amount of cloud cover and good contrast between tones.

High-level photos were used in this initial interpretation to cover the large study area efficiently. Black and white photos were used to minimize the amount of noise witnessed through the stereoscope. Enlarged laser quality photocopies of the air photocopies were made and used to record the observed features. These copies were enlarged to a scale of 1:30,000, allowing for many of the smaller features to be identified. The enlarged copies were bound together and used for interpretation. The copies were useful during the analysis as they could be marked on. As features were recognized, they were marked on the copied photos with one of four coloured markers, blue, green, orange, and purple (Figure 7.2). These colours corresponded to the defined activity levels of Active, Dormant, Abandoned, and Modified. Active failures were defined by the relative freshness of the failures, marked by a distinct tone change due to the difference in vegetation. Dormant slides were those that were older with toes currently being eroded and abandoned were those that did not appear to be currently eroded. Many modified slopes were previously defined by geotechnical reports. Very Active slides were not defined using the 1986 air photos, as they had not occurred yet. Very Active slides were

defined outlined by fieldwork during the summers of 2002 and 2003. As the features were delineated on the photocopies, slide types were recognized and defined (Section 6). All failures outlined to that point were then classified and the same classification applied to all future failures outlined.

7.4 Database

During the interpretation, a set of information was developed and then recorded in a database for every failure. This information set includes: Failure Identity, Slope Segment Identity, Activity Level, Failure Type, Waterway of Occurrence, Air Photo Identity, Probable Material of Basal Shear Plane, Length, Width, Area, Aspect, and Additional Information (Appendix IV). This information was placed within a database of all failures. The waterway of occurrence is simply the river or creek on which the failure occurred. The air photo on which the failure occurred was also recorded so that the photos could be easily retrieved. The probable material of the basal shear plane was identified on the basis of the bedrock geology map created by Stott (1982), the surficial geology map created by Mathews (1978), and by field observations. Both the bedrock and the Pleistocene sequence are made of materials that are horizontally bedded within highly plastic layers. This leads to a similar morphology in landslides within both materials. The similar morphology can make the determination of the materials involved very difficult. Only the basal shear plane material was inferred, as many of the failures have multiple shear planes and only the basal plane can be estimated. The length and the width were estimated from the air photos and the area was calculated from these values. The aspect of the failures was also measured off the air photos in order to help analyze

slide type trends. The additional information category incorporates any other pertinent information.

Once the initial inventory was completed using the air photos from 1986, the data was checked against air photos from two other time periods, 1945 and 1997 at a scale of approximately 1:10,000.

This allowed the slides marked as “Active” to be cross-checked against the older photos and the “Very Active” slides to be cross-checked by the younger photos. The larger scale photos also allowed for selected sites to be examined in more detail. Unfortunately, the entire area was not checked and only partial cross-checking could be accomplished. The activity of some of the landslides was very hard to determine as the slopes have all experienced failure since deglaciation. Some of the failures appear to have undergone a slow deformation with periodic larger movements. It is possible that many of the failures are actually younger than they appear on the photographs, with small movements remaining undetected.

After the database had been updated and cross-checked by the secondary photographs, the slide outlines and features were transferred from the photocopies to the hardcopy 1986 TRIM maps (Figure 7.3). This was done manually, drawing the features on the map as they appear on the photocopied air photographs using coloured pencils corresponding to the colours of the activity classification. Once the landslides were transferred to the map sheets, they were digitized in order to create a digital version of the landslide inventory.

The landslides were digitized at a 1:20,000 scale using a Summagraphics[®] SummaSketch[®] II Professional Tablet (Model: MM II 1812) into AutoCAD[®] 2002. This

Figure 8.2 Landslide Outlines on Enlarged Photographs



Figure 8.3 Landslide Outlines on TRIM map

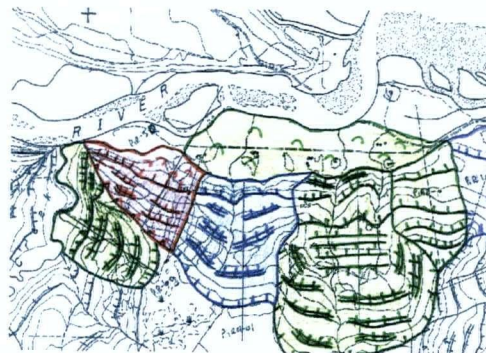
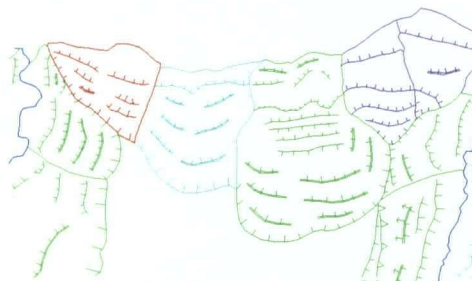


Figure 8.4 Digitized Landslide Outlines



produced a digitized version of the landslide inventory within a single file. All features and outlines were reproduced as they appear on the map sheet (Figure 7.4). The landslide locations were normalized by the Universal Transverse Mercator Projection (UTM) using North American Datum – NAD 83. Within the AutoCAD program, normalizing with UTM allows the slides exact location to be given only in UTM co-ordinates. Map boundaries and labels were also reproduced to serve as a locating device. Only the map sheets in which failures have been found to exist were included. The water way(s) in contact with the landslide were digitized as well.

7.5 Results

7.5.1 Distribution

The total number of slides identified during this inventory is 1610. The analysis of all of the slope failures is done with respect to the divisions defined during the initial stages of the inventory and as an entire area. All results are summarized in Appendices IV and V. The divisions that contain the largest number of slides are the Peace River Valley containing 465 slides, roughly 28.9% of all slides, and the Beatton River Valley containing 407, approximately 25.3%. The higher values for these two rivers are due to the length in which they occur within the pre-glacial valley limits. Within these boundaries, the presence of valley fill and rebound features within the shale bedrock are more prominent than outside of the boundaries. The rest of the slides are allocated as follows, Southern Tributaries contains 255 failures (15.9%), Halfway River West contains 216 failures (13.4%), Beatton River East contains 137 failures (8.5%), and Halfway River to Beatton River contains 130 failures (8.1%) (Table V-3; Appendix V).

As the total number of kilometers was measured during the initial investigation, an average ratio of slide per kilometer was determined for each division (Table V-3). The highest ratios of slide per kilometer are 1.82 occurring within the Peace River Valley and 1.60 within the Beatton River Valley. This ratio for the Peace River Valley is slightly underestimated as there were many slides coalescing and indistinguishable becoming long slide complexes, often persisting longer than several kilometers, especially along the south valley wall. The Beatton River ratio is also somewhat misleading as the ratio is skewed by large tracts of valley walls in the northern part of the river and bedrock canyons within the middle of the river that do not show many failures. The southern section of the river alone would have a much higher ratio. The remaining sections have the following ratios, Southern Tributaries (1.28), Halfway River West (0.82), Beatton River East (0.71) and Halfway River to Beatton River (1.38). The total failed area was also calculated for each division as well as the ratio of area per slide (Table V-3).

Even though the Peace River Valley and the Beatton River Valley have a similar number of failures, the area covered by the Peace River (258.92 km²) failures far exceeds that of the Beatton River (151.98 km²) (Table V-3). This suggests larger failures on the Peace River (0.56 km²/slide) than the Beatton River (0.37 km²/slide); likely due to pre-sheared bedding planes protruding further into the valley wall. These longer pre-sheared surfaces are likely due to the size of the post-glacial river and the relatively quicker rate of excavation. There are also a larger number of rock falls identified within the Beatton Canyons, which generally have small volumes. The Southern Tributaries division has a 0.59 km²/slide ratio with an overall area covered of 151.62 km². The failures within the Halfway River West section cover a total area of 116.79 km² with an average size of 0.54

km². The Beatton River East sector covers a total of 33.01 km² with failures and an average of 0.25 km² per slide. Sandstone bedrock canyons along the Doig and Alces Rivers control this small average failure size. The Halfway to Beatton River section has a total area of 53.13 km², which is covered by failures with an average size of 0.49 km².

7.5.2 Activity

An activity value was assigned to each slide during the investigation. This allows for the activity to be divided with respect to area, failure type, aspect, and material involved. The results are given as percentages with respect to the respective divisions and as an average through out the entire area (Table V-1; Appendix V). This relative comparison allows for separate divisions to be compared to the whole study area. The dominant activity throughout the entire map sheet is “Low Activity-Dormant” with 61.0% of all failures. This activity has a high in the Halfway to Beatton River division with 80.8%. “Very Active” failures comprise of 1.9% of all failures, or 30 landslides. The area with the highest activity is the Beatton River Valley with 4.2% of all failures in the division being “Very Active” and over half of all of the “Very Active” failures in the region. “Active” failures comprise 12.9% of all failures, with the Beatton River East (16.8%), Halfway to Beatton River (15.4%), and Beatton River Valley (15.2%) with higher than average active failures. Combining “Active” and “Very Active” failures together, allows for the analysis of every slide that has failed within the past 50 years. These activities together comprise 14.7% of all failures in the area, making the Peace Region one of the most active areas in British Columbia, over that time period with 237 landslides. “Low Activity-Abandoned” failures include 22.9% of the areas landslides,

with a high of 40.7% in the Halfway River West division. There are 23 “Modified” slides over the area with a majority of that (12) with the Peace River Valley.

The area for each activity level was calculated and compared within the division and throughout the entire study area (Table V-7; Appendix V). The area distribution of the landslides is similar to the location of the slides with respect to activity with Activity 3, “Low Activity-Dormant” dominating with 58.6% and “Low Activity-Abandoned” with 30.7%. The increase with respect to “Low Activity-Abandoned” suggests that the average size of failures is decreasing. This is supported by the “Active” and “Very Active” failures representing smaller areas as a percentage than in number as a percentage. This indicates that the active failures are smaller than the failures with a lower activity. “Modified” slides are represented by a similar percentage in size and number, 1.5%.

The failure type for each activity was determined and compared within the division and throughout the entire study area (Table V-9; Appendix V). “Very Active” slides are dominantly multi-leveled failures, which consist of 73.3% of that activity level. The number of type 6 failures along the Beaton and Peace River Valleys heavily influences this number. “Active” failures are also dominated by multi-leveled failures, 43.0%, however, there is a larger component of rotational, 20.8%, and retrogressive rotational failures, 23.7%, which resembles the values for the overall type values. The values slightly change when analyzing “Low Activity-Dormant” failures as the percentage of retrogressive failures actually falls to 12.6%, and Compound failures rises to 7.3%. Multi-leveled failures and rotational failures consistently comprise of a large percentage of the failures at 48.8% and 25.8%. “Low Activity-Abandoned” failures show a similar distribution of failure types with 44.0% being multi-rotational landslides, and

33.4% being rotational. The value for rotational failures is likely inflated by the conservative classification of a deep-shear plane. A large majority of multi-level failures exists within the activity 5 grouping, "Modified" with a total of 17 failures, which represents 73.9% of this activity. There are no strong trends within this analysis, therefore, it cannot be said that failure type is changing with time. It can however be said that retrogressive rotational failures and multi-level failures are harder to recognize as the features become muted, thus being classified as a rotational failure.

The slope aspect for each activity level was determined and compared within the division and throughout the entire study area (Table V-11; Appendix V) using the groupings described in 8.4.5. 30% of all "Very Active" landslides had a northern aspect, which is twice the average and corresponds to 9 failures. This is opposite to the over all trend of landslides with respect to aspect, which showed a higher number of failures on the south-facing slopes. "Active" slides are fairly evenly distributed with slightly higher values of south-facing, 17.9%, and east-facing, 18.4%, slopes. "Low Activity-Dormant" failures and "Low Activity-Abandoned" failures follow the overall trend displaying a higher proportion of south-facing slopes with both activity levels displaying 19.3% of their landslides having a southern aspect. "Modified" failures are generally on the south-facing valley walls, 26.1%, as a majority of the infrastructure runs along south-facing slopes. This value is influenced far more by location of infrastructure than landslide activity.

The material, suspected or known, containing the basal shear plane for each activity level was determined by geologic maps and field investigations. The values were then compared within the division and throughout the entire study area (Table V-10,

Appendix V). “Very Active” slides are predominantly within the Quaternary sediments, with 73.3% of failures of this activity. Most of these failures, 63.6%, occur within the Beatton River Valley, north of the Sandstone canyons. “Active” failures are more evenly distributed between the materials, with 43.5% of the slides within the shale bedrock, and 56.5% in the pre-glacial sediments. The dominant activity, “Low Activity-Dormant”, displays an even distribution of materials, with 47.2% of failures in shale and 52.8% in the Quaternary materials. Two exceptions to this average are the Southern Tributaries, with 66.4% of failures in shale, and Beatton River East, with 64.9% of failures in shale. Of the “Low Activity-Abandoned” failures, 62.8% of the failures are within the Pleistocene sequence, while only 37.2% are in the shale bedrock. The same divisions, Southern Tributaries and Beatton River East, show a dominance of bedrock failures. “Modified” failures occur evenly within the shale, 43.5%, and the silt, sand, and clay, 56.5%.

7.5.3 Failure Type

The failure type of each slide was assigned during the investigation. Each failure type was then divided with respect to area, activity, aspect, and material involved. The results are given as percentages with respect to the respective divisions and as an average through out the entire area (Table V-2, Appendix V). This relative comparison allows for separate divisions to be compared to the whole study area. The dominant failure type within the Charlie Lake map sheet (NTS 94A) is the multi-level failure with 47.8% of all landslides, which corresponds to 769 slides. The Peace River and Beatton River Valleys display higher values than average for this type of failure. This number may be slightly underestimated as slides in which the mode of failure is uncertain are conservatively

designated as a deep-seated failure, either single rotational or retrogressive rotational landslide. Conversely, the values of the deep-seated single rotational and retrogressive failures may be bolstered by this conservatism with values of 26.3% and 13.0% respectively. The Halfway River West division has a very high component of single rotational failures, 44.9%, due to the homogenous nature of the pre-glacial material in the area. Compound slides with one shear plane are fairly uncommon in the Peace Region due to the low number of weak shear planes available for failure. Compound failures comprise 6.5% of all failures with a high of 15.4% within the Halfway to Beatton River section. The remaining failures make up a total of 6.4%, with raveling at 0.6%, mudflows at 2.1%, earth flows at 2.4%, and shallow retrogressive failures at 1.4%.

The area for each failure type was also calculated and compared within the division and throughout the entire study area (Table V-8; Appendix V). As expected from above, the failure type, which covers the most area, is the multi-leveled failure with 53.1%. Even though compound failures do not represent many failures in terms of number, they do represent a larger number in terms of area, 13.8%. This suggests that compound failures are larger than other failures, which is supported by the average size calculated for each type of failure. The average size for a compound failure is 1.0 km²/slide, whereas the average size of the next largest type slides, earth flows, is 0.61 km², and multi-leveled failures with an average size of 0.53 km².

The activity levels for each failure type was determined and compared within the division and throughout the entire study area (Table V-12, Appendix V). Most retrogressive rotational failures are "Low Activity-Dormant" representing 59.1%, of all slides of this type, while 23.3% are "Active". Rotational failures are also have a dominant

activity of “Low Activity-Dormant” at 59.8% with the “Low Activity-Abandoned” activity representing 29.1%. Multi-level failures have a higher than average amount of “Very Active” failures at 2.9%, however, the failure type is also dominated by the “Low Activity-Dormant” activity level, 62.3%. Compound failures have a very high component of the “Low Activity-Dormant” activity level at 68.8%, with 21.0% represented by “Low Activity-Abandoned” failures. There are very few active compound failures.

The slope aspect for each failure type was determined and compared within the division and throughout the entire study area (Table V-14; Appendix V) using the groupings described previously. Raveling occurs with all aspects, although more have been identified on south-facing scarps, 30.0%, than other aspect groups. Of the aspect groups analyzed, retrogressive rotational failures occur at a higher frequency on the east-facing slopes, 19.5%, and west-facing slopes, 19.5%, than on the north-facing slopes and south-facing slopes. Rotational failures occur more often on south-facing slopes, 20.8%, than on the other aspect groups examined. Both mobile mudflows and earth flows are also dominantly on the south facing slopes, with 27.7% and 21.1% respectively. South-facing slopes have more multi-leveled failures, 19.9%, than the other aspect groups. Of the compound failures within the study area, 31.4% of them occur on north-facing slopes, with only 6.7% occur on north-facing slopes. This may in fact be due to the regional dip of the bedrock towards the northeast. 31/8% of all shallow retrogressive failures occur on west-facing slopes, mostly within the western portion of the map sheet.

The material suspected or known to contain the basal shear plane for each failure type was determined by geologic maps and field investigations and compared within the division and throughout the entire study area (Table V-13, Appendix V). Mobile

mudflows, earth flows, and shallow retrogressive failures are all completely within the Quaternary materials or colluvium derived from these sediments. The dominant failure type, multi-leveled failures, is evenly distributed between the shale, 51.9% and the silt, sand, and clay, 48.1%. The Southern Tributaries and Beatton River East divisions heavily influence the overall average, with values of 90.7% and 82.1% respectively. Compound failures are dominantly within the shale bedrock, 61.9%, especially within the Peace River Valley, 79.3%, and the Halfway to Beatton River section, 75.0%. Single rotational failures are mainly found within the Quaternary sequence, 64.8%, heavily influenced by the Peace and Beatton River Valleys. Retrogressive rotational failures have basal shear planes with in both materials evenly with shale containing 41.4% and silt, sand, and clay with 58.6%.

7.5.4 Material Involved

Each slide was assigned a material suspected or known to be involved with the basal shear plane during the desk study using geologic and surficial maps and field investigation. This allows for the materials to be divided with respect to activity, failure type, and aspect. The results are given as percentages with respect to the respective divisions and as an average through out the entire area (Table V-4, Appendix V). This relative comparison allows for separate divisions to be compared to the whole study area. Both the consolidated shale bedrock and the pre-glacial sediments have a propensity for failure as observed by the distribution of materials involved in failure. Shale represents 43.7% of material in which the basal shear plane is located, while the Pleistocene sequence represents 56.3%. This equal distribution is one of the reasons why landslide investigation is so difficult in the Peace River Region. The material in which the failures

shear plane is not witnessed can be difficult to determine. During this investigation, surficial geology maps and bedrock maps were consulted along with field investigation, but often what is needed is geotechnical drilling. Divisions that have a higher percentage of shale failures are Southern Tributaries with 65.3% and Beatton River East with 60.6%. Quaternary failures dominate the Beatton River Valley, 65.1%, particularly in the northern section, Peace River Valley, 60.4%, Halfway River West, 65.7%, and Halfway to Beatton River, 56.9%.

The activity for each basal shear plane material was determined and compared within the division and throughout the entire study area (Table –15, Appendix V). Mirroring the overall activity, both the shale and the Quaternary sequence are dominated by “Low Activity-Dormant” failures. The value for shale failures that are dormant is slightly higher, with 65.9% than the silts, sand, and clay at 57.5%. Shale and Quaternary failures show similar amounts of “Active” and “Modified” failures, and Quaternary failures have a higher percentage of active failures at 2.4% compared to 1.1% within the shale.

The failure type for each material involved was determined and compared within the division and throughout the entire study area (Table V-16, Appendix V). Multi-rotational failures make up 56.5% of all shale failures, with rotational failures and retrogressed rotational failures comprising 21.1% and 12.4% respectively. Failures in shale have a much higher component of compound failures with 9.3%, over double of the amount within the Pleistocene sediments, 4.4%. The distribution of failures in the silt, sand, and clay is more widely distributed as mudflows, earth flows, and shallow retrogressive failures occur within the sequence. The dominant failure type within the

surficial material is multi-leveled failures as well, with 40.7%. The unconsolidated sediments have a higher percentage of rotational failures, 30.3%, and retrogressed rotational failures, 13.6%. The Halfway River West division shows a large component of rotational failures within both of the bedrock, 48.6%, and the surficial materials, 43.0%.

The slope aspect for shale and surficial failures was determined and compared within the division and throughout the entire study area (Table V-17; Appendix V) using the above groupings. This was done primarily to observe the difference between aspects in the Cretaceous bedrock, as it has a slight regional dip towards the northeast. The shale bedrock does show slightly more failures in the south facing and west facing slopes with 18.3% and 17.9% respectively. Conversely, the north-facing and east-facing slopes have below average values at 13.8% and 14.9%, respectively. To understand this, the analysis of type and aspect has to be revisited, as there were a large number of compound failures on the north and east facing slopes. Also, if the analysis of area and type is re-examined, then it can be seen that the average size of compound slides is much larger than others. Therefore, the lower number of shale failures facing the north and east is due to larger compound failures that occur in that direction. Overall, there appear to more failures on the south and east facing slopes, likely related to the size of failures.

7.5.5 Slide Aspect

A slide aspect was determined for each slide using maps and field investigation during the inventory. These aspects were then grouped into north-facing slopes (330° - 30°), east-facing slopes (60° - 120°), south-facing slopes (150° - 210°), and west-facing slopes (240° - 300°). This allows for the aspect to be divided with respect to activity, failure type, and material involved. The results are given as percentages with respect to

the respective divisions and as an average through out the entire area (Table V-5, -6; Appendix V). This relative comparison allows for separate divisions to be compared to the whole study area. If landslides occurred in every direction equally, the average value of occurrence would be 16.7% as the aspects were assembled into 60⁰ groups. Therefore, aspect groupings greater than this average have a higher density of failures. These grouping are the east, south, and south-east facing slopes, the highest of which is the south-facing group at 19.1%. The north-facing slopes have a smaller percentage due to the number of failures that coalesce to form complexes.

The activity levels for the aspect groupings were established and compared within the division and throughout the entire study area (Table V-18; Appendix V). Of the north-facing failures, 56.6% are "Low Activity-Dormant", 26.1% are "Low Activity-Abandoned", 12.4% are "Active" and 4.0% are "Very Active". This aspect grouping has a relatively high percentage of "Very Active" failures, when compared to other aspects. East-facing slopes have a higher percentage of "Low Activity-Dormant" slides, with 66.5%, as well as a higher percentage of "Active" failures at 15.3%. South-facing failures also have a high component of "Low Activity-Dormant" failures with 62.4% as well as a high percentage of "Low Activity-Abandoned" failures at 23.4%. West-facing slopes have a similar distribution with respect to activity with 63.6% of the failures being "Low Activity-Dormant" and 20.1% being "Low Activity-Abandoned". No obvious trends of the slide aspect exist with respect to activity.

Failure types for each of the aspect groupings were established and compared within the division and throughout the entire study area (Table V-19; Appendix V). For north-facing slopes, the majority of the failures are multi-leveled failures making up

48.1%, with the conservative value of 19.5% for rotational failures. Compound failures comprise 14.3% of the north-facing slopes, while they only make up 2.2% of the south-facing slopes. Multi-leveled failures account for 47.3% of the south-facing slopes with rotational failures making up 27.8%. East-facing failures have a high percentage of shallow retrogressive failures, 13.2% and conversely a relatively very low percentage of multi-leveled failures, 37.5%. West-facing slopes have a high number of multi-leveled failures, 45.1%, and rotational failures, 27.8%. The trend seen from this analysis is that there are more compound failures on north-facing slopes, whereas all other aspects seem to be fairly evenly distributed between types with higher values belonging to multi-leveled failures.

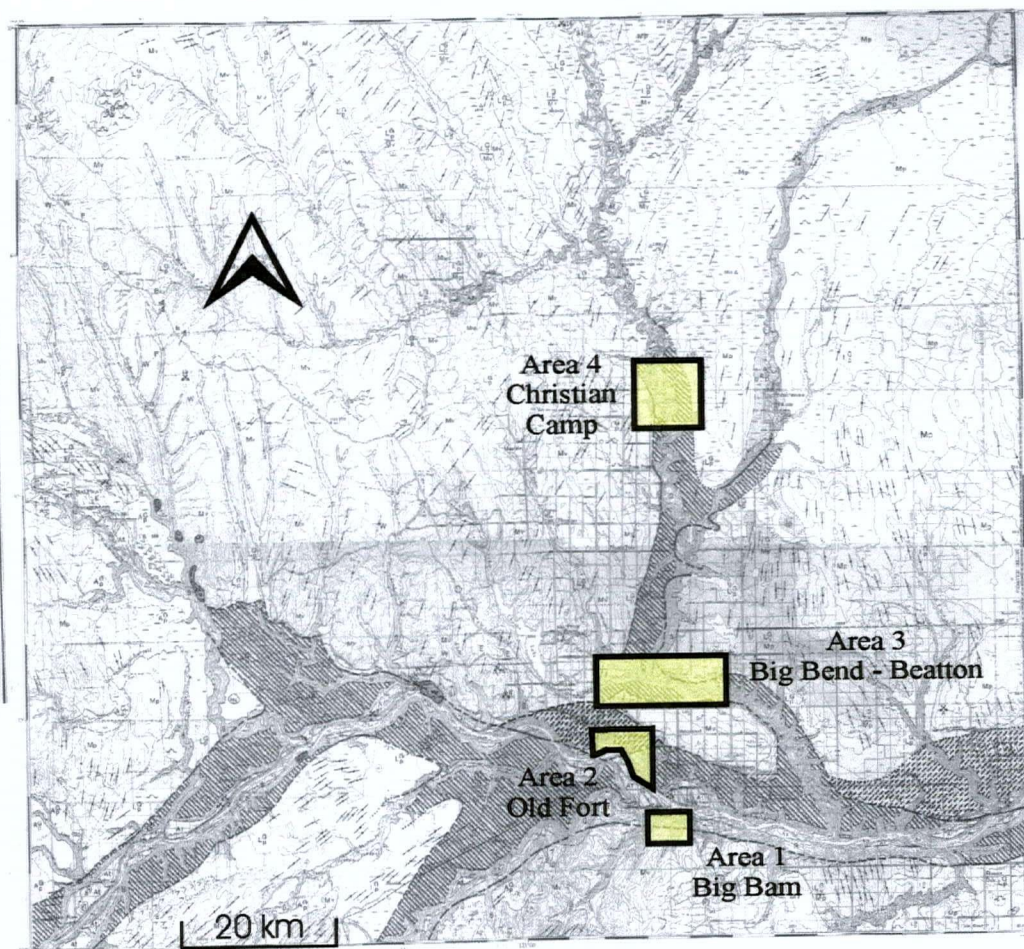
The material suspected or known to contain the basal shear plane for each aspect grouping was determined by geologic maps and field investigations and compared within the division and throughout the entire study area (Table V-20; Appendix V). All of the aspect groupings have a fairly even distribution of materials, with the Quaternary materials being slightly higher in each case. This mirrors the overall allocation of materials involved in basal shear planes. 55.8% of all north-facing failures are within the Quaternary materials, dominated by the Beatton River Valley, 72.3%, and the Halfway River West division with 91.7%. South-facing failures are similarly distributed with 58.0% of all failures within the silt, sand, and clay. East-facing failures have the Pleistocene sequence involved in 57.3%, while west-facing failures are slightly lower at 51.9%. There are no distinct trends involving the material with respect to the aspect of the slide.

8.0 DETAILED INVESTIGATION OF SELECTED LANDSLIDES AREAS

For a directed field study, four areas were chosen to be investigated in detail (Figure 8.1). These areas were named : 1) Big Bam Ski Area, 2) Old Fort, 3) Beaton – Big Bend, and 4) Christian Camp. These areas were chosen due to their importance to the area, both sociologically and scientifically, the variety of materials within these sites, the variety of landslide types, their accessibility, and their proximity to the city of Fort St. John.

Each area is described in detail including location, stratigraphy, and profiled landslides. A generalized stratigraphy was created from exposures mapped during the field season and published geologic information on the area. Each slide was investigated in detail with a description and an interpretation within this section. Profiles were conducted using an inclinometer, a range finder, and a 50m measuring tape during the summer of 2003. Given the stratigraphy and the interpreted mechanisms of the landslides, a generalized geologic model was produced to better explain slope movements within that area. These models were then considered at the scale of the map sheet in an attempt to explain landsliding in the Fort St. John area.

Figure 8.1 Detailed Landslide Study Areas



8.1 Slide Area 1

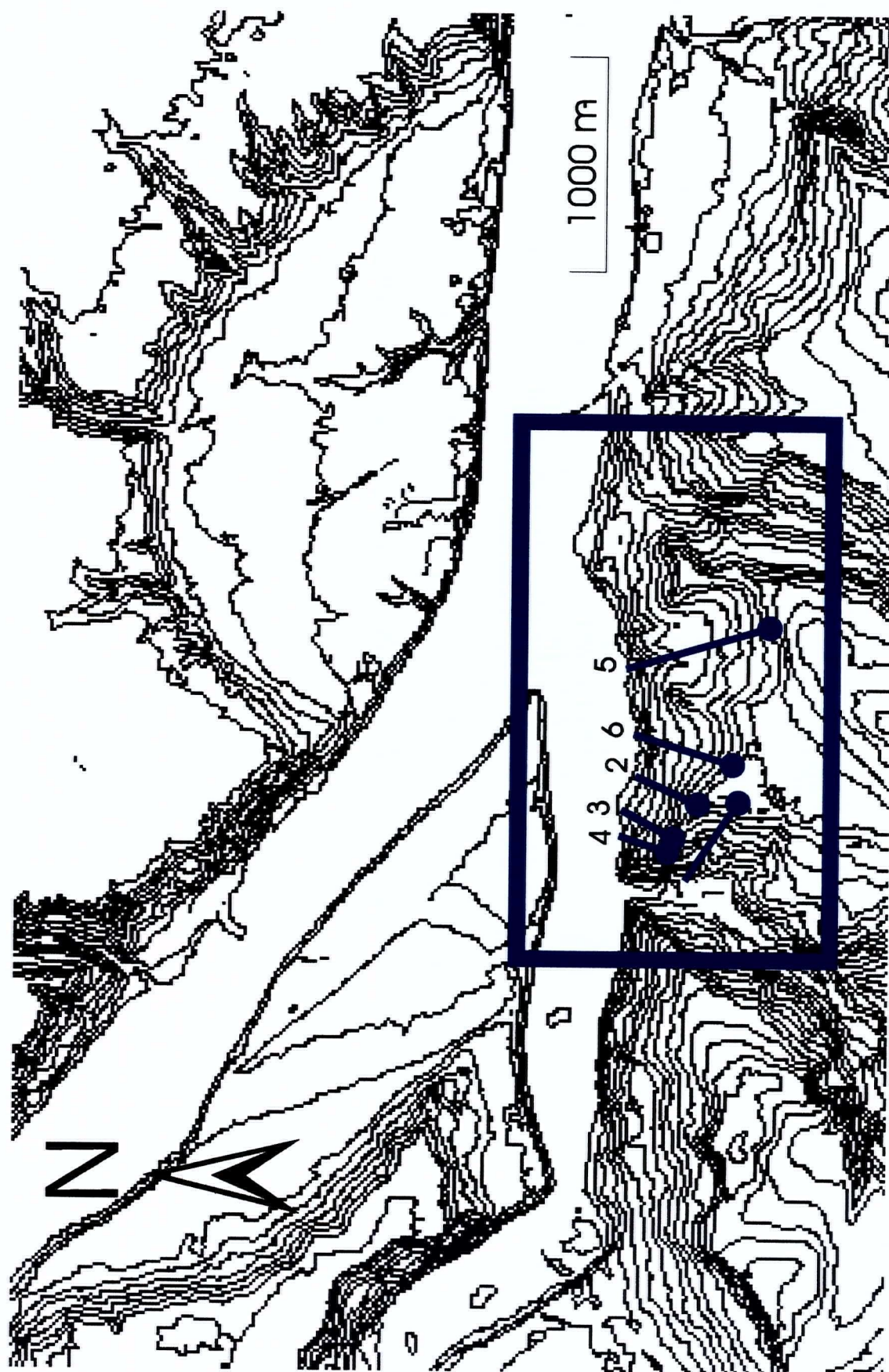
8.1.1 Location

Six slopes were profiled near the old Big Bam Ski Hill approximately 4 kilometers south west of the town of Taylor on the south bank of the Pine River at the confluence with the Peace River (Figure 8.2). The area extends for 3.5 km along the north-facing slope and encompasses the ski hill, part of Septimus Creek, and an abandoned residential area. At the upper elevations, the head scarps of these slides border on a farm field. Much of the area is overgrown except for where the younger landslides have a disturbed surface and where the vegetation has been cleared for the ski hill paths. The ski hill was in operation as late as mid-1996 (Zandbergen, 1999). The area was chosen due to the relative easy accessibility and it's proximity to a highly active, much-discussed area (Hardy, 1952; Maber and Stewart, 1976). The area is also a very distinct landmark with in the Peace River area as many local residents still use the area for recreation purposes such as horseback and ATV riding.

8.1.2 Local Stratigraphy

This site is within the pre-glacial valley (Mathews, 1978) and is underlain by Shaftesbury Shale. The shale bedrock elevation has been noted at three slides (Slides 1-1, 1-5, 1-6) at a constant elevation of approximately 397 meters a.s.l. with a small variation of 0.3 meters. Bedrock was seen in near vertical exposures mirroring the steep sided bedrock exposure on the opposite side of the Pine River. This angle likely persists to the depth of the current river.

Figure 8.2 Landslide Area 1

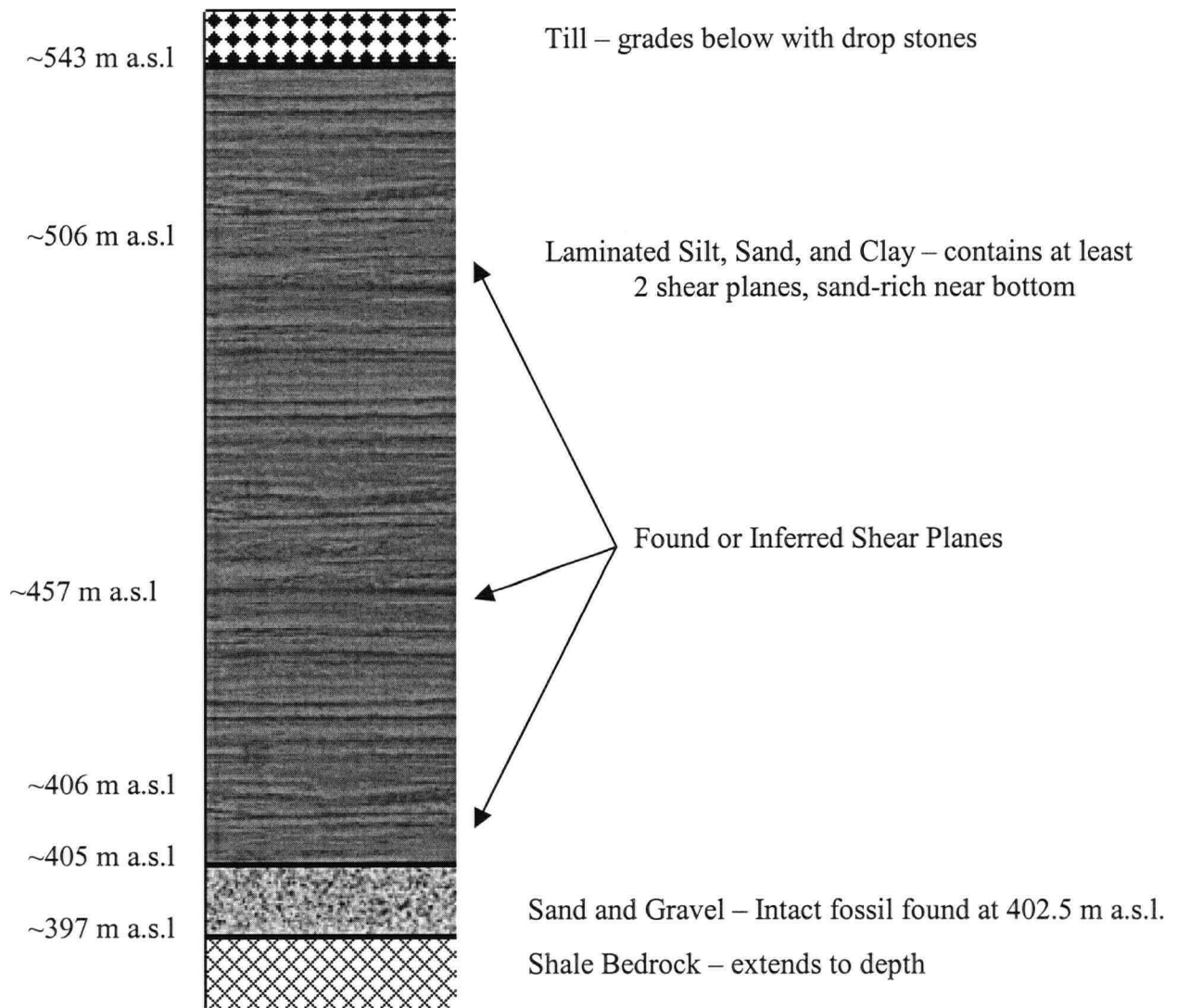


Above the bedrock lies a bed of Pleistocene sand and gravel with a fairly constant thickness of 8.5 meters with a variation of approximately meter to an elevation of 405 meters a.s.l. The sand and gravels were noted at three of the six profiles completed (Slides 1-1, 1-5, 1-6). The unit was lightly cemented and iron stained and the well-rounded gravel ranged in size from 4 cm to 12 cm and contained small lenses of sand 20 cm thick. A fossil was found intact within these gravels on Slide 1-6 at 402.5 a.s.l. (Figure 8.6).

Above the gravels lies a thin bed of sand seen in 2 of the profiles. In the west (Slide 1-1), the bed has a thickness 0.2 m and in the east (Slide 1-6) the bed is 0.1m thick. In the two profiles where this unit was seen directly above the gravels, the beds do not show the same facies suggesting the pre-glacial sand, silt, and clay unit is variable at the base. A different section of this unit was seen in Slide 1-4 at 446 m a.s.l. The section was completely dominated by weakly bedded sand with a thickness of 15 m. No shear plane could be found at this locale above the sand although one is inferred to be at approximately 457 m a.s.l. Another intact section of weakly bedded sand was observed in the same profile (Slide 1-4) at 489 m a.s.l. The sand unit was 16.5 m thick extending to an elevation of 505.5 m a.s.l. Above the sand is a 4 cm bed of massive clay (sample 1-4B) followed by a 4 cm bed of laminated silt and clay. Directly above the clay and silt beds is a 1 mm thick softened clay shear plane (sample 1-4A) in which highly polished shear surfaces were observed. Above the shear plane was massive debris made mostly of sand with some silt and clay. These same beds of sand-rich deposits were observed to the west (Slide 1-3) at 493.6 meters a.s.l. This exposure became silt rich near the top (522.6 m a.s.l.) with frequent interbeds of clay. At the top of this unit is a bed of silt with drop

stones that resembles till seen at an elevation of 543 m a.s.l. Lake Peace clay and till were not witnessed at this location; however, the silt near the top of Slide 1-2 and Slide 1-3 appears heavily disturbed and could be near the upper limit of the pre-glacial unit. The presence of drop stones suggests a close proximity of ice when deposited. The top 10 metres very well may be till, however, it was difficult to tell the till from the underlying strata. An approximate stratigraphic column is produced (Figure 8.3).

Figure 8.3 Area 1 – Stratigraphic Profile (not to scale)



8.1.3 Landslides

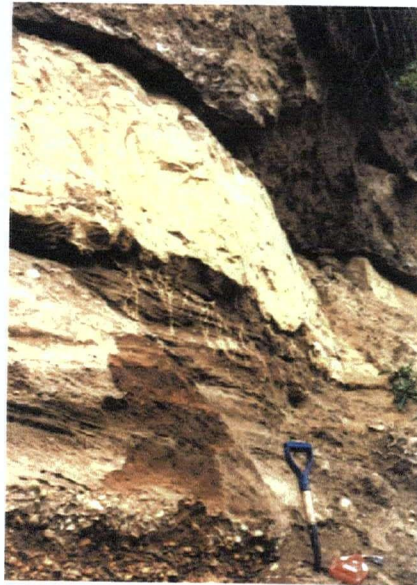
Slide 1-1

Slide 1-1 is on a northwest-facing slope, within the Septimus Creek valley (Figure 8.7). The profile starts within a field and continues down slope at an azimuth of 290° . Using the previously defined activity classification, the upper section is active showing some fresh features, the middle section is dormant, and the toe is very active. The slide contains three failure plains. The upper two planes were both covered with debris and interpreted from surface morphology. Near the head scarp, the features are generally small suggesting that the failure planes are shallow instead of one deep plane. There are minor amounts of back tilt suggesting that the failures are compound. Small mobilized debris flows occurred near the head of the slide, obliquely cutting the profile. A rounded diapiric structure was observed 50 meters off the profile to the east for which an explanation is still not clear.

Then there is a lengthy section with a disturbed hummocky surface with a small overall gradient. The area is thought to be a compound section because of the muted horst and graben features witnessed during the traverse. At the end of this section a remobilization of the debris has occurred which is expressed by major scarp in debris. A shear plane was examined at 407 m a.s.l., 20 m to the northwest. This is thought to be the same failure surface controlling the profiled failure. A sample of this shear plane was sampled and found to be a clayey silt of low plasticity with a silt fraction of 61. Figure 8.4 shows the pre-glacial stratigraphy directly below the shear plane. Below the debris, intact rubbly shale was observed. The bedrock surface likely drops at a steep angle to the level of the current creek elevation, as this was likely a cut bank before the slope above it

failed. The debris at the creek level is remobilized debris activated by erosion from the present creek. Downstream of this failure on the opposite bank, a failure within the shale mass was witnessed at 10 metres above the creek bed.

Figure 8.4 Intact Pre-glacial Stratigraphy (Slide 1-1)



Slide 1-2

The second profile in the area was done along the tower line of the Big Bam Ski Hill that failed in 1997 closing operations permanently (Figure 8.5). The profile started near a farmer's field and continued down the north-facing slope on an azimuth of 345° . There were no exposures of shear planes or intact materials found during the traverse, however, many of the features can be interpreted from surface morphology and comparisons with nearby slopes. The slide is understood as having three failure planes, all within the pre-glacial sand, silt, and clay unit (Figure 8.8). During the upper part of the failure, the features were small indicating that the failure planes were relatively shallow.

There was enough displacement in the upper portion of the failure to force many of the ski towers to fall over or tilt, however, the movement appeared to have been compound with little rotational component. Further down slope, a large horst was observed, suggesting a new failure surface much deeper with little or no rotational displacement being observed. The bedrock is suspected to underlie the deep compound failure and then sharply dropping off causing debris to slide over and producing a steeper slope within the debris. The toe of the slide is remobilized debris.

Figure 8.5 Former Big Bam Ski Hill Tower Line (Slide 1-2)



Slide 1-3

This active slide is at the western edge of the Big Bam ski area and has a north-facing slope. The profile starts on a ridge caused by an abutting failure on Septimus Creek. The profile goes down slope at an azimuth of 7^0 . The failure consists of two successive compound slides within the pre-glacial sand, silt, and clay unit (Figure 8.9).

No shear planes or intact sections were observed as they were covered by debris, but rather inferred by slope morphology. Both failures show a definite component of back tilting and the resultant ponding of water as well as horst and graben structures. Intact slopes underneath the debris are expressed as steeper slopes within the profile. The start of the profile is at the exposed head scarp of till above bedded silt-rich pre-glacial silts of a rotational component of the failure. The two failure plane hypothesis is supported by the small nature of most of the features seen on the slide. Locally, the bedrock is inferred from neighbouring slides, bedrock elevation across the Pine River, and that the site is completely within the pre-glacial valley (Mathews, 1978). The toe is made of remobilized debris that is heavily treed and shows small-scale back tilting and compound features.

Slide 1-4

Slide 1-4 is an active failure consisting of three failure planes within the pre-glacial sand, silt, and clay unit (Figure 8.10). It is located in between Slides 1-2 and 1-3 and also has a north-facing slope. The profile started on an undisturbed surface near a field at the top of the failure and traversed at an azimuth of 0^0 to the toe. The head scarp of the slide is made entirely of till and the debris near the top was slightly back tilted suggesting a rotational component to the failure. The upper shear plane was observed and sampled (1-4A) during the traverse, however, the lower two failures are an interpretation. The upper failure plane lies directly above a massive intact sand bed with minor interbeds of silt and clay. The shear plane has a clay fraction of 38 with a liquid limit of 51% and a plasticity index of 33%.

The middle failure shows back tilting and ponding so it is also considered a compound failure. The debris of this failure had small features suggesting the failure

surface is shallow. Below this debris, another intact section of sand with clay beds was observed, however the shear surface above it could not be found. One of the intact beds of clay was determined to be mostly silt at 72%, with a liquid limit of 41% and a plasticity index of 24%. The horst and graben features within the debris mass of the lowest failure below the second intact bed of sand were also small so a third, shallow failure plane is inferred here. The bedrock surface is again assumed from other profiles to be below this failure plane forcing the debris to slide and remobilize below. An interesting element of this slide was a series of laterally extending ridges within the remobilized debris, which appeared to be compressional features as if the debris is being loaded on to and pushed toward the river.

Slide 1-5

This failure is at the eastern end of the area and has a north-facing slope. The slide is dormant with three shear planes within the pre-glacial unit (Figure 8.11). The upper shear planes are interpreted from slide morphology and the lower shear was seen although not accessible. The profile starts in a farmer's field at an azimuth of 323° and continues down slope to an abandoned property. Within the field, a small crack was observed which might be a tension crack, however with the amount of seasonal modification caused by plowing of the field, features within a field can be deceptive. The head scarp for the upper failure is shallow at 23.5 and all the features are overgrown and muted. This is likely a compound failure with a shallow failure plane with little to no rotational movement. The trees on the slope are J-shaped and have an estimated age of 60 to 80 years. Wide scale ponding marks the head of the next compound failure and represents subsidence of a graben. Several large horst and graben structures were

observed representing a deep compound failure. The third failure plane is at a very low angle and is quite long, roughly 9000 meters, and is represented by large-scale horst and graben features and hummocky terrain. The toe of this section is remobilized and shows a rotational failure. The shear plane was covered along the traverse but it was detected in a ravine next to the profile, making sampling impossible. Gravel was also seen at this section as well as the intact shale below. Elevations were taken obliquely off the profile. The bedrock surface dips steeply toward the current river level. The debris below is remobilized and highly disturbed. The slide surface was modified at the toe due to the development of a residence.

Slide 1-6

Slide 1-6 is located just west of Slide 1-5 and is very similar to it (Figure 8.12). It is dormant near the top of the slide and active at the toe and within the debris apron. The profile starts within the same farmers' field and goes down slope at an azimuth of 330° towards an abandoned property. There are three shear planes within the pre-glacial sand, silt, and clay. The two upper failure surfaces are inferred from Slide 1-6 and surface morphology. These are shallow compound slides in a successive manner with small horst and graben features along with shallow head scarps. There is little to no back tilt, however, trees with ages up 60 years are J-shaped suggesting that this failure is a continuous slow movement. The lower failure is a long, deep compound failure interpreted from the large horst and graben features. The toe is actually failing with fresh scarps in a rotational manner. The toe is very hummocky and disturbed with a gully obliquely cutting the profile exposing the lower gravels. In the bank of the gully, a fossil was found intact within the gravels, location marked in Figure 8.6. The fossil has

tentatively been identified as a Pleistocene bison's 4th cervical vertebra and was dated as being $26,530 \pm 320$ years BP (Hartman, In Progress). Off the debris on the other side of the gully, intact gravels and sands were found, however, no shear plane could be located. A smaller bone fragment was also found at this location. Below the gravels, intact rubbly shale was detected to dip steeply into the slope, likely to the present river level. The toe was made of remobilized debris. Previous development caused the slope to be modified. This property was to be later abandoned due to landslide hazard.

Figure 8.6 Bison Vertebrae found in place (Slide 1-6)



Figure 8.7 Landslide Profile 1-1

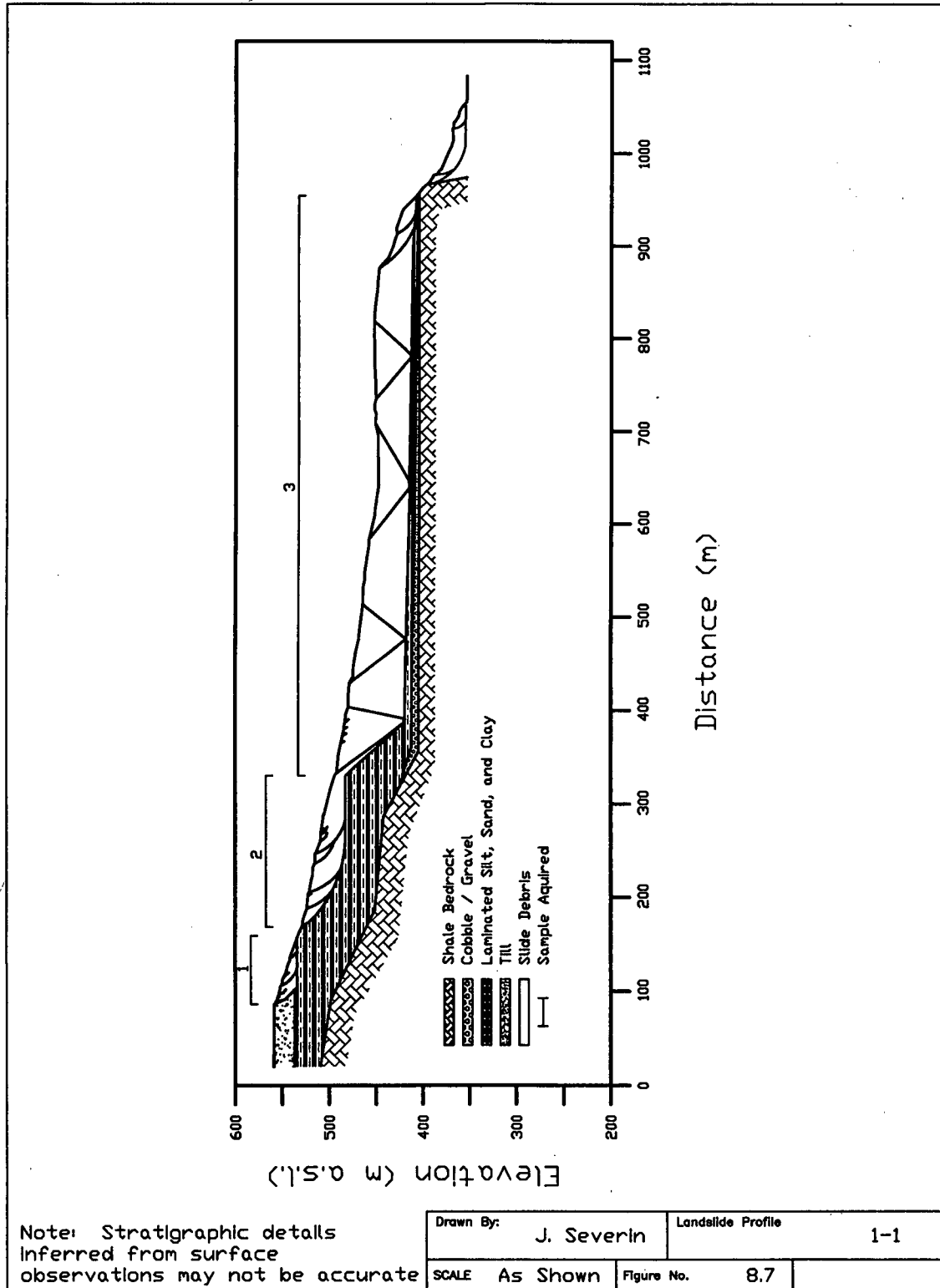


Figure 8.8 Landslide Profile 1-2

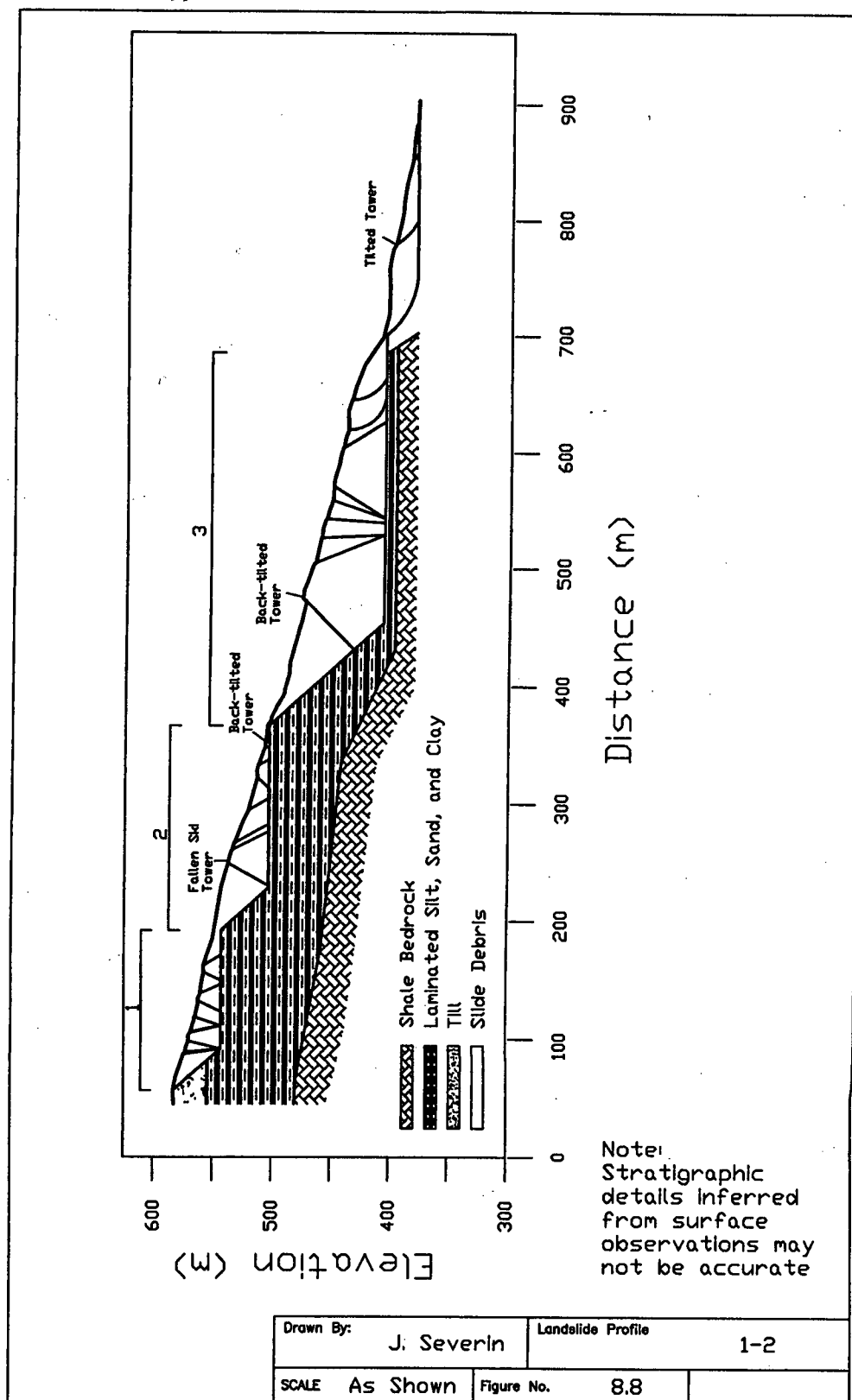


Figure 8.9 Landslide Profile 1-3

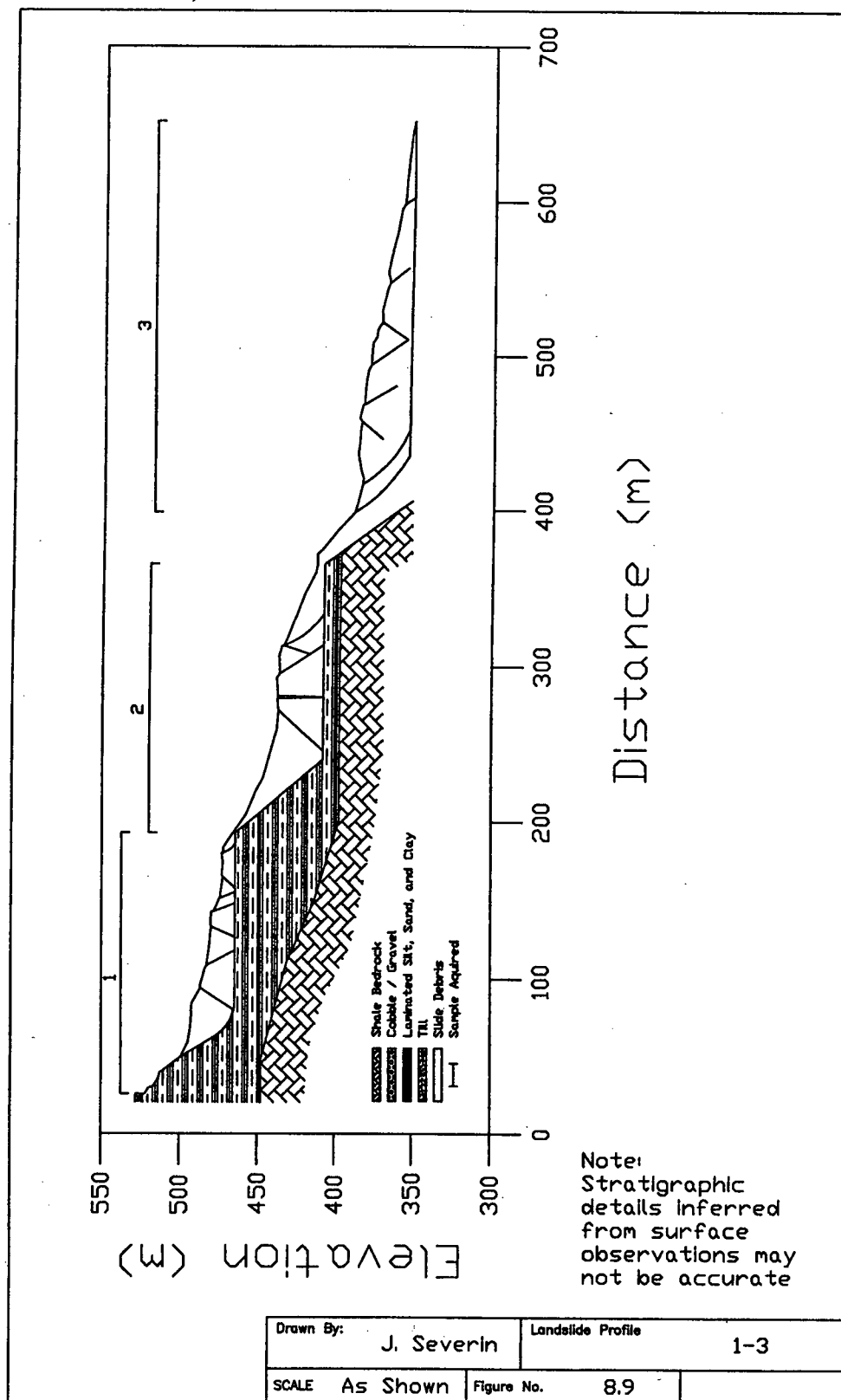


Figure 8.10 Landslide Profile 1-4

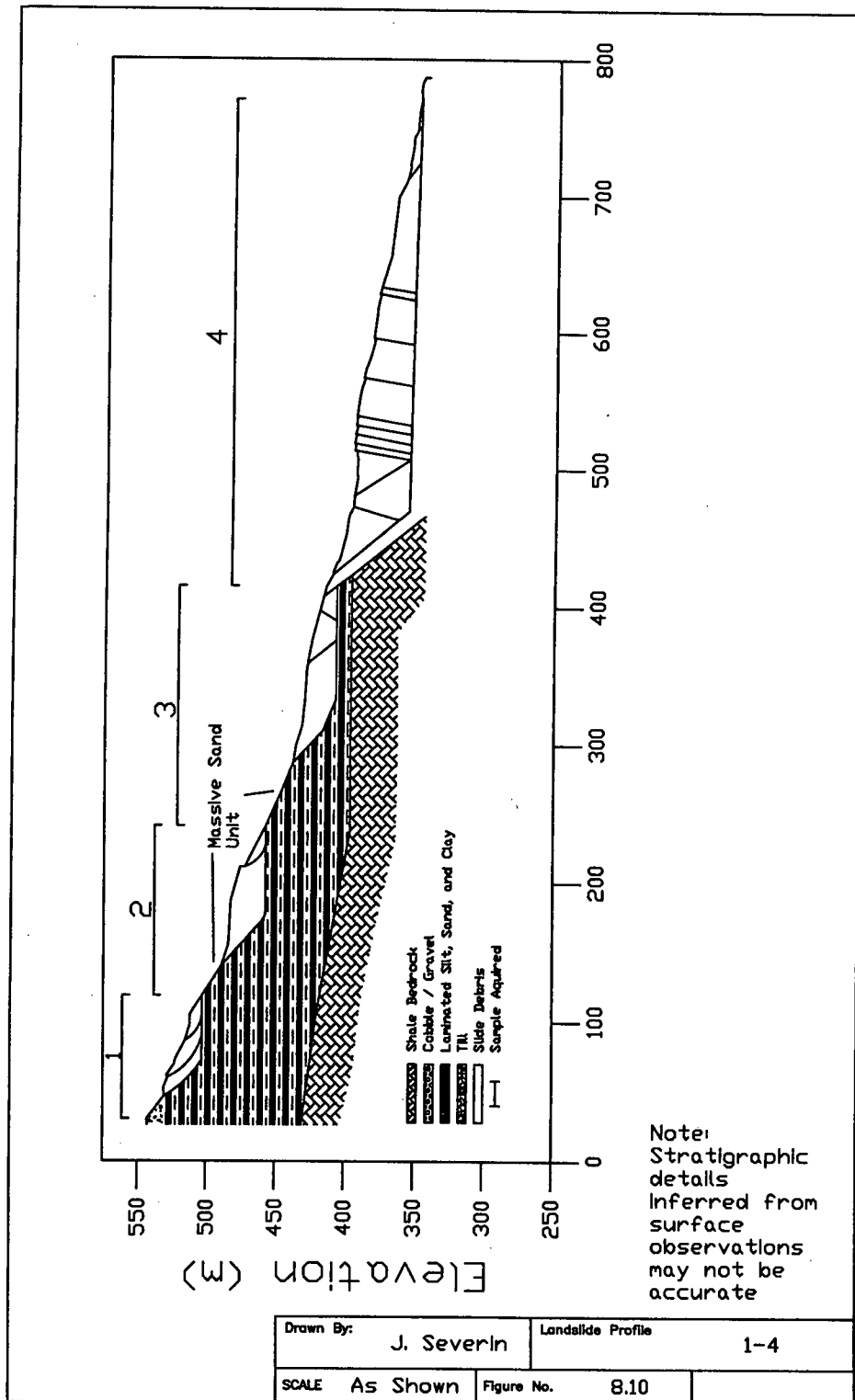
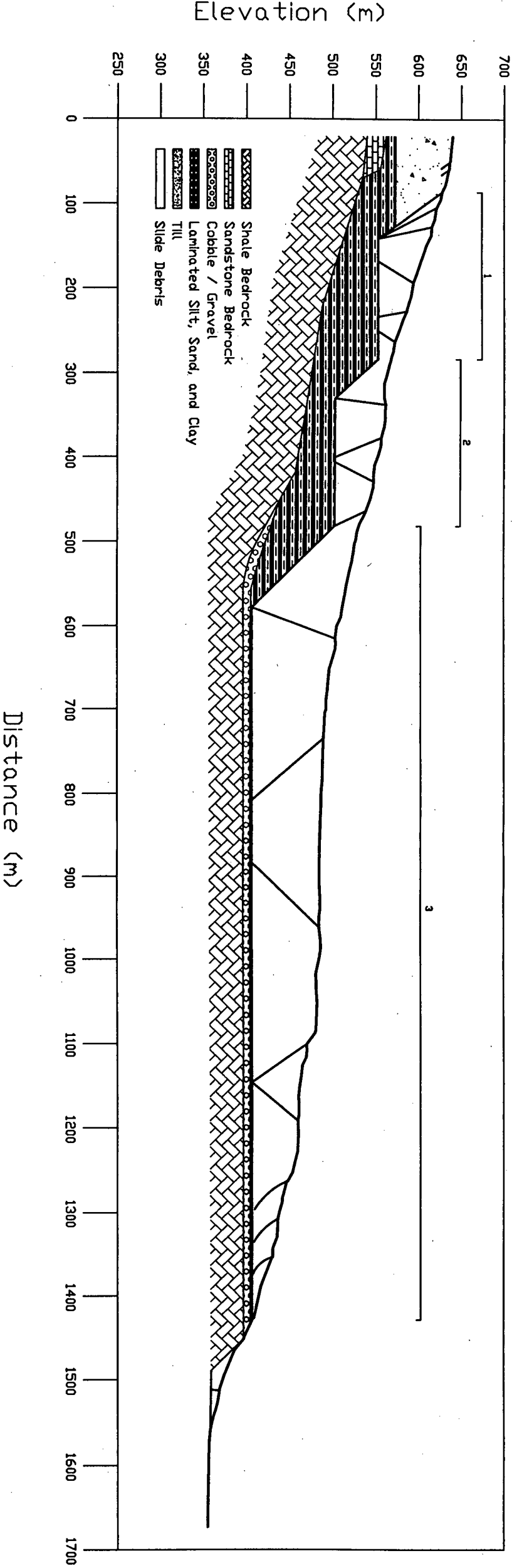


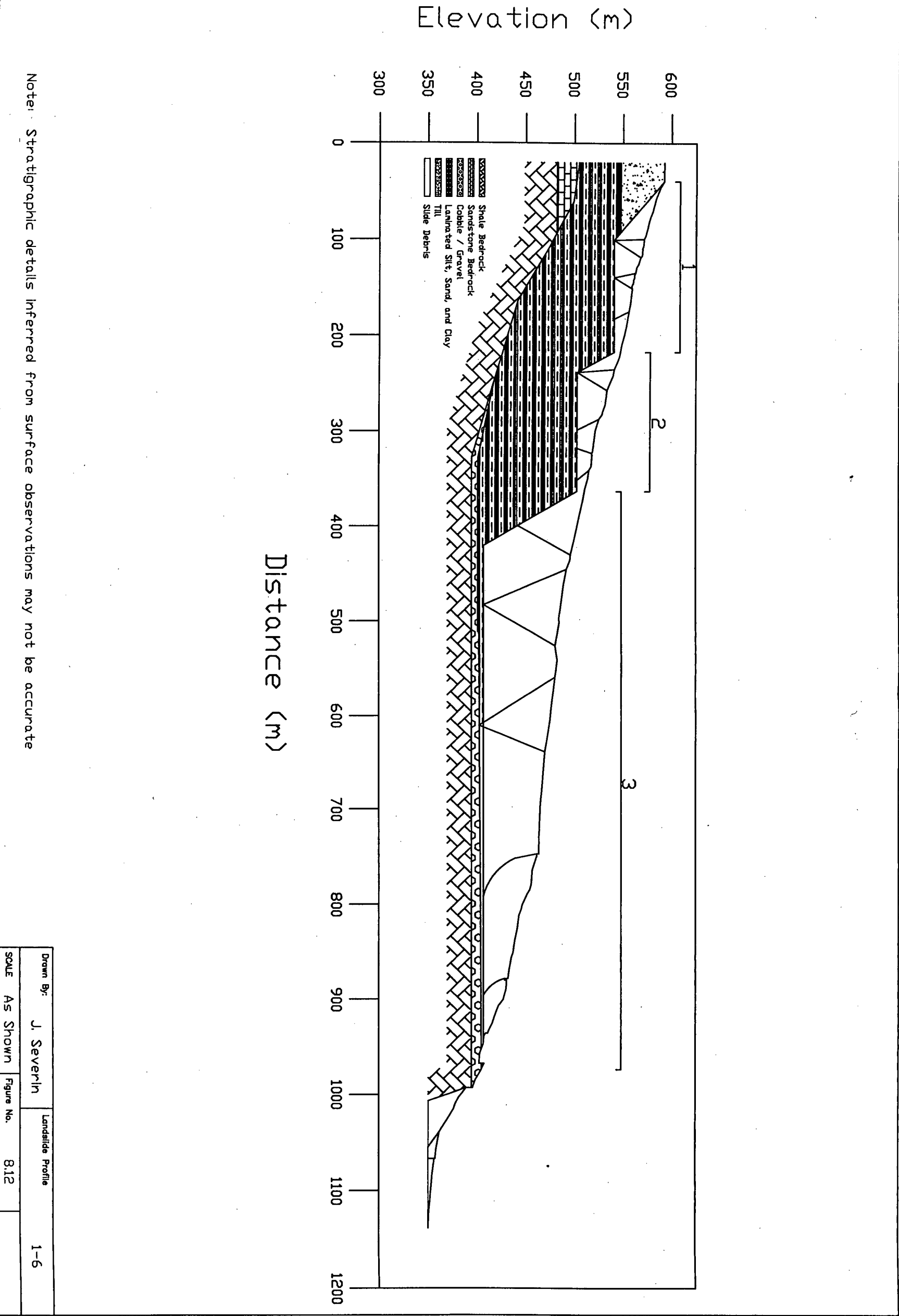
Figure 8.11 Landslide Profile 1-5



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By: J. Severin		Landslide Profile		1-5
SCALE	As Shown	Figure No.	8.11	Page No. 115

Figure 8.12 Landslide Profile 1-6

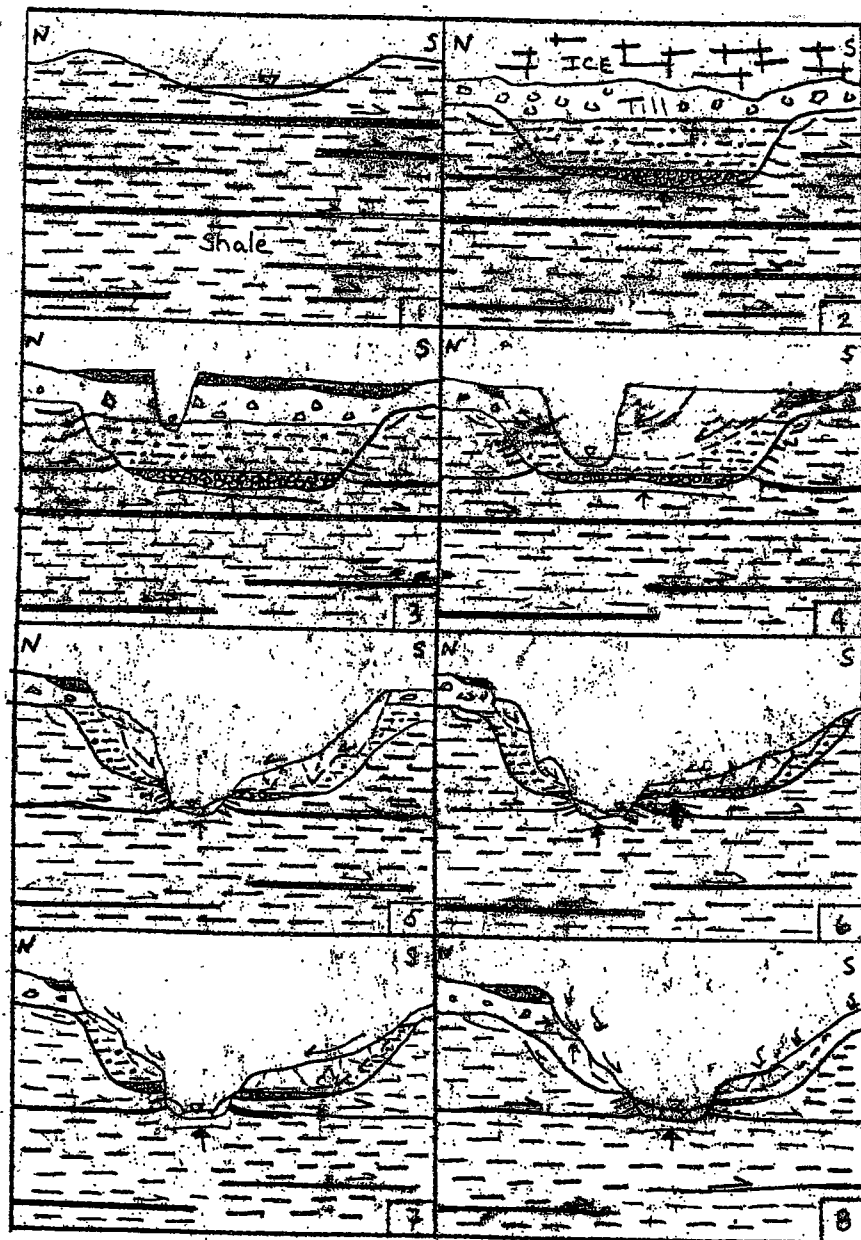


8.1.4 Area Model

This area is located completely (Figure 8.13) within the pre-glacial valley (1), which has undergone glaciation (2), re-established its drainage (3) and eroded to a lower elevation (4) (397 metres a.s.l). During this erosional event several landslides likely occurred, starting the valley widening process. The slides that occur in this area are completely within the Pleistocene sediments with at least two failure planes and sometimes three. The rapid downcutting into the soft sediments of the Pine River and Septimus Creek has exposed weak clay layers (4), on which the slides occur. The clay layers are pre-sheared surfaces, although the cause of the shearing is not obvious. The pre-shearing could have occurred due to glacial loading, syn-erosional slumping, or the release of lateral stresses during valley rebound. The valley expanded with a migrating river constantly eroding the resultant debris. As the river cut down into the bedrock, this abandons the basal shear surface (5). The first-time failures in this area are likely due to erosion from the river, however, reactivation of failures can occur if the debris mass is eroded and the river once again attacks the intact slope (6). Most continued failures are likely due to increased pore pressure from precipitation, softening of the clay layers, and simple creep along these pre-sheared failures (7 and 8).

In this area, there seems to be one major shear surface within a meter of the gravel contact, which was sheared in all the landslides. At the eastern edge of this area the plane is extended for quite a distance, allowing for a more translational movement. Several planes appear to extend over a few slides but not the whole area. These are smaller localized beds with limited area. There is another failure plane near the top of the

Figure 8.13 Landslide Area 1 - Geologic Model



1) Broad, stable pre-glacial bedrock channel; 2) Aggregating deltaic environment into encroaching ice-dammed lake followed by glaciation; 3) Drainage re-establishes; 4) Post-glacial river erodes down into till leaving steep banks, once softer, laminated sediments below are reached, initial failures occur; 5 & 6) Failures continue to occur, on older, upper lower bedding planes and lower planes once exposed; 7) Debris at base acts as a berm for debris on slope; 8) Erosion of debris and intact bank and increases in pore pressure in pre-glacial units cause

sequence, which appears in all slides that reach this elevation. Bedrock plays little or no part in these failures except to provide an intact ledge for the debris to slide over.

In general, this area has a few clay beds within the Pleistocene sequence that seem to be more apt to failure; however, failure is not confined to these beds. As the lowest failure plane usually contains the largest slide, it seems unlikely that the failures are successive, but rather retrogressive. The lower slide would fail, unloading the toe for the next failure in sequence.

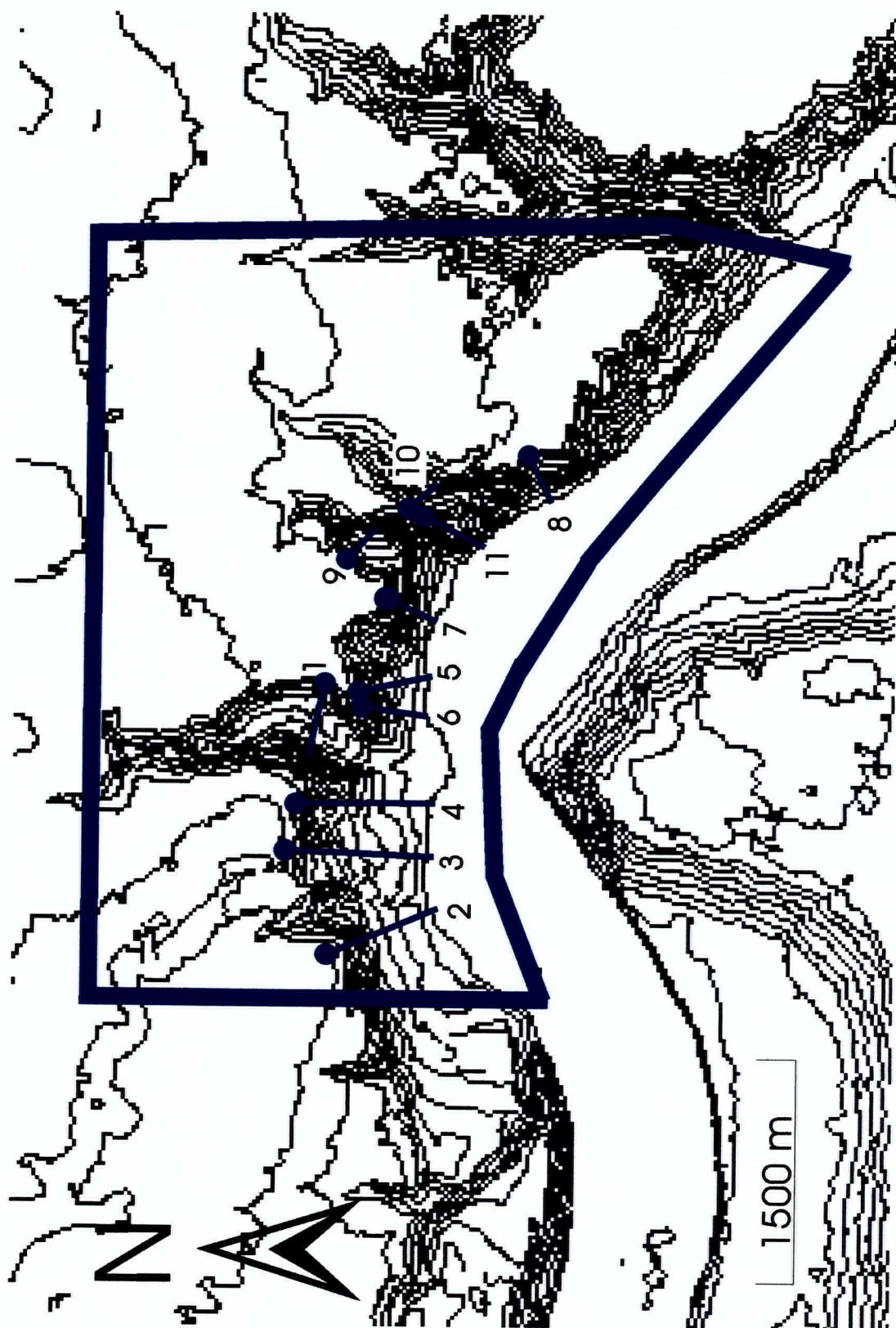
8.2 Slide Area 2

8.2.1 Location

Eleven slopes were profiled near Old Fort, approximately 2 kilometers south of the City of Fort St. John, on the north bank of the Peace River (Figure 8.14). The area extends for 5 km along the south-facing slope and encompasses the settlement of Old Fort, several coulees, and the Peace River Valley. At the upper elevations, the head scarps of these slides border on a bison farm owned by Mr. Bouffieux, a sewage treatment plant, a gravel pit, and a garbage dump. Most of the slopes are south-facing.

Much of the area is developed and only has localized tree cover at the base of the slope. The area was chosen due to the relative easy accessibility, the location of a human settlement, the location of a pipeline crossing and a few oil wells, and its distinct geologic setting. The area is also a very important historical landmark within the Peace River region as it is the location of an original location of Fort. St. John. Residents and tourists alike visit this location to view the scenic Peace River Valley.

Figure 8.14 Landslide Area 2



8.2.2 Local Stratigraphy

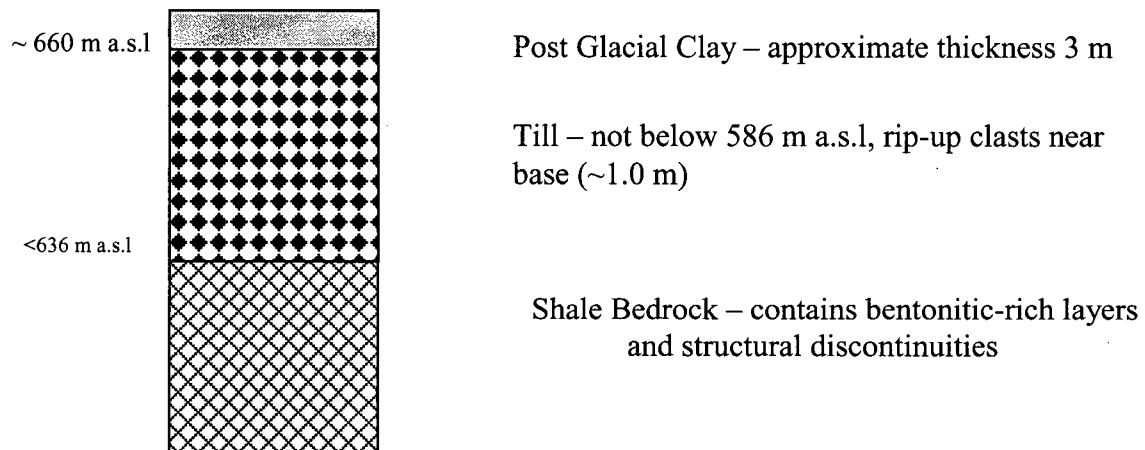
The local stratigraphy at this site is very complex and poorly understood. Most of the section is outside of the pre-glacial valley so there is a distinct lack of pre-glacial sediments at these locals. Shaftesbury shale, observed in 9 of the 12 slides, underlies the area, outcropping quite high in the profiles. The actual height of the Shaftesbury shale is hard to determine as in most sections the top was not actually seen, but rather a shear surface and not actually representative of the maximum shale height. The highest elevation at which the shale was seen at was 636 metres a.s.l (Slide 2-3). The top of the shale unit is irregular as intact till and clay are seen below this elevation.

The material directly above the shale was seen intact once (Slide 2-5) where rip-up clasts of shale were observed within till. The debris directly above the shear plane is thought to be disturbed till which contains clay, silt, and minor amounts of sand and gravel. The till unit was seen at varying elevations but never lower than 568 metres a.s.l (Slide 2-11). The till has a maximum thickness of 44 meters (Slide 2-7), which may actually contain till from the previous glacial advance. The locations of the till are controlled by the pre-glacial bedrock topography. Above the till unit lies a blanket of post-glacial clay seen at most locations at approximately 660 meters a.s.l., with an average thickness of 3 meters.

The pre-glacial sediments are seen again at Slide 2-8 in the east, marking the extent of the pre-glacial valley.

An approximate stratigraphic column is produced (Figure 8.15).

Figure 8.15 Area 2 – Stratigraphic Profile (not to scale)



8.2.3 Landslides

Slide 2-1

Slide 2-1 is located on a west-facing slope within Bouffieux Coulee. The profile starts at the top within a field and continues down slope at an azimuth of 262° . The failure is currently dormant except near the toe where some of the debris has remobilized. The failure is interpreted as having three failure planes within shale (Figure 8.17). The lowest failure plane was the only one observed, however it could not be sampled. The upper two failure planes are simply inferred due to surface morphology and neighbouring slides. A vertical face of post-glacial clay was seen near the top at an elevation of 663 meters a.s.l, within the head scarp of a small compound failure. The profile goes down a covered intact scarp. The scarp is likely intact because the slope is likely too steep for the debris to maintain. Large horst and graben structures appear along the next failure plane. The depth of this failure plane is interpreted from the size of the features. The toe of this failure slides over another intact scarp where a large compound failure occurs. Back tilting and ponding occur within the slide mass at this point, suggesting a rotational

component. At the base of this failure, intact shale is seen overlain by debris within a ravine. There is no sign of the pre-glacial sand, silt, and clay so the bedrock surface in the area must be controlled by failure surfaces.

Slide 2-2

Slide 2-2 has a south facing profile down the Peace River Valley at an azimuth of 138°. The profile begins at a common sight seeing location directly above Old Fort (Figure 8.14) where there has been a great amount of modification in order to incorporate parking. Garbage within the landslide debris, including several automobile bodies, suggests that this may have been a trash disposal site in the past. Several shear planes, four or five, all of which are in the shale bedrock, control the dormant landslide (Figure 8.18). At the head scarp of the slide, a 10m intact section of Quaternary materials was found to have 4 m of glacial Lake Peace clay and 6 m of till. The upper most shear plane was observed within the shale and sampled (sample 2-2A). The shear plane had undergone some alteration and shale clasts were seen within the debris. The shear plane consisted mainly of silt, 70%, and has a liquid limit of 40% and a plasticity index of 20%. A sample of the shear plane within the debris was also sampled and found to have the exact composition but with a slightly higher liquid limit, 43%, and plasticity index, 24%. Below the shear plane a displaced slump block with a heavy concentration of garbage was encountered. No controlling shear plane was observed beneath this failure, however, below the debris, intact but rubbly shale was found allowing the shear plane to be inferred within a certain elevation range. A third shear plane is also inferred below based on surface morphology and thought to be in shale as well. This part of the failure may be interpreted as two failure planes. Below this, the slide has a large translational

component, made up largely of remobilized debris with a few large-scale horst and graben features. Along the profile, ponding and highly disturbed terrain was encountered. At the toe, the debris is remobilized in a rotational slump, thought to represent the edge of the underlying intact shale.

Slide 2-3

This slide is on a south-facing slope within the Peace River Valley. The profile starts at a fence line near the sewage lagoon and heads down slope at an azimuth of 192 . Even though this slide has an active upper section, overall it would be classified as a dormant slide. This slide has multiple failure planes, four or five, all thought to be within the shale bedrock (Figure 8.19). The depth of the failure planes are inferred from surface morphology and the material is inferred from neighbouring slide reports. The upper most failure is interpreted as a compound failure with till and glacial Lake Peace clay within the head scarp. This was the only section of unconsolidated sediments found during the profile. There was one intact section of shale seen during the profile below the upper failure. Further down in the profile, the debris is very hummocky with small-scale features. The location of the sewage lagoon makes this an important failure to study as further movement could cause rupture of the lagoon leading to a higher risk in failure.

Slide 2-4

Slide 2-4 is a dormant slide on the south-facing slope of the Peace River Valley, with an active upper most section. The profile starts on the fence of the sewage lagoon 50 meters east of the beginning of profile 2-3 and continues down slope at an approximate azimuth of 160 . The top of the profile is within a plowed field so some features may

have been disturbed. The intact section at the head scarp of this upper failure shows glacial Lake Peace clay above till within an 8-meter section. The upper most failure plane is within shale so the failure planes lower in the slide are also thought to be within the shale unit. The upper failure is a retrogressed rotational slide with large, tilted slump blocks. The shale failure plane was located about 30 meters off the profile (Figure 8.16) and sampled (sample 2-4 A and B). Sample 2-4B consisted of 32% clay, 49% silt, and 19% sand. The shear plane has a liquid limit of 47% with a plasticity index of 28%. The failure plane was partially altered to a gouge that was also sampled (sample 2-4 C and I) and tested. The gouge had an average of 67% silt and 27% sand; it has a high liquid limit

Figure 8.16 Shear Plane found and sampled (Slide 2-4)



of 53% and a low plasticity index of 23%. Below the gouge, a sheared surface in the shale was observed and found to consist mostly of silt, 85%, with a liquid limit of 41% and a plasticity index of 22%. Another intact section was beneath the upper failure made of weather shale. The rest of the slope is made of 2 inferred failure planes, both showing small, muted compound features (Figure 8.20). This slide shows 2 interesting features not seen elsewhere. The first is a large amount of cobbles near the base of a gully at the top

of the slide. This is simply a collection of weathered cobbles that have been released from the till unit. The second, a banded diamicton found near the top of the slide within the debris, is thought to represent several surges of debris that have been deposited and then broken up by a later failure.

Slide 2-5

Slide 2-5 is on a southeast-facing slope within the Peace River Valley. The profile starts at the edge of a field, same field as Slide 2-1, and continues down slope at an azimuth of 132°. The slide is mostly dormant and contains three failure planes all within the Shaftesbury Shale (Figure 8.21). The head scarp of the slide shows a thick layer of glacial Lake Peace clay of 9.5 meters above a till unit of 25 meters in thickness. At the base of the till unit is the contact with the shale below. This contact is a zone of shale rip-up clast dominated till with a clay-rich matrix of about a meter in thickness. Below this contact is a 2.5-meter vertical section of intact shale above the talus of the slope. At the base of this intact section is a large slump block of a rotational failure. The failure plane was not observed, however, below the failure another section of intact shale was found suggesting that the failure is in shale. The intact section of shale is approximately 50 meters thick and below this is two compound failures interpreted from surface morphology with several small horst and graben features. The toe of this failure is a modified river terrace that has since been farmed that shows no natural features.

Slide 2-6

This slide is dormant with a south-facing aspect in the Peace River Valley and an azimuth of 154°. The profile starts on a knife-edge with Bouffieux Coulee, so a stable top

section is not possible to attain. The head scarp is completely grassed so no intact section was observed, although the small flake slides within the head scarp and the neighbouring slide suggest that the head is made of till and post-glacial clay. The slide is composed of two independent failure planes within the slope (Figure 8.22). The upper failure plane is abandoned and currently stable and represented by a small dip in the profile. No failure surface was observed, but rather interpreted from surface morphology and neighbouring slides to be within the shale. At the base of the main scarp, a small, rotated block was found with shear planes (sample 2-6 A), was observed. The shear plane has a clay fraction of 34 and a silt fraction of 57. The liquid limit of the sample is 48% with a plasticity index of 28%. Below this, a layer of gouge within the shale, which does not look sheared, was located (sample 2-6 B). This sample consisted of 38% sand, 40% silt, and 22% clay. The sample displayed a liquid limit of 45% and a plasticity index of 22%. The second failure is a retrogressed rotational failure that has been weathered and the features are now muted, although ponding is still prevalent in the debris mass. No intact sections or failure planes were observed during this profile. The toe of the slide has been modified by a river terrace and subsequent farming activity.

Slide 2-7

Slide 2-7 is south-facing with an aspect of 188° in the Peace River Valley. The profile begins on a hummocky, plowed field within a Bison Ranch belonging to Mr. Bouffieux. This is an active failure with three shear planes within the shale (Figure 8.23). The head scarp of the failure shows an anomalous section of Quaternary sediments. This section is at least 56 meters thick and dominated by a unit of till. The top of the section displays a 10-meter unit of glacial clay underlain by 44 meter thick of till and at least 2

meters of sand and gravels at the base. The till displays glaciotectionic shears near the base, however, these shears don't seem to control the stability of the vertical till face. There is a rotational failure below the intact face with a shear plane in the shale. Below this failure, an intact shale face was encountered that leads to the debris mass of the middle failure on the slope. This failure is also a rotational failure with shale found directly below in a nearby gully. A third rotational failure exists below this intact face. The debris of this lower failure flows over an intact shale bed below, with the toe being remobilized debris. This slide has three failure planes allowing for independent rotational failures. All failure planes are inferred as none were witnessed during the profile.

Slide 2-8

This slide represents the edge of the pre-glacial valley of the Peace River and is on the eastern edge of the slide area. The profile has a western aspect with an azimuth of 252 and starts upon a flat where several pump jacks and a field exists. The slide is active and contains three failure planes, however, only one is in shale (Figure 8.24). There is a small rise within the field and back tilted trees, which may be a filled tension crack. The main head scarp is dominated by till with a thin layer of glacial clay near the surface. The displaced debris is slightly back-tilted from a rotational movement. The shear plane for this failure was located within a clay bed above a thick unit of silt. Both the failed clay bed and intact clay beds below were sampled (sample 2-8A and 2-8B). The failed clay consisted of 57% clay and 43% silt, whereas the intact clay below consisted of 32% clay and 68% silt. The Liquid Limit for the failed clay was 47% with the Plasticity Index at 28%. The intact clay showed a higher Liquid Limit of 69% and a Plasticity Index of 48%. The silt unit has some smaller beds of clay, up to 3 cm in thickness, and lenses of sand,

up to 45 cm thick. The failure below this intact section is a rotational failure moving along a clay layer within the pre-glacial sediments (sample 2-8 D). This surface occurs 0.1 meters above the gravel contact and is composed of 36% clay and 63% silt. The Plasticity Index of the failed clay is 26% and a Liquid Limit of 48%. A bed of intact clay below the slide plane was sampled (sample 2-8 AA) and found to be 57% clay with 43% silt. The intact bed has a Liquid Limit of 51% and a Plasticity Index of 31%. The gravel unit is anomalously thick in this section at 36 meters with an over steepened angle of 45°. Below this intact section, highly disturbed debris with ponding and minor back tilting suggests another failure plane within the shale. The steeper slope within this debris is likely caused by the presence of an intact scarp below. The toe along the river's edge is very active, failing within the last 5 years.

Slide 2-9

The profile of slide 2-9 begins in a field near many bent, twisted, and tilted trees and heads down slope at an azimuth of 104°. The slide occurs within Crawfords Coulee near the eastern edge of the area with an eastern aspect. The 6-meter vertical head scarp shows glaciolacustrine clay for most of the face with till composing the bottom 0.2 meters. The head scarp shows distinct pinnacles of till and clay 3 to 5 meters high that have moved several meters. The failure is a retrogressed rotational failure with a slide surface that daylights in the scarp below (Figure 8.25). There is a large amount of back tilting and small scarps within the debris. The failure plane occurs within a clay bed of the pre-glacial sediments (sample 2-9 A). Below the shear plane, the quaternary sediment sequence is dominated by silt with minor amounts of sand and discrete clay beds. The lower failure is a compound failure displaying steps within the debris and extends the rest

of the slope. The toe of the failure is very active with fresh features and disturbed material. A lower shear plane within in the shale was located within the ravine that the debris mass is sliding on.

Slide 2-10

This small slide has an eastern aspect, occurring within a coulee east of Crawfords Coulee. The profile begins on a knife-edge between two coulees and continues down slope at an azimuth of 96 . The profile goes down into a large tension crack and back to a vertical face of a diamicton, which may be disturbed till. The failure is active and has one failure plane within the shale (Figure 8.26). The head scarp is steep and the debris below exhibits major back tilt within several steps. The slide is interpreted as a retrogressed rotational failure within the shale bedrock. The debris steps are large suggesting that there is a deep failure plane. The toe of the slide has been reactivated with a disturbed surface and small pockets of ponding. No shear surfaces or intact sections were observed in the slide mass, however, a shale scarp was witnessed nearby extending above the elevation that the shear plane is interpreted to be at.

Slide 2-11

This short failure occurs within the Peace River Valley on a south-facing slope. The profile begins on a ridge between coulees and follows along it and down slope on an azimuth of a 192 . The failure is very active and has two failure planes within the Shaftesbury Shale (Figure 8.27). The head scarp is composed of 9 meters of glaciolacustrine clay above 5 meters of exposed till. Pre-glacial sediments were not

Figure 8.17 Landslide Profile 2-1

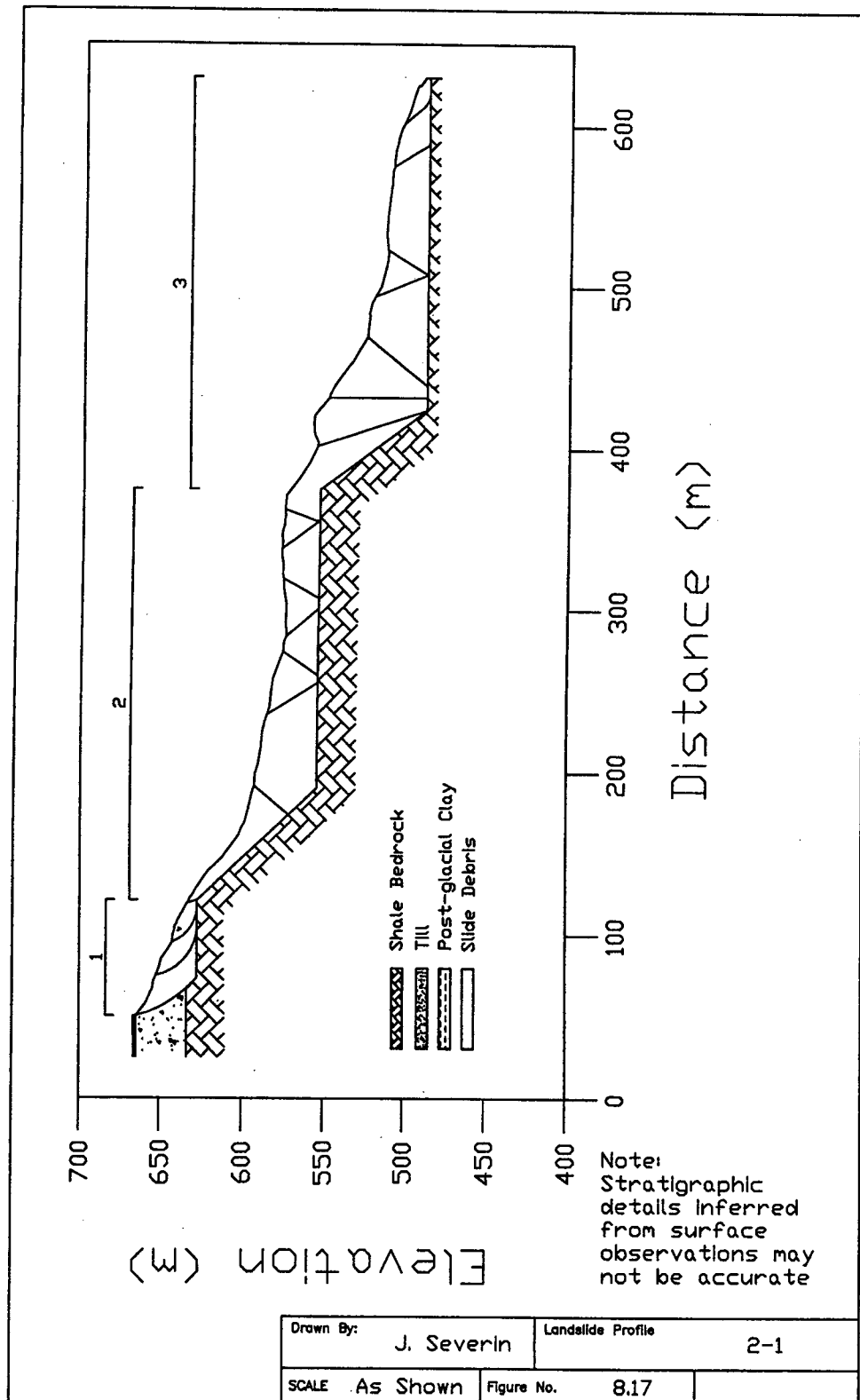


Figure 8.18 Landslide Profile 2-2

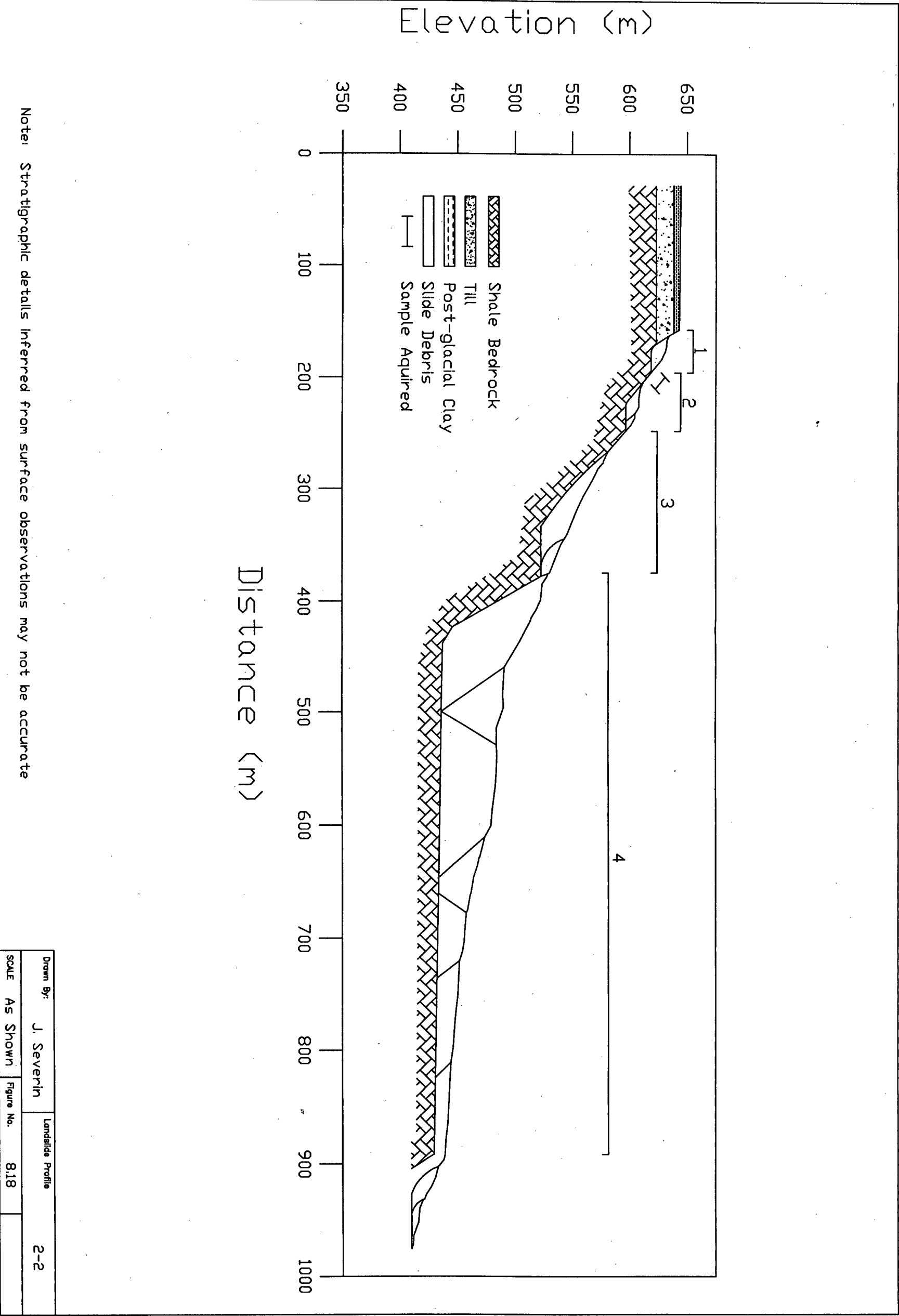


Figure 8.19 Landslide Profile 2-3

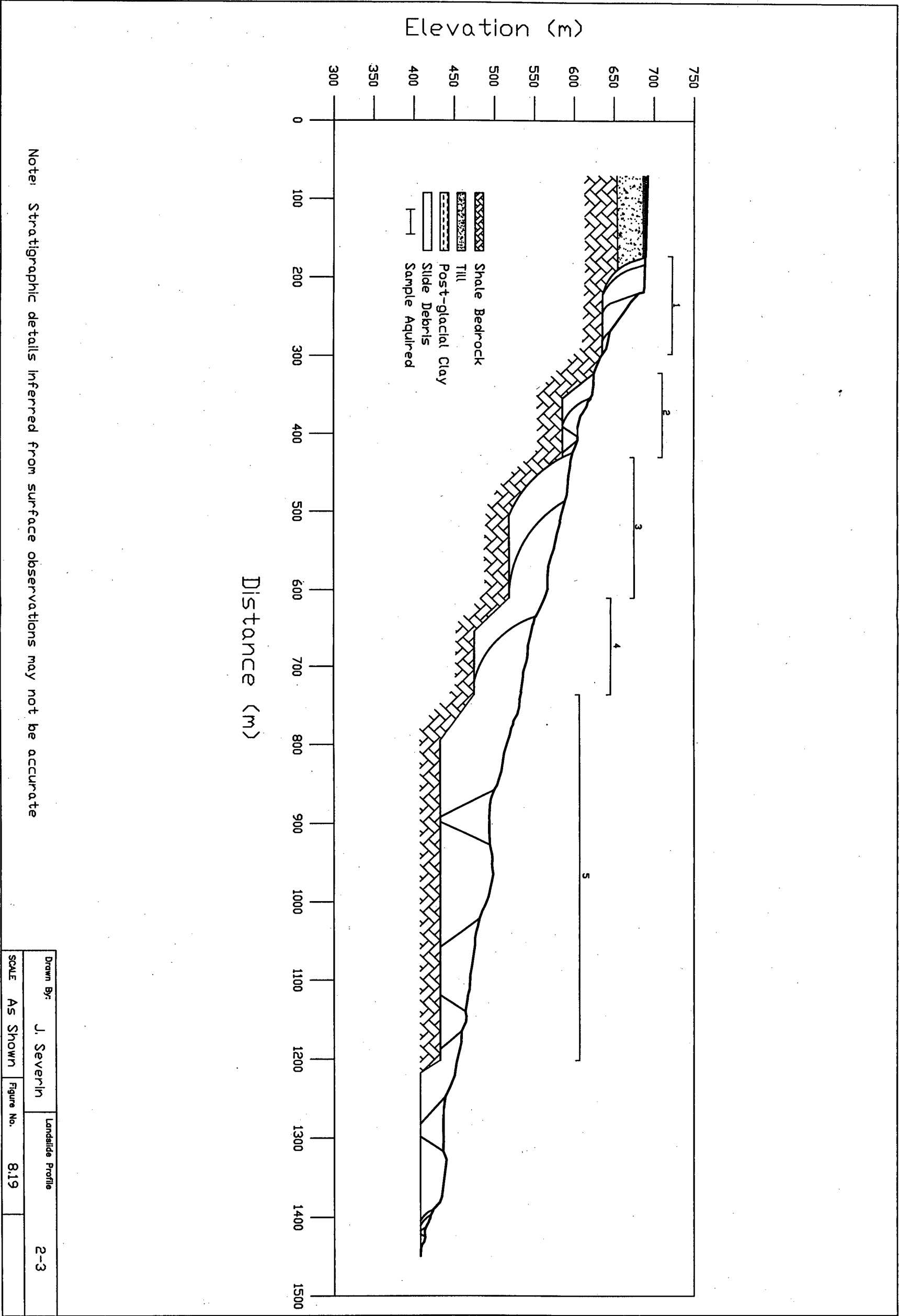
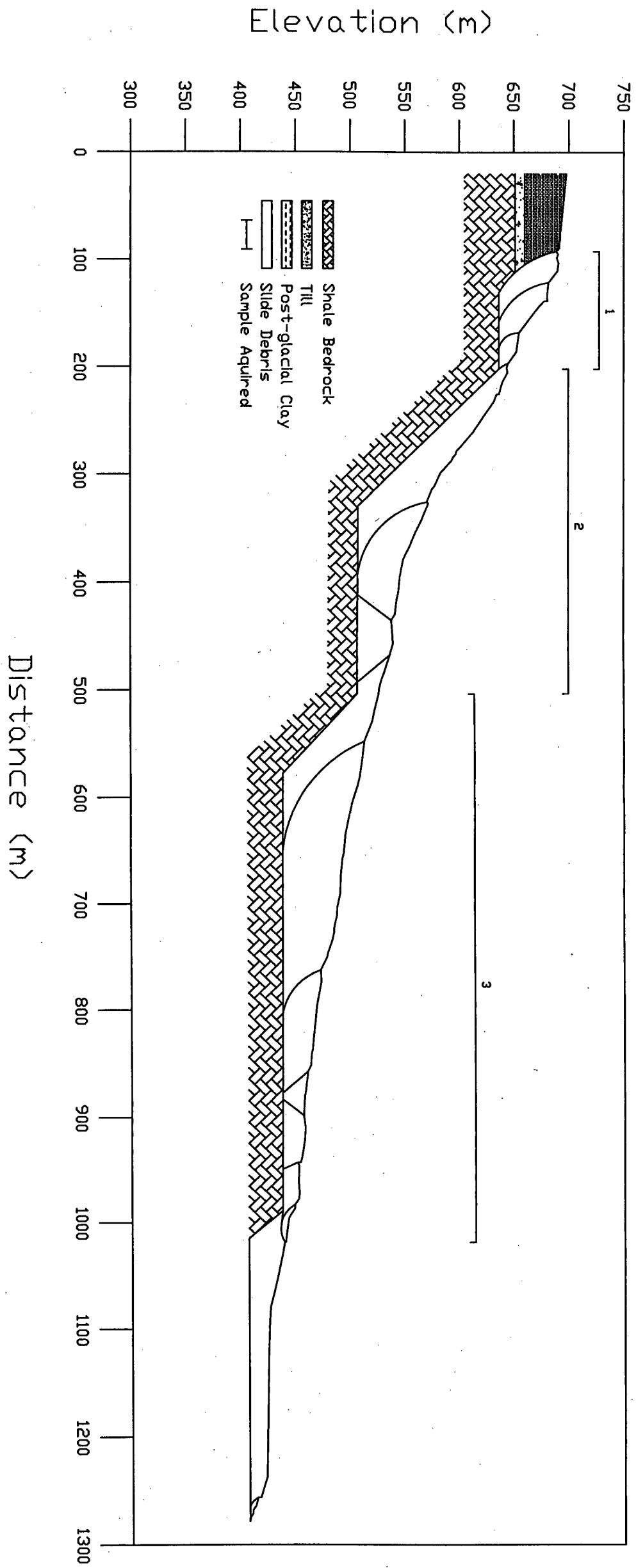


Figure 8.20 Landslide Profile 2-4



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By: J. Severin		Landslide Profile		2-4	
SCALE	AS SHOWN	Figure No.	8.20		

Figure 8.21 Landslide Profile 2-5

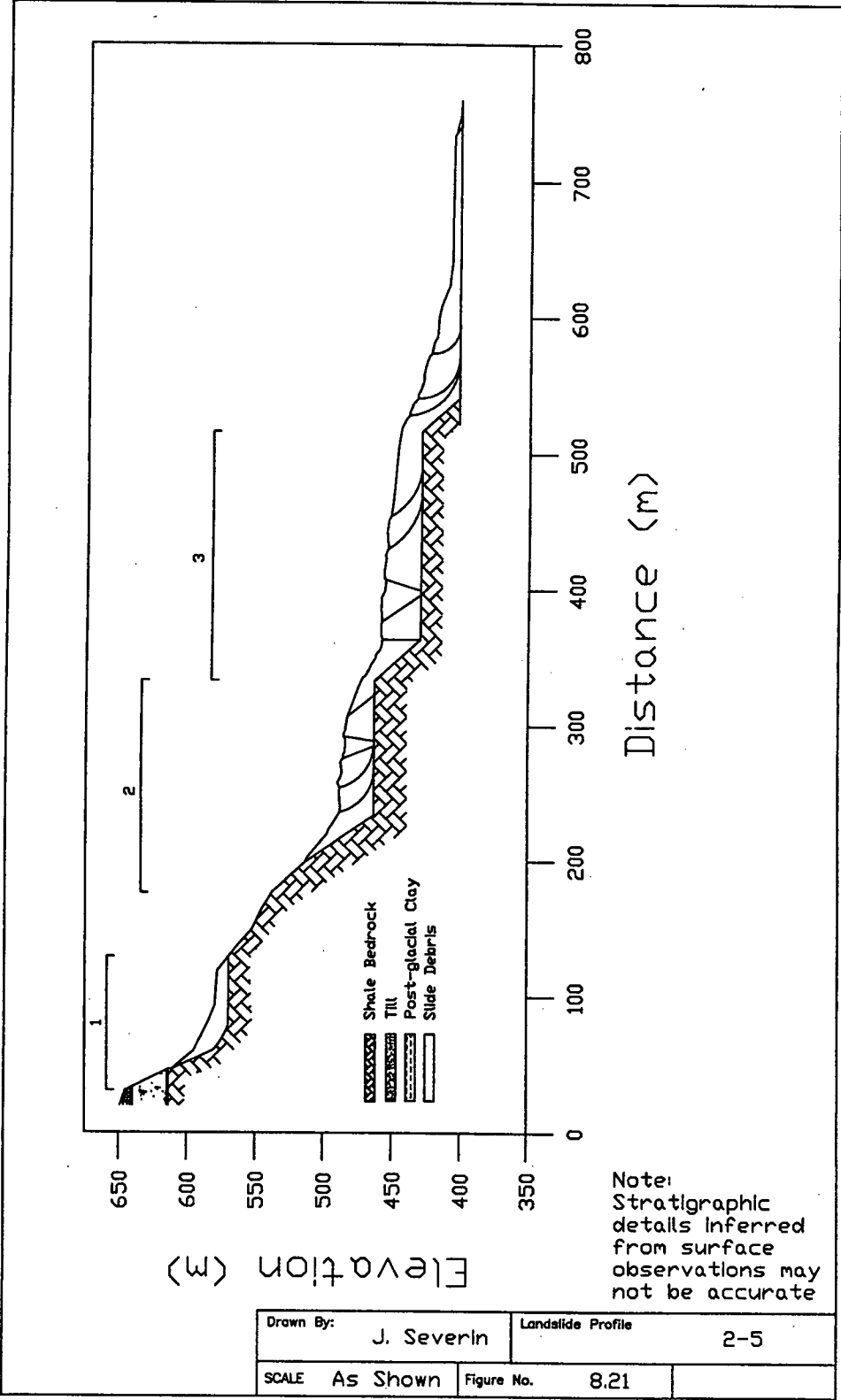


Figure 8.22 Landslide Profile 2-6

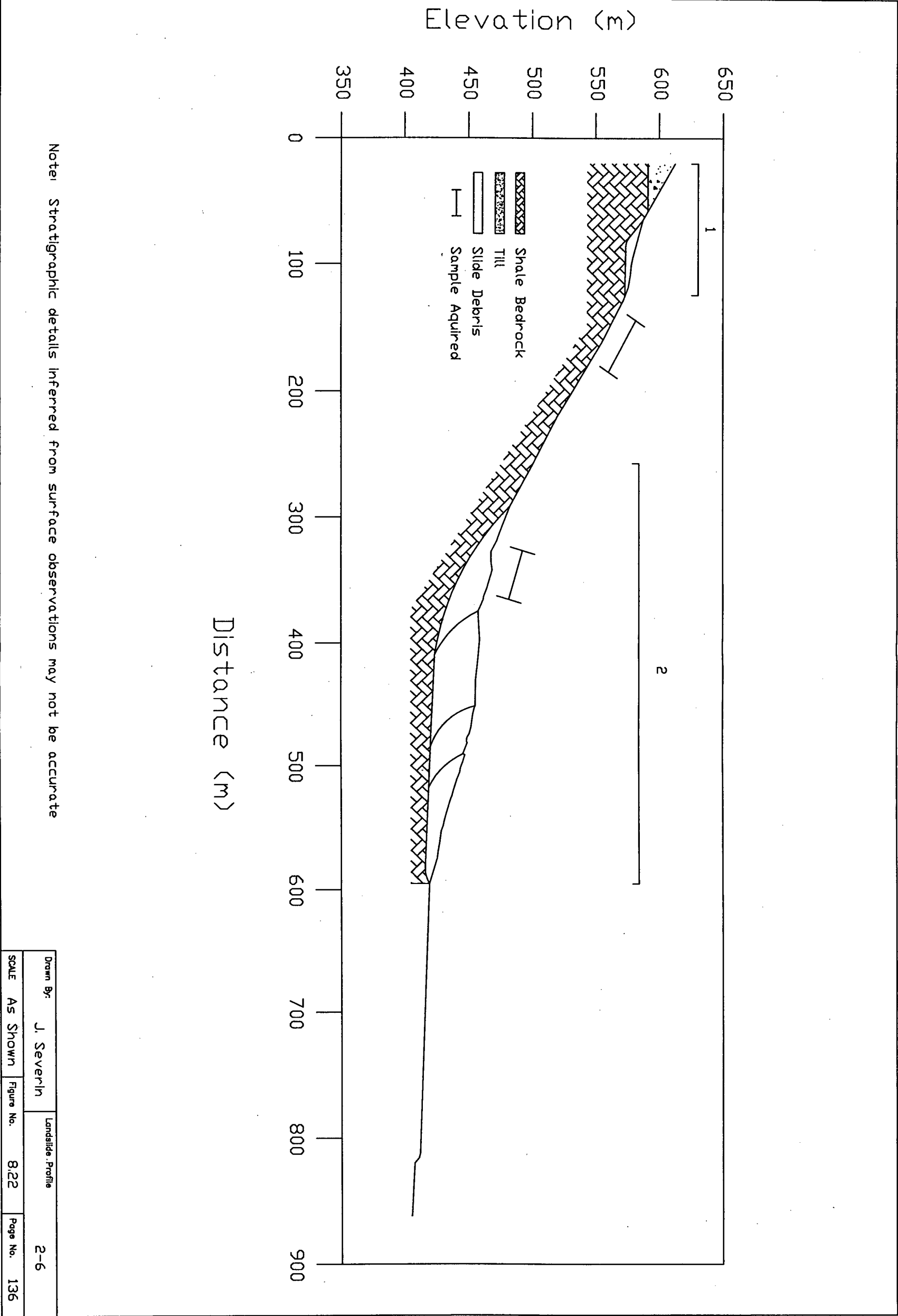


Figure 8.23 Landslide Profile 2-7

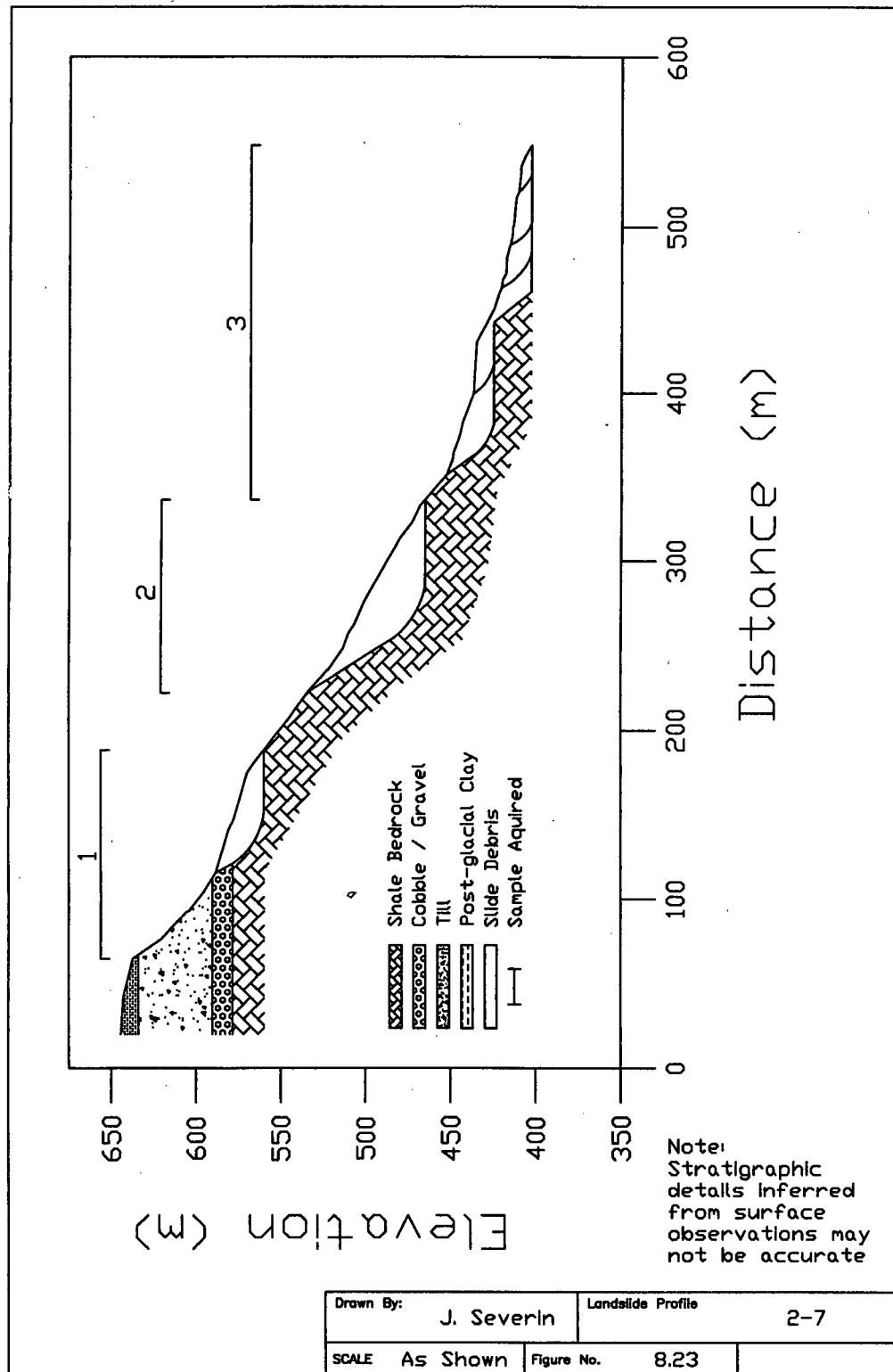


Figure 8.24 Landslide Profile 2-8

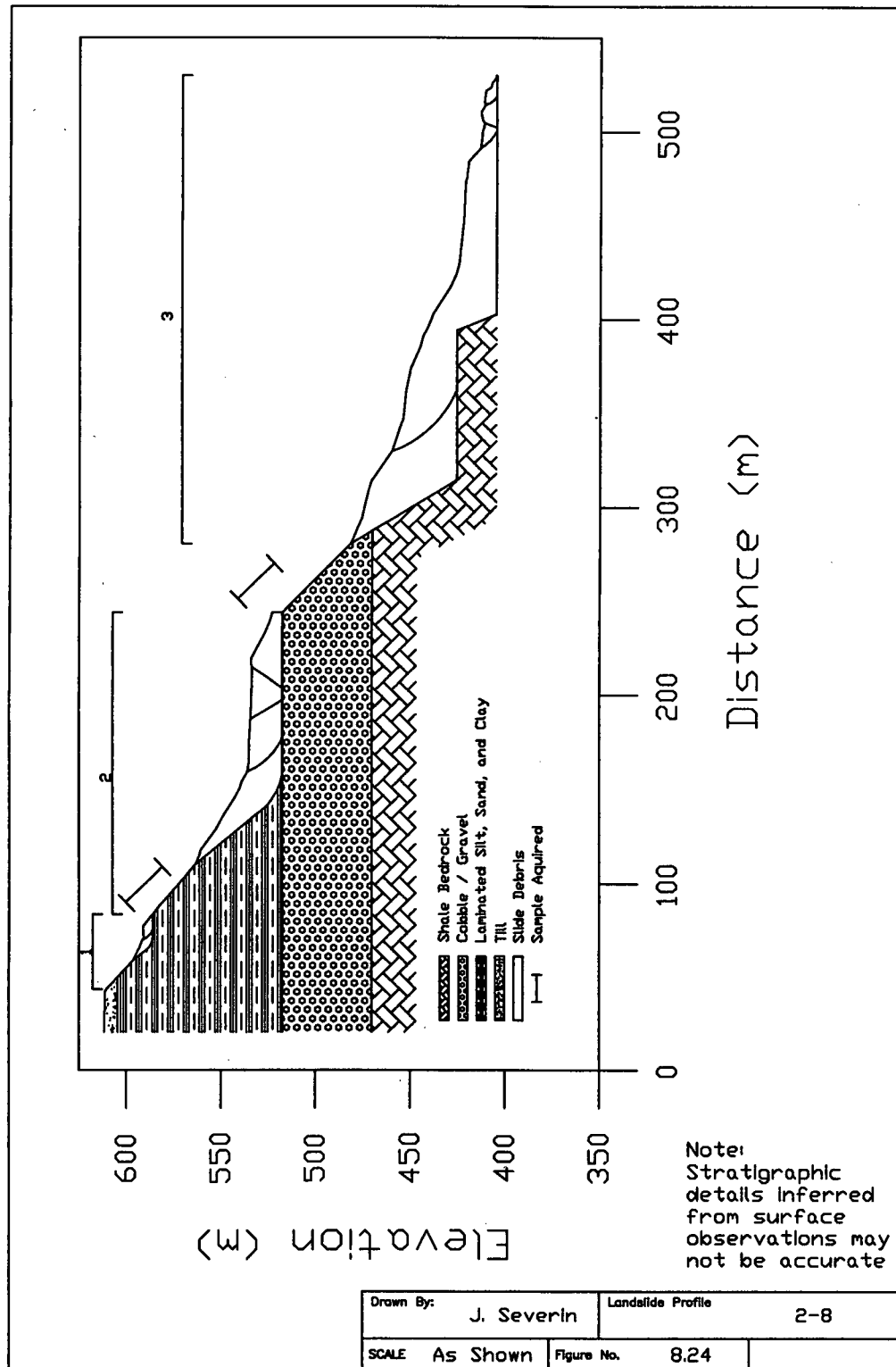


Figure 8.25 Landslide Profile 2-9

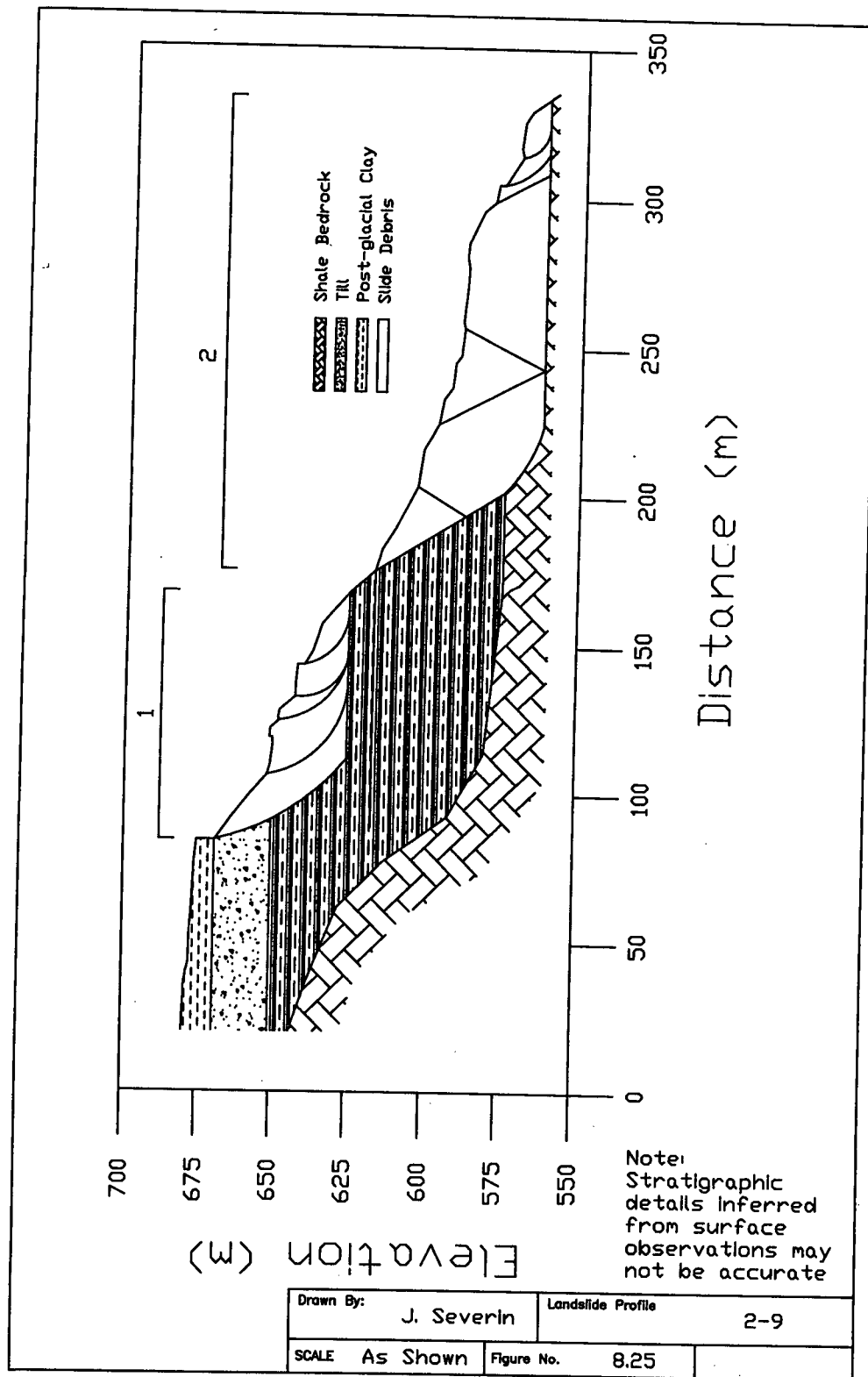


Figure 8.26 Landslide Profile 2-10

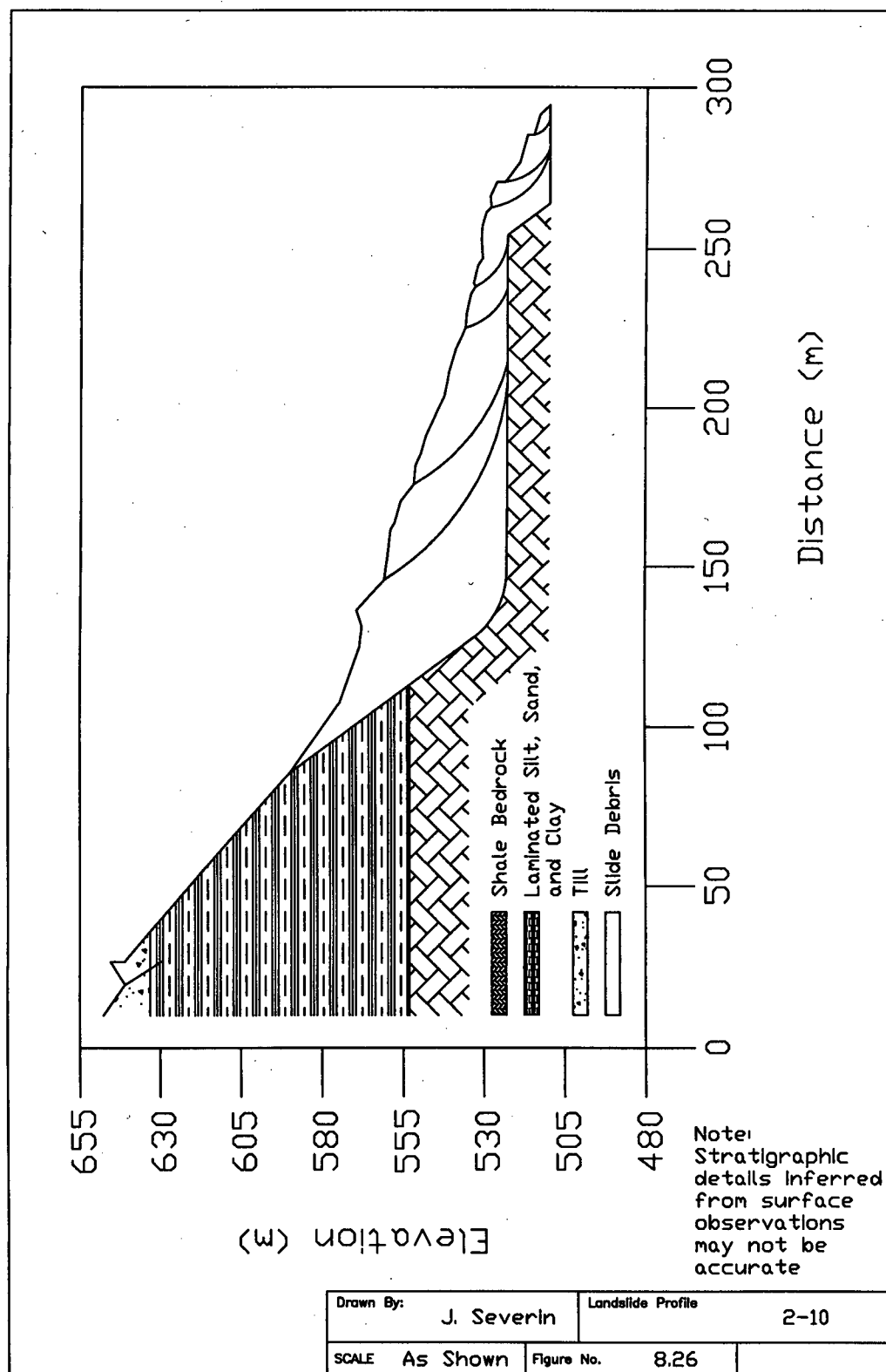
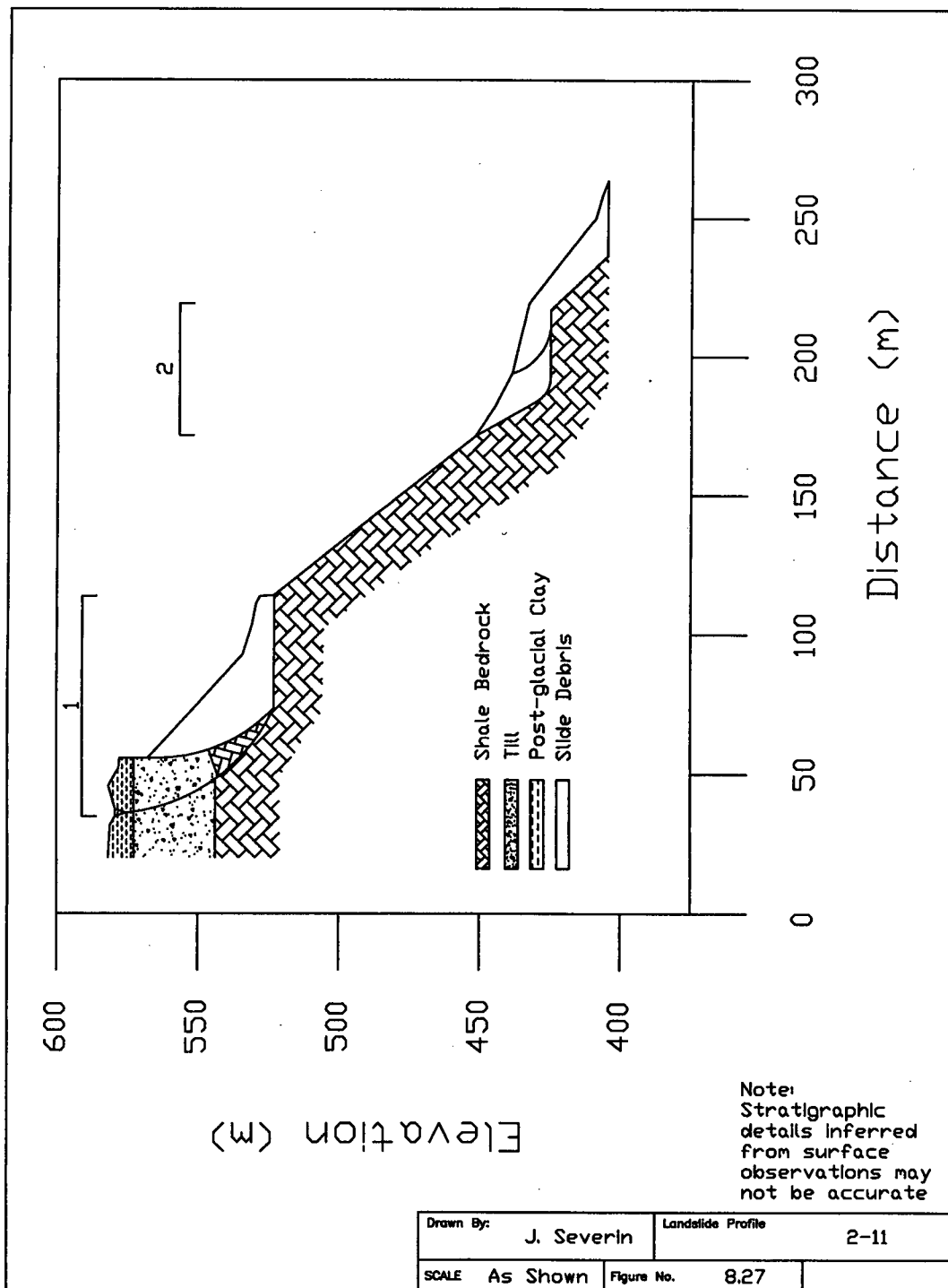


Figure 8.27 Landslide Profile 2-11



detected during the profile. Behind the head scarp is a large crack thought to be a tension crack connecting with the failure plane used for the upper failure. It is likely that this whole section is unstable. The talus below the vertical section is till-like and ends at a bench within the slide. The upper failure is a rotational failure completely separated from the lower failure. The failure plane was observed in the shale but could not be sampled. Below this shear plane, an intact section of weathered shale was found and extends until a debris bench overlying a stable bedrock layer likely containing another shear plane within the shale.

8.2.4 Area Model

This area occurs completely within the Shaftesbury Shale boundaries making the more resistant Dunvegan Sandstone irrelevant to slope stability studies (Figure 8.28). The highest intact elevation that the shale was observed at was 636 meters a.s.l.; however, in most cases the exposures were truncated by a shear plane. This area is unique within the map sheet because it represents a location in which the present river valley does not follow the pre-glacial valley (1), shown by the high shale elevation and distinct lack of pre-glacial sediments. Only one slide to the east is within the boundaries of the pre-glacial valley. The area was then glaciated (2), represented by till and glaciolacustrine clay in the headscarp and debris through out the slide profiles. Drainage re-established itself within the pre-glacial valley (3) and eventually migrated to erode the bedrock shale, exposing the weak strata (4). As the site is outside of the pre-glacial valley, the shale is found at a high elevation and the shear planes are located within the bedrock. The shale, being a marine deposit, has much less variability than the pre-glacial sand, silt, and clay.

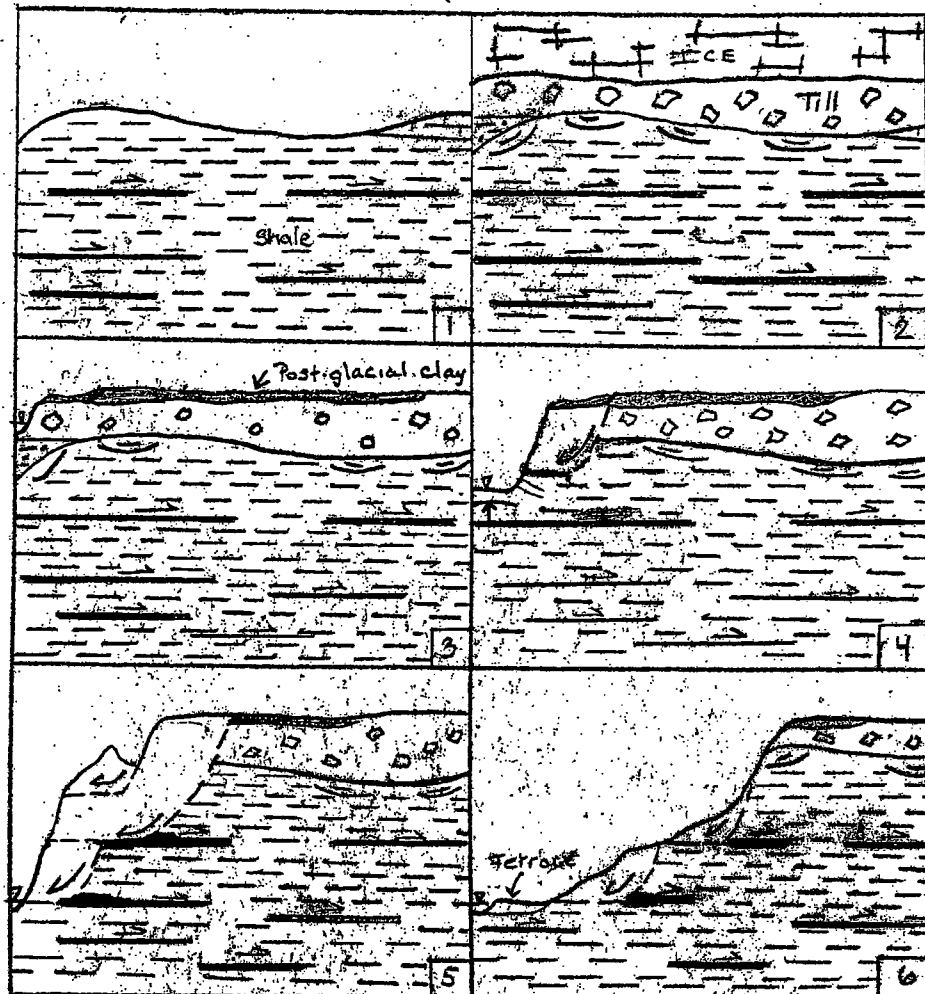
With this in mind, the shear planes are more likely to be correlated over a small area as the lithology plays a role in the slope stability. As the river migrated throughout its channel, proven by current terraces, it also eroded into the bedrock valley wall exposing older weak planes and creating new ones through valley rebound (5). Eventually the river became smaller and controlled by the upstream dam, the lower shear planes were abandoned.

Near the bottom of the river valley, there is a 14-meter thick zone, between the elevations of 420 meters a.s.l and 433 meters a.s.l., in which all profiles that extend to that elevation show a failure. This widespread failure zone is likely a bentonite-rich section of the Shaftesbury Shale reported by others (Sargent and Moore, 1988; Imrie, 1991). Another failure zone exists between 465 meters a.s.l and 478 meters a.s.l. in which 6 of the 9 profiles that are at that depth show a failure plane. Within this study site, there are four more such groupings that can be made that correlate failure planes from at least three profiles. These groupings occur near 522 meters, 555 meters, 585 meters, and 633 meters a.s.l. These shear plane groupings are expected in a material within such a uniform depositional environment. The amount of groupings suggests that bentonite is quite prevalent within the formation, confirmed by other reports on the Shaftesbury Shale and other Cretaceous shales. This bentonite-rich shale occurs in discreet zones roughly correlatable because of the sporadic nature of deep-sea deposition. The shale will fail along the weakest of these zones given the proper setting or else it will fail along the next weakest layer. The bentonite does not occur in discrete beds but rather dispersed within a larger zone. The slides occur along planes that have been sheared due to one of many causes. These planes may be pre-sheared due to relaxation along bedding planes due to

valley rebound, over steepening of valley walls during erosion causing syn-erosional slumping, and larger horizontal stresses cause by the orogenic events to the west. The Peace River area is close enough to the mountains that horizontal stresses cannot be ruled out. With the amount of weak zones within the unit and the number of possible causes for pre-shearing, it is no surprise to see individual slides with multiple shear planes. The slides occurring within the coulees generally only have one to three failure planes, simply because the lower failure zones have not been reached by the downcutting of the small streams. One interesting feature of this area is the asymmetry of the coulee slopes. The coulees are generally aligned north-south, however the asymmetry is not consistent. In some of the coulees, the shallower slope is to the east, and in others, to the west. The failures not occurring in the shale show a similar morphology to the slide area around Big Bam Ski Area. A weak clay layer less than a meter above the gravel contact and another failure plane occurring higher in the pre-glacial succession.

In general, there are several weak layers within the shale due to the inclusion of bentonite allowing the shale to fail at many elevations. All of these failures occurred post-glacially as the current valley wall did not exist and as scouring action of the glaciers eroded previous landslide deposits. Therefore, the rapid erosion of the current Peace River Valley, caused relaxation of the valley walls along bedding planes, pre-sheared surfaces caused by tectonic forces, and perhaps glacio-tectonics. As the river level dropped, many of the slides became abandoned and some debris features were destroyed by river erosion.

Figure 8.28 Landslide Area 2 – Geologic Model



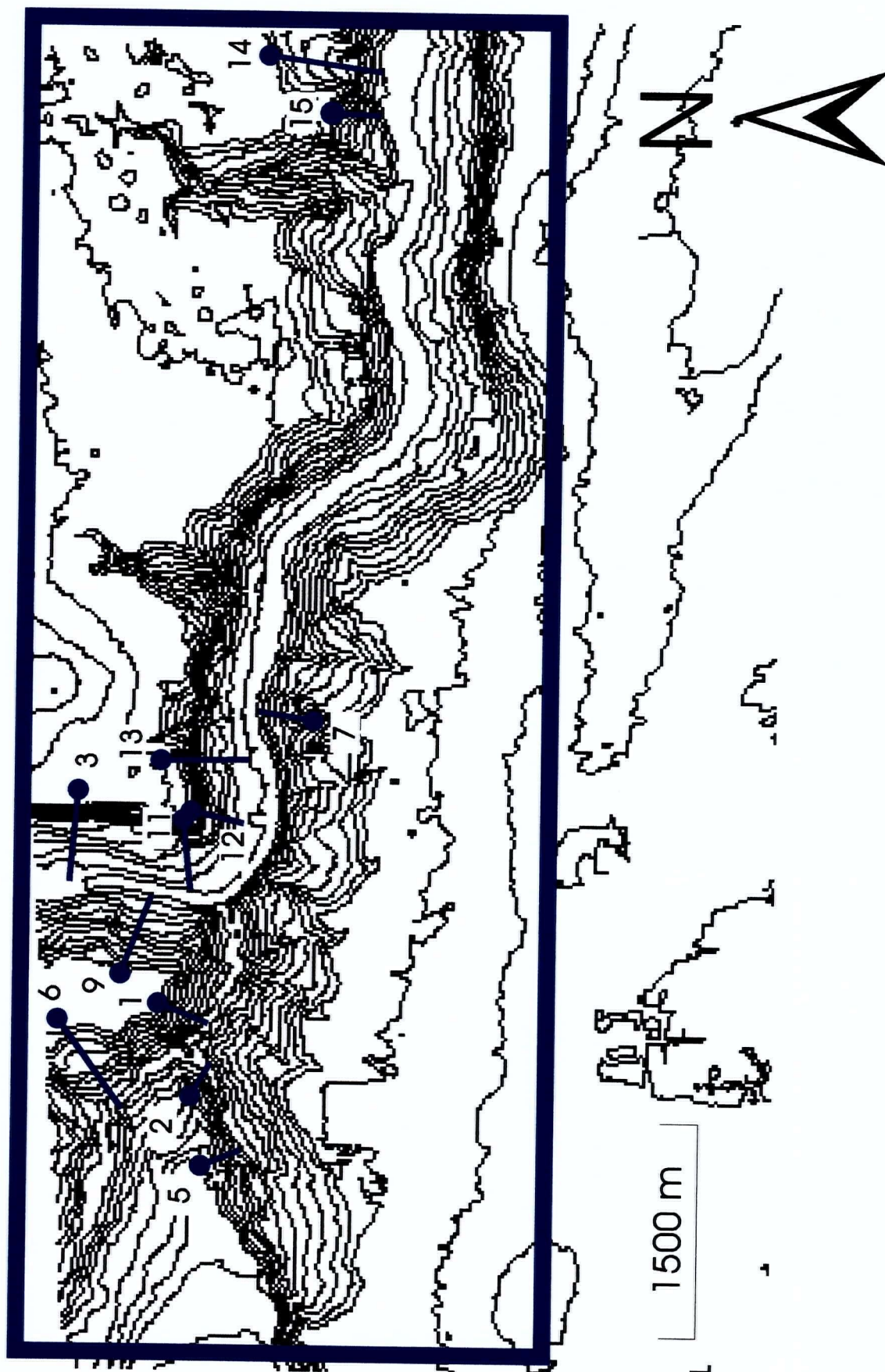
1) Bedrock dominated plain near edge of pre-glacial river valley; 2) Aggregating deltaic environment into encroaching ice-dammed lake followed by glaciation; 3) Drainage re-establishes in pre-glacial river valley; 4) Migrating river allows intact material to be eroded and failures to occur exposing weak strata; 5) Migrating river continues to expose weak strata allowing failures to occur; 6) Debris from failures acts as a stabilizing berm of unstable material, although erosion of debris may cause reactivation of failure plane.

8.3 Slide Area 3

8.3.1 Location

The third area chosen was approximately 12 kilometers long and located near the confluence of the Beatton River and St John Creek (Figure 8.29). The area encompasses the large bend in the river where general course changes from the south to the east, roughly 7 km to the northeast of the City of Fort St. John. It also includes most of the lower St. John Creek, Stoddart Creek, and some sites further downstream on the Beatton River. Within the area, 15 slides were profiled within the study site encompassing 5 kilometers of valleys. The area was chosen because of the extensive infrastructure located within it. It contains Cecil Lake Road 103, which has two bridges crossing the Beatton River and St. John Creek, a pump station owned and operated by Devon Gas Services, approximately 15 operating pump jacks, and several pipelines crossing the river valleys. One bridge crossing the Beatton River has been rendered inactive due to the failures making the approach unstable. The area constantly undergoes movements that affect the roadway and pipelines and is currently undergoing slope stabilization project. The area was also very accessible because of the amount of infrastructure. The south-facing slopes show little to no vegetation and steeper slopes while the north-facing slopes are heavily vegetated and generally much gentler slopes producing a distinct asymmetry to the riverbanks.

Figure 8.29 Landslide Area 3



8.3.2 Local Stratigraphy

As in many cases, when the study area is larger, the complexity of the stratigraphy is increased. The stratigraphy of Slide Area 3 is best described by dividing the area into three sections, the north and east walls of the Beaton River, the south and west walls of the Beaton River, and the valley walls of St. John and Stoddart Creeks.

The first section is the north and east valley walls and incorporates the left-hand side of the valley wall, facing the down stream direction. Six profiles were completed within this section. This section contains both Shaftesbury Shale and Dunvegan Sandstone (Stott, 1988) and is interpreted as being the boundary of the pre-glacial valley (Mathews, 1978). The Shaftesbury Shale extends to an elevation of approximately 560 meters a.s.l.; however, because the contact is gradational and exact elevation is hard to determine. The Dunvegan Sandstone reaches to an elevation of approximately 590 meters a.s.l. with a local high of 621 meters a.s.l. on landslide profile 3-13. Above the sandstone is a silt-rich unit of the pre-glacial sequence. This unit contains interbedded clay and sand on which the upper failures occur. This unit was not witnessed intact but rather inferred from the material within the colluvium and within the debris. This unit is capped by a till blanket which extends to an approximate elevation of 710 meters a.s.l. The post-glacial clay was not observed at this site. An approximate stratigraphic column is produced (Figure 8.30).

Four slide profiles were completed in the second section, the south and west valley walls, which is the right hand side of the valley when looking down stream. This section is within the pre-glacial valley (Mathews, 1978) and therefore the sandstone unit is missing and the shale unit has been eroded down to an elevation of 460 meters a.s.l.

Above the shale is a thin unit of gravels with a thickness of 2 meters. The gravels are clean, uncemented and has a reddish colour. Above the gravels are the pre-glacial bedded sand, silt, and clay beds observed to extend to an elevation of approximately 592 meters a.s.l. At this elevation, the unit becomes clay-rich and contains drop stones with the bedded material to an elevation of approximately 609 meters a.s.l. An approximate stratigraphic column is produced (Figure 8.31).

The third section has five profiles and encompasses the valley walls of St. John and Stoddart Creeks. This section has a similar stratigraphy to the south wall of the Beaton River. The shale was found at an elevation of 487 meters a.s.l on the north side of the valley and lower at 455 meters a.s.l on the south walls. This section is also thought to be within the pre-glacial valley (Mathews, 1978) and is therefore missing the Dunvegan Sandstone in all sections except the north wall of Stoddart Creek. The gravel unit is located above the shale with a thickness of 4 meters. The gravels are partially cemented and contain cross bedding indicating an aggrading condition during deposition. Above the gravels, with a gradational contact, is the pre-glacial sequence, dominated here by silt and clay. The sequence continues to an estimated elevation of 650 meters a.s.l. A till blanket of unknown thickness is located at the top of the plateau. An approximate stratigraphic column is produced (Figure 8.32).

8.3.3 Landslides

Slide 3-1

The first slide studied within this area was a multi-leveled failure on a south-facing slope with an azimuth of 198°. The profile began on the upper plateau and

Figure 8.30 Area 3 – Stratigraphic Profile – North and East Valley Walls (not to scale)

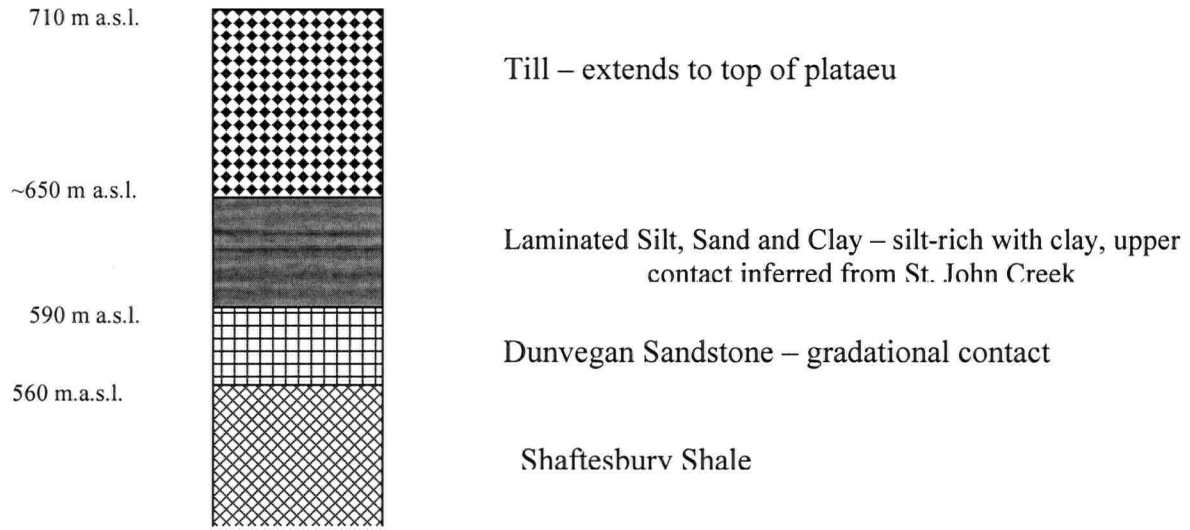


Figure 8.31 Area 3 – Stratigraphic Profile – South Valley Wall (not to scale)

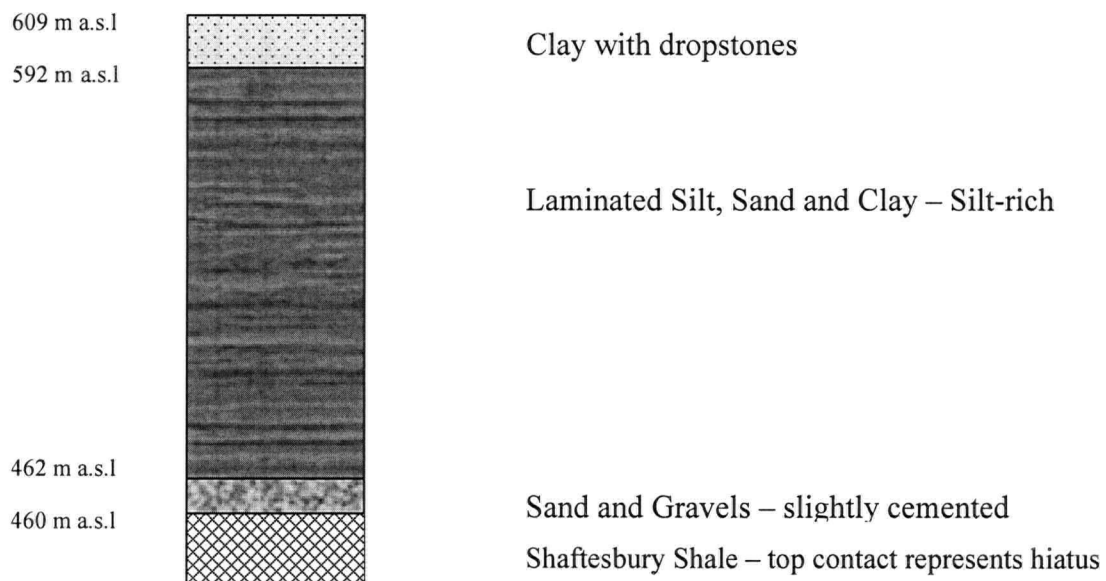
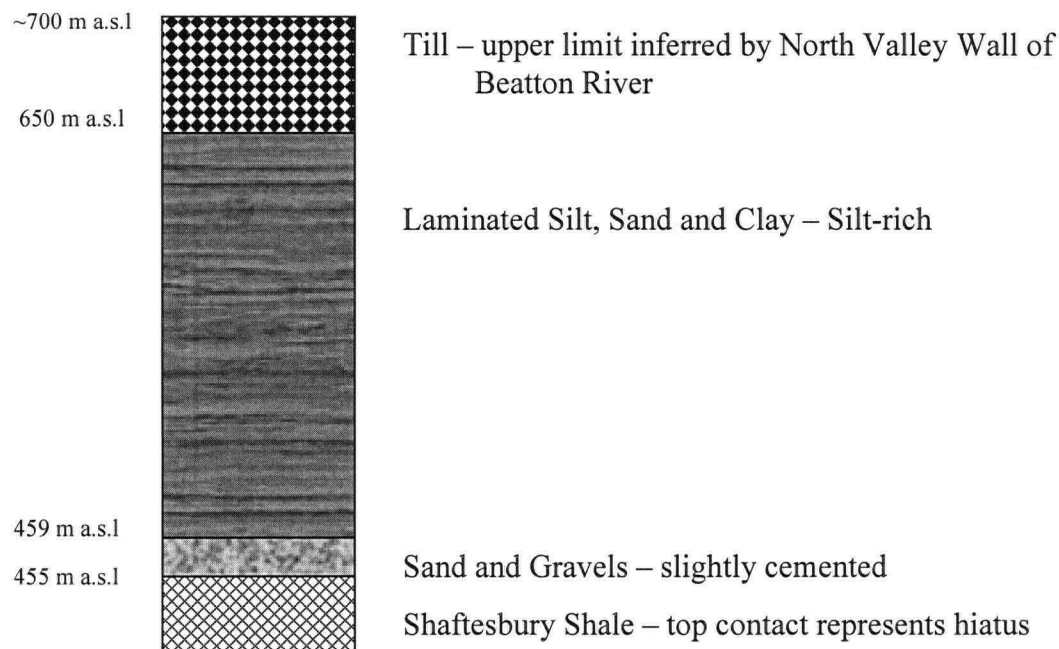


Figure 8.32 Area 3 – Stratigraphic Profile – Stoddart & St. John Creeks



headed down slope towards St. John Creek, near its confluence with Stoddart Creek (Figure 8.29). The slide has two failure planes both within the pre-glacial sequence (Figure 8.34). The top failure is abandoned and completely independent of the lower, very active bottom failure. The head scarp is completely grassed over and appears stable, though morphology indicates that this part of the slope was previously active. The upper failure plane was not observed but rather inferred from the terrain. The lower failure has fresh features even though the main scarp is grassy and appears stable. The lower failure is rotational with many scarps within the debris mass. The failure plane was observed with the vertical face visible from the roadway and sampled (sample 3-1A). Slickensided surfaces were located near the bottom of the pre-glacial unit, 3 meters above the gravel. The debris mass protruded up to 7 cm from the intact section. The shear plane consisted of 47% clay and 52% silt. The liquid limit of the layer is 54% and a plasticity index of

34%. The gravel unit extends for 5 meters until the shale was reached. The toe of the slide, on which a roadway is located, is remobilized debris. This toe has been armored at the creek level in order to stop the remobilization of the debris and to add stability to small slumps that threaten the roadway.

Slide 3-2

Slide 3-2 is multi-level failure located on an east-facing slope on Stoddart Creek near its confluence with St. John Creek (Figure 8.29). The profile begins along a knife-edge and continues down slope at an azimuth of 100°. This slide is a very active part of a larger, south-facing slide complex extending further to the west. The slide incorporates two failure planes, both within the pre-glacial sequence above the bedrock (Figure 8.35). Neither shear plane was observed, but rather their location is inferred from slide morphology. The failure planes are thought to be in the unconsolidated sediments because of the low elevation that the shale is found around the area. The shale outcrops only 2.5 meters above the creek level. The material within head scarp of the upper failure appears to have been disturbed from the abutting slide to the north. Even though the material is disturbed, it can still be noted that it is dominated by silt and sand with minor clay. The upper failure is a retrogressed rotational failure displaying back tilt of a slump block and scarps within the debris. This failure appears to be independent of a lower failure, which is more of a compound failure with the typical horst and graben extensional features. The toe of the failure had failed earlier in the summer of 2003 in as a rotational slump.

Slide 3-3

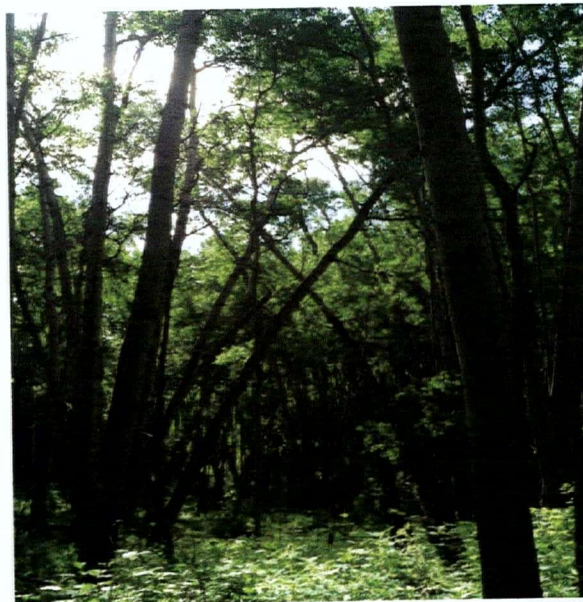
Even though this large landslide has two distinct failure planes, it is best classified as a compound failure (Figure 8.36). The failure occurs within the Beatton River Valley on a slope that has a western aspect (Figure 8.29). The profile begins within a field on the upper plateau and continues down slope at an azimuth of 262°. The failure is dormant with a few sections that are active to very active. The upper failure is a small rotational slump within a silt-dominated sequence of the pre-glacial unit that appears abandoned. This failure is above a steep face controlled by the Dunvegan Sandstone, which acts as a head scarp for the lower compound failure. The large pond at the base of the scarp abuts against a large horst block with smaller scale hummocky terrain within in. Other large-scale horst and graben features are seen further down slope including a second pond near the road, which services a pump jack within the debris. Sandstone blocks were seen through out much of the failure up to the location of the second pond. The toe of the failure is at a low angle and drops near the current river level. This is interpreted as the edge of the stable bedrock below. The failure is thought to be deep as the size of the features are quite large and must be in shale due to the presence of the sandstone strata. This type of failure is common along the Beatton River Valley within this section of the river.

Slide 3-4

This failure profile begins at the fence of a bison ranch south of Stoddart Creek (Figure 8.29) and continues down slope at an azimuth of 336°. The ranch is within the flat, stable plateau above the river valley underlain by the Dunvegan Sandstone. The profile follows along a seismic line that has cut through the dense vegetation on the

north-facing slope. The slide has some small active sections, although in general it would be classified as dormant. The slide is interpreted conservatively as having one dominant basal shear plane within the shale bedrock allowing for a large compound failure (Figure 8.37). This surface was not seen, but rather inferred from the slide terrain. No intact sections or failure planes were seen along this profile, however the bedrock elevation is projected from neighbouring intact sections. Much of the morphology of this slide is interpreted from the tree cover, as there was several sections within this slide that displayed a distinct cross tilting of the trees (Figure 8.33). The cross tilting is interpreted as an extensional feature where discrete blocks are separating from each other. Several muted horst and graben features were encountered during this profile supporting the compound interpretation.

Figure 8.33 Cross-Tilting of trees on Landslide Profile 3-4



Slide 3-5

The profile of Slide 3-5 on the south-facing slope occurs opposite of Slide 3-4 on Stoddart Creek (Figure 8.29). The profile starts upon the stable plateau above the creek near a cluster of pump jacks and continues down slope at an azimuth of 129°. This multi-level slide has three distinct failure planes independent of each other with the upper failure being abandoned and the lower failures being very active (Figure 8.38). The upper failure has no distinct features, however, from the shape of the slope the failure might have been a compound failure. The sandstone face below the failure indicates that the failure plane is within the pre-glacial sediments. The second failure, below the sandstone face, is a small rotational failure with a shear plane that is within the shale bedrock. The active debris shows minor back tilt although the features are fairly muted. There were sandstone blocks scattered throughout the debris of this section from the bedrock above and below. The shear plane for this failure was not found here although debris was observed directly above intact shale. The failure plane was located deep within the slope and could be sampled with a shovel; however, the same plane was sampled (sample 3-5A) approximately 200 meters giving enough field evidence to support that the failure plane was located at this level. This shear plane was found to consist mostly of silt, 68%, with minor clay, 19%. The Liquid Limit of the shear was found to be 53% with a Plasticity Index of 35%. The intact shale below the failure plane was heavily weathered and produces a large talus cone of raveled shale. A gouge layer within the shale was sampled (sample 3-5B) and tested. The gouge consisted of 32% sand, 58% silt, and 10% clay. The layer has a Liquid Limit of 52% and a Plasticity Index of 29%. Below this section, distinct horst and graben features were observed indicating a translational

movement of the debris. Near the river, intact shale was observed again signifying that the lowest shear plane is also within the shale and not simply remobilized debris.

Slide 3-6

Slide 3-6 is a multi-level failure occurring on a southwest-facing slope within the valley created by St. John Creek (Figure. 8.29). The profile begins at the base of a pump jack within a field on the stable plateau and continues down slope at an azimuth of 206 . The slide is classified as dormant although the upper sections are likely to be abandoned. The slide has three distinct failure planes independent of each other (Figure 8.39). The upper most failure is abandoned and has muted features making the failure type hard to distinguish. It likely a compound failure based on the slope morphology, however, no shear plane was found. A steeper section of the slope is interpreted as being a stable back scarp for a lower compound failure. The debris within this section shows large-scale horst and graben features with little to no back tilt observed. The slope in this section has a low angle until a large scarp is reached. This large scarp was vegetated with grass and shrubs and thought to be the head scarp for the third and lowest failure. Originally it was thought that the debris below this scarp was remobilized debris from the failure above, however, a failure plane was located within the shale near a vertical section by the creek. This lower failure is a retrogressed rotational failure displaying back tilt and scarps within the debris. As most of the slide had no exposure, the shale elevation was inferred by projecting nearby elevations found. The inferred shale elevation leads to the conclusion that the upper two failures occur within the pre-glacial material while the lower most failure occurs within the shale.

Slide 3-7

This is an active, multi-level failure on the north-facing slope along the Beaton River (Figure 8.40). The profile begins along a ridge between a gully towards the west and a large bowl-shaped failure to the east, and continues down slope at an azimuth of 350 (Figure 8.29). This failure has partially destroyed a road that approached a now decommissioned bridge making it impassable. This occurred some years ago and has now been partially reclaimed. There is still danger associated with this slope as there are still a few residents serviced by this road further upslope. The failure incorporates three failure planes all within the pre-glacial sediments. The upper most failure is an independent, dormant rotational slide. It is heavily modified as heavy equipment is in the process of moving earth and really can only be recognized by the hummocky terrain and the existence of shear planes found in the scarp of the next failure. The vertical face of the next failure is of intact clay-rich material with drop stones through the mass. A distinct shear plane was observed and sampled (sample 3-7A) for a slide that occurs next to the one profiled. A similar failure plane likely exists for the above failure, however was not actually observed. The second failure shows back tilting and scarps within the debris indicating a retrogressed rotational failure. The failure plane for this section was not observed either, although because the section is active, it can be inferred due to the fresh features and the total lack of shale within the debris. An intact scarp below the debris is interpreted from a small scarp within the debris that begins a steeper section of the slope. It is unknown whether this failure is within the Quaternary sequence or within the bedrock, however it is believed to be within the pre-glacial progression as little to no shale was found within the debris. This is the approximate location of the intact road

section on the slope. The toe of the failure is interpreted as being remobilized debris from the above failures. This failure is located on an outlying section of pre-glacial material, as failures have retrogressed the slopes on either side of this section.

Slide 3-8

Slide 3-8 is compound failure on a north-facing slope along the Beaton River (Figure 8.41). The slide profile starts on the plateau above the river and mostly follows the edge of a pipeline right of way down slope at an azimuth of 340°. The failure is dormant and many of the slide features are small and muted. The profile was taken 20 meters off the right of way so that disturbance from the pipeline would not be added to the data. The slope is interpreted as having one major basal shear plane within the shale and another near the top within the unconsolidated material. The upper shear plane was not actually observed, although the surface expression of the slope would indicate that a failure had occurred in that location. The only section of intact material observed was at the bottom of the slope within a ravine less than 10 meters of the profile. The ravine revealed a section of intact shale that was rubbly and slaked. Near the bottom of the shale section, more intact shale was found. Within this more competent shale, several shear planes were located suggesting that the shale was undergoing creep within this section. This suggesting the presence of a deep-seated shear plane on which the shale mass is moving, albeit, very slowly. Above the shale was a slightly disturbed bed of sands and gravels, common to this side of the river valley. The material was loose and easily excavated. The controlling shear plane of the failure is thought to occur below this and movement along it must be very slow as not to cause the gravel bed to be disturbed more

than it was. The material above the gravel was bedded silt with plumes of gravel intruding from below.

Slide 3-9

This is a multi-leveled failure with three distinct failure areas (Figure 8.42) on an east-facing slope across from Slide 3-11 within the Beaton River valley. The profile for the slide starts upon the stable plateau on an access road and heads down slope at an azimuth of 80 (Figure 8.29). This slide is entirely dormant and is currently used by recreational vehicles. None of the shear planes were observed, however their location was inferred from surface morphology. The upper two shear planes are thought to be within the Quaternary materials while the lower failure is thought to be in shale. The lower shear plane was placed in the shale due to the amount of shale within in the debris near the base of the slope. Shale was not witnessed in other sections of the slide. The upper most failure is dormant and many of the features were either muted or disturbed to make a decision on what type of failure it may have been. The debris of the middle failure displayed back tilting with little to no extensional features so it was likely a rotational failure. The lowest compound failure was longer than the upper two and did show horst and graben features with little rotation. The debris at the toe of the slide was vegetated and appeared to be stabilized debris from the above failures. The elevation of the shale is low in this area due to the pre-glacial valley so many of the shear planes in this area are within the Pleistocene sequence.

Slide 3-11

Slide 3-11 is a large compound failure occurring on a southwest-facing slope within the large bend of the Beaton River Valley (Figure 8.29). The profile begins on the corner of the upper plateau above the river and heads down slope at an azimuth of 241 . The failure is dormant and is controlled by one dominant failure plane (Figure 8.43). The slope morphology suggests a second failure plane that may be in the intact material or within the debris. The main head scarp is covered in grass and colluvium and shows signs of minor shallow slumping. The debris close to the head scarp shows decided back tilting and translational horst and graben features further down the profile. The toe of the failure is modified for the right of way of a pipeline. If the debris remobilized along the shear plane, this pipeline would be under extreme hazard. Near the river, a series of scarps within the debris show that the debris has moved in the past and may move again in the future.

Slide 3-12

This slide profile is on a south-facing slope at the large bend within the Beaton River Valley on an azimuth of 184 (Figure 8.29). The profile begins within a field and continues along a pipeline right of way down slope and ends at the gas plant fence line. The slide, like others around it, has one dominant basal shear plane and a smaller, independent failure above the bedrock face (Figure 8.44). The failure is dormant with no recent deep-seated movements, although there are some shallow failures within the colluvium cover forced at an unstable angle due to the underlying bedrock. The upper shear plane was not observed as the slope had a thin cover of colluvium but the slope morphology suggests an abandoned slide. Several reflectors were set up on the slope to

monitor movement by an unknown party. An intact sandstone scarp was encountered, beneath which debris from the deep-seated failure within the shale begins. The bottom part of this failure mirrors that of Slide 3-13, although the features along this profile were much more muted than the neighbouring slide. The roadway also crosses this profile increasing the risk of this failure but more importantly, the gas plant is located on the debris of this old failure. The profile was truncated by the presence of the plant and finished from topographical data.

Slide 3-13

Slide 3-13 is a large failure on a south-facing slope along the Beatton River near a pump house for a pipeline that transects the area (Figure 8.45). The profile starts near a pump jack up on the stable plateau within a field and continues down slope at an azimuth of 344°. The failure resembles Slide 3-3, in that it should be classified as a multi-level failure but because a majority of the movement occurs along one shear plane, it can be considered a compound failure. The upper two failures that occur within the Pleistocene unit are dormant and covered in light vegetation. They are compound failures and neither shear plane was witnessed but rather inferred from surface morphology. They are believed to occur within the Pleistocene sequence, as the vertical face below is composed of Dunvegan Sandstone and not likely to failure. The dominant lower failure is a compound failure occurring along within the Shaftesbury Shale. The debris has recently become active, failing along the same deep-seated shear plan. A sample of the remolded shale was collected (sample 3-13A) and found to be 78% silt and 19% sand. The Liquid Limit of the shale is 42% with a Plasticity Index of 24%. The debris is very hummocky with major ponding occurs where is an intact scarp beneath the debris. The toe of the

slide has been disturbed as the debris that blocked the roadway was removed and used as fill in another location. The landslide also damaged a transmission line and forced one of the wells off line.

Slide 3-14

Slide 3-14 is on a south-facing slope of the Beaton River approximately 7 kilometers down stream of the St. John Creek confluence (Figure 8.29). The profile begins on the stable plateau near the edge of the slide and continues down slope at an azimuth of 180°. This is a multi-level failure with three shear planes with the upper section being abandoned and lower two being active (Figure 8.46). The upper most failure is distinct and should be considered of a special type. It is bowl-shaped failure with a very low angle, somewhat resembling quick clay failures occurring within the Leda Clay in Eastern Ontario and Quebec (Mitchell, 1977). The failure has the appearance of an abandoned meander scar, however there is a distinct set of sub-parallel ridges, thought to be debris scarps. The interpretation of the upper failure is that it is a shallow retrogressed rotational failure that has receded for an extensive distance with highly mobile debris. The failure has occurred within the pre-glacial unit, which makes it unique. Most of the failures that have this morphology within the map sheet occur within the post-glacial clay. The middle failure is a retrogressed rotational slide that occurs within the shale unit. The failure plane was sampled (sample 3-14 A, B, C) and a thin unit of remobilized shale was found above the bottom shear plane. Both the shear plane found in the shale (sample 3-14A) and the remobilized material (sample 3-14B) consisted dominantly of silt 77% and 74%, respectively and minor clay, 22% and 25%. Both materials have a Liquid Limit of 41% and a Plasticity Index of 21%. The debris contains

till, sand, silt, and pockets of weathered shale. Near the edge of the slide, an intact layer of cemented gravel was found above the location of the shear also supporting the location of the failure plane within the shale. Below the middle failure, an intact scarp in which the failure plane daylights, is made of bedded shale. At the base of this scarp is a large, intact, rotated block of shale indicating that a compound failure below the scarp. This bottom failure plane was not observed, however it can be placed with confidence within the shale. The debris mass is very disturbed and is mostly made-up of shale with minor a minor amount of till and sandstone. The underlying intact shale bed abruptly stops where the debris thins toward the toe.

Slide 3-15

Slide 3-15 is on a south-facing slope of the Beatton River less than 1 kilometer up stream from Slide 3-14 (Figure 8.29). The profile starts at the river level and continues up to the plateau at an azimuth of 350 . This is a multi-level slide incorporating two independent failures, with the upper section being abandoned and the lower section being active (Figure 8.47). The upper failure has a steep, grassy head scarp with a disturbed and back tilted debris mass. The failure by itself could be described as a single rotational failure. The shear plane of this failure was not observed as it likely daylights behind the lower grassed scarp below the ledge. The total lack of sandstone and shale within the debris, allows the assumption that the failure is within the pre-glacial unit to be made. The grassed scarp below the first failure is the head scarp of the active lower failure. The lower failure is a compound slide with a shear plane with in the shale. This shear plane was located and sampled (sample 3-15A) in the vertical face below the second debris mass. The shear plane consists of 22% sand, 57% silt, and 21% clay. The Liquid Limit of

the sheared layer is 55% with a Plasticity Index of 40%. A distinct layer of remolded shale was also observed and sampled (sample 3-15B). This layer had high plasticity with a plasticity index of 51% and a liquid limit of 80% and contained fragments of intact shale. Above this layer, a disturbed layer of gravels, often found in pockets or included was found within the debris above suggesting that the movement was slow enough not to completely disturb the pre-slide stratigraphy. The failure occurred near the top of the shale unit with the section of disturbed shale being mostly transported out of the slope. Secondary shears also occurred within the Pleistocene sequence, however these were not the controlling structures within the slope.

Figure 8.34 Landslide Profile 3-1

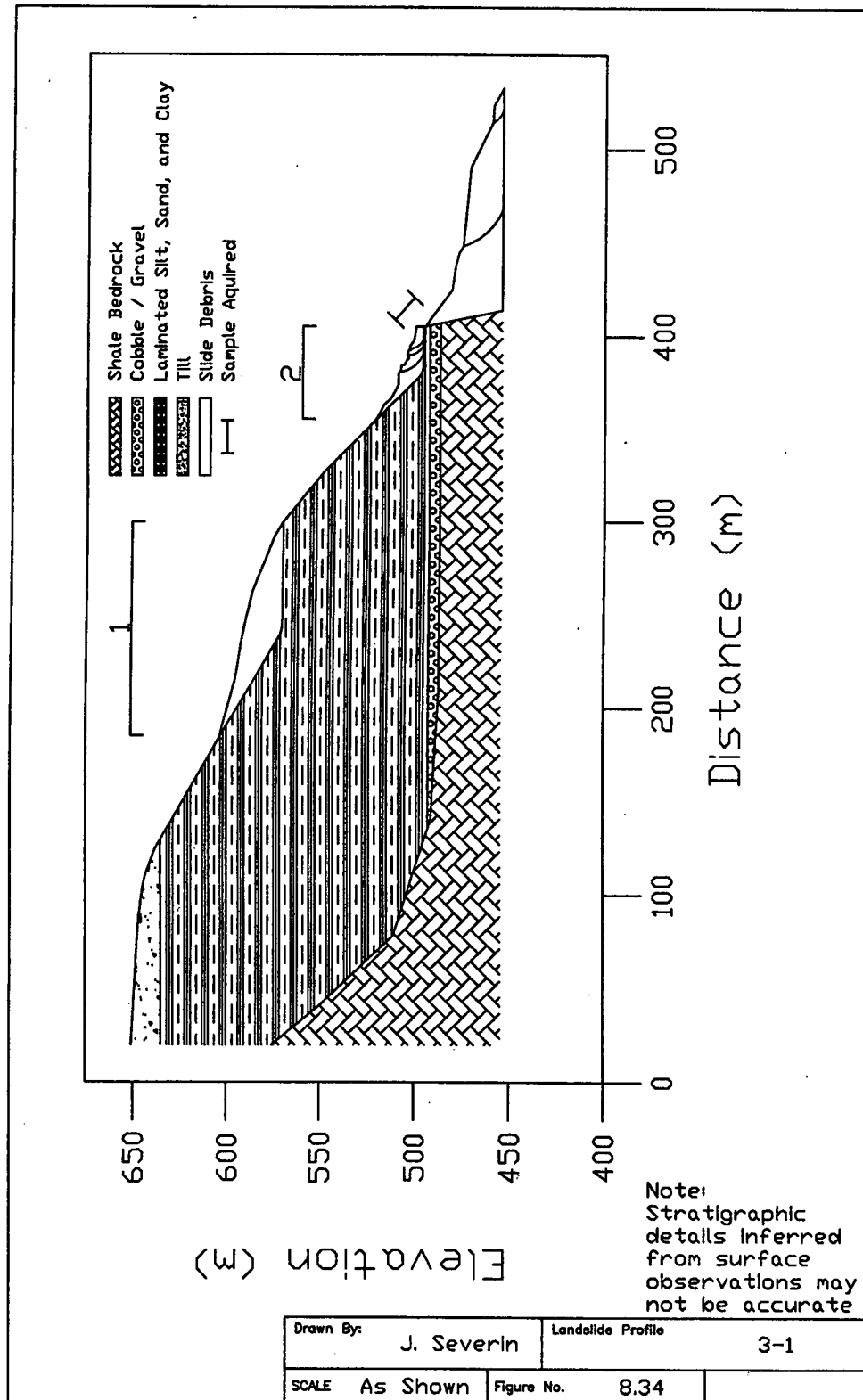


Figure 8.35 Landslide Profile 3-2

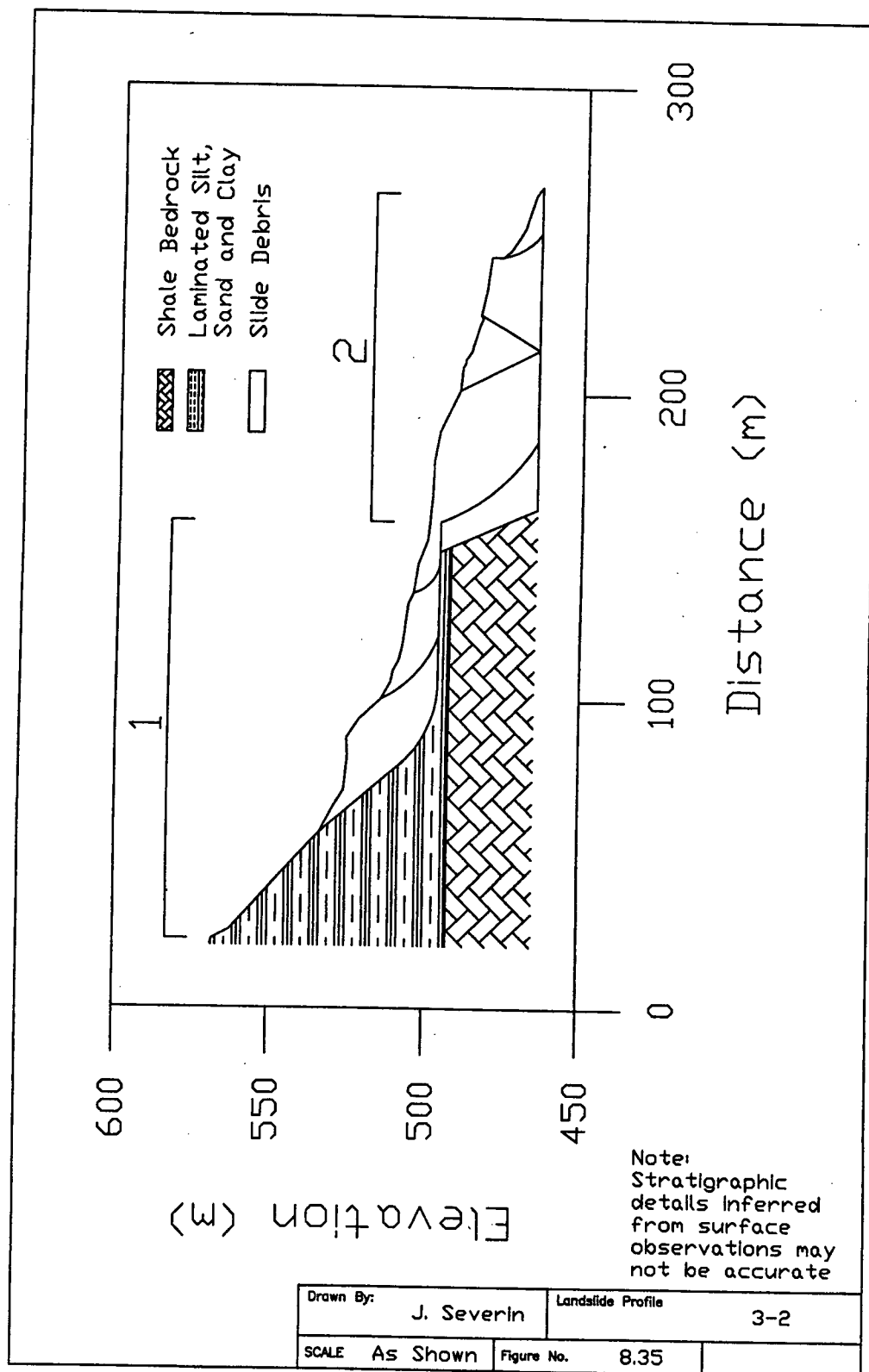


Figure 8.36 Landslide Profile 3-3

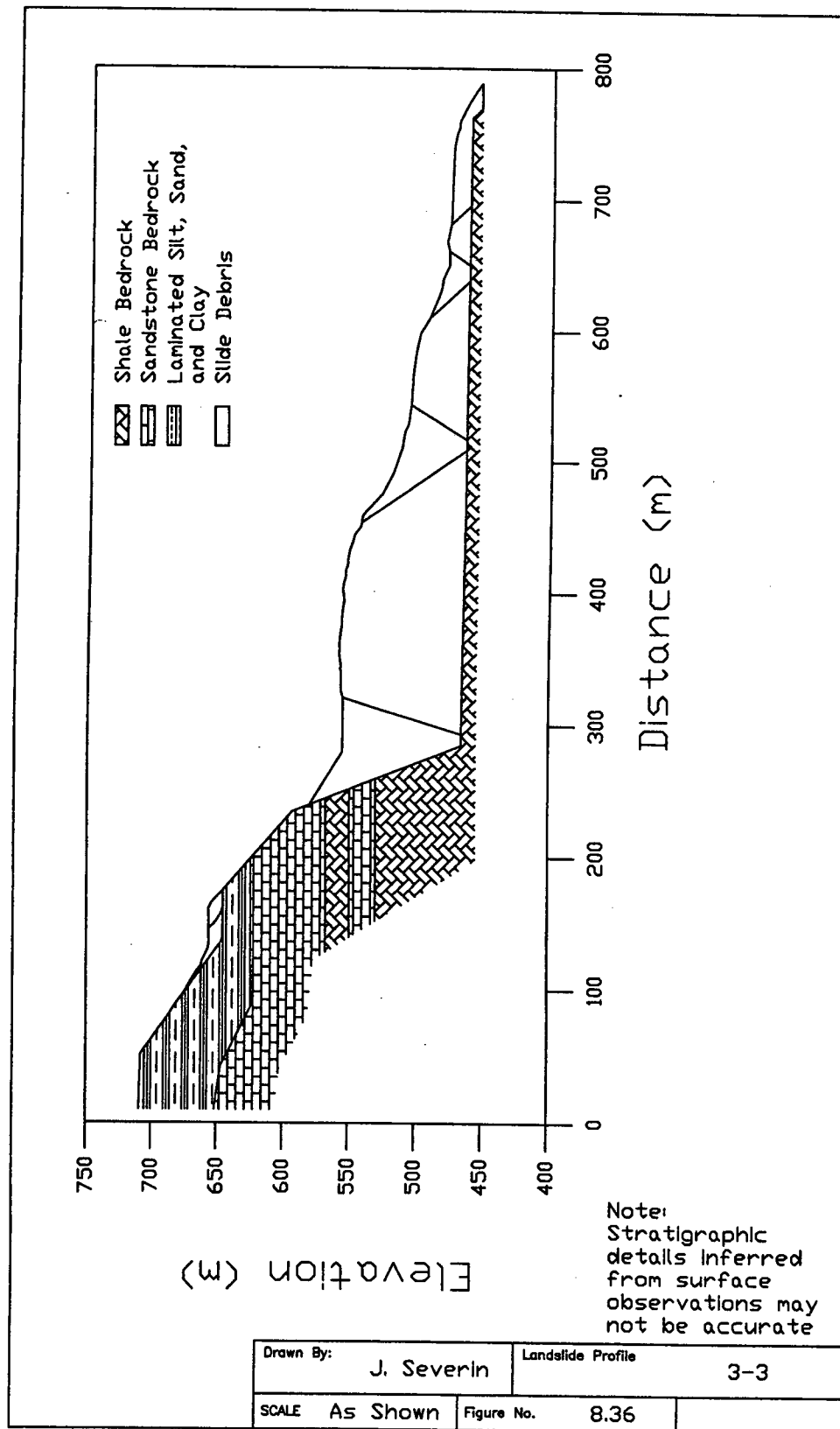
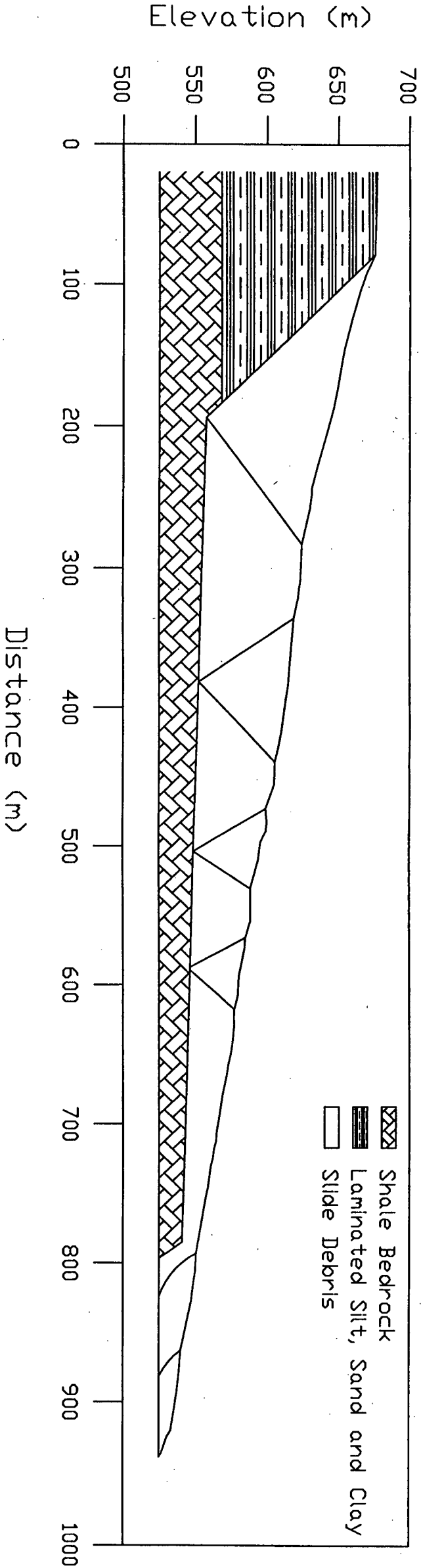


Figure 8.37 Landslide Profile 3-4



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By:	J. Severin	Landslide Profile	3-4
SCALE	AS SHOWN	Figure No.	8.37

Figure 8.38 Landslide Profile 3-5

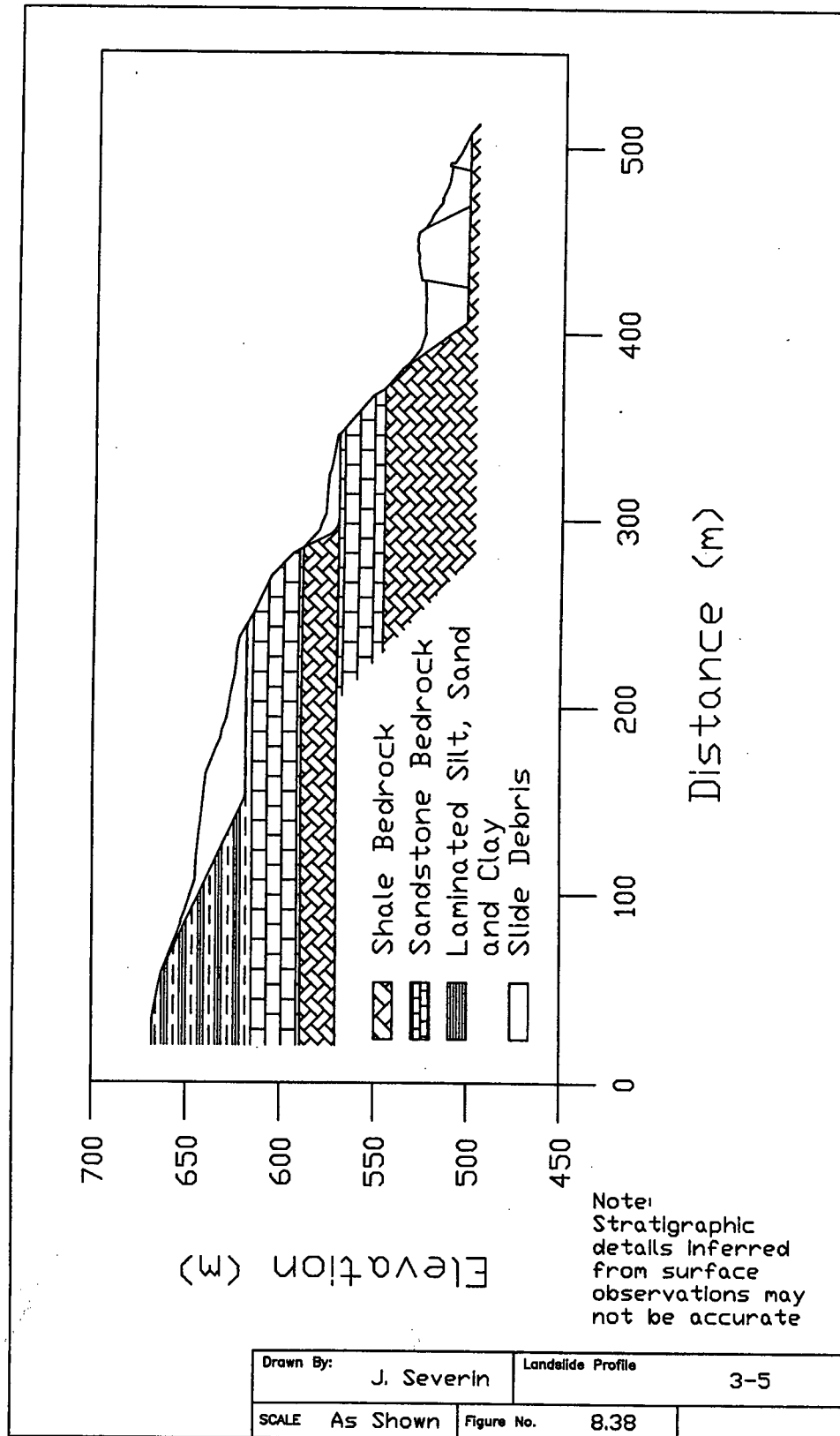
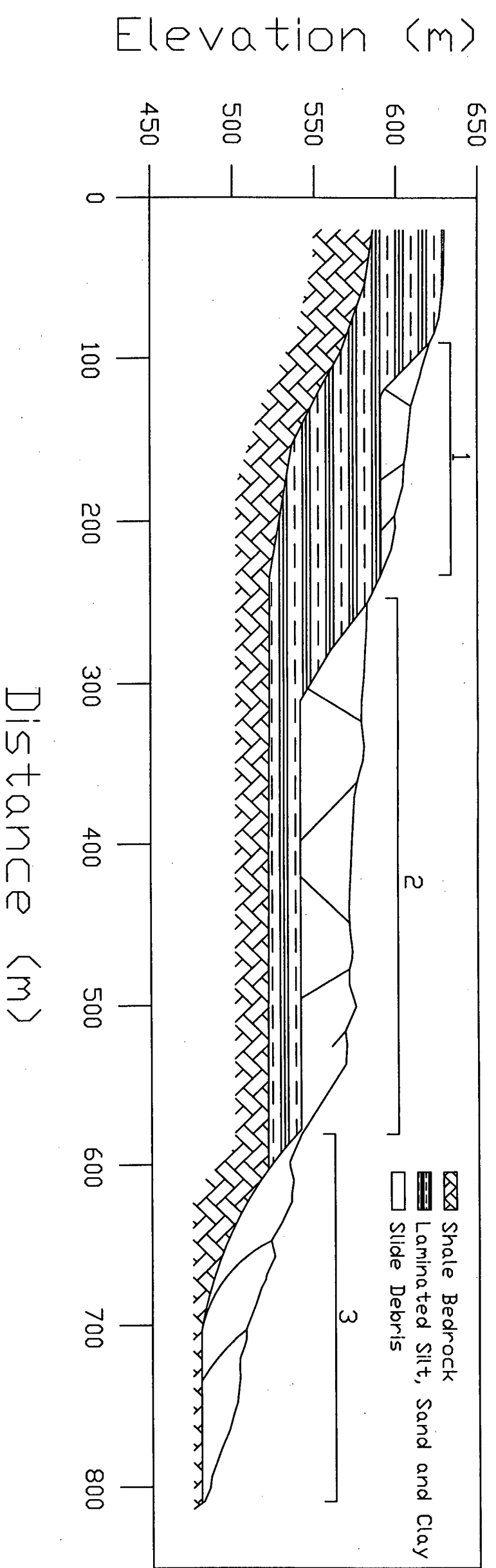


Figure 8.39 Landslide Profile 3-6



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By: J. Severin		Landslide Profile	
		3-6	
SCALE	As Shown	Figure No.	8.39

Figure 8.40 Landslide Profile 3-7

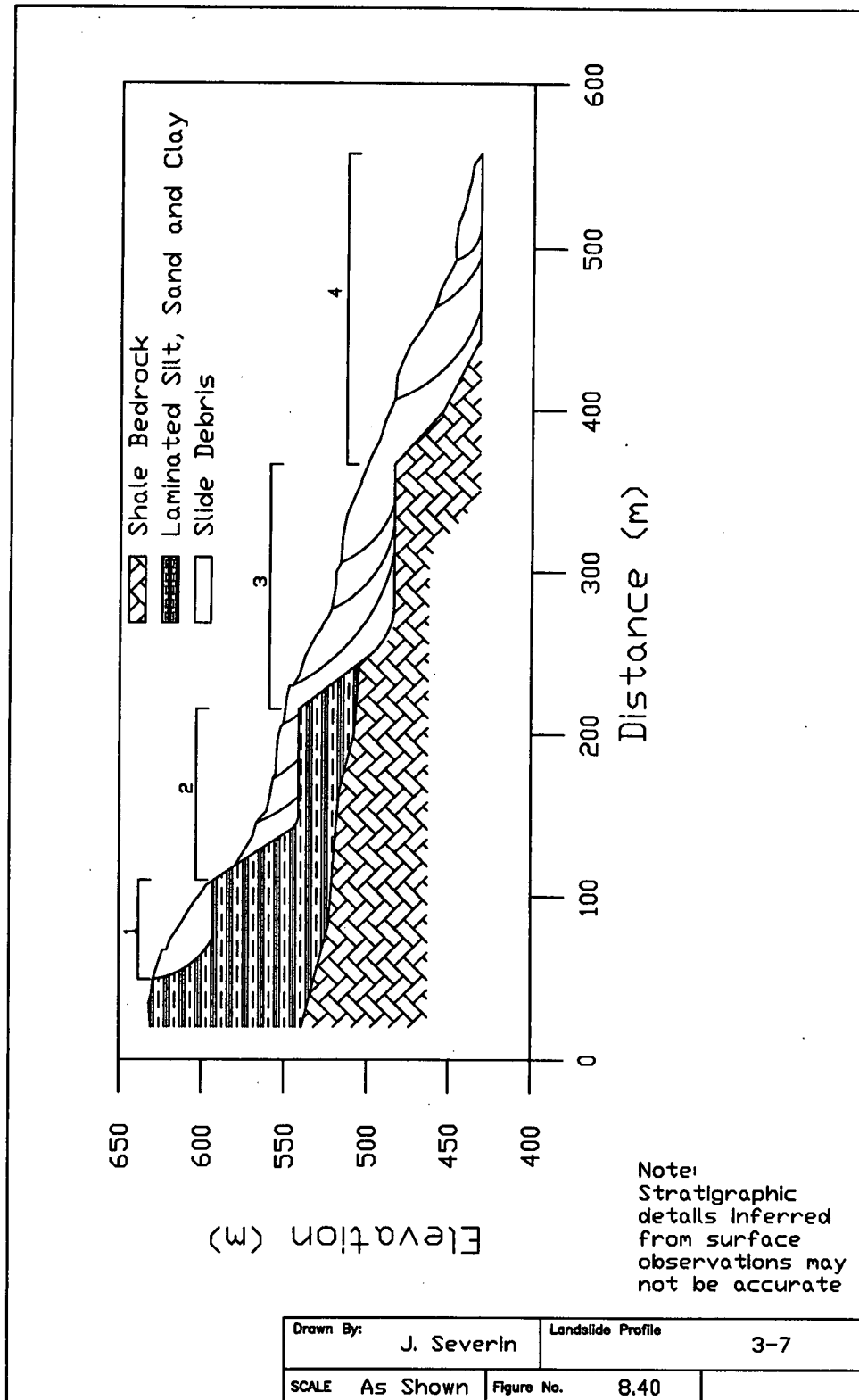
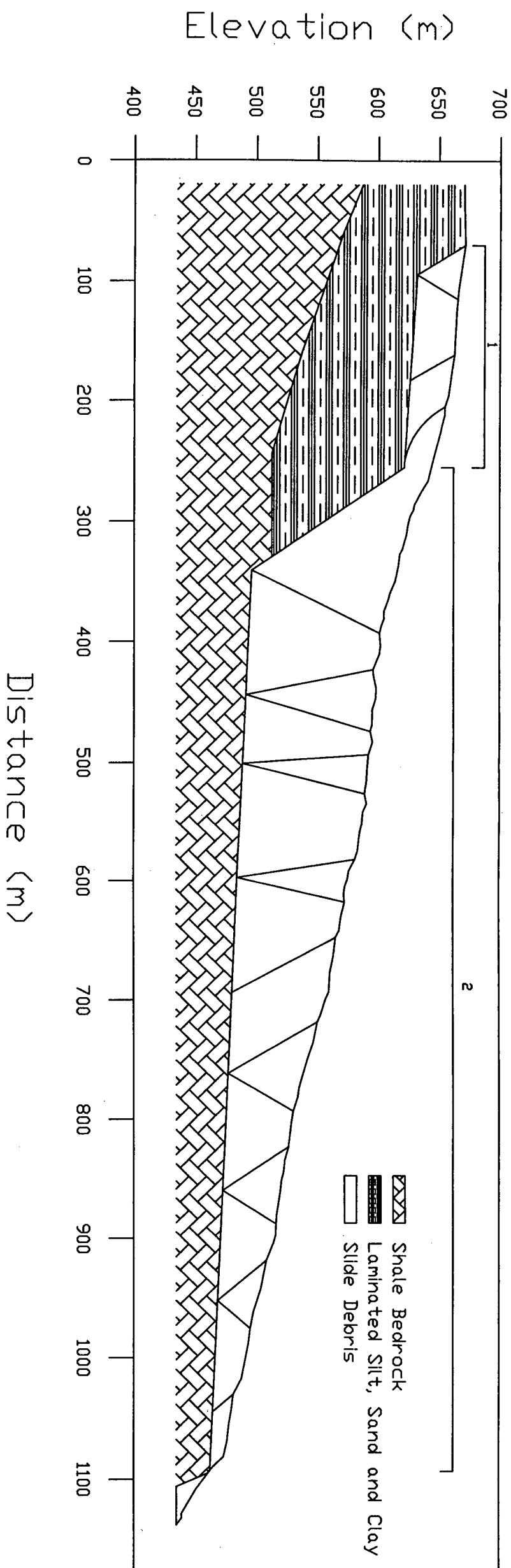


Figure 8.41 Landslide Profile 3-8



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By:	J. Severin	Landslide Profile	3-8
SCALE	As Shown	Figure No.	8.41

Figure 8.42 Landslide Profile 3-9

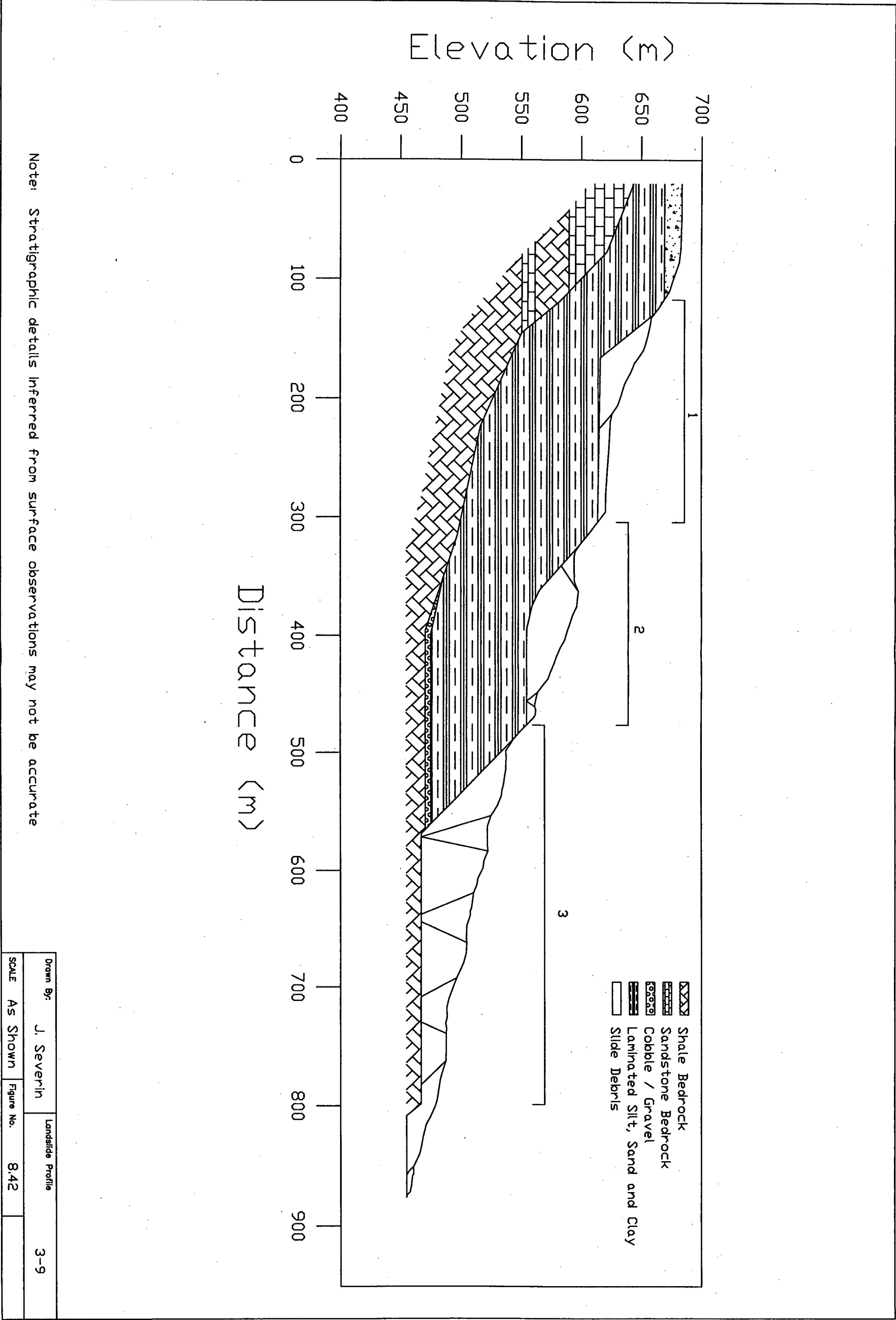


Figure 8.43 Landslide Profile 3-11

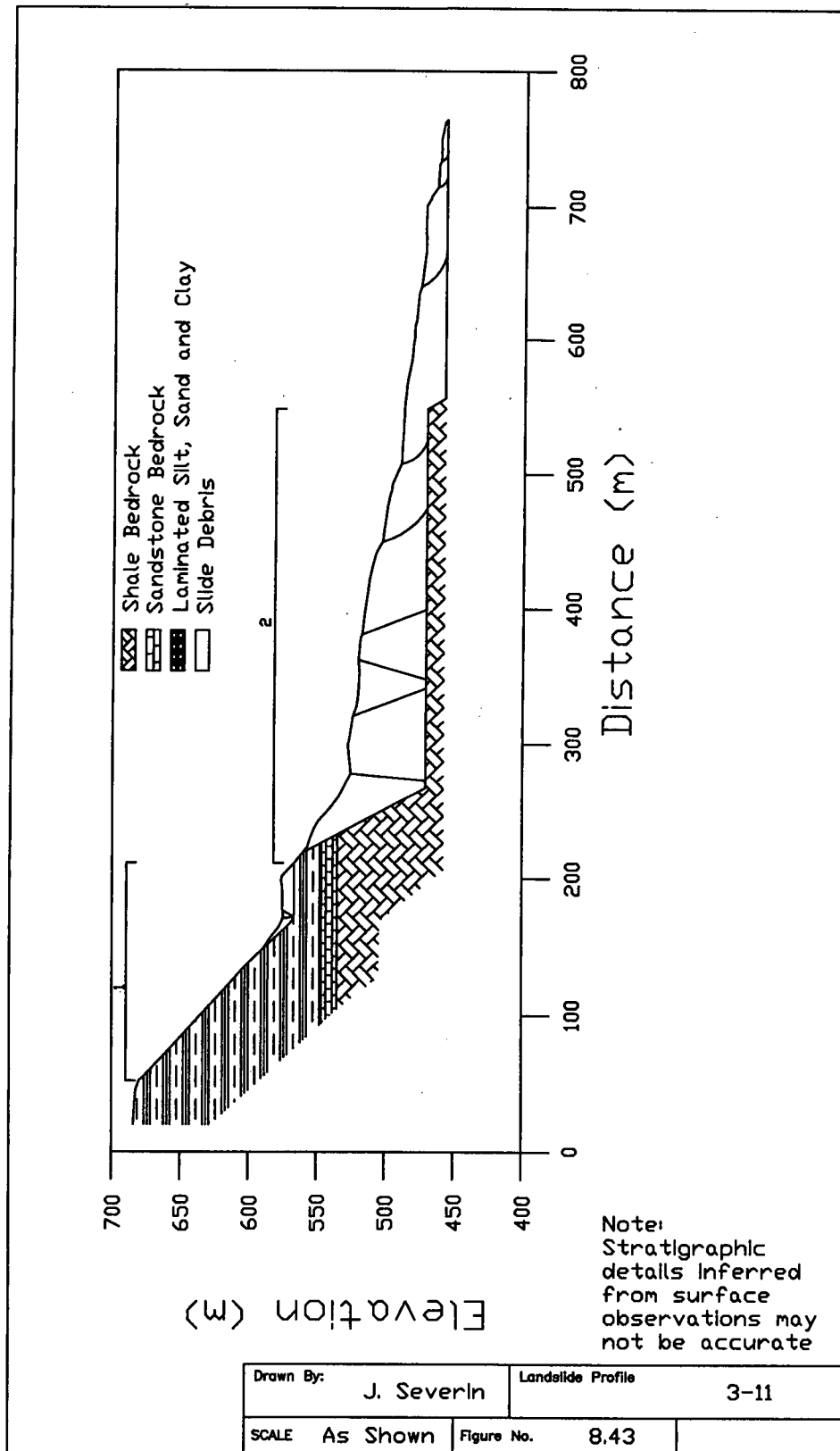


Figure 8.44 Landslide Profile 3-12

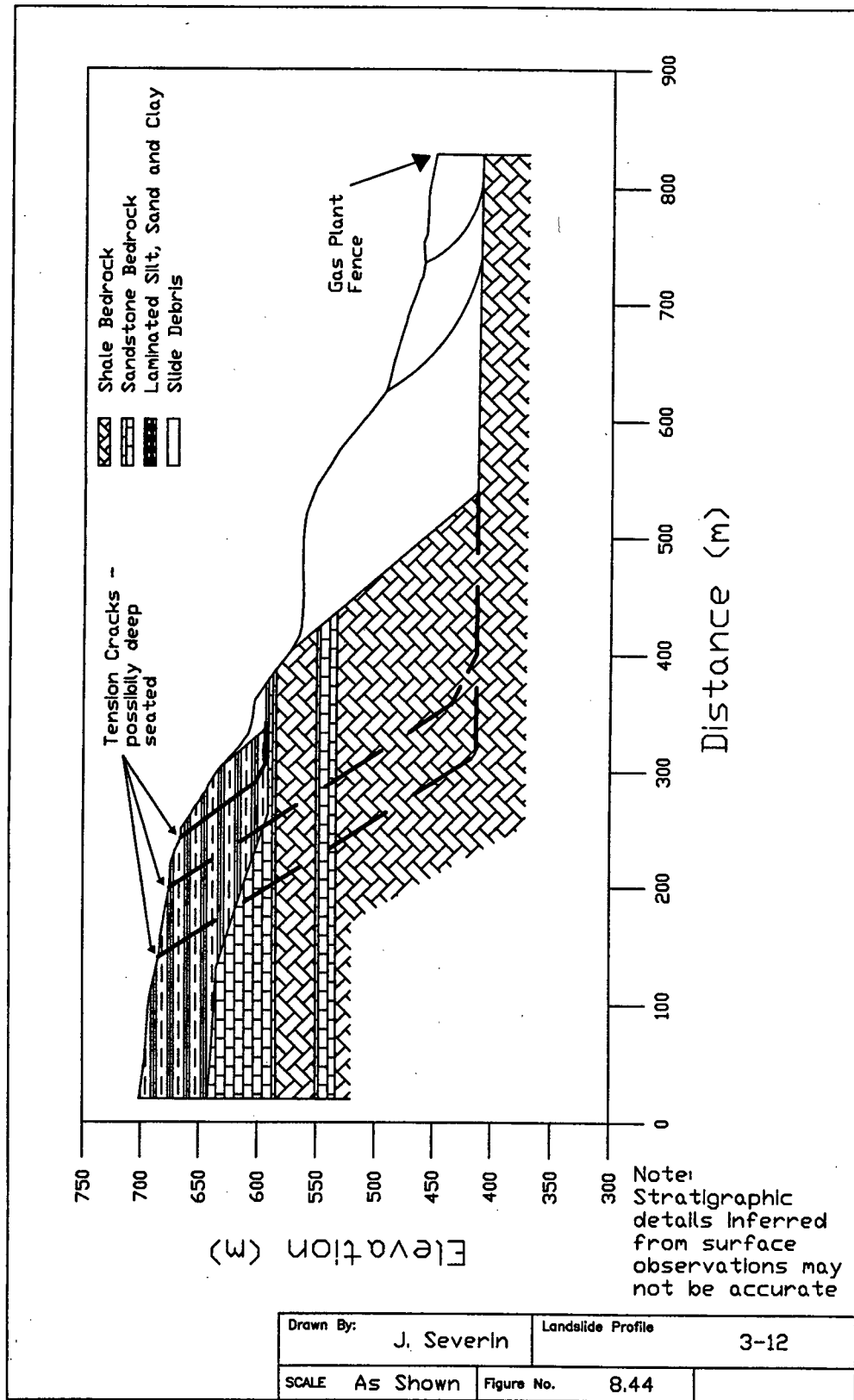


Figure 8.45 Landslide Profile 3-13

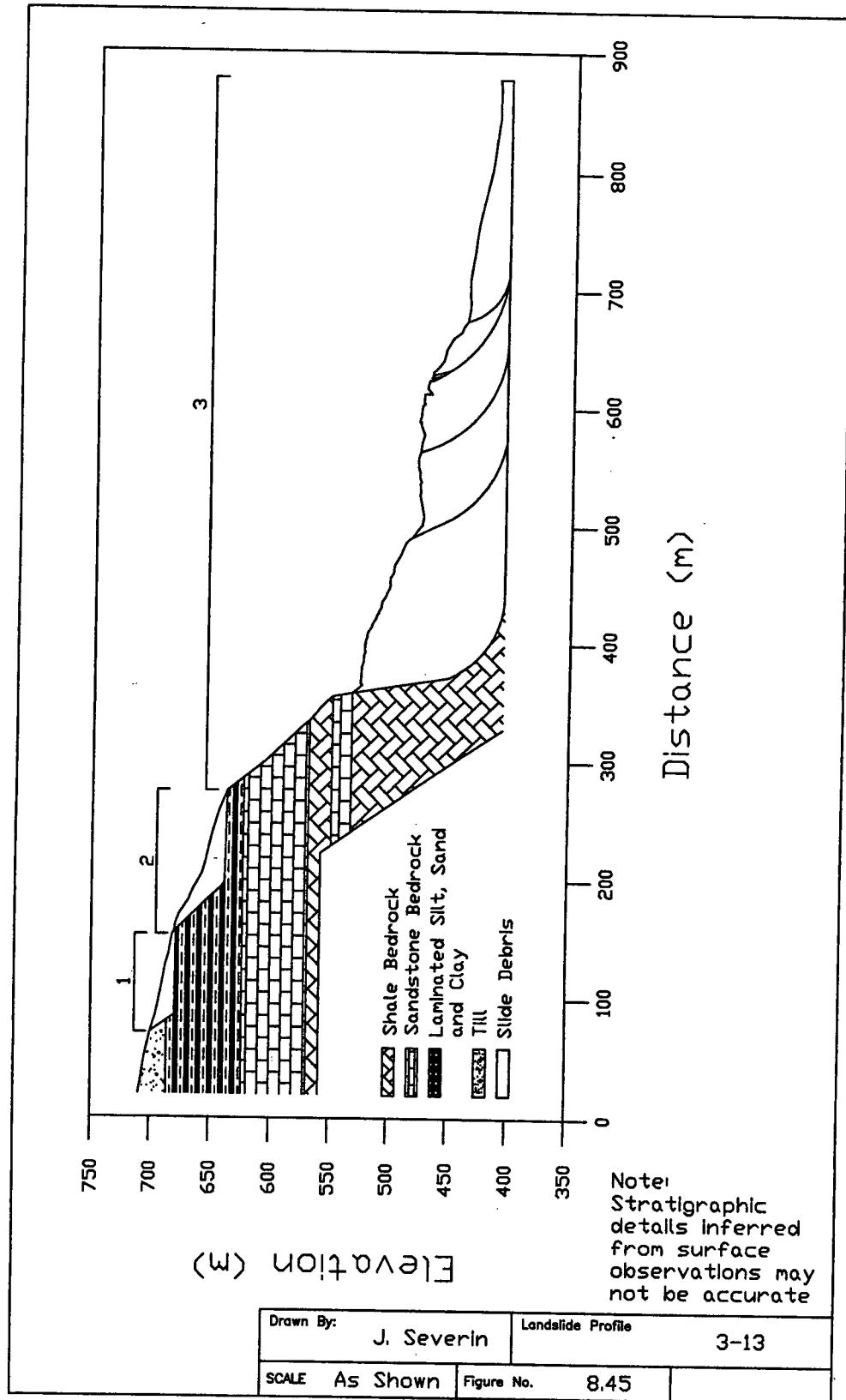
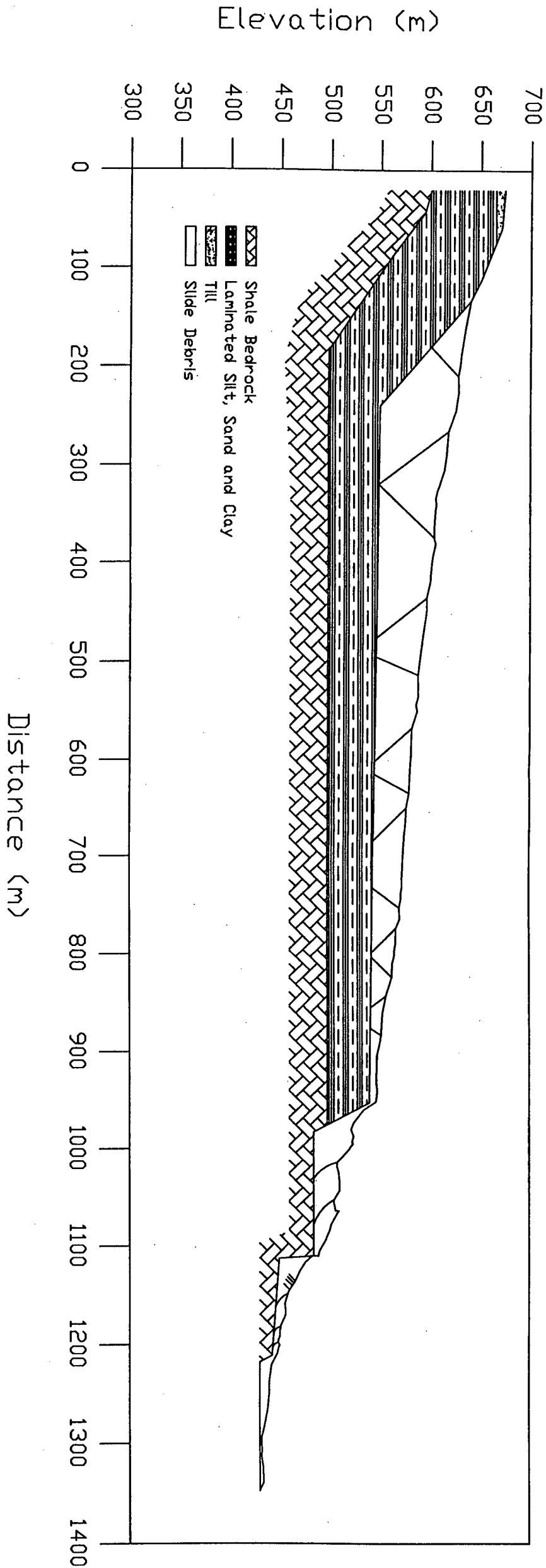


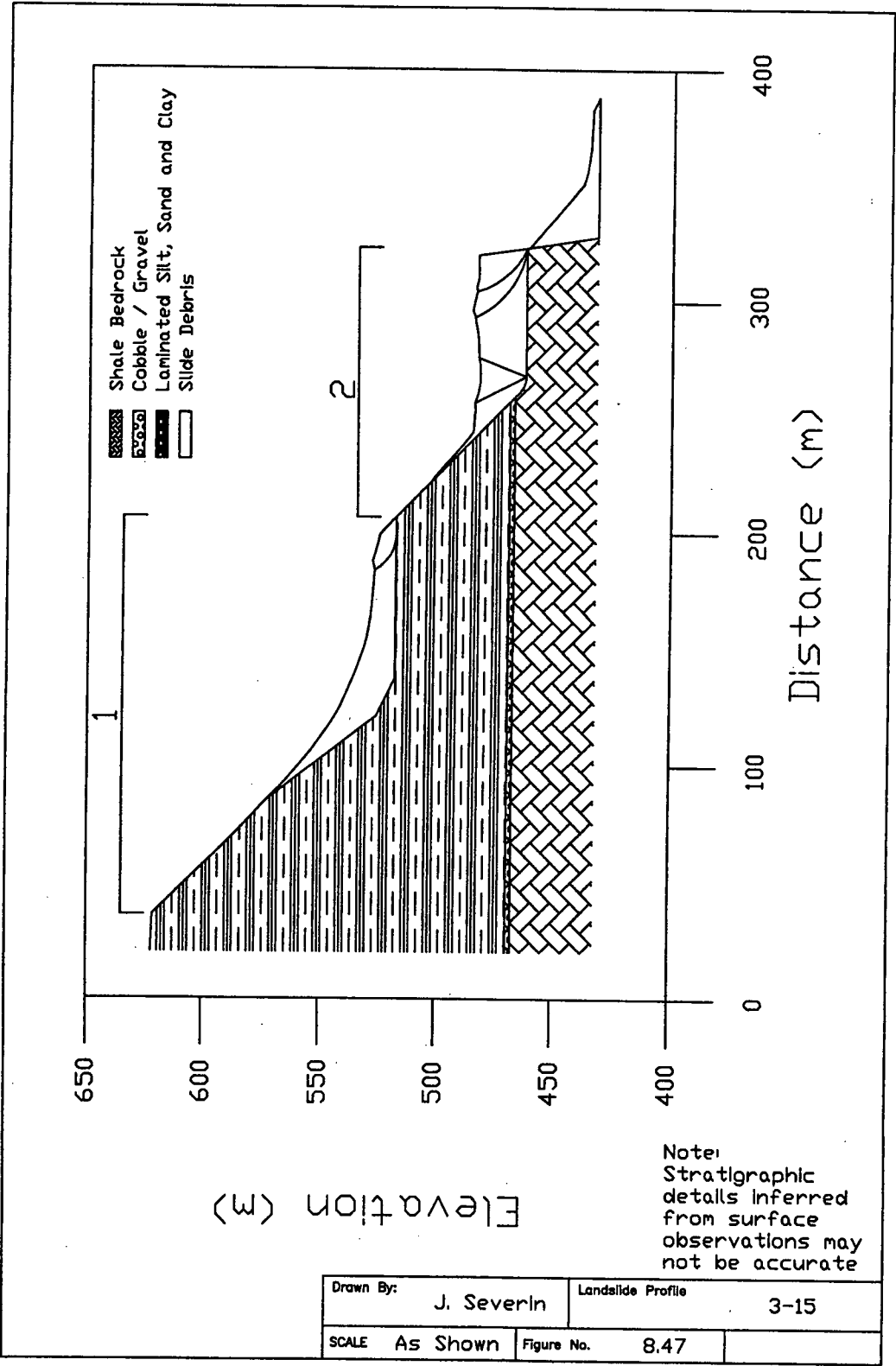
Figure 8.46 Landslide Profile 3-14



Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By: J. Severin		Landslide Profile	
SCALE AS Shown		Figure No.	8.46
		3-14	

Figure 8.47 Landslide Profile 3-15



8.3.4 Area Model

The most interesting feature of this slide area is the marked asymmetry of all the river valley walls. Three models are presented to explain both the landslides in the area and the valley wall asymmetry. The three models presented are for; 1) south-facing slopes, 2) north-facing slopes, and 3) slopes that are at an oblique angle to the previous two.

South-facing Slopes (Model 1)

There are six profiles within the area that have south-facing slopes, four on the Beatton River, and a single profile on each of St. John and Stoddart Creeks. Common features seen in all of these slopes are that they all contain multiple failure planes, have steep overall angles, and are only lightly vegetated.

When the north wall is outside of the pre-glacial valley boundary, the failures show a dominant basal failure plane in the shale and smaller failures within the Quaternary sequence. Since the shale has very weak and often pre-sheared bedding planes, a long compound failure would be expected. This does not occur in the south-facing slopes of the Beatton River because of the regional dip of the strata and the presence of the overlying sandstone. The Cretaceous bedrock dips gently to the northeast, (into the slope) at a value of 5° . If the slope were made entirely of shale, then there would be several short failure surfaces utilizing the bedding planes even with the bedrock dip into the slope. This insignificant angle provides enough friction to make a long compound failure impossible, but would still allow for a steeper failure. The sandstone cap and sandstone interbeds within the shale acts as a stabilizing factor allowing for a much steeper angle and to act as a cohesive agent to keep the material above a weak

plane from failing. Lower in the slope, where the sandstone interbeds occur less frequently, the weak bedding planes are mobilized making deep failures with steep head scarps possible (Figure 8.48).

The initial river valley (1) was filled with ice-dammed pre-glacial sediments and eventually glaciated (2). The formation of the failures occurs by an initial stage of river erosion near the edge of the pre-glacial valley (3) causing the nearby pre-glacial sediments to fail (4). Eventually the river erodes to a weak layer that can be mobilized (5). The slide may utilize a slip surface caused by horizontal stress relief during the valley rebound process (Matheson and Thomson, 1973). After the slope fails, the river adjusts its course around the debris and active erosion takes place (6). After enough erosion has occurred, a remobilization of the debris will occur (7). Eventually the failure plane will be allowed to fail again after the buttressing debris is removed (8). The local dip and the sandstone layers allow the south-facing slopes to stand at much steeper angles than their north-facing counterparts and only allow for short failure planes.

When the north wall is inside the boundary of the pre-glacial valley, a number of situations could occur. If the shale is stable and the failure planes are only within the silt and clay, then the failures can occur at any level and often several failure planes are mobilized. The failure planes still appear to be much shorter and there is an absence of the long compound failures. When the basal failure is in shale, the shear surface is short due to the local dip making a forced rotational failure. If the dip were not present, the failure plane would retrogress further into the slope.

North-facing Slopes (Model 2)

There are three slide profiles on south-facing slopes within the area, two within the Beatton River and one within Stoddart Creek. This section is wholly within the pre-glacial valley (Mathews, 1978) and is also affected by the process of valley rebound. In general, the south facing slopes are gentle and highly vegetated and contain mostly ancient compound failures (Figure 8.49). These slopes have a dominant shear surface affected by bedrock creep within the shale. The post-glacial valley down cutting and pre-glacial valley down cutting affects these slopes. The process of valley rebound is well-defined (Matheson and Thomson, 1973) and in this area, two cycles of valley rebound may affect the stability of the slope. The initial valley formation eroded to the top of the Shaftesbury Shale. In this location, crosscutting shears and vertical relaxation joints are of minor importance because of the competence of the Dunvegan Sandstone. The most important feature created during this first down cutting is the upward arching of the riverbed within the shale. The north-facing slopes within the valley are currently above the pre-glacial riverbed. The discontinuities created by upward arching now affect the shale slope allowing for creep to occur along the horizontal planes. Upward arching affects a large area under the riverbed, allowing a large failure to occur along the shale bedding. The second series of valley down cutting exposed this riverbed and also allows for horizontal bedding plane joints to shear. These horizontal bedding plane joints allow the arching to be mobilized within the slope.

The pre-glacial valley existed with steep valley walls due to the competency of the sandstone bedrock (1). The area became filled with ice-damed, pre-glacial valley fill sediments and eventually became glaciated (2). Drainage re-established itself near the

north end of the pre-glacial valley (3) and eroded down until a layer of weak material could be mobilised (4). These layers were very weak and had only a small amount of time to consolidate under the load of the glacial ice. As the river continued to erode down, more weak layers were sheared and mobilized (5) until eventually the river channel was confined by the shale bedrock. The overlying weight has allowed movement along the pre-sheared layers within the shale allowing for a bedrock creep to occur forcing the slope to move slowly toward the river (6). Failure in the shale likely occurs along weak bedding planes; therefore the structural properties of the rock control the slope angle and movement.

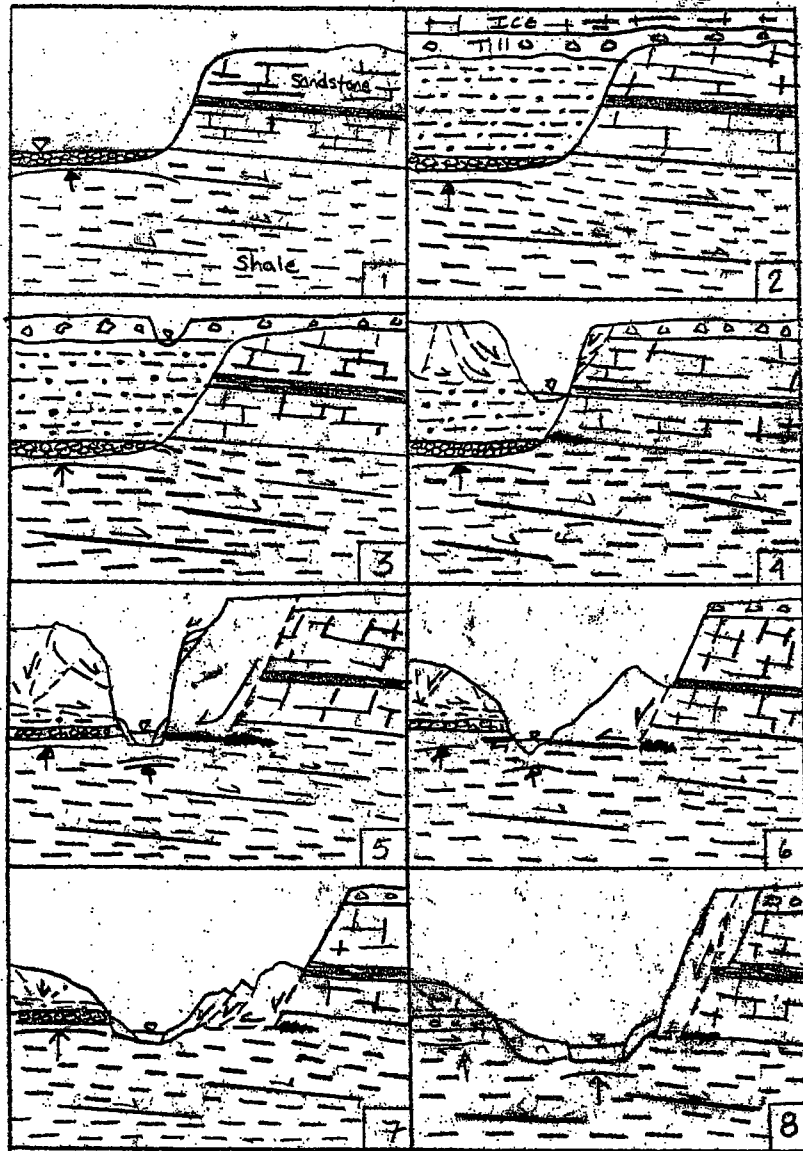
Oblique Slopes (Model 3)

Within this area, there are six slide profiles where the slides are at oblique angles to the regional dip, three east facing and three west facing. Of the west facing slopes, two of these are outside of the pre-glacial valley. These failures show the same mechanism and morphology as the south facing slopes outside of the valley. They are deep-seated failures within the shale with slightly longer failure planes. The other west-facing slope has a basal shear plane within the shale and upper shear planes within pre-glacial sequence. The shear plane within the shale is of an intermediate length, however, the failure planes within the pre-glacial materials are longer as they do not correspond to the bedrock geology.

The west facing slopes are all within the pre-glacial valley and therefore the shear planes are mostly within the Pleistocene sequence. The failure planes within the shale also have an intermediate length and those within the surficial material are variable.

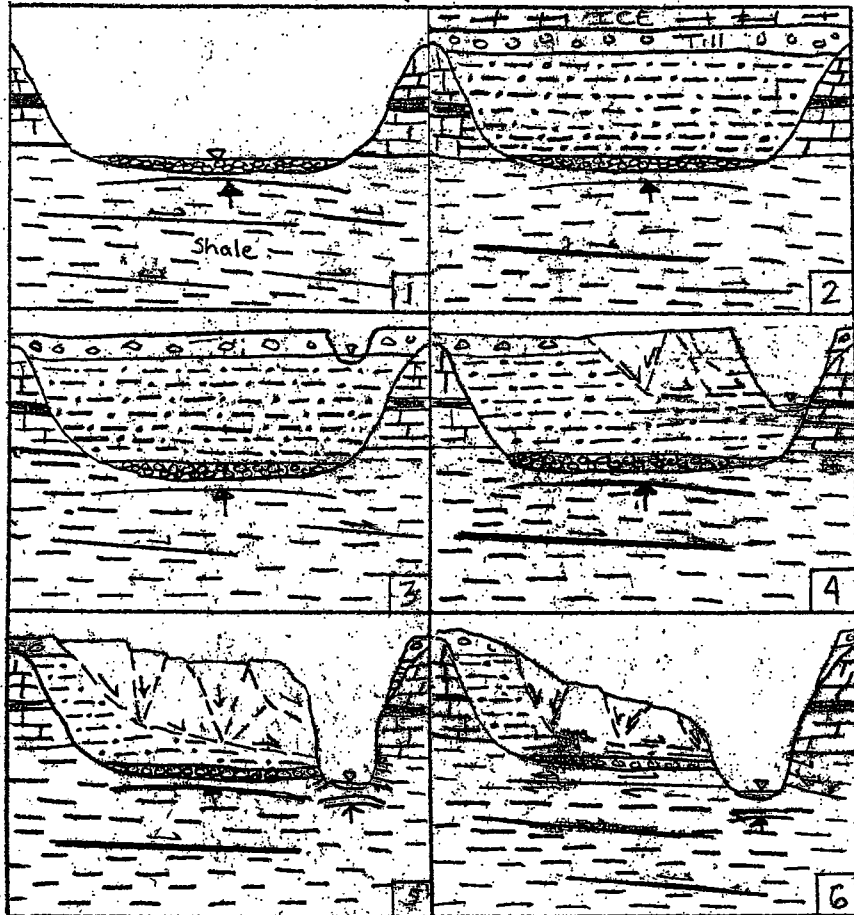
In general, if the slopes are at an oblique angle to the north and south, the failure planes within the shale are longer than those on south-facing slopes and shorter than those on north-facing slopes. Failure planes within the Pleistocene sequence are variable and do not correspond to any trends.

Figure 8.48 Landslide Area 3 – Model 1



1) Broad, stable pre-glacial bedrock channel; 2) Aggregating deltaic environment into encroaching ice-dammed lake followed by glaciation; 3) Drainage re-establishes; 4) Post-glacial river erodes down into till and sandstone leaving steep banks; 5) Once softer shale is reached, initial failures occur along near-horizontal surface unloading slope; 6) Failures continue to occur, on older, upper bedding planes and lower planes once exposed; 7) Debris at base acts as a berm for debris on slope, internal deformation of debris occurs; 8) Erosion of debris and intact bank cause failure.

Figure 8.49 Landslide Area 3 – Model 2



1) Broad, stable pre-glacial bedrock channel; 2) Aggregating deltaic environment into encroaching ice-dammed lake followed by glaciation; 3) Drainage re-establishes; 4) Post-glacial river erodes down into till and sandstone leaving steep banks, once softer, laminated sediments below are reached, initial failures occur; 5) Failures continue to occur, on older, upper lower bedding planes and lower planes once exposed, debris at base acts as a berm for debris on slope; 6) Erosion of debris and intact bank and increases in pore pressure in pre-glacial units cause failure.

8.4 Slide Area 4

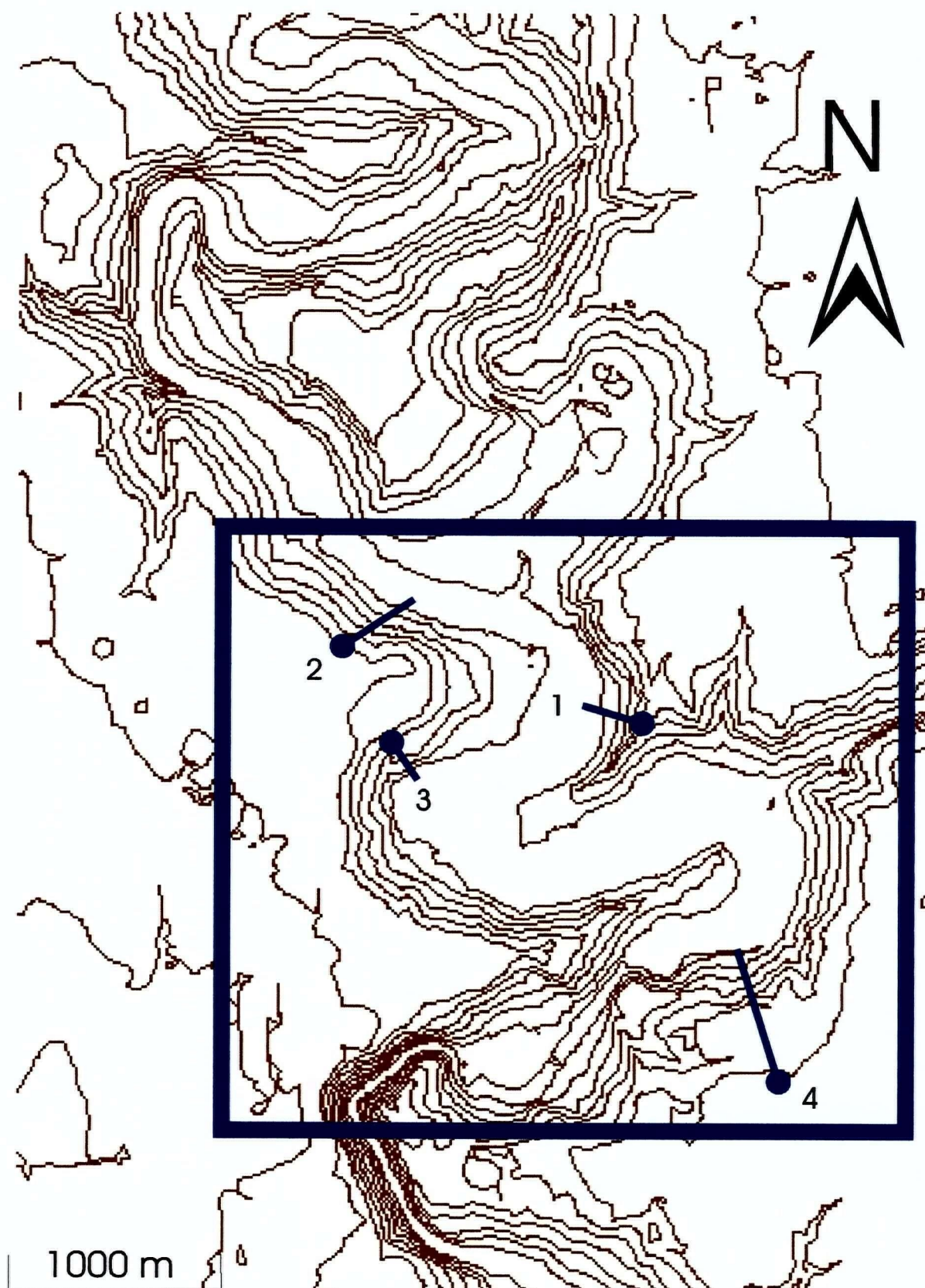
8.4.1 Location

The fourth area chosen to study was along the Beatton River, north of the town of North Pine (Figure 8.50). The area was chosen due to its accessibility and because all of the landslides are completely within the Pleistocene sequence. In fact, the shale bedrock is at or beneath the river level and does not play a role in slope stability in the area. Both sides of the river are easily reached as there is a bridge in the immediate area and the approach to the bridge has undergone recent failure, undermining the foundation of the road. The road services a petroleum extraction area to the north east of the site. This locale was also selected due to the high number of very active and active slides in the area. Four slides were profiled in this area. The area encompasses both banks along an 8-kilometer stretch of the Beatton River. This region is also home to 'Kings Valley Christian Camp' which provide valuable assistance in accessing areas and providing witnessed accounts of some of the local failures.

8.4.2 Local Stratigraphy

This area is located within the boundaries of the pre-glacial valley near its northern end (Mathews, 1978). The local stratigraphy is pieced together from the four slides in the area (Slide 4-1, 4-2, 4-3, and 4-3). The bedrock extends to an elevation of 531 meters a.s.l if the slide is located near the edge of the pre-glacial valley; otherwise, the bedrock is not present. This location is within the Dunvegan Formation as the regional dip places the Shaftesbury Shale at an elevation not yet reached by the river. Above the bedrock occurs a thin unit of gravel (<1m) within two of the profiles. All of

Figure 8.50 Landslide Area 4



the slides show good intact sequences within some of the scarps, where all the detailed stratigraphy is noted. Unfortunately, the stratigraphy of the Quaternary deposits is complex and cannot be correlated from one section to another. The pre-glacial unit is variable, laterally and vertically, within these separate sections so it is not likely that beds match up over longer distance. This area is interpreted as being part of a deltaic sequence due to the silt and sand cross beds observed and the variability of the materials over a small distance. The intact section neighbouring Slide 4-3 shows a detailed deltaic sequence supporting this interpretation (Figure 8.51). A till cap was witnessed at an elevation of 640 meters a.s.l. A hypothetical stratigraphic column is produced (Figure 8.52).

Figure 8.51 Deltaic Sequence (Slide 4-3, * G.Hartman-notes)

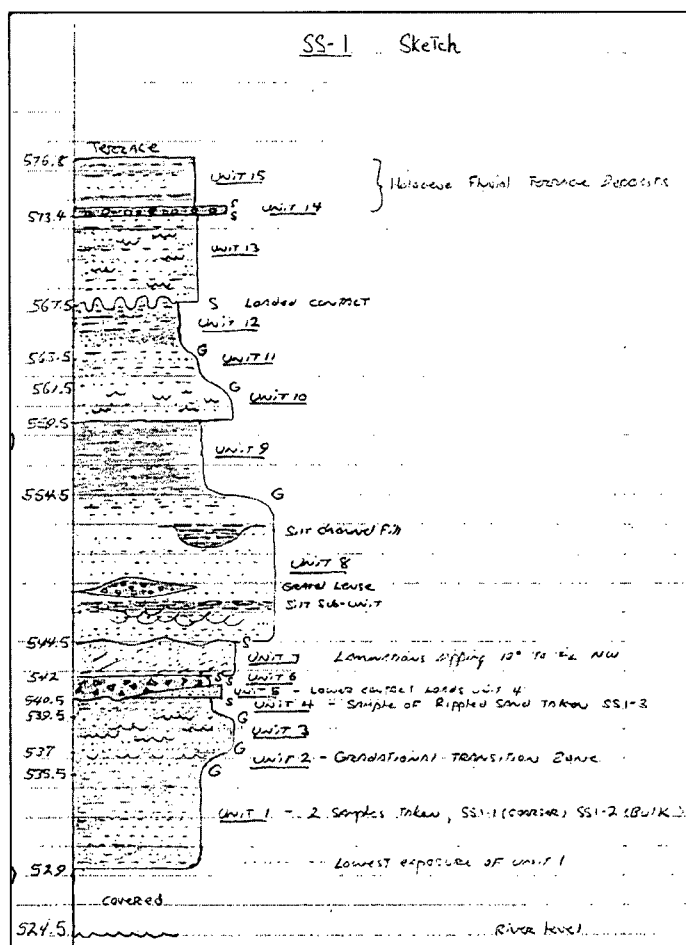
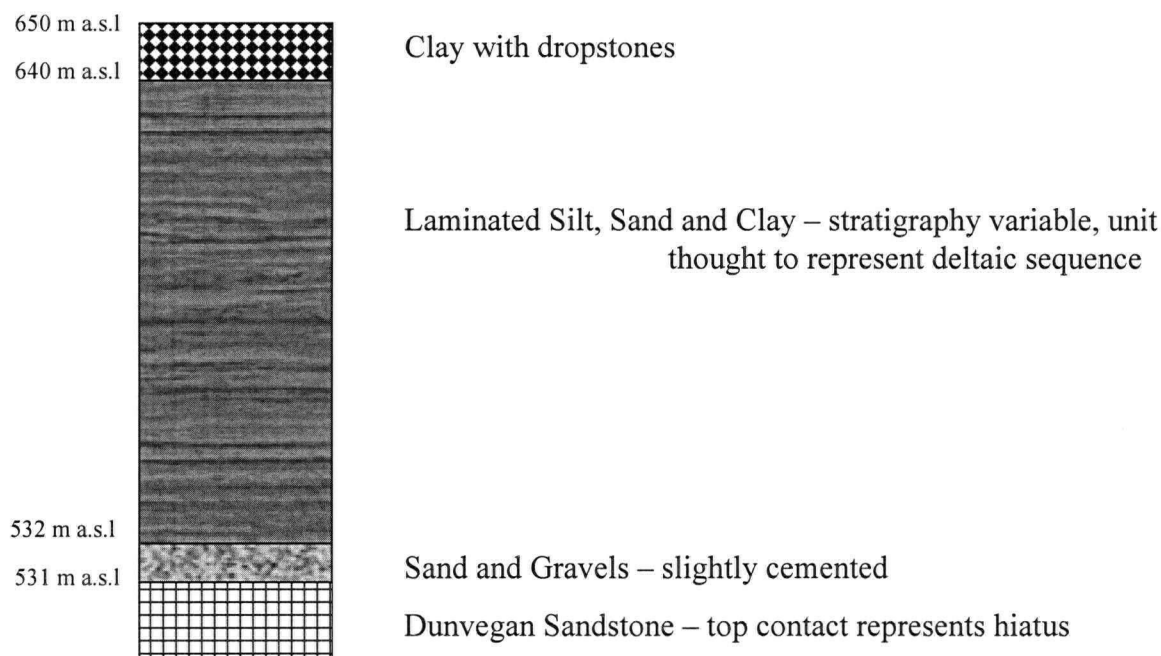


Figure 8.52 Area 4 – Stratigraphic Profile



8.4.3 Landslides

Slide 4-1

Slide 4-1 is an active, multi-leveled failure with two slide planes within the pre-glacial unit (Figure 8.55). The slide occurs on a west-facing slope along the Beaton River directly across the river from the Christian Campground (Figure 8.50). The profile begins on a knife-edge with a larger dormant slide with an aspect that is opposite of the 256° azimuth of this slide. The head scarp is made of a disturbed material likely caused by the older abutting failure. The upper shear zone was observed to be within silt dominated facies of the pre-glacial unit at an elevation of 595.65 meters a.s.l. A slickensided surface was not found within this degraded and modified shear zone, however, a sample of the clay zone was taken (sample 4-1A). An intact section of the facies was found 30 meters to the north (Figure. 8.53) and was found to be sand dominated with larger cross beds and few clay layers. The fine-grained beds below the

shear surface have smaller-scale cross beds with a higher percentage of clay. The second failed mass shows some back tilt and horst and graben structures although the shear plane was not observed. The morphology of the slope suggests that the debris slides over an intact section and the toe appears to be remobilized debris. A bedrock elevation was found at an outcrop near the toe of the failure with a thin (< 1 m) intact unit of gravel above along with a sand-dominated pre-glacial section.

Figure 8.53 Intact Section Photo (Slide 4-1)



Slide 4-2

This failure is a very active multi-level failure within the Pleistocene sequence on a north facing slope on the Beatton River (Figure 8.56). The failure is located approximately 500 meters upstream from the bridge within the area. The profile begins upon a stable plateau underlain by till and heads down slope at an azimuth of 26°. The failure contains three failure planes all within the sand-dominated pre-glacial sediments. Most of the upper section of the slope is covered in colluvium and the shear planes are

Most of the upper section of the slope is covered in colluvium and the shear planes are not visible. The upper failures are interpreted from slope morphology to have rotational and translational failure mechanisms. Most of the features are muted and older than the toe. Below the upper failures is an intact section of massive sand with small interbeds of silt and clay. The lower failure is fresh with a shear surface (sample 4-2 A) within a silt-rich package within the sequence (sample 4-2 B). The shear plane is composed mostly of silt (77%) with some clay (18%). The plasticity of the sheared material is fairly low, with a liquid limit of 39% and a plasticity index of 24%. The intact clay bed sampled is actually 97% silt, which has low plasticity with a liquid limit of 28% and a plasticity index of 8%. Below the intact section where the shear plane was found, is a toe of remobilized debris.

Slide 4-3

Slide 4-3 is a small, very active, rotational failure on a south-facing slope on the Beaton River (Figure 8.57). The profile begins mid-slope on a stable bench and heads down towards the river level at an azimuth of 147 (Figure 8.50). The slope above this failure is likely part of a separate failure. It could not be profiled, as the roadway that bisects the slope was heavy with traffic. The debris surface is fresh with ponding and intact slump blocks within the debris mass. The head scarp has a complex stratigraphy with silt, gravel, and massive clay. The single failure plane for this slide is within the pre-glacial unit with the bedrock missing at this section. The basal shear plane was not found but is interpreted as being slightly below the river level. The slump blocks within the debris have matching stratigraphy found in the head scarp and in an intact section near the slide.

Slide 4-4

Slide 4-4 occurs on a north-facing slope on the Beatton River roughly 3 kilometers south of the Christian Camp (Figure 8.50). The profile begins on a stable section of the slope. It is presumed stable due to the lack of any landslide features and the low angle of the slope (1°). The profile continues down the slope at an azimuth of 330°. The slide has two failure planes, both within the pre-glacial sediments (Figure 8.58). The upper failure is a deep, compound failure approximately 700 meters long and 50 meters deep. The debris mass exhibits dormant, translational features, i.e. horst, grabens, and cross-tilted trees. A rotational failure occurs behind the toe of the upper slide because of an intact scarp in which the shear plane for the compound failure was located (sample 4-4 A). The shear plane is made of silt, 76%, and clay, 13% and has a medium plasticity with a liquid limit of 58% and a plasticity index of 35%. Below the main shear, smaller shear planes were found and sampled (sample 4-4 B). These shear planes are also mostly made up of silt, 86%, with minor clay, 10%. The secondary shears are much more plastic with a Liquid Limit of 73% and a Plasticity Index of 53%. The intact face shows a complex stratigraphy (Figure 8.54) with several shear planes and massive clay unit with drop stones and gravel lenses. Below this intact face exists a second debris mass with rotational slide features, including ponding and back tilted slump blocks and trees. This lower failure has fresh features and is very active as it occurred 2 years ago. The toe of the slide is assembled of remobilized debris.

Figure 8.54 Complex Stratigraphy (Slide 4-4)



Figure 8.55 Landslide Profile 4-1

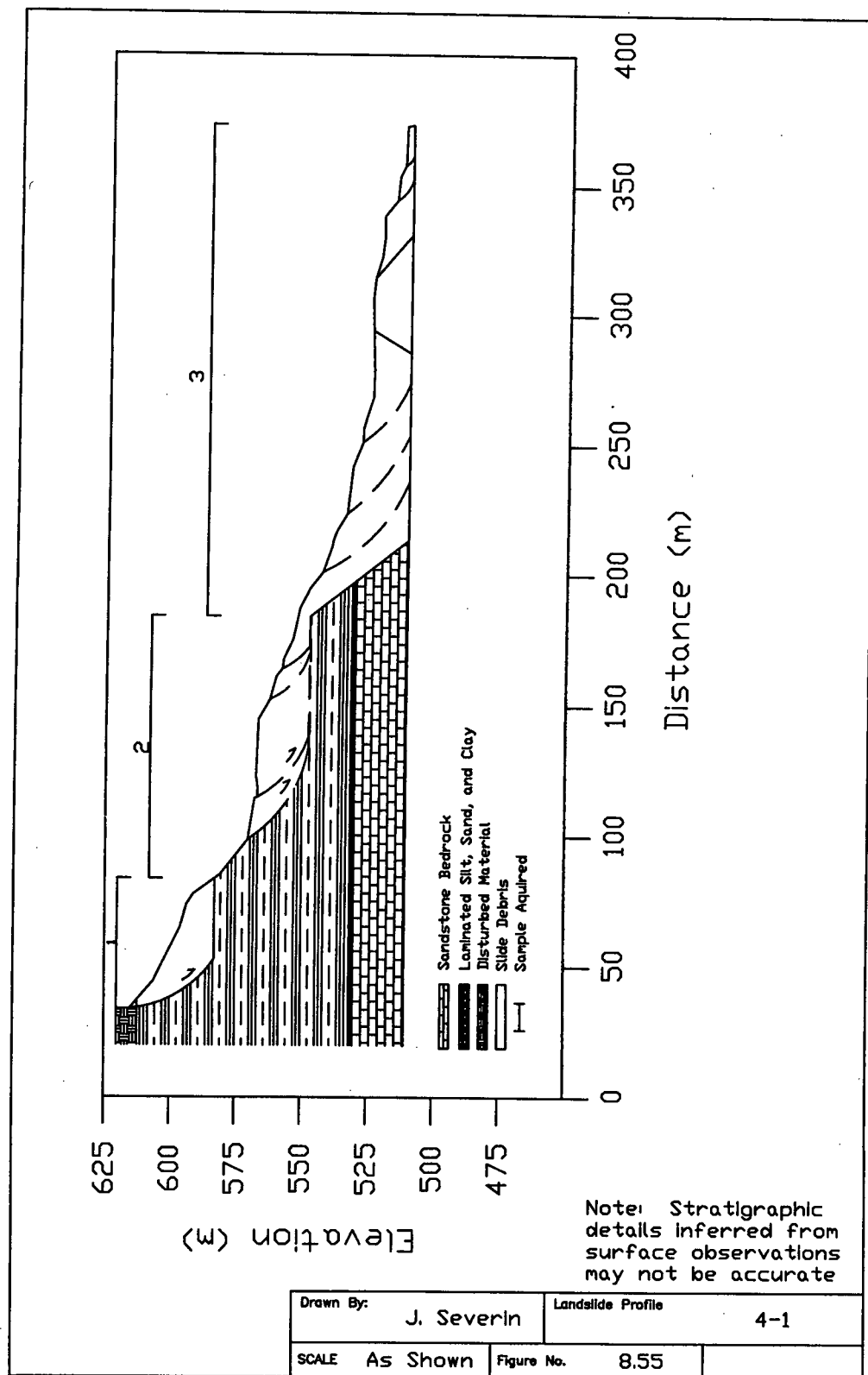


Figure 8.56 Landslide Profile 4-2

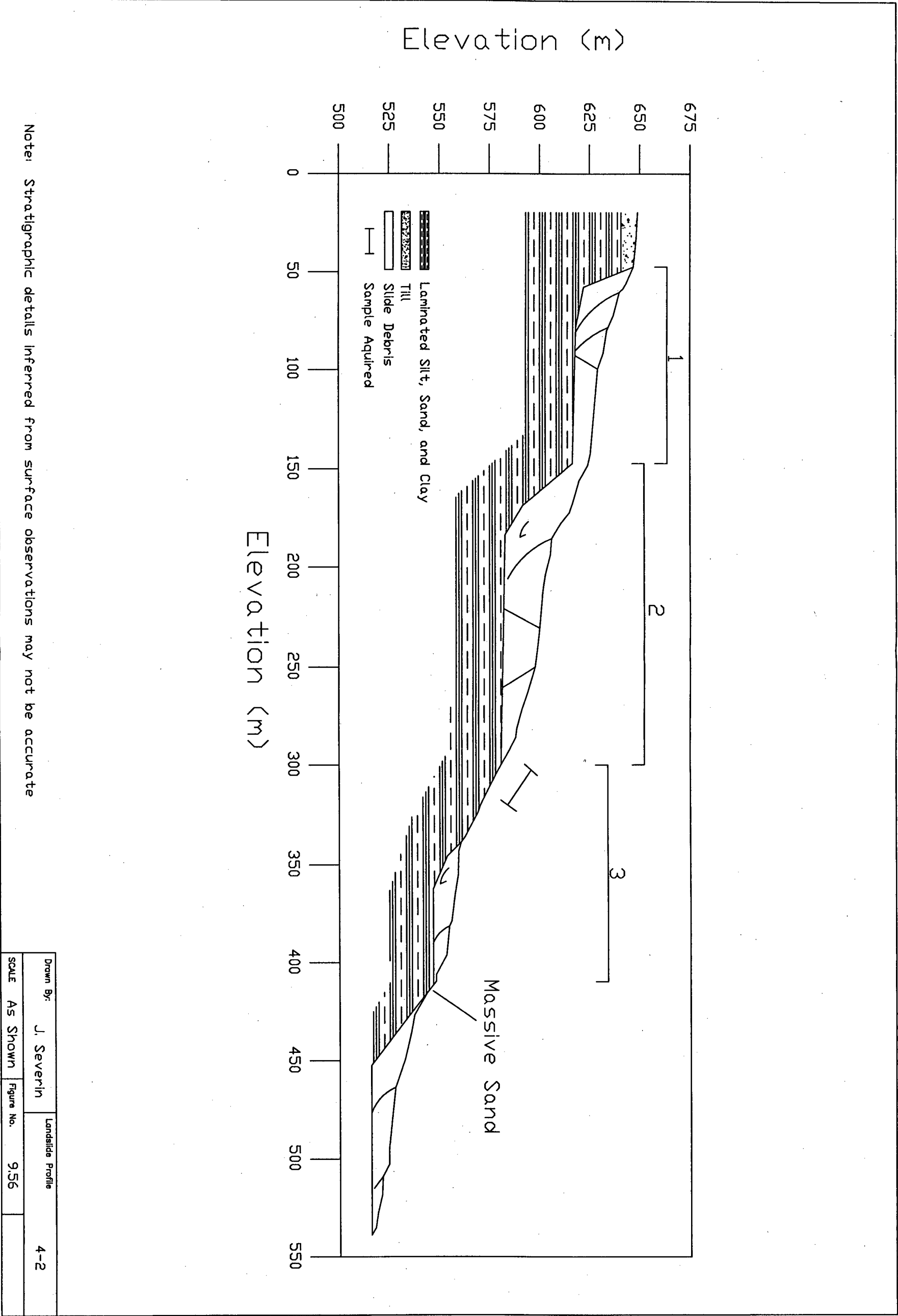


Figure 8.57 Landslide Profile 4-3

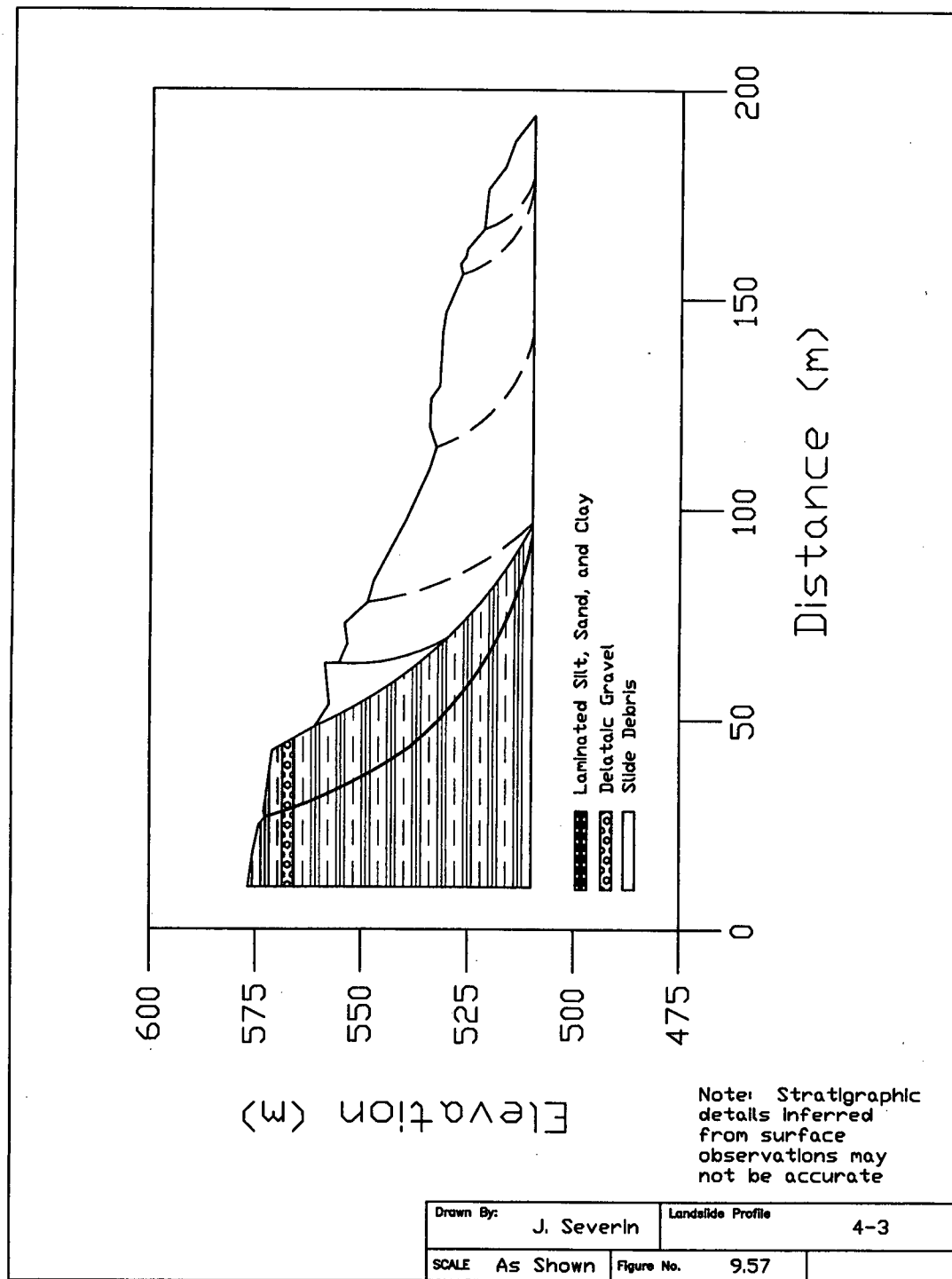
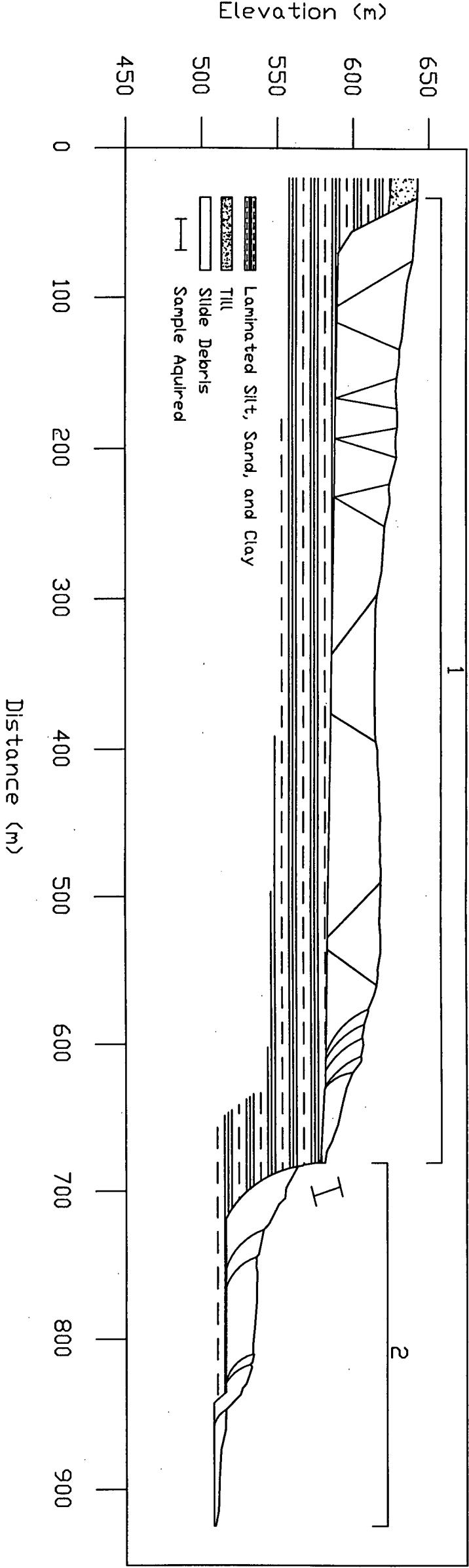


Figure 8.58 Landslide Profile 4-4



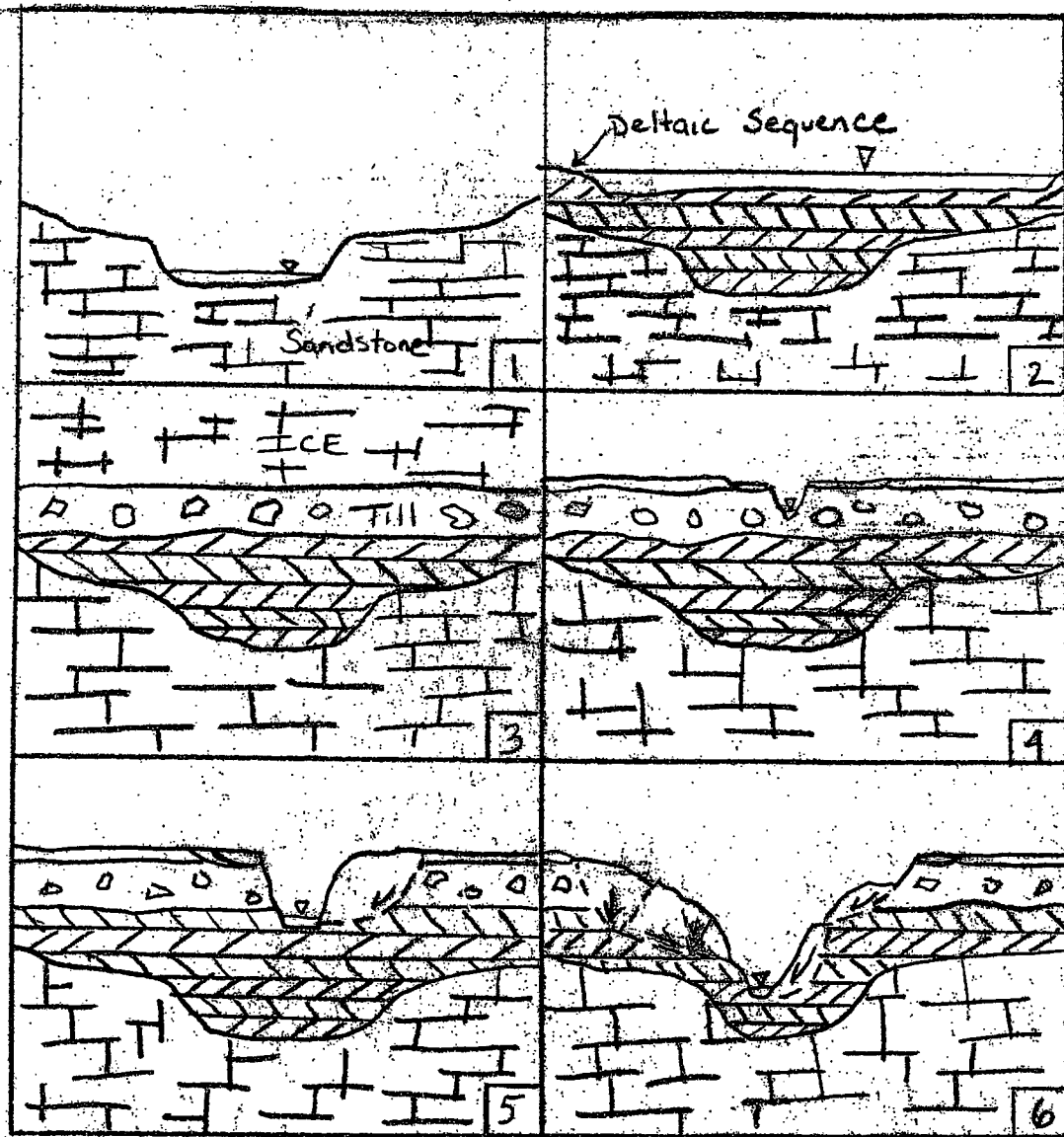
Note: Stratigraphic details inferred from surface observations may not be accurate

Drawn By: J. Severin		Landslide Profile 4-4	
SCALE AS Shown	Figure No. 8.58		

8.4.4 Area Model

This area represents a deltaic sequence within the pre-glacial materials above a stable bedrock channel of sandstone (1). The materials of this sequence are variable and therefore the failure planes cannot be correlated over a longer distance (Figure 8.59). The pre-glacial unit represents a deltaic environment as the Beatton River continued to transport sediment into an expanding ice-dammed lake within the river valley (2). The area then became glaciated (3) as evidenced by the till and post-glacial lake sediments. Drainage became re-established (4) and eventually cut into the deltaic silts, sands, and clays. The river channel at this time likely had steep walls, as the till in this area is mostly competent. Once the river eroded down into the pre-glacial sediments, layers of weaker, more plastic materials were exposed and failures occurred along them. The debris mass likely acted as a protection berm until it was eroded. As the river continued to cut downward, new layers were exposed and failures occurred along several failure planes as the intact material weakened and softened. This process is still occurring, shown by the high activity of landslides found on this river. Several of the failure planes occur at similar elevations, so there may be larger beds which can extend over a large area, but for the most part, failures that occur at similar levels at this site do so by coincidence. The failures at this location are generally rotational with shorter shear planes than in other areas. The variability of the materials is directly responsible for the shorter shear planes. The valley was created by rapid erosion and down cutting. This process has exposed the plastic layers and allowed them to fail. The clay beds within the sequence may have been pre-sheared by glaciotectionics and by horizontal relaxation during the valley rebound. During valley rebound, the weakest layers undergo the most deformation; in this case, the

Figure 8.59 Landslide Area 4 – Geologic Model



- 1) Stable pre-glacial bedrock channel; 2) Aggregating deltaic environment into encroaching ice-dammed lake;
- 3) Glaciation; 4) Drainage re-establishes; 5) Post-glacial river erodes down into till leaving steep banks, once softer, laminated sediments below are reached, initial failures occur; 6) Failures continue to occur, on older, upper lower bedding planes and lower planes once exposed. Debris at base acts as a berm for debris on slope.

plastic clay layers are the victims. As downcutting occurs, these failure planes are exposed and allowed to fail. Often the upper sections of the slope contain older, dormant failures, suggesting that toe erosion is the major trigger of landslides in this area.

9.0 DISCUSSION

9.1 Landslide Mechanisms and Distribution

This study of slope movements in the Peace River area has led to the formation of several interesting discussion points. The most important one is the understanding of the river valley slope morphology in shale and unconsolidated valley fill sediments. This project also helps to characterize the geotechnical properties of the sediments located within the Peace River Valley and associated tributary valleys. This project is also the first attempt to systematically map and take inventory of landslides within the Peace River Valley, one of the many valleys in the Canadian Prairies, which shows ubiquitous landsliding throughout its length.

The first major result of this project is that a majority of the failures contain more than one shear plane on which the landslides mobilize. These multiple failure planes are located both within the Cretaceous shale and the Pleistocene laminated silts, sands, and clay. Further, there are several elevations within the shale that are apt to fail, that is to say that there is no one unique elevation at which all failures occur within the shale. There are several bentonite-rich layers within the shale that are weak enough to fail. No pure bentonite layers were found within this extensive field-based project, however, as there are numerous failures, most of which with covered shear planes, it is possible that such bentonite layers exist. Many slides, especially those that occur outside the preglacial

valley, noted in the “Old Fort” area, have multiple shear planes within the shale. In other areas, located within the pre-glacial valley, there are often failure surfaces within both the Cretaceous bedrock and Pleistocene sediments, or often exclusively within the unconsolidated silts, sands, and clay.

Another result from this project is the recognition that morphology can rarely be used to determine the material in which the shear plane is located. Failures in both Cretaceous shale and Pleistocene sediments are morphologically similar with comparable features and sizes. Even though the materials are geologically different from each other, their engineering properties, which control the stability of the slopes and the failure mechanisms, can be similar. During this investigation, the location of the failure surface was often inferred by an understanding of material distribution and deposit geometry as understood by past geologic investigations of the area (Stott, 1982, and Mathews, 1978).

From the detailed landslide map and resultant inventory, a number of trends can be noticed. As mentioned in Section 8.4, a total of 1610 slope failures covering 1262 km were analyzed in terms of their distribution, activity, type, material involved, and aspect.

The Peace river contained nearly a third (28.9%) of all failures catalogued and together with the Beatton River, contains over a half of all failures, 54.2%. The average slide per kilometer ratio is 1.28 slide/km, with the Peace and Beatton Rivers showing the highest values of 1.82 and 1.60 respectively. The overall area covered by all failures was 765.5 km² with an average area per slide of 0.48 km².

The dominant activity level throughout the entire map sheet is “Low Activity – Dormant” with 61.0% of all failures classified as this level. “Very Active” and “Active” failures comprise 1.9% and 12.9% of all failures, respectively. “Very Active” and

“Active” slides are also both dominantly multi-leveled failures, 73.3% and 43.0%, respectively. Approximately 30.0% of all “Very Active” failures had a northern aspect, twice the average percentage and are dominantly in the Quaternary sedimentary sequence (73.3%).

The dominant failure type within the study area is the multi-leveled failure comprising nearly half of all failures, 47.8%, or over 769 failures. This value is thought to be conservatively low as the worst-case scenario, or deeper failure plane was assigned if doubt existed as to its location. Multi-rotational failures are the most active with 2.9%, which is to be expected due to the distribution of landslide types. Compound failures occur mostly on north-facing slopes, likely a reflection of the regional dip in the bedrock towards the northeast. East and west-facing slopes are dominated by retrogressive rotational failures each with 19.5% on slopes of those aspects. South facing slopes are dominated by multi-level failures, with 19.9 %. The dominant failure type, multi-leveled failures, is evenly distributed between the shale, 51.9%, and the Pleistocene sequences, 48.1%. Compound failures occur mostly within the shale bedrock (61.9%). This is probably due to the relative ease with which the overlying Quaternary sand, silt and clay can be sheared through in order to produce a curved head scarp. The higher number of single rotational failures occurring in the unconsolidated sediments, 64.8%, supports this fact.

Shale is the basal material in 43.7% of all failures, while the Pleistocene sequence represents 56.3%. This equal distribution is one of the reasons why landslide investigation is so difficult in the Peace River Region. The value for shale failures is

slightly higher in north-facing failures as the regional dip of the bedrock is more favorable for failure.

9.2 Generalized Geologic Model

A series of generalized geologic models depicting the occurrence of landslides in the Peace River Region has been produced. Each investigated site fits into an aspect of this model depending on local conditions. The model starts with the deposition of the low energy sediments into an inland sea during the Cretaceous Period. During this time, several volcanic events occurred, associated with the orogenic activity to the west, depositing ash in the deep sea environment. This was followed by continued deposition, which allowed the sediments to compact into shale incorporating the weathered ash, or bentonite, into laminations of the shale. During the Tertiary and Quaternary Periods, a long period of erosion allowed broad valleys to form in to a mature drainage system. It is possible that these broad valleys developed by extensive landsliding that developed along the weak layers within the shale to retrogress back from the river and become stable at a very low angle. During the ice advances of recent geologic time, the valleys have been plugged with valley fill sediments, such as silt, sand, and clay followed by a glacial diamicton unit that overlies the sedimentary sequence. The weight and movement of the glaciers may have caused further failure in the shale and shearing within the unconsolidated sediments below. Upon retreat of the glaciers, a post-glacial lake occurred, and another unit was deposited of normally consolidated clays. Once the ice or sediment dam, which retained the post-glacial lake, was breached, a rapid downcutting through the Pleistocene sequence occurred, uncovering the sheared planes and allowing

syn-erosional slumping to occur. The rapid downcutting may have also caused some new shear planes to develop as riverbanks became too steep to be supported. Valley rebound features occurred within both the horizontal shale bedrock along the bentonitic-rich layers and within the Quaternary sedimentary package along the plastic clay layers. As further erosion occurred, the current river valley began to take shape with slow moving failures along the pre-defined shear planes. The valley developed as a series of landslide cycles, in which the valleys walls retrogress back, stabilize, are eroded, and retrogress back again. A similar sequence involving the London Clay and wave erosion was noted near the Thames estuary in England (Hutchinson, 1975) and has been postulated to occur in Alberta (Cruden et al., 1998). Of course, several (of the) parameters would have to be taken into account; however, the same type of cycle could be responsible for the development of the pre-glacial and current Peace River Valleys.

A secondary conclusion, based on air photograph interpretation and field investigation, is that the river morphology of the Peace River is completely dependent on the slope morphology of the river valley slopes. Several of the large islands within the Peace River are at the base of failed slopes. The large slide at Attachie in 1973, allows us to examine the effects of failures on the river morphology. Smaller areas of the slide debris have stabilized within the riverbed, making islands at the edges of the river. Conversely, the islands themselves can help the investigation of slopes as well. The islands may indicate that a large amount of debris has been deposited denoting the presence of a large landslide. In the Peace River, it can then be said that the slope was unstable and may still be. The slope may consist of several shear planes and can then be

marked as an abandoned failure even though the slide morphology has been completely destroyed.

10.0 CONCLUSIONS

One of the main objectives of this research project was to complete a comprehensive landslide inventory and map to be used in future engineering and geological work done in the Peace River region. The landslide map, containing over 1600 slope movements, is one of the main products of this project. The detail contained within the map, including activity and morphological features, make it a valuable document to be included in any desk or field study with respect to the Peace River region.

The detailed inventory database, created to accompany the map, includes: River Segment, Activity, Failure Type, Slide Aspect, Likely basal shear material, Length, Width, Area, and Morphological features (Appendix II). This inventory, based on air photo interpretation and field proofing, was used to analyze the landslides with respect to these characteristics, in order to make some basic conclusions about landslides in the Charlie Lake map sheet.

The most important conclusion derived from an analysis of this data was that a majority of the failures were multi-leveled, containing at least two shear planes. Almost 48% (769) of all failures were multi-level. This number is thought to be conservative as many of the failures are now dormant and their features obscured. In this case, the worst case scenario was used, i.e., a single large failure, therefore, the number of multi-level failures is likely to be higher and the number of rotational, retrogressive rotational, and compound failures would be lower than shown. A corollary to this conclusion is that

instead of having one single weak layer within the region, there are many relatively weak horizons both in the unconsolidated material and in the bedrock. Based on laboratory testing of both failed and intact materials, there was little difference in bulk properties between the two types of material tested and the failed materials were not highly plastic. Of the 28 samples collected, four were a gouge material, eight were of sheared or disturbed shale, and 16 were of clay, sheared and intact. The average value of the Liquid Limit of the intact clay beds was 47.4%, while the sheared beds had a Liquid Limit of 49.8%. The Plasticity Indices of the intact and sheared beds are 30.3% and 27.9%, respectively. The sheared beds are silt dominated, 70% and 63%, respectively. The shale has a slightly higher Liquid Limit of 49.1% and a Plastic Index of 29.2%. Thus, the physical differences between weaker and stronger horizons is subtle. The preferences for failure occurring on certain horizons is probably due to pre-shearing as a result of flexural slip.

Another important result was that the Beatton River seems to be the most active of all areas. Over half of all the Very Active failures were found within the Beatton River Valley. This thought to be because the rate of erosion is slightly higher than the Peace River, with the same weak materials. The higher rate of erosion is due to the steeper gradient of the river.

The third conclusion is that north-facing slopes appear to be more active than their south facing counterparts. 226 failures (14%) occurred on north-facing slopes as opposed to the 113 (7%) on south-facing slopes. The reason is thought to be the gentle northeastern dip of the underlying bedrock. The dip of the shale bedrock into the valley allows for more failures to occur than would otherwise be allowed at such shallow

angles. This gentle dip can also dramatically affect the type of slide observed in the river valleys. On the north-facing slopes, a distinct majority of long, compound failures were found. These failures are thought often to utilize weak layers in the Cretaceous shale rather than the Quaternary sequence above. Unfortunately, this is not always the case, therefore it can not be a rule, but rather a generality. Often, morphology alone can be a deciding factor in determining which type of material has failed. This can occur only if the preferred failure mechanism it is known for a material in that situation. This study presents the opposite situation in which knowing the type of failure can not really help determine which material had failed. This is due to the similarities in the failure setting for both the shale and the pre-glacial clays. The Peace River region provides an example where making assumptions on failed material based on morphology could prove incorrect.

The final conclusion to be made from this work is derived from field work completed during the summers of 2002/2003, when a total of 40 landslides were profiled within the Peace River Area. These profiles were completed in four landslide investigation areas; Big Bam Ski Hill, Old Fort, Beatton Big Bend, and Beatton Christian camp. These areas were investigated in greater detail in the field with profiles of individual slides and several samples being collected and tested. From this work, geologic models were created for each of the areas and then combined to produce a generalized model for failures in the Peace River region. The generalized model is discussed in Section 9.2.

The results of the research indicate how a landslide inventory can be compiled, the necessary steps that must be taken in order to make it valid and useful, and what types of information can be harvested from such an inventory.

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APPENDIX I

Geotechnical Lab Results

Grain Size Analysis

Atterberg Limits

SUMMARY OF HYDROMETERS



TO: Geological Survey of Canada
601 Booth Street
Ottawa ON K1A 0E8

OFFICE: Fort St. John
PROJECT NO: GX04091
CLIENT: Geological Survey of Canada
COPIES TO: Mr. Jordan Severin

ATTENTION: Mr. Larry Dyke

PROJECT: Peace & Beatton Rivers

SAMPLE NO.	DATE	SOIL DESCRIPTION	Sand	Silt	Clay	D30 mm	D60 mm
1-1C	July 20/03	Shale Failure Plane	25%	61%	14%	.0054	.023
1-4A	June 21/03	Shear Surface #1 ICS	3%	59%	38%	.0011	.0086
1-4B	July 23/03	Massive Clay	4%	72%	24%	.0029	.018
2-2A	June 26/03	Shale/Shear?	7%	70%	23%	.0032	.046
2-2B	June 27/03	Debris/shear	7%	70%	23%	.0030	.027
2-4B	June 29/03	Debris Shear Plane	19%	49%	32%	.0018	.026
2-4C	June 29/03	Gouge	30%	66%	4%	.021	.061
2-4D	June 29/03	Shale Shear Plane	4%	85%	11%	.0066	.033
2-4I	July 23/03	Shear Gouge	23%	68%	9%	.014	.054
2-6A	June 30/03	Failed Clay	9%	57%	34%	.0015	.011
2-6B	June 30/03	Shear Gouge in Shale	38%	40%	22%	.0042	.070
2-8A	July 3/03	Upper Shear Plane	0%	50%	50%	.0010	.0032
2-8AA	July 23/03	Intact Clay II	0%	43%	57%	.00082	.0025
2-8B	July 3/03	Intact Clay Beneath Shear (5cm)	0%	68%	32%	.0018	.051
2-8D	July 3/03	Lower Slide Plane	1%	63%	36%	.0014	.013
3-1A	July 6/03	Shear Plane ICS	1%	52%	47%	.00088	.0037
3-5A	July 8/03	Adjacent Shear	13%	68%	19%	.0056	.030
3-5B	July 8/03	Gouge	32%	58%	10%	.016	.055
3-13A	July 12/03	Remolded Shale	19%	78%	3%	.045	.063
3-14A	July 13/03	Shale Shear	1%	77%	22%	.0035	.027
3-14B	July 13/03	Remolded Clay? Middle Material	1%	74%	25%	.0026	.0088
3-15A	July 13/03	Shale Shear	22%	57%	21%	.0038	.044
3-15B	July 13/03	Remolded Shale	14%	50%	36%	.0012	.023
4-2A	July 19/03	ICS Shear	5%	77%	18%	.0063	.038
4-2B	July 19/03	Intact Clay	2%	97%	1%	.016	.031
4-4A	July 22/03	Shear Upper ICS	11%	76%	13%	.028	.060
4-4B	July 22/03	Shear ICS	4%	86%	10%	.0031	.0059
PI-1A	June 24/03	Shear Plane	4%	57%	39%	.0012	.0055

PROJECT	SAMPLE <u>P1-1A June 24/03 Shear Plane.</u>
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	<u>0</u>
WT. PASS. #10 & PAN	<u>0</u>
WT. PAN	<u>0</u>
WT. PASSING #10	<u>259.4</u>
TOTAL WT.	<u>259.4</u>
HYDROMETER SAMPLE	
TARE NO.	
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	<u>50.10</u>
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.40	
40	0.36	
60	0.25	
100	0.26	
200	0.79	

WATER CONTENT	
TARE NO.	<u>429 P1-1A</u>
WT. TARE & WET SOIL	<u>20.53</u>
WT. TARE & DRY SOIL	<u>20.06</u>
WT. TARE	<u>1.25</u>
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME — 9:00

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	"A" FACTOR				
				COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
21	9:01	1	49					
21	9:02	2	48					
21	9:04	4	47.5					
21	9:08	8	47					
21	9:15	15	46.5					
21	9:30	30	44					
21	10:00	60	39.5					
21	11:00	120 ^{2h}	36					
21	1:00	240 ^{4h}	32					
21	5:00	480 ^{8h}	29.5					
21	9:00	1440 ^{24h}	25					

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PROJECT

LAB ORDER

SAMPLE 1-1C July 20/03 Shale failure plane
TEST HOLE across from ~~DEPTH~~ 1-1 downstream

TECHNICIAN

DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	0.16
WT. PASS. #10 & PAN	0
WT. PAN	0
WT. PASSING #10	433.5
TOTAL WT.	440.1

HYDROMETER SAMPLE	
TARE NO.	
WT. TARE & WET SOIL	50.37
WT. TARE	
WT. WET SOIL	50.37
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	3.69	
40	3.59	
60	1.79	
100	1.47	
200	1.53	

WATER CONTENT	
TARE NO.	FA 1-1C
WT. TARE & WET SOIL	29.20
WT. TARE & DRY SOIL	28.58
WT. TARE	1.22
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME - 8:55

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR DIAMETER	PERCENT PASSING
21	8:56	1	47					
21	8:58	2	46					
21	8:59	4	38					
21	9:03	8	35					
21	9:10	15	31					
21	9:25	30	29					
21	9:55	60	26					
21	10:55	120 ^{2h}	23					
21	12:55	240 ^{4h}	20					
21	4:55	480 ^{8h}	18					
21	8:06	1440 ^{24h}	14					

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PROJECT	SAMPLE 1-4A June 21/03 Shear Surface #1 IC
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE

TOTAL SAMPLE

TOTAL WT. RET. ON #10	3.40
WT. PASS. #10 & PAN	872.0
WT. PAN	551.9
WT. PASSING #10	320.1
TOTAL WT.	323.5

HYDROMETER SAMPLE

TARE NO.	①
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	50.51
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
#4		
#10		
#20	0.11	
#40	0.13	
#60	0.14	
#100	0.31	
#200	0.77	

WATER CONTENT

TARE NO.	1-4A
WT. TARE & WET SOIL	27.86
WT. TARE & DRY SOIL	27.33
WT. TARE	1.24
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME - 10:05 "K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR DIAMETER	PERCENT PASSING
20	10:06	1	51					
20	10:07	2	49					
20	10:09	4	47					
20	10:13	8	44.5					
20	10:20	15	42.5					
20	10:35	30	39.5					
20	11:05	60	37					
20.5	12:05	120 ^{2h}	34					
21	13:05	240 ^{4h}	31.5					
21	16:25	480 ^{8h}	29					
21	18:00	1440 ^{24h}	25.5					

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PROJECT GX04091

LAB ORDER

SAMPLE 1-4B Massive Clay

TEST HOLE

DEPTH

TECHNICIAN

DATE July 23/03

TOTAL SAMPLE

TOTAL WT. RET. ON #10 —
WT. PASS. #10 & PAN —
WT. PAN —
WT. PASSING #10 —
TOTAL WT. —

HYDROMETER SAMPLE

TARE NO. 1-4B
WT. TARE & WET SOIL —
WT. TARE —
WT. WET SOIL 49.85
WATER CONTENT —
WT. DRY SOIL —

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.20	
40	0.41	
60	0.35	
100	0.41	
200	0.59	

WATER CONTENT

TARE NO. 1-4B
WT. TARE & WET SOIL 17.93
WT. TARE & DRY SOIL 17.46
WT. TARE 1.24
WT. WATER —
WT. DRY SOIL —
WATER CONTENT % —

START TIME — 10:00

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
21	10:01	1	47					
	10:02	2	43					
	10:04	4	41					
	10:08	8	38					
	10:15	15	36					
	10:30	30	33.5					
	11:00	60	31					
	12:00	120 ^{2h}	28					
✓	14:00	240 ^{4h}	25					
21.5	18:00	480 ^{8h}	22					
21	10:00	1440 ^{24h}	19					

Reporting of these test results constitutes a testing service only. Engineering interpretation or evaluation of the test results is provided only on written request.

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PROJECT _____
LAB ORDER _____
SAMPLE 2-2A Jan 26/03 Shale/shear?
TEST HOLE _____ DEPTH _____
TECHNICIAN _____ DATE _____

TOTAL SAMPLE
TOTAL WT. RET. ON #10 — 9.46
WT. PASS. #10 & PAN — 1107.6
WT. PAN — 327.9
WT. PASSING #10 — 779.7
OTAL WT. — 789.16

HYDROMETER SAMPLE
TARE NO. — 2
WT. TARE & WET SOIL —
WT. TARE —
T. WET SOIL — 50.74
WATER CONTENT —
WT. DRY SOIL —

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4"		
10		
20	0.33	
40	0.75	
60	0.57	
100	0.59	
200	1.12	

WATER CONTENT
TARE NO. — 2-2A
WT. TARE & WET SOIL — 38.43
WT. TARE & DRY SOIL — 37.57
WT. TARE — 1.26
WT. WATER —
WT. DRY SOIL —
WATER CONTENT % —
612-9403

A" TIME —		"K" FACTOR		HYDROMETER TEST			"A" FACTOR		
°C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING	
10	10:01	1	38.5						
20	10:02	2	37						
30	10:04	4	36						
40	10:08	8	34.5						
50	10:15	15	33						
60	10:30	30	31.5						
70	11:00	1 hr	29.5						
80	12:00	2 hr	26						
90	13:00	4 hr	25						
100	18:25	8 hr	22						
	13:00	24 hr	19.5						

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70 - 88 / 09

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70 - 88 / 09

PROJECT

LAB ORDER

SAMPLE 2-28 June 27/03 Debris/shear?

TEST HOLE

DEPTH

TECHNICIAN

DATE

TOTAL SAMPLE

TOTAL WT. RET. ON #10 - 38.38
WT. PASS. #10 & PAN - 843.4
WT. PAN - 388.0
WT. PASSING #10 - 515.4
TOTAL WT. - 553.78

HYDROMETER SAMPLE

TARE NO. 3
WT. TARE & WET SOIL - 49.64
WT. TARE -
WT. WET SOIL - 49.64
WATER CONTENT
WT. DRY SOIL

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
#1"		
3/4"		
1/2"		
3/8"		
#4		
#10		
#20	1.01	
#40	0.80	
#60	0.39	
#100	0.38	
#200	0.75	

WATER CONTENT

TARE NO. 2-28
WT. TARE & WET SOIL - 22.92
WT. TARE & DRY SOIL - 22.49
WT. TARE - 1.25
WT. WATER -
WT. DRY SOIL -
WATER CONTENT % -

START TIME 9:50 10-4

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
19	9:51	1	42					
19	9:52	2	40.5					
19	9:54	4	39					
19	9:58	8	37.5					
20.5	10:05	15	35.5					
20.5	10:20	30	34					
20.5	10:50	60	31					
20.0	11:50	120	29					
20.0	14:15	240	24.5					
21.0	17:50	480	22.0					
20	9:50 10-5	1440	18.5					

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PROJECT GX04091

LAB ORDER

SAMPLE June 29/03 2-4B Debris Slur

TEST HOLE

DEPTH

TECHNICIAN

DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	<u>66.68</u>
WT. PASS. #10 & PAN	<u>921.66</u>
WT. PAN	
WT. PASSING #10 Dry	
TOTAL WT.	
HYDROMETER SAMPLE	
TARE NO.	<u>2-4B</u>
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	<u>50.53</u>
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"	8.03	
#4	22.48	
#10	36.17	
#20	0.74	
#40	0.71	
#60	1.37	
#100	1.69	
#200	2.24	

WATER CONTENT	
TARE NO.	<u>2-4B</u>
WT. TARE & WET SOIL	<u>32.08</u>
WT. TARE & DRY SOIL	<u>31.43</u>
WT. TARE	<u>1.25</u>
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME 9:45

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR	DIAMETER	PERCENT PASSING
-0.5	9:46	1	44						
	9:47	2	43						
	9:49	4	41						
	9:53	8	40						
✓	10:00	15	38.5						
20	10:15	30	37						
↓	10:45	60	34						
20.5	11:45	120 ^{2h}	32						
20.5	13:45	240 ^{4h}	29						
21	17:45	480 ^{8h}	27						
20.5	9:45	1440 ^{24h}	24						

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BLT 70 - 88 / 09

PROJECT	SAMPLE 2-4C Gouge June 29/03
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	7.44
WT. PASS. #10 & PAN	860.7
WT. PAN	307.7
WT. PASSING #10	553.0
TOTAL WT.	554.44
HYDROMETER SAMPLE	
TARE NO.	1
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	51.25
WATER CONTENT	
WT. DRY SOIL	

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
#4		
#10		
#20	0.03	
#40	0.26	
#60	0.65	
#100	3.37	
#200	10.09	

WATER CONTENT	
TARE NO.	2-4C
WT. TARE & WET SOIL	26.11
WT. TARE & DRY SOIL	24.73
WT. TARE	1.25
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME — 10:00				"K" FACTOR		HYDROMETER TEST				"A" FACTOR	
TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING			
19.5	10:01	1	34								
19.5	10:02	2	30								
19.5	10:04	4	26								
20.0	10:08	8	23								
20.0	10:15	15	21								
20.0	10:30	30	18								
20.5	11:00	60	17								
21.0	12:00	120	15.5								
20	14:00	240									
21	17:55	480									
20	10:00	1440	11								

Recording of these test results constitutes a testing service only. Engineering interpretation or evaluation of the test results is provided only on written request.

PROJECT	SAMPLE <u>240</u>	DATE <u>June 29, 03</u>	TEST HOLE	DEPTH
LAB ORDER	TECHNICIAN	DATE		

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	—
WT. PASS. #10 & PAN	—
WT. PAN	—
WT. PASSING #10	—
TOTAL WT.	—
HYDROMETER SAMPLE	
TARE NO.	<u>240</u>
WT. TARE & WET SOIL	—
WT. TARE	—
WT. WET SOIL	<u>50.25</u>
WATER CONTENT	—
WT. DRY SOIL	—

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10	<u>0.15</u>	
20	<u>0.16</u>	
40	<u>0.09</u>	
60	<u>0.14</u>	
100	<u>1.32</u>	
200		

WATER CONTENT	
TARE NO.	<u>240</u>
WT. TARE & WET SOIL	<u>26.98</u>
WT. TARE & DRY SOIL	<u>26.31</u>
WT. TARE	<u>1.26</u>
WT. WATER	<u>2.78</u>
WT. DRY SOIL	—
WATER CONTENT %	—

START TIME — 9:45

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20	9:46	1	42					
20	9:47	2	39					
20	9:49	4	38					
20	9:53	8	36					
20	10:00	15	31					
20	10:15	30	29					
20	10:45	60	24					
20	11:45	120 ^{2h}	22					
20	1:45	240 ^{4h}	20 20					
20	5:45	480 ^{8h}	16					
20	9:45	1440 ^{24h}	15					

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HLT 70 - 88/09 1.200K 12

PROJECT	SAMPLE 2-41	July 23/03	Shear Gauge
LAB ORDER	TEST HOLE	DEPTH	
	TECHNICIAN	DATE	

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	13.96
WT. PASS. #10 & PAN	817.3
WT. PAN	471.2
WT. PASSING #10	346.1
TOTAL WT.	360.06
HYDROMETER SAMPLE	
TARE NO.	2
WT. TARE & WET SOIL	
WT. TARE	1.24
WT. WET SOIL	50.34
WATER CONTENT	
WT. DRY SOIL	

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.06	
40	0.09	
60	0.54	
100	3.03	
200	7.58	

WATER CONTENT	
TARE NO.	2-41
WT. TARE & WET SOIL	23.65
WT. TARE & DRY SOIL	23.41
WT. TARE	1.24
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME - 9:55 "K" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
19	9:56	1	36.5					
19	9:57	2	33					
19.5	9:59	4	29.5					
19.5	10:03	8	27					
20	10:10	15	24.5					
20	10:25	30	22					
20.5	10:55	1hr	20.5					
21.0	11:55	2hr	19					
20	14:15	4hr	17					
21	18:55	8hr	15					
20	11:55	24hr	14					

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HLBT 70 - 88 / 09

PROJECT	SAMPLE 2-6A June 30/03 failed clay
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE

TOTAL SAMPLE		SIEVE ANALYSIS			WATER CONTENT	
TOTAL WT. RET. ON #10	—	SIEVE SIZE	WT. RETAINED	PERCENT PASS.	TARE NO.	—
WT. PASS. #10 & PAN	—	1"			WT. TARE & WET SOIL	— 28.11
WT. PAN	—	3/4"			WT. TARE & DRY SOIL	— 27.35
WT. PASSING #10	— 579.2	1/2"			WT. TARE	— 1.24
TOTAL WT.	— 579.2	3/8"			WT. WATER	—
HYDROMETER SAMPLE		4			WT. DRY SOIL	—
TARE NO.	—	10			WATER CONTENT %	—
WT. TARE & WET SOIL	—	20	0.80			
WT. TARE	—	40	0.91			
WT. WET SOIL	— 50.07	60	0.75			
WATER CONTENT	—	100	0.85			
WT. DRY SOIL	—	200	1.00			

START TIME — 9:05		"K" FACTOR		HYDROMETER TEST					"A" FACTOR		
TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT. PASSING			
21	9:06	1	44								
21	9:07	2	43								
21	9:09	4	43								
21	9:13	8	42								
21	9:20	15	37.5								
21	9:35	30	37								
21	10:05	60	34								
21	11:05	120 ^{2h}	32								
21	1:05	240 ^{4h}	29.5								
21	5:05	480 ^{8h}	27								
21	8:07	1440 ^{24h}	24								

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HBLT 70 - 88/09

PROJECT

LAB ORDER

SAMPLE 2-6B June 30/03 Shear gouge in shaft

TEST HOLE

DEPTH

TECHNICIAN

DATE

TOTAL SAMPLE

TOTAL WT. RET. ON #10 — 80.72
WT. PASS. #10 & PAN — 601.1
WT. PAN — 337.4
WT. PASSING #10 — 263.7
TOTAL WT. — 284.42

HYDROMETER SAMPLE

TARE NO. — ③
WT. TARE & WET SOIL —
WT. TARE —
WT. WET SOIL — 93.81
WATER CONTENT —
WT. DRY SOIL —

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	12.27	
40	10.69	
60	5.04	
100	3.33	
200	3.02	

WATER CONTENT

TARE NO. — ③
WT. TARE & WET SOIL — 28.74
WT. TARE & DRY SOIL — 27.79
WT. TARE — 1.24
WT. WATER —
WT. DRY SOIL —
WATER CONTENT % —

START TIME — 10:30

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR DIAMETER	PERCENT PASSING
20	10:31	1	52					
20	10:32	2	50					
20	10:34	4	48					
20	10:38	8	46					
20	10:45	15	44					
20	11:00	30	42					
20	11:30	60	40					
20.5	12:30	120	37					
20	4:30	240	35.34					
21.20	13:30	480	30					
5	9:25	24hr	25.5					

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BLT 70 - 88 / 09

PROJECT _____		SAMPLE <u>2-8A July 3/03 Upper Shear Plane</u>	
LAB ORDER _____		TEST HOLE _____	DEPTH _____
TECHNICIAN _____		DATE _____	

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	<u>5.95</u>
WT. PASS. #10 & PAN	<u>881.0</u>
WT. PAN	<u>469.0</u>
WT. PASSING #10	<u>411.0</u>
TOTAL WT:	<u>416.95</u>
HYDROMETER SAMPLE	
TARE NO.	<u>(2)</u>
WT. TARE & WET SOIL	_____
WT. TARE	_____
WT. WET SOIL	<u>50.30</u>
WATER CONTENT	_____
WT. DRY SOIL	_____

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20		
40	<u>0.01</u>	
60	<u>0.05</u>	
100	<u>0.08</u>	
200	<u>0.06</u>	

WATER CONTENT	
TARE NO.	<u>2-8A</u>
WT. TARE & WET SOIL	<u>27.83</u>
WT. TARE & DRY SOIL	<u>27.49</u>
WT. TARE	<u>1.25</u>
WT. WATER	_____
WT. DRY SOIL	_____
WATER CONTENT %	_____

START TIME - 10:35 "K" FACTOR _____

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	"A" FACTOR		
						EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20	1	10:36	<u>53</u>					
20	2	10:37	<u>52.5</u>					
20	4	10:39	<u>51.5</u>					
20	8	10:43	<u>50.5</u>					
20	15	10:50	<u>49</u>					
20	30	11:05	<u>47</u>					
20	1hr	11:35	<u>45</u>					
20.5	2hr	12:35	<u>42.5</u>					
21	4hr	14:35	<u>38</u>					
21	.8hr	10:35	<u>34</u>					
9.5	24hr	9:25	<u>29</u>					

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HBLT 70 - 88 / 09

PROJECT 6X04091

SAMPLE 2-8AA

Intact Clay II

LAB ORDER

TEST HOLE

DEPTH

TECHNICIAN

DATE 23 July 2003

TOTAL SAMPLE	
TOTAL WT. RET. ON #10 —	
WT. PASS. #10 & PAN —	
WT. PAN —	
WT. PASSING #10 —	
TOTAL WT: —	

HYDROMETER SAMPLE	
TARE NO.	
WT. TARE & WET SOIL —	
WT. TARE —	
WT. WET SOIL —	<u>50.14</u>
WATER CONTENT —	
WT. DRY SOIL —	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20		
40		
60		
100		
200	<u>0.0</u>	

WATER CONTENT	
TARE NO.	<u>928</u>
WT. TARE & WET SOIL —	<u>16.14</u>
WT. TARE & DRY SOIL —	<u>15.91</u>
WT. TARE —	<u>1.25</u>
WT. WATER —	
WT. DRY SOIL —	
WATER CONTENT % —	

ART TIME — 8:30

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20	8:31	1	52					
20	8:32	2	50.5					
20	8:34	4	50					
20	8:38	8	50					
20	8:45	15	49					
20	9:00	30	48.5					
20	9:30	60	46					
20	10:30	120 ^{2h}	44					
20	12:30	240 ^{4h}	41					
21	16:30	480 ^{8h}	38					
19.5	8:30	1440 ^{24h}	32					

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HBLT 70 - 88 / 09

PROJECT
LAB ORDERSAMPLE 2-8B July 3/03 intact clay, beneath shear
TEST HOLE
TECHNICIAN
DEPTH (6cm)
DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	0.34
WT. PASS. #10 & PAN	603.9
WT. PAN	331.4
WT. PASSING #10	272.5
TOTAL WT.	272.84

HYDROMETER SAMPLE	
TARE NO.	2-8B
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	49.96
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.01	
40	0.01	
60	0.01	
100	0.02	
200	0.04	

WATER CONTENT	
TARE NO.	2-8B
WT. TARE & WET SOIL	24.20
WT. TARE & DRY SOIL	23.68
WT. TARE	1.24
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

RT TIME - 8:25

EMP °C

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
8:26	1	38					
8:27	2	33					
8:30	4	31					
8:33	8	30					
8:40	15	29.5					
8:50	30	29					
9:25	60	28					
10:25	120	28					
12:30	240	27					
4:025	480	26					
8:30	1440	22.5					

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2-88/09

PROJECT Gx04091
LAB ORDER

SAMPLE 2-8D Lower Slide Pane

TEST HOLE DEPTH

TECHNICIAN

DATE July 3/02

TOTAL SAMPLE	
TOTAL WT. RET. ON #10 —	
WT. PASS. #10 & PAN —	
WT. PAN —	
WT. PASSING #10 —	
TOTAL WT. —	

HYDROMETER SAMPLE	
TARE NO. —	2-8D
WT. TARE & WET SOIL —	
WT. TARE —	
WT. WET SOIL —	49.93
WATER CONTENT —	
WT. DRY SOIL —	

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.02	
40	0.03	
60	0.04	
100	0.06	
200	0.08	

WATER CONTENT	
TARE NO. —	2-8D
WT. TARE & WET SOIL —	19.09
WT. TARE & DRY SOIL —	18.77
WT. TARE —	1.25
WT. WATER —	
WT. DRY SOIL —	
WATER CONTENT % —	

START TIME — 8:35

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	"A" FACTOR				
				COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
22	8:36	1	44.44					
22	8:37	2	43					
22	8:39	4	42					
22	8:42.3	8	40					
22	8:50	15	39					
22	9:05	30	37					
22	9:35	60	36					
22	10:35	120 ^{2h}	32					
22	12:35	240 ^{4h}	30					
22	4:35	480 ^{8h}	28					
22	8:35	1440 ^{24h}	24					

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HBLT 70 - 88/09

PROJECT	SAMPLE 3-1A July 6/03 Shear Plane (10	
LAB ORDER	TEST HOLE	DEPTH
	TECHNICIAN	DATE

TOTAL SAMPLE		SIEVE ANALYSIS			WATER CONTENT	
TOTAL WT. RET. ON #10	5.20	SIEVE SIZE	WT. RETAINED	PERCENT PASS.	TARE NO.	3-1A
WT. PASS. #10 & PAN	701.7	1"			WT. TARE & WET SOIL	25.49
WT. PAN	299.6	1/2"			WT. TARE & DRY SOIL	24.94
WT. PASSING #10	402.1	3/8"			WT. TARE	125
TOTAL WT.	407.3	4			WT. WATER	
HYDROMETER SAMPLE		10			WT. DRY SOIL	
TARE NO.	3	20	0.09		WATER CONTENT %	
WT. TARE & WET SOIL		40	0.14			
WT. TARE		60	0.13			
WT. WET SOIL	50.02	100	0.13			
WATER CONTENT		200	0.22			
WT. DRY SOIL						

START TIME — 9:55 "K" FACTOR				HYDROMETER TEST "A" FACTOR				
TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20	9:56	1	55					
20	9:57	2	55					
20	9:59	4	53.5					
20	10:03	8	52					
20	10:10	15	50					
20	10:25	30	47					
20	10:55	60	44					
20.5	11:55	120	40					
21	13:55	240	36.5					
20	18:25	480	33					
21	13:00	1440	29					

PROJECT	SAMPLE 3-5A	DATE JUN 8 2005	ADJACENT SHEAR
LAB ORDER	TEST HOLE	DEPTH	
	TECHNICIAN	DATE	

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	1.08
WT. PASS. #10 & PAN	1002.8
WT. PAN	566.0
WT. PASSING #10	436.8
TOTAL WT.	437.88
HYDROMETER SAMPLE	
TARE NO.	1
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	50.47
WATER CONTENT	
WT. DRY SOIL	

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.26	
40	0.49	
60	0.74	
100	1.39	
200	3.40	

WATER CONTENT	
TARE NO.	3-5A
WT. TARE & WET SOIL	34.94
WT. TARE & DRY SOIL	33.4
WT. TARE	1.24
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

ART TIME — 10:40		"K" FACTOR		HYDROMETER TEST					"A" FACTOR		
TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING			
20	1	10:41	42								
20	2	10:42	39.5								
20	4	10:44	37								
20	8	10:48	34								
20	15	10:55	31								
20	30	10:10	30								
20	1hr	11:40	25.5								
20	2hr	12:40	23								
20	4hr	14:40	21.5								
21	8hr	18:15	19.5								
21.5	24hr	19:25	19								

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HBLT 70 - 88 / 09

PROJECT <u>GX04091</u>	SAMPLE <u>3-5B</u> <u>Gouge</u>
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE <u>July 8/08</u>

TOTAL SAMPLE	
TOTAL WT. RET. ON #10 —	_____
WT. PASS. #10 & PAN —	_____
WT. PAN —	_____
WT. PASSING #10 —	_____
TOTAL WT. —	_____
HYDROMETER SAMPLE	
TARE NO. <u>3-5B</u>	_____
WT. TARE & WET SOIL —	_____
WT. TARE —	_____
WT. WET SOIL —	<u>99.81</u>
WATER CONTENT —	_____
WT. DRY SOIL —	_____

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
¾"		
½"		
¼"		
4		
10		
20	1.28	
40	1.37	
60	1.17	
100	8.56	
200	18.04	

WATER CONTENT	
TARE NO. <u>3-5B</u>	_____
WT. TARE & WET SOIL —	<u>25.28</u>
WT. TARE & DRY SOIL —	<u>24.06</u>
WT. TARE —	<u>1.26</u>
WT. WATER —	_____
WT. DRY SOIL —	_____
WATER CONTENT % —	_____

START TIME — 8:40

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
22	8:41	1	59					
22	8:42	2	53					
22	8:44	4	48					
22	8:48	8	36					
22	8:55	15	34					
22	9:10	30	30					
22	9:40	60	26					
22	10:40	120 ^{2h}	24					
22	12:40	240 ^{4h}	20					
22	4:40	480 ^{8h}	19					
22	08:30	1440 ^{24h}	18					

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HBLT 70 - 88 / 09

PROJECT
LAB ORDER

SAMPLE 3-13A July 12/03 Remolded shale

TEST HOLE

DEPTH

TECHNICIAN

DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	29.73
WT. PASS. #10 & PAN	1250.6
WT. PAN	552.0
WT. PASSING #10	698.6
TOTAL WT.	728.33

HYDROMETER SAMPLE	
TARE NO.	3-13A
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	49.82
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	3.58	
40	2.68	
60	1.04	
100	0.82	
200	1.12	

WATER CONTENT	
TARE NO.	3-13A
WT. TARE & WET SOIL	35.17
WT. TARE & DRY SOIL	34.40
WT. TARE	1.24
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

START TIME — 8:30

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
21	8:31	1	25					
21	8:32	2	23					
21	8:34	4	23					
21	8:38	8	22					
21	8:45	15	21					
21	8:50	60 30	19.5					
21	9:30	120 60	18					
21	10:30	240 120	16					
21	12:30	480 240	14					
21	4:30	480	12					
	8:32	1440	10					

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HBLT 70 - 88 / 09

PROJECT (20401)

LAB ORDER

SAMPLE 3-14A

Shale Shear

TEST HOLE

DEPTH

TECHNICIAN

DATE 13 July 2003

TOTAL SAMPLE

TOTAL WT. RET. ON #10 —

WT. PASS. #10 & PAN —

WT. PAN —

WT. PASSING #10 —

TOTAL WT. —

HYDROMETER SAMPLE

TARE NO. —

WT. TARE & WET SOIL —

WT. TARE —

WT. WET SOIL — 50.1

WATER CONTENT —

WT. DRY SOIL —

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	3.02	
40	0.05	
60	0.06	
100	0.10	
200	0.30	

WATER CONTENT

TARE NO. —

WT. TARE & WET SOIL — 22.89

WT. TARE & DRY SOIL — 22.46

WT. TARE — 1.25

WT. WATER —

WT. DRY SOIL —

WATER CONTENT % —

START TIME — 8:35

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20	8:36	1	40					
20	8:37	2	38.5					
20	8:39	4	37					
20	8:43	8	35					
20	8:50	15	34					
20	9:05	30	31					
20	9:35	60	29					
20	10:35	120 ^{2h}	27					
20	12:35	240 ^{4h}	24					
21	16:35	480 ^{8h}	22					
19.5	8:35	1440 ^{24h}	10					

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HBLT 70 - 88/09

PROJECT G804091
LAB ORDER

SAMPLE July 13/03 3-14B Shear remolded Clay
TEST HOLE DEPTH middle material
TECHNICIAN DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	—
WT. PASS. #10 & PAN	—
WT. PAN	—
WT. PASSING #10	—
TOTAL WT.	—

HYDROMETER SAMPLE	
TARE NO.	<u>3-14B</u>
WT. TARE & WET SOIL	—
WT. TARE	—
WT. WET SOIL	<u>49.84</u>
WATER CONTENT	—
WT. DRY SOIL	—

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.07	
40	0.08	
60	0.09	
100	0.10	
200	0.17	

WATER CONTENT	
TARE NO.	<u>3-14B</u>
WT. TARE & WET SOIL	<u>28.22</u>
WT. TARE & DRY SOIL	<u>27.70</u>
WT. TARE	<u>1.25</u>
WT. WATER	—
WT. DRY SOIL	—
WATER CONTENT %	—

START TIME — 9:55

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
20.5	9:56	1	51					
	9:57	2	50					
	9:59	4	48					
	10:03	8	46					
✓	10:10	15	42					
20	10:25	30	38.5					
↓	10:55	60	34					
20.5	11:55	120 ^{2h}	30					
20.5	13:55	240 ^{4h}	26.5					
21	17:55	480 ^{8h}	23					
20.5	9:55	1440 ^{24h}	18					

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HBLT 70 - 88/09

PROJECT GX04091
LAB ORDER

SAMPLE July 13/03 3-15A Shale Slur
TEST HOLE DEPTH
TECHNICIAN DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10 —	<u>45.55</u>
WT. PASS. #10 & PAN <u>Wet</u>	<u>425.52</u>
WT. PAN —	—
WT. PASSING #10 Dry —	—
TOTAL WT. —	—

HYDROMETER SAMPLE	
TARE NO.	<u>3-15A</u>
WT. TARE & WET SOIL —	—
WT. TARE —	—
WT. WET SOIL —	<u>49.94</u>
WATER CONTENT —	—
WT. DRY SOIL —	—

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4	<u>4.22</u>	
10	<u>41.33</u>	
20	<u>0.48</u>	
40	<u>0.42</u>	
60	<u>0.34</u>	
100	<u>0.75</u>	
200	<u>4.55</u>	

WATER CONTENT	
TARE NO.	<u>3-15A</u>
WT. TARE & WET SOIL —	<u>24.14</u>
WT. TARE & DRY SOIL —	<u>23.64</u>
WT. TARE —	<u>1.27</u>
WT. WATER —	—
WT. DRY SOIL —	—
WATER CONTENT % —	—

START TIME — 9:50

"K" FACTOR

HYDROMETER TEST

TEMP °C		TIME		TIME INTERVAL		HYD. RDG.		COMP. CORR.		R		EFFECTIVE DEPTH		DIAMETER		PERCENT PASSING	
<u>20.5</u>		<u>9:51</u>		<u>1</u>		<u>42</u>											
		<u>9:52</u>		<u>2</u>		<u>39.5</u>											
		<u>9:54</u>		<u>4</u>		<u>37.5</u>											
		<u>9:58</u>		<u>8</u>		<u>36</u>											
		<u>10:05</u>		<u>15</u>		<u>35</u>											
<u>20</u>		<u>10:25</u>		<u>30</u>		<u>32</u>											
		<u>10:50</u>		<u>60</u>		<u>29.5</u>											
<u>20.5</u>		<u>11:50</u>		<u>120</u> ^{2h}		<u>27.5</u>											
<u>20.5</u>		<u>13:50</u>		<u>240</u> ^{4h}		<u>24.5</u>											
<u>21</u>		<u>17:50</u>		<u>480</u> ^{8h}		<u>22.5</u>											
<u>20.5</u>		<u>9:50</u>		<u>1440</u> ^{24h}		<u>19.0</u>											

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HBLT 70 - 88/09

Earth & Environmental

GRAIN SIZE ANALYSIS

42

PROJECT 6X0409

LAB ORDER

SAMPLE

3-1508

Remolded Shale

TEST HOLE

DEPTH

TECHNICIAN

DATE 13 July 2003

TOTAL SAMPLE

TOTAL WT. RET. ON #10 —

WT. PASS. #10 & PAN —

WT. PAN —

WT. PASSING #10 —

TOTAL WT. —

HYDROMETER SAMPLE

TARE NO. —

WT. TARE & WET SOIL —

WT. TARE —

WT. WET SOIL —

WATER CONTENT —

WT. DRY SOIL —

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
¾"		
½"		
¼"		
#4		
#10		
#20	0.65	
#40	0.36	
#60	0.21	
#100	0.77	
#200	4.83	

WATER CONTENT

TARE NO. —

WT. TARE & WET SOIL — 23.71

WT. TARE & DRY SOIL — 22.87

WT. TARE — 1.24

WT. WATER —

WT. DRY SOIL —

WATER CONTENT % —

START TIME — 8:25

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR DIAMETER	PERCENT PASSING
20	8:26	1	45					
20	8:27	2	42					
20	8:29	4	39					
20	8:33	8	37					
20	8:40	15	36					
20	8:55	30	34.5					
20	9:25	60	33					
20	10:25	120 ^{2h}	31					
20	12:25	240 ^{4h}	29					
21	16:25	480 ^{8h}	28					
19.5	8:25	1440 ^{24h}	25.5					

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HBLT 70 - 88 / 09

PROJECT 0X04091	SAMPLE 4-2A ICS Shear
LAB ORDER	TEST HOLE DEPTH
	TECHNICIAN DATE July 19, 03

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	—
WT. PASS. #10 & PAN	—
WT. PAN	—
WT. PASSING #10	—
TOTAL WT.	—
HYDROMETER SAMPLE	
TARE NO.	4-2A
WT. TARE & WET SOIL	—
WT. TARE	—
WT. WET SOIL	50.17
WATER CONTENT	—
WT. DRY SOIL	—

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.28	
40	0.22	
60	0.28	
100	0.40	
200	1.35	

WATER CONTENT	
TARE NO.	4-2A
WT. TARE & WET SOIL	19.96
WT. TARE & DRY SOIL	19.43
WT. TARE	1.24
WT. WATER	—
WT. DRY SOIL	—
WATER CONTENT %	—

START TIME 8:30

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
22	8:31	1	40					
22	8:32	2	37.5					
22	8:34	4	36					
22	8:38	8	34					
22	8:45	15	31					
22	9:00	30	28.28					
22	9:30	60	24.24					
22	10:30	120 ^{2h}	22					
22	12:30	240 ^{4h}	20					
22	4:30	480 ^{8h}	19					
22	8:30	1440 ^{24h}	18					

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HBLT 70 - 88 / 09

PROJECT 6X04091
LAB ORDER

SAMPLE 4-2B Intact Clay Silt + Clay
TEST HOLE DEPTH
TECHNICIAN
DATE July 19/03

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	—
WT. PASS. #10 & PAN	—
WT. PAN	—
WT. PASSING #10	—
TOTAL WT.	—

HYDROMETER SAMPLE	
TARE NO.	4-2B
WT. TARE & WET SOIL	—
WT. TARE	—
WT. WET SOIL	50.12
WATER CONTENT	—
WT. DRY SOIL	—

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.06	
40	0.07	
60	0.17	
100	0.33	
200	0.53	

WATER CONTENT	
TARE NO.	4-2B
WT. TARE & WET SOIL	17.04
WT. TARE & DRY SOIL	16.75
WT. TARE	6.26
WT. WATER	0.06
WT. DRY SOIL	—
WATER CONTENT %	—

START TIME — 10:10

"K" FACTOR

HYDROMETER TEST

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	"A" FACTOR DIAMETER	PERCENT PASSING
21	10:11	1	44					
	10:12	2	39					
	10:14	4	32.5					
	10:18	8	25.5					
	10:25	15	20					
	10:40	30	15					
	11:10	60	13					
	12:10	120 ^{2h}	12.5					
	14:10	240 ^{4h}	11					
21.5	18:10	480 ^{8h}	10					
21	10:10	1440 ^{24h}	9					

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HBLT 70 - 88/09

PROJECT	SAMPLE 4-4A July 22/03 Shear upper ICS	
LAB ORDER	TEST HOLE	DEPTH
	TECHNICIAN	DATE

TOTAL SAMPLE	
TOTAL WT. RET. ON #10	10.35
WT. PASS. #10 & PAN	793.6
WT. PAN	323.6
WT. PASSING #10	470.0
TOTAL WT:	480.35
HYDROMETER SAMPLE	
TARE NO.	4-4A
WT. TARE & WET SOIL	
WT. TARE	
WT. WET SOIL	49.96
WATER CONTENT	
WT. DRY SOIL	

SIEVE ANALYSIS		
SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
4		
10		
20	0.26	
40	0.73	
60	1.16	
100	1.58	
200	1.71	

WATER CONTENT	
TARE NO.	4-4A
WT. TARE & WET SOIL	25.96
WT. TARE & DRY SOIL	24.82
WT. TARE	1.23
WT. WATER	
WT. DRY SOIL	
WATER CONTENT %	

ART TIME — 8:20

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
21	8:21	1	26.5					
21	8:22	2	25					
21	8:24	4	24					
21	8:28	8	23					
21	8:35	15	22.5					
21	8:50	30	22					
21	9:20	60	21					
21	10:20	120	19.5					
21	12:30	240	18.5					
21	4:20	480	16.5					
21	8:30	1440	14					

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HBLT 70 - 88/09

PROJECT GX04091 SAMPLE 4-4B Shear 10S
LAB ORDER _____ TEST HOLE _____ DEPTH _____
TECHNICIAN _____ DATE July 22/03

TOTAL SAMPLE
TOTAL WT. RET. ON #10 — _____
WT. PASS. #10 & PAN — _____
WT. PAN — _____
WT. PASSING #10 — _____
TOTAL WT. — _____

HYDROMETER SAMPLE
TARE NO. 4-4B
WT. TARE & WET SOIL — _____
WT. TARE — _____
WT. WET SOIL — 49.98
WATER CONTENT — _____
WT. DRY SOIL — _____

SIEVE ANALYSIS

SIEVE SIZE	WT. RETAINED	PERCENT PASS.
1"		
3/4"		
1/2"		
3/8"		
#4		
#10		
#20	0.09	
#40	0.25	
#60	0.60	
#100	0.45	
#200	0.37	

WATER CONTENT
TARE NO. 4-4B
WT. TARE & WET SOIL — 21.03
WT. TARE & DRY SOIL — 19.86
WT. TARE — 1.21
WT. WATER — _____
WT. DRY SOIL — _____
WATER CONTENT % — _____

START TIME — 10:05

"K" FACTOR

HYDROMETER TEST

"A" FACTOR

TEMP °C	TIME	TIME INTERVAL	HYD. RDG.	COMP. CORR.	R	EFFECTIVE DEPTH	DIAMETER	PERCENT PASSING
21	10:06	1	46					
	10:07	2	45					
	10:09	4	44					
	10:13	8	43					
	10:20	15	42.5					
	10:35	30	40.5					
	11:05	60	38					
	12:05	120 ^{2h}	33.5					
✓	14:05	240 ^{4h}	19					
21.5	18:05	480 ^{8h}	8.5					
21	10:05	1440 ^{24h}	4					

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HBLT 70, 88/09

311

TO: Geological Survey of Canada
601 Booth Street
Ottawa ON K1A 0E8

OFFICE: Fort St. John
PROJECT NO: GX04091
CLIENT: Geological Survey of Canada
COPIES TO: Mr. Jordan Severin

ATTENTION: Mr. Larry Dyke

PROJECT: Peace & Beatton Rivers

SAMPLE NO.	DATE	SOIL DESCRIPTION	LL	PL	PI	MUSC
1-1C	July 20/03	Shale Failure Plane	39.4	21.1	18.3	CI
1-4A	June 21/03	Shear Surface #1 ICS	50.9	17.8	33.1	CH
1-4B	July 23/03	Massive Clay	41.2	17.4	23.8	CI
2-2A	June 26/03	Shale/Shear?	40.0	19.6	20.4	CI
2-2B	June 27/03	Debris/shear	43.1	19.5	23.6	CI
2-4B	June 29/03	Debris Shear Plane	47.4	19.0	28.4	CI/CH
2-4C	June 29/03	Gouge	49.8	29.6	20.2	CI/CH
2-4D	June 29/03	Shale Shear Plane	40.9	19.0	21.9	CI
2-4I	July 23/03	Shear Gouge	55.7	30.4	25.3	CH
2-6A	June 30/03	Failed Clay	47.6	19.6	28.0	CI/CH
2-6B	June 30/03	Shear Gouge in Shale	44.9	22.7	22.2	CI
2-8A	July 3/03	Upper Shear Plane	47.4	19.4	28.0	CI/CH
2-8AA	July 23/03	Intact Clay II	50.9	19.6	31.3	CI/CH
2-8B	July 3/03	Intact Clay Beneath Shear (5cm)	69.4	21.2	48.2	CH
2-8D	July 3/03	Lower Slide Plane	47.6	21.2	26.4	CI/CH
3-1A	July 6/03	Shear Plane ICS	53.7	19.9	33.8	CI/CH
3-5A	July 8/03	Adjacent Shear	52.9	18.2	34.7	CI/CH
3-5B	July 8/03	Gauge	52.3	22.9	29.4	CI/CH
3-13A	July 12/03	Remolded Shale	41.9	18.0	23.9	CI
3-14A	July 13/03	Shale Shear	40.6	19.9	20.7	CI
3-14B	July 13/03	Remolded Clay? Middle Material	40.6	19.2	21.4	CI
3-15A	July 13/03	Shale Shear	55.4	15.9	39.5	CH
3-15B	July 13/03	Remolded Shale	80.4	29.6	50.8	CH
4-2A	July 19/03	ICS Shear	38.8	14.6	24.2	CI
4-2B	July 19/03	Intact Clay	28.1	19.9	8.2	CL
4-4A	July 22/03	Shear Upper ICS	57.5	22.7	34.8	CH
4-4B	July 22/03	Shear ICS	73.2	20.7	52.5	CH
PI-1A	June 24/03	Shear Plane	51.1	18.1	33.0	CI/CH

LL = Liquid Limit
PL = Plastic Limit
PI = Plasticity Index
MUSC = Modified Unified Classification System

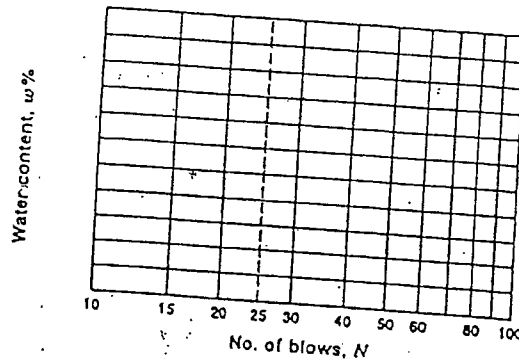
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04811
 Location of Project _____ Boring No. _____ Sample No. 1-1C
 Description of Soil Shale Failure Plane (20 July)
 Depth of Sample _____ Tested By _____ Date 24.9.2003

Liquid Limit Determination

Can no.	A	B				
Wt. of wet soil + can	28.95	24.35				
Wt. of dry soil + can	21.11	17.84				
Wt. of can	1.24	1.23				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	39.5	39.2				
No. of blows, N	26	24				
	39.7	39.0				



Flow index $F_i =$ _____
 Liquid limit = 39.4
 Plastic limit = 21.1
 Plasticity index $I_p =$ 18.3

CI

Plastic Limit Determination

Can no.	C	D		
Wt. of wet soil + can	8.65	6.35		
Wt. of dry soil + can	7.28	5.46		
Wt. of can	1.24	1.24		
Wt. of dry soil	6.04			
Wt. of moisture				
Water content, $w\% = w_p$	21.0	21.1		

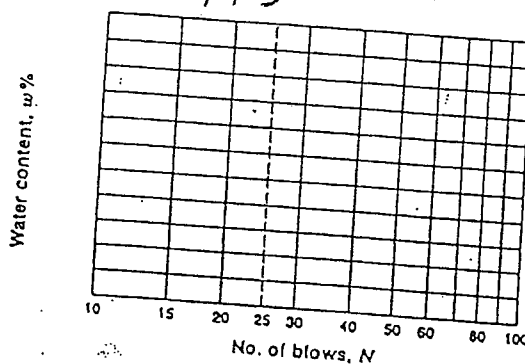
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 1-4A Sample No. _____
 Description of Soil Shear Surface #1
 Depth of Sample _____ Tested By _____ Date July June 21

Liquid Limit Determination

Can no.	1	2					
Wt. of wet soil + can	30.809	31.14					
Wt. of dry soil + can	21.26	21.97					
Wt. of can	1.24	1.24					
Wt. of dry soil							
Wt. of moisture							
Water content, w%	44.1	44.2					
No. of blows, N	26	29					
	44.3						



Flow index $F_i =$ _____
 Liquid limit = 50.9
 Plastic limit = 17.8
 Plasticity index $I_p =$ 33.1

CH

Plastic Limit Determination

Can no.	3	4				
Wt. of wet soil + can	4.27	4.04				
Wt. of dry soil + can	3.82	3.61				
Wt. of can	1.25	1.23				
Wt. of dry soil						
Wt. of moisture						
Water content, w% = w_p	17.5	18.1				

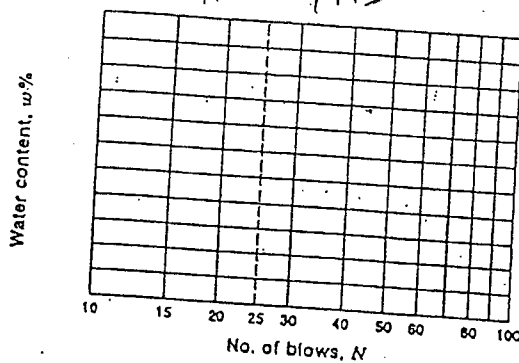
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. Gx04091
 Location of Project _____ Boring No. 1-4B Sample No. _____
 Description of Soil Massive Clay Tested By _____
 Depth of Sample _____ Date July 23

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	34.52	29.62				
Wt. of dry soil + can	24.65	21.20				
Wt. of can	1.84	1.85				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	42.2	42.2				
No. of blows, N	20	21				
	41.1	41.3				



Flow index $F_i = 41.2$
 Liquid limit = 41.2
 Plastic limit = 17.4
 Plasticity index $I_p = 23.8$

C1

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	9.55	8.16		
Wt. of dry soil + can	8.31	7.14		
Wt. of can	1.24	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	17.5	17.3		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

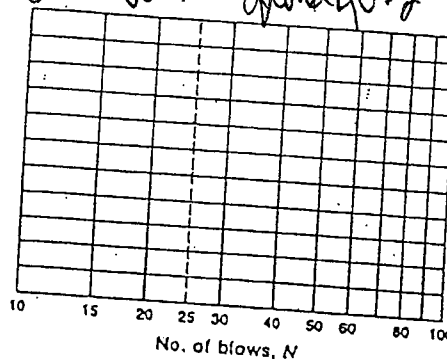
Project _____ Job No. GX04091
 Location of Project _____ Boring No. 2-2A Sample No. _____
 Description of Soil Shale/Shear?
 Depth of Sample _____ Tested By _____ Date Jan 26/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	31.92	32.58				
Wt. of dry soil + can	23.01	23.45				
Wt. of can	1.24	1.24				
Wt. of dry soil						
Wt. of moisture	40.9	41.1				
Water content, w%	40.9	41.1				
No. of blows, N	20	21				

39.8 ~~20.5~~ 40.2

Water content, w%



Flow index $F_i =$ _____

Liquid limit = 40.0

Plastic limit = 19.6

Plasticity index $I_p =$ 20.4

cl

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	4.37	4.55		
Wt. of dry soil + can	3.86	4.01		
Wt. of can	1.24	1.25		
Wt. of dry soil				
Wt. of moisture	19.5	19.6		
Water content, w% = w_p	19.5	19.6		

ATTERBERG LIMITS DETERMINATION

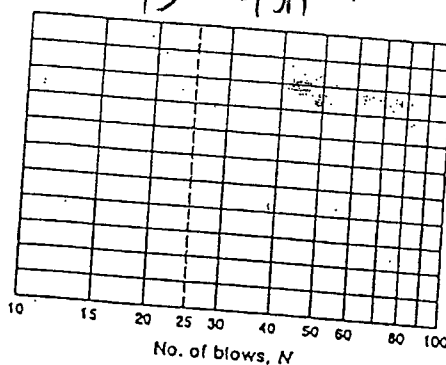
Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 2-2B Sample No. _____
 Description of Soil Debris / Shear
 Depth of Sample _____ Tested By _____ Date June 27

Liquid Limit Determination

Can no.	<u>1</u>	<u>2</u>				
Wt. of wet soil + can	<u>27.23</u>	<u>34.75</u>				
Wt. of dry soil + can	<u>19.49</u>	<u>24.79</u>				
Wt. of can	<u>1.23</u>	<u>1.25</u>				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	<u>42.4</u>	<u>42.3</u>				
No. of blows, N	<u>28</u>	<u>29</u>				
	<u>43</u>	<u>43.1</u>				

Water content, $w\%$



Flow index $F_i =$ _____
 Liquid limit $=$ 43.1
 Plastic limit $=$ 19.5
 Plasticity index $I_p =$ 23.6

1

Plastic Limit Determination

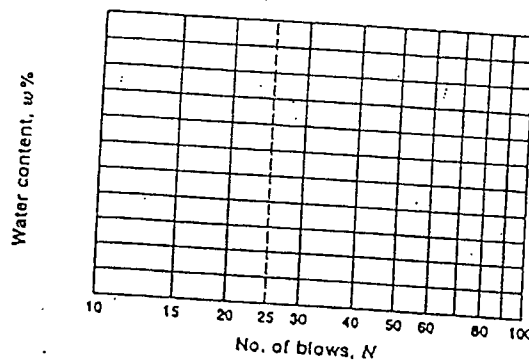
Can no.	<u>3</u>	<u>4</u>		
Wt. of wet soil + can	<u>5.09</u>	<u>4.17</u>		
Wt. of dry soil + can	<u>4.46</u>	<u>3.70</u>		
Wt. of can	<u>1.26</u>	<u>1.25</u>		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	<u>19.1</u>	<u>19.2</u>		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. _____ Sample No. Z-4B
 Description of Soil Debris Shear Plane 29 June 2003
 Depth of Sample _____ Tested By _____ Date _____
 Liquid Limit Determination

Can no.	A	B				
Wt. of wet soil + can	31.43	31.87				
Wt. of dry soil + can	21.68	22.02				
Wt. of can	1.24	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	47.7	47.4				
No. of blows, N	26	23				
	47.9	46.9				



Flow index $F_i =$ _____
 Liquid limit = 47.4
 Plastic limit = 19.0
 Plasticity index $I_p =$ 28.4

CI/CH

Plastic Limit Determination

Can no.	C	D		
Wt. of wet soil + can	3.40	3.28		
Wt. of dry soil + can	3.07	2.94		
Wt. of can	1.24	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	18.0	20.0		

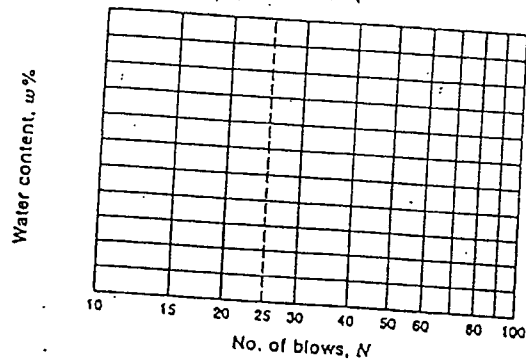
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 2-4C Sample No. _____
 Description of Soil Googe
 Depth of Sample _____ Tested By _____ Date June 29/03

Liquid Limit Determination

Can no.	1	2			
Wt. of wet soil + can	34.24	33.29			
Wt. of dry soil + can	23.60	22.45			
Wt. of can	1.25	1.24			
Wt. of dry soil					
Wt. of moisture					
Water content, w%	51.0	51.1			
No. of blows, N	20	20			
	49.7	49.8			



Flow index $F_i =$ _____
 Liquid limit = 49.8
 Plastic limit = 29.6
 Plasticity index $I_p =$ 20.2

d

Plastic Limit Determination

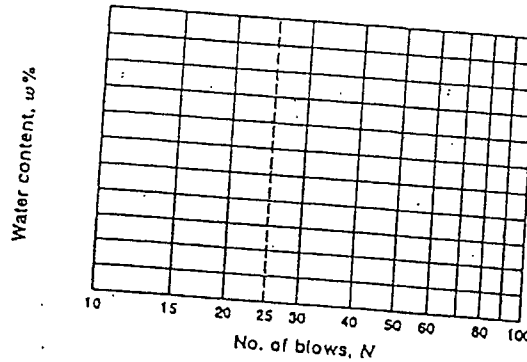
Can no.	3	4		
Wt. of wet soil + can	6.99	6.87		
Wt. of dry soil + can	5.68	5.59		
Wt. of can	1.27	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	29.7	29.5		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. _____ Sample No. 240
 Description of Soil Shale Shear Plane, 29 June 2003
 Depth of Sample _____ Tested By _____ Date _____
 Liquid Limit Determination

Can no.	A	B				
Wt. of wet soil + can	22.92	30.43				
Wt. of dry soil + can	16.53	21.84				
Wt. of can	1.24	1.23				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	41.6	41.6				
No. of blows, N	23	20				
	41.2	40.5				



Flow index $F_i =$ _____
 Liquid limit = 40.9
 Plastic limit = 19.0
 Plasticity index $I_p =$ 21.9

CI

Plastic Limit Determination

Can no.	C	D		
Wt. of wet soil + can	3.86	3.71		
Wt. of dry soil + can	3.44	3.32		
Wt. of can	1.25	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	19.2	18.8		

ATTERBERG LIMITS DETERMINATION

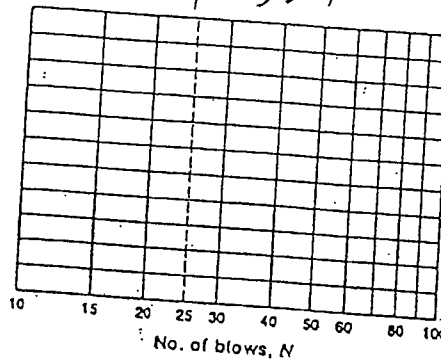
Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. 2-4E Sample No. _____
 Description of Soil Shear Gouge
 Depth of Sample _____ Tested By _____ Date July 23/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	32.30	29.98				
Wt. of dry soil + can	21.36	19.84				
Wt. of can	1.24	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	54.4	54.5				
No. of blows, N	30	30				
	55.7	55.7				

Water content, $w\%$



Flow index $F_i =$ _____
 Liquid limit = 55.7
 Plastic limit = 30.4
 Plasticity index $I_p =$ 25.3

CK

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	7.13	7.98		
Wt. of dry soil + can	5.77	6.39		
Wt. of can	1.24	1.23		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	30.0	30.8		

ATTERBERG LIMITS DETERMINATION

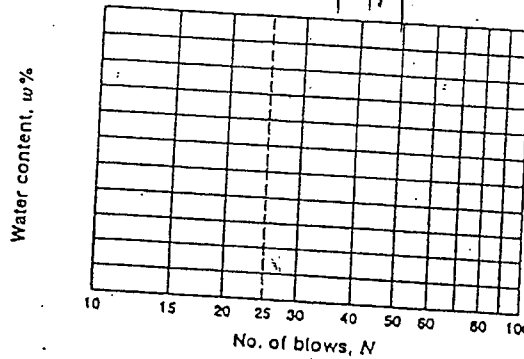
Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 2-6A Sample No. _____
 Description of Soil Filled Clay
 Depth of Sample _____ Tested By _____ Date June 30/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	31.80	32.71				
Wt. of dry soil + can	21.97	22.60				
Wt. of can	1.24	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	47.4	47.4				
No. of blows, N	27	25				

47.8 47.4



Flow index $F_i =$ _____

Liquid limit = 47.6

Plastic limit = 19.6

Plasticity index $I_p =$ 28

CI/CH

Plastic Limit Determination

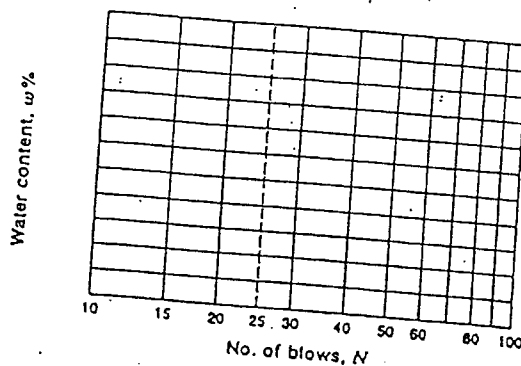
Can no.	3	4		
Wt. of wet soil + can	9.33	10.25		
Wt. of dry soil + can	8.02	8.76		
Wt. of can	1.25	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	19.4	19.8		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 40 SX04091
 Location of Project _____ Boring No. 2-6A Sample No. _____
 Description of Soil Shale
 Depth of Sample _____ Tested By _____ Date June 30/03
 Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	30.32	30.10				
Wt. of dry soil + can	21.50	21.12				
Wt. of can	1.25	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	43.0	45.8				
No. of blows, N	29	26				
	44.3	45.4				



Flow index $F_i =$ _____
 Liquid limit = 44.9
 Plastic limit = 22.7
 Plasticity index $I_p =$ 22.2

CI

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	6.76	6.90	6.36	6.90
Wt. of dry soil + can	5.85	5.42	5.42	5.85
Wt. of can	1.24	1.25	1.24	1.25
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	18.0	35.5	22.5	22.8

Retest

ATTERBERG LIMITS DETERMINATION

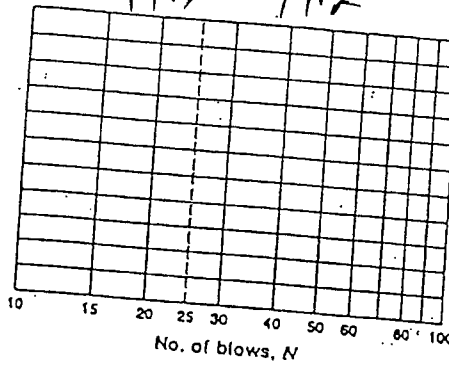
Data Sheet 3

Project _____ Job No. EX04091
 Location of Project _____ Boring No. 2-8A Sample No. _____
 Description of Soil Upper Shear Pile
 Depth of Sample _____ Tested By _____ Date July 3/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	28.38	31.24				
Wt. of dry soil + can	19.52	21.44				
Wt. of can	1.24	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	48.5	48.5				
No. of blows, N	21	20				
	47.5	47.2				

Water content, w%



Flow index $F_i =$ _____

Liquid limit = 47.4

Plastic limit = 19.4

Plasticity index $I_p =$ 28 *OK*

Plastic Limit Determination

Can no.	3	4			
Wt. of wet soil + can	7.85	8.62			
Wt. of dry soil + can	6.78	7.42			
Wt. of can	1.25	1.24			
Wt. of dry soil					
Wt. of moisture					
Water content, w% = w_p	19.3	19.4			

ATTERBERG LIMITS DETERMINATION

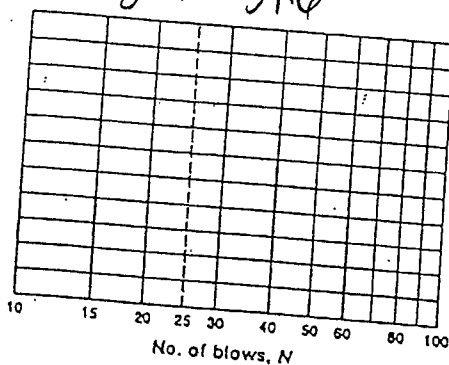
Data Sheet 3

Project _____ Job No. 4091
 Location of Project _____ Boring No. 2-8AA Sample No. _____
 Description of Soil Intact Clay II
 Depth of Sample _____ Tested By _____ Date _____

Liquid Limit Determination

Can no.	1	2			
Wt. of wet soil + can	27.27	28.55			
Wt. of dry soil + can	18.46	19.32			
Wt. of can	1.24	1.26			
Wt. of dry soil					
Wt. of moisture					
Water content, $w\%$	51.2	51.1			
No. of blows, N	22	27			
	50.1	51.6			

Water content, $w\%$



Flow index $F_i =$ _____

Liquid limit = 50.9

Plastic limit = 19.6

Plasticity index $I_p =$ 31.3

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CI/CH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	4.52	5.31		
Wt. of dry soil + can	3.98	4.65		
Wt. of can	1.24	1.26		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	19.7	19.5		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

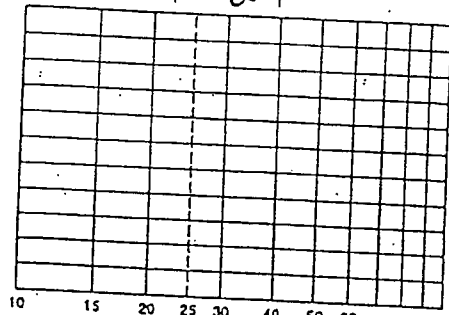
Project _____ Job No. Gx0 4091
 Location of Project _____ Boring No. 2-8B Sample No. _____
 Description of Soil Intact Clay Beneath Shoor (5cm)
 Depth of Sample _____ Tested By _____ Date July 3/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	30.27	30.01				
Wt. of dry soil + can	18.24	18.67				
Wt. of can	1.24	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	70.8	70.8				
No. of blows, N	22	20				

69.7 69

Water content, $w\%$



No. of blows, N

Flow index $F_i =$ 69.4

Liquid limit = 21.2

Plastic limit = 21.2

Plasticity index $I_p =$ 48.2

CH

Plastic Limit Determination

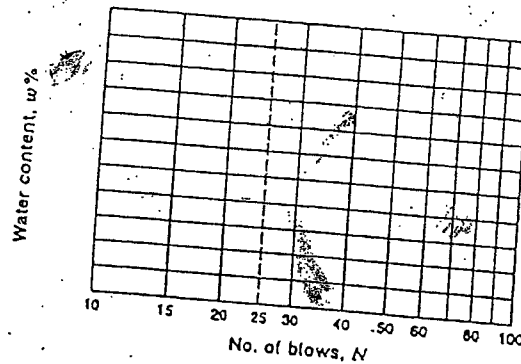
Can no.	3	4		
Wt. of wet soil + can	6.13	4.99		
Wt. of dry soil + can	5.28	4.33		
Wt. of can	1.23	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	21	21.4		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. _____ Sample No. 2-8D
 Description of Soil Lower Slide Plane Date 3 July 2003
 Depth of Sample _____ Tested By _____ Date 9 Dec 03
 Liquid Limit Determination

Can no.	A	R				
Wt. of wet soil + can	25.48	24.23				
Wt. of dry soil + can	17.67	16.82				
Wt. of can	1.25	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	47.6	47.6				
No. of blows, N	24	26				
	47.4	47.8				



Flow index $F_i =$ _____
 Liquid limit = 47.6
 Plastic limit = 21.2
 Plasticity index $I_p =$ 26.4

CI

Plastic Limit Determination

Can no.	C	D		
Wt. of wet soil + can	6.86	6.44		
Wt. of dry soil + can	5.81	5.60		
Wt. of can	1.26	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = $\frac{w}{p} \times 100$	23.1	19.3		

ATTERBERG LIMITS DETERMINATION

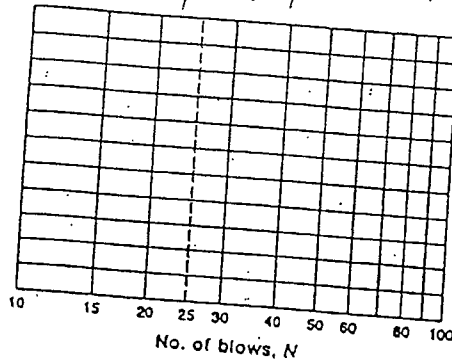
Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 3-1A Sample No. _____
 Description of Soil Shear Plane 1C5
 Depth of Sample _____ Tested By _____ Date _____

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	31.56	38.08				
Wt. of dry soil + can	21.13	25.36				
Wt. of can	1.25	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	52.5	52.8				
No. of blows, N	29	30				
	53.4	54				

Water content, $w\%$



Flow index $F_i =$

Liquid limit = 53.7

Plastic limit = 19.9

Plasticity index $I_p =$ 33.8

C/KH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	5.62	7.54		
Wt. of dry soil + can	4.90	6.49		
Wt. of can	1.25	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	19.7	20.0		

ATTERBERG LIMITS DETERMINATION

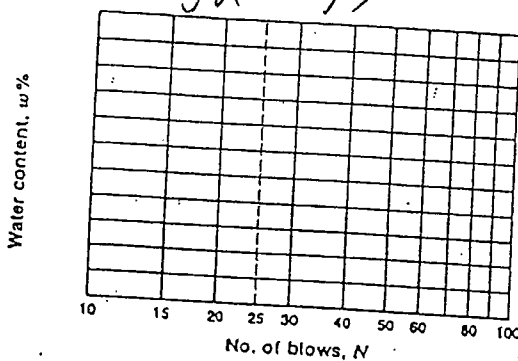
Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 3-5A Sample No. _____
 Description of Soil Adjacent Shear
 Depth of Sample _____ Tested By _____ Date July 8/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	29.03	25.79				
Wt. of dry soil + can	19.26	17.17				
Wt. of can	1.25	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	54.2	54.1				
No. of blows, N	20	21				

52.5 53



Flow index $F_i =$
 Liquid limit = 52.9
 Plastic limit = 18.2
 Plasticity index $I_p = 34.7$

C/KH

Plastic Limit Determination

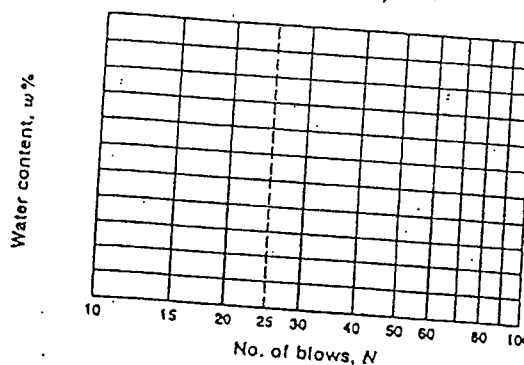
Can no.	3	4		
Wt. of wet soil + can	4.03	3.88		
Wt. of dry soil + can	3.60	3.48		
Wt. of can	1.26	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	18.4	17.9		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. _____ Sample No. 3-5B
 Description of Soil Gauge 8 July 2003
 Depth of Sample _____ Tested By _____ Date 9 Dec 2003
 Liquid Limit Determination

Can no.	A	B				
Wt. of wet soil + can	<u>31.83</u>	<u>33.78</u>				
Wt. of dry soil + can	<u>21.13</u>	<u>22.50</u>				
Wt. of can	<u>1.25</u>	<u>1.24</u>				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	<u>53.8</u>	<u>53.1</u>				
No. of blows, N	<u>21</u>	<u>20</u>				
	<u>52.6</u>	<u>51.7</u>				



Flow index $F_i =$ _____
 Liquid limit $=$ 52.3
 Plastic limit $=$ 22.9
 Plasticity index $I_p =$ 29.4

CI/CH

Plastic Limit Determination

Can no.	C	D		
Wt. of wet soil + can	<u>5.85</u>	<u>5.22</u>		
Wt. of dry soil + can	<u>5.00</u>	<u>4.48</u>		
Wt. of can	<u>1.25</u>	<u>1.26</u>		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	<u>22.7</u>	<u>23.0</u>		

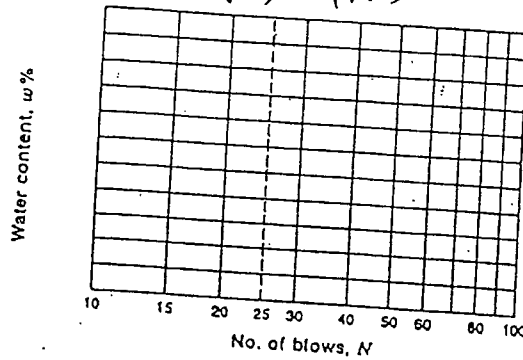
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. Gx04091
 Location of Project _____ Boring No. 3-13A Sample No. _____
 Description of Soil Remolded Shale
 Depth of Sample _____ Tested By _____ Date July 12/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	35.76	34.56				
Wt. of dry soil + can	25.49	24.66				
Wt. of can	1.25	1.26				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	42.4	42.3				
No. of blows, N	21	25				
	41.5	42.3				



Flow index $F_i =$ 41.9
 Liquid limit = 18
 Plastic limit = 18
 Plasticity index $I_p =$ 23.9

C

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	5.11	7.27		
Wt. of dry soil + can	4.52	6.35		
Wt. of can	1.25	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	18	14		

ATTERBERG LIMITS DETERMINATION

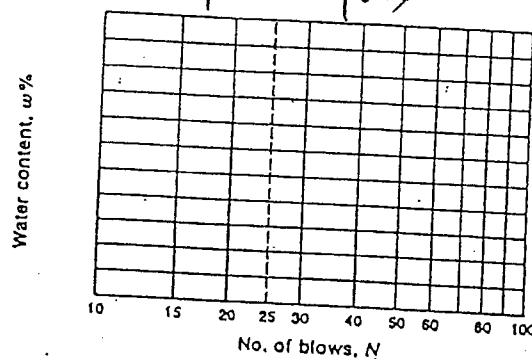
Data Sheet 3

Project _____ Job No. 4091
 Location of Project Shale Shear Boring No. 3-14A Sample No. _____
 Description of Soil _____

Depth of Sample _____ Tested By _____ Date July 13/03

Liquid Limit Determination

Can no.	1	2					
Wt. of wet soil + can	33.69	34.74					
Wt. of dry soil + can	24.47	25.24					
Wt. of can	1.23	1.25					
Wt. of dry soil							
Wt. of moisture							
Water content, w%	39.7	39.6					
No. of blows, N	30	30					
	40.6	40.5					



Flow index $F_i =$ _____
 Liquid limit = 40.6
 Plastic limit = 19.9
 Plasticity index $I_p =$ 20.7

✓

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	6.44	6.96		
Wt. of dry soil + can	5.58	6.01		
Wt. of can	1.23	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	19.8	20.		

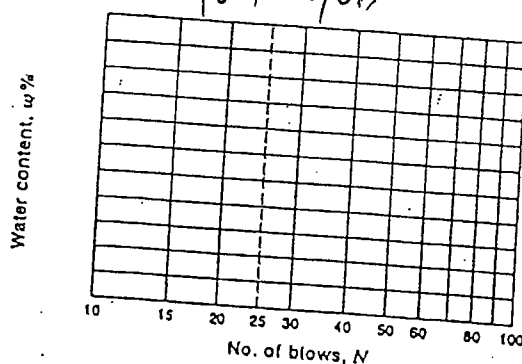
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 5X04091
 Location of Project _____ Boring No. 3-148 Sample No. _____
 Description of Soil Remolded Clay 2 Middle Material
 Depth of Sample _____ Tested By _____ Date July 13/03

Liquid Limit Determination

Can no.	1	2					
Wt. of wet soil + can	32.09	34.67					
Wt. of dry soil + can	23.11	24.93					
Wt. of can	1.24	1.25					
Wt. of dry soil							
Wt. of moisture							
Water content, $w\%$	41.1	41.1					
No. of blows, N	23	22					
	40.1	40.5					



Flow index $F_i =$ 40.6
 Liquid limit = 40.6
 Plastic limit = 19.2
 Plasticity index $I_p =$ 21.4

C1

Plastic Limit Determination

Can no.	3	4				
Wt. of wet soil + can	7.92	11.28				
Wt. of dry soil + can	6.86	9.64				
Wt. of can	1.24	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\% = w_p$	18.9	19.4				

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. Gx04091
 Location of Project _____ Boring No. 3-15A Sample No. _____
 Description of Soil Shale Shale

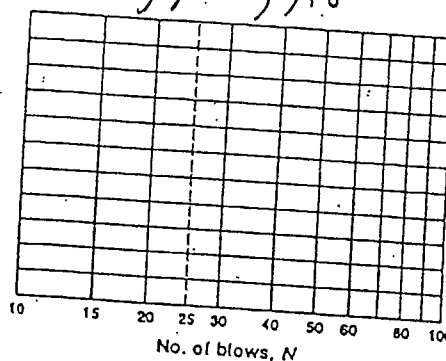
Depth of Sample _____ Tested By _____ Date July 13/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	30.22	28.12				
Wt. of dry soil + can	19.88	18.53				
Wt. of can	1.24	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	55.5	55.5				
No. of blows, N	23	26				

55 55.8

Water content, $w\%$



Flow index $F_i =$ _____

Liquid limit = 55.4

Plastic limit = 15.9

Plasticity index $I_p =$ 39.5

CH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	10.59	10.85		
Wt. of dry soil + can	9.31	9.53		
Wt. of can	1.24	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	15.9	15.9		

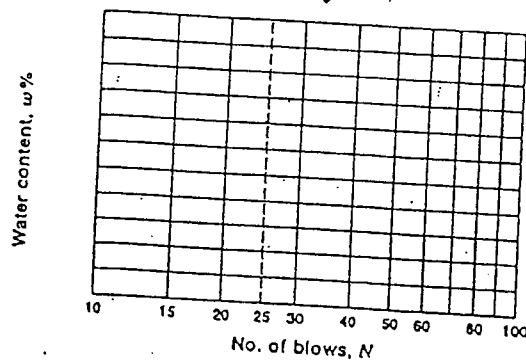
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. Gx04091
 Location of Project _____ Boring No. 3-15B Sample No. _____
 Description of Soil Remolded Shale
 Depth of Sample _____ Tested By _____ Date July 13/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	26.48	26.87				
Wt. of dry soil + can	15.29	15.41				
Wt. of can	1.25	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	79.7	80.9				
No. of blows, N	21	30				
	78.0	82.7				



Flow index $F_i =$ _____
 Liquid limit = 80.4
 Plastic limit = 29.6
 Plasticity index $I_p =$ 50.8

CH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	5.66	4.57		
Wt. of dry soil + can	4.99	4.06		
Wt. of can	1.26	1.25		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	14	18.1		

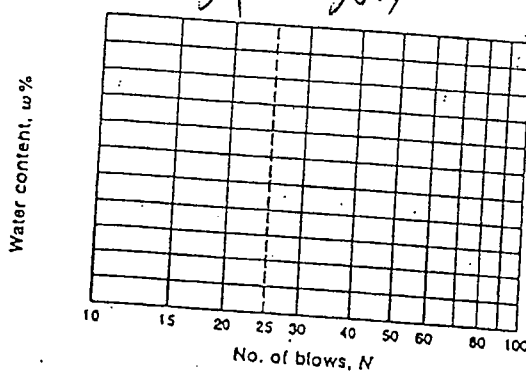
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. GX04091
 Location of Project _____ Boring No. 4-2A Sample No. _____
 Description of Soil 105 Shear

Depth of Sample _____ Tested By _____ Date July 19/03
 Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	33.41	36.96				
Wt. of dry soil + can	24.44	27.00				
Wt. of can	1.24	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	38.7	38.1				
No. of blows, N	27	24				
	39	38.5				



Flow index $F_i =$ _____
 Liquid limit = 38.8
 Plastic limit = 14.6
 Plasticity index $I_p =$ 24.2

Plastic Limit Determination

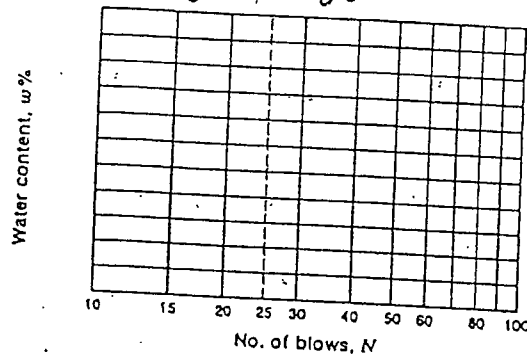
Can no.	3	4		
Wt. of wet soil + can	7.54	6.79		
Wt. of dry soil + can	6.73	5.57		
Wt. of can	1.25	1.23		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	14.8	14.3		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. 6X04091
 Location of Project _____ Boring No. 4-2B Sample No. _____
 Description of Soil Intact Clay
 Depth of Sample _____ Tested By _____ Date July 19/03
 Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	36.06	36.97				
Wt. of dry soil + can	28.28	28.99				
Wt. of can	1.25	1.23				
Wt. of dry soil						
Wt. of moisture						
Water content, w%	28.6	28.1				
No. of blows, N	20	20				
	28.1	28				



Flow index $F_i =$ _____
 Liquid limit = 28.1
 Plastic limit = 19.9
 Plasticity index $I_p =$ 8.2

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	13.27	10.58		
Wt. of dry soil + can	11.26	9.05		
Wt. of can	1.24	7.24 1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	20.1	19.6		

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

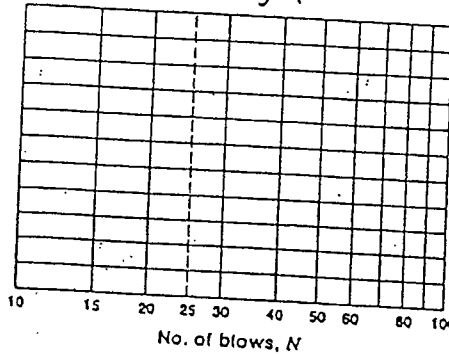
Project _____ Job No. BX04091
 Location of Project _____ Boring No. 4-4A Sample No. _____
 Description of Soil Shear Upper ICS
 Depth of Sample _____ Tested By _____ Date July 22/03

Liquid Limit Determination

Can no.	1	2			
Wt. of wet soil + can	29.06	29.06 32.63			
Wt. of dry soil + can	19.00	21.26			
Wt. of can	1.24	1.24			
Wt. of dry soil					
Wt. of moisture					
Water content, w%	56.6	56.8			
No. of blows, N	27	29			

57.1 57.8

Water content, w%



Flow index $F_i =$

Liquid limit = 57.5

Plastic limit = 22.7

Plasticity index $I_p =$ 34.8

CH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	5.39	7.48		
Wt. of dry soil + can	4.63	6.32		
Wt. of can	1.25	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, w% = w_p	22.5	22.9		

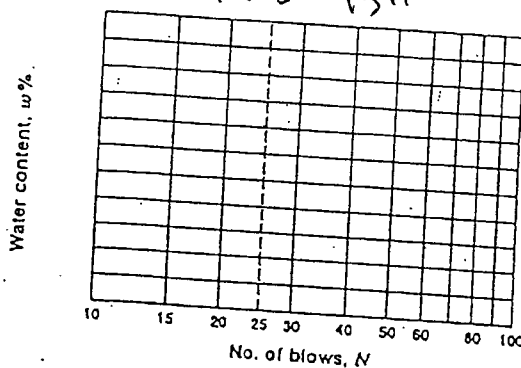
ATTERBERG LIMITS DETERMINATION

Data Sheet 3

Project _____ Job No. Gx04091
 Location of Project _____ Boring No. 4-4B Sample No. _____
 Description of Soil Shear 1CS
 Depth of Sample _____ Tested By _____ Date July 22/03

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	27.57	27.53				
Wt. of dry soil + can	16.26	16.25				
Wt. of can	1.25	1.24				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	75.3	75.1				
No. of blows, N	20	20				
	73.3	73.1				



Flow index $F_i =$ _____
 Liquid limit = 73.2
 Plastic limit = 20.7
 Plasticity index $I_p =$ 52.5

 CH

Plastic Limit Determination

Can no.	3	4			
Wt. of wet soil + can	4.71	5.46			
Wt. of dry soil + can	4.12	4.73			
Wt. of can	1.24	1.23			
Wt. of dry soil					
Wt. of moisture					
Water content, $w\% = w_p$	20.5	20.9			

ATTERBERG LIMITS DETERMINATION

Data Sheet 3

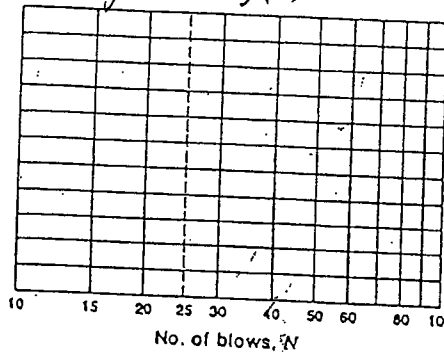
Project _____ Job No. 6X04091
 Location of Project _____ Boring No. P1-1A Sample No. _____
 Description of Soil Shear Plane
 Depth of Sample _____ Tested By _____ Date June 24

Liquid Limit Determination

Can no.	1	2				
Wt. of wet soil + can	29.41	27.41				
Wt. of dry soil + can	19.94	18.62				
Wt. of can	1.25	1.25				
Wt. of dry soil						
Wt. of moisture						
Water content, $w\%$	50.1	50.6				
No. of blows, N	25	29				

50.1 51.5

Water content, $w\%$



Flow index $F_i =$ _____
 Liquid limit = 51.1
 Plastic limit = 18.1
 Plasticity index $I_p =$ 33

C/KH

Plastic Limit Determination

Can no.	3	4		
Wt. of wet soil + can	5.88	5.29		
Wt. of dry soil + can	5.17	4.67		
Wt. of can	1.24	1.24		
Wt. of dry soil				
Wt. of moisture				
Water content, $w\% = w_p$	18.1	18.1		

APPENDIX II

Air Photograph List

Appendix II - Table 1
Air Photograph Sets Used during Initial Investigation

Reference Area	Flight Line No.	Photograph No.
Peace River	BC 86047	60 - 64, 75 - 82, 87 - 97, 102 - 108, 122 - 133, 139, 140, 155 - 162
	BC 86047	90 - 92, 132 - 136, 150 - 154
	BC 86075	15 - 18, 61, 62, 81 - 83
	BC 86074	163 - 166, 209 - 211, 232 - 234, 253, 254
Southern Tributaries	BC 86047	60 - 82, 88 - 103, 125 - 131
	BC 86047	87, 88, 136 - 140, 145 - 148
	BC 86075	8 - 15, 61 - 69, 75 - 79
	BC 86074	254 - 261, 226 - 231, 213 - 218, 267 - 273
Beatton River - East	BC 86047	105 - 110, 118 - 125, 162 - 168
	BC 86075	26 - 32, 47 - 52, 151 - 155
	BC 86099	171 - 173
	BC 86047	153 - 162
Halfway River to Beatton River	BC 86075	16 - 28, 51 - 61, 87 - 91, 136 - 149
	BC 86074	151 - 165, 181 - 210, 232 - 238, 244 - 250
	BC 86099	173 - 177

Appendix II - Table 2
Air Photograph Sets Used during Activity Determination

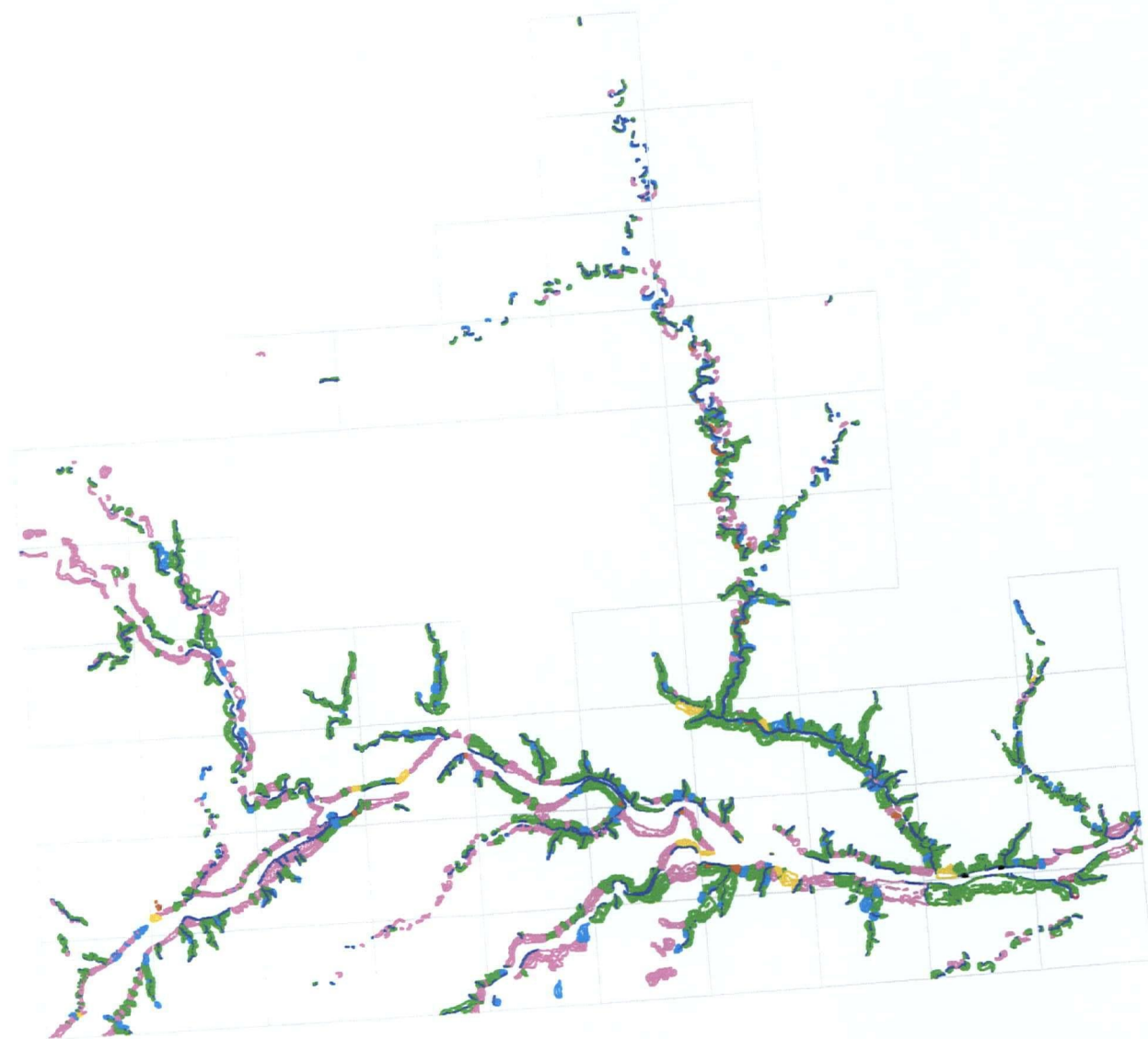
Date	Flight Line No.	Photograph No.
1997	30BCC 97159	1 - 290
	A 7904	71 - 75
	A 8000	30 - 55, 60 - 66, 68 - 72, 74 - 97
	A 8001	1 - 6
	A 8002	100 - 106
	A 8050	1 - 6, 38 - 55, 70 - 84
	A 8101	1 - 10
	A 8103	57 - 66
	A 8104	1 - 34, 61 - 70, 77 - 108
	A 8105	5 - 26, 37 - 44
	A 8108	80 - 101
	A 8109	17 - 30
	A 8109	20 - 30, 44 - 47, 56 - 78, 80 - 92
	A 8141	16 - 35
	A 8145	8 - 16, 28 - 38, 45 - 59, 69 - 79, 88 - 97
	A 8146	9 - 14, 24 - 29, 56 - 73
	A 8148	1 - 10, 18 - 35, 49 - 53, 65 - 77
	A 8149	14 - 30, 43 - 48, 58 - 67, 73 - 75, 77 - 80, 82 - 84, 89 - 92
	A 8150	14 - 26, 40 - 50, 80 - 88
	A 8151	5 - 15, 27 - 55, 61 - 71
	A 8153	17 - 21, 25 - 33, 37 - 39, 81 - 86
	A 8155	13 - 25, 40 - 57, 60 - 80
	A 8156	26 - 29, 34 - 44
	A 8288	1 - 14, 17 - 29, 43 - 47, 53 - 64, 88 - 96
	A 8291	22 - 25, 29 - 32, 38 - 41, 52 - 54, 58 - 60, 85 - 95
	A 8292	45 - 56
	A 8294	5 - 15, 25 - 40, 57 - 70, 74 - 78
	A 8296	3 - 10
	A 8302	3 - 23, 80 - 89
	A 8305	3 - 20, 65 - 76
	A 8349	38 - 47
	A 8350	46 - 60
	A 8353	70 - 75
1945	A 8402	1 - 10, 20 - 43, 53 - 59
	A 8403	1 - 26, 40 - 54
	A 8404	15 - 23, 47 - 52, 57 - 72
	A 8407	10 - 15, 74 - 80
	A 8410	27 - 33
	A 8411	59 - 64
	A 8440	3 - 18
	A 8450	42 - 45
	A 8504	74 - 77, 80 - 85
	A 8507	5 - 12
	A 8514	1 - 10
	A 8528	50 - 60, 65 - 74
	A 8531	27 - 36, 46 - 56
	A 8532	32 - 45, 76 - 80
	A 8537	62 - 68
	A 8540	55 - 67
	A 8541	76 - 81
	A 8542	66 - 72
	A 8543	69 - 74
	A 8544	15 - 24
	A 8577	1 - 5
	A 8580	10 - 15
	A 8587	76 - 83
	A 8597	17 - 23
	A 8598	67 - 70
	A 8600	39 - 45
	A 9111	11 - 17
	A 9137	15 - 18
	A 10429	5 - 12
	A 10431	33 - 37, 51 - 55
	A 10434	56 - 67
	A 10457	65 - 69
	A 10461	1 - 10, 24 - 31

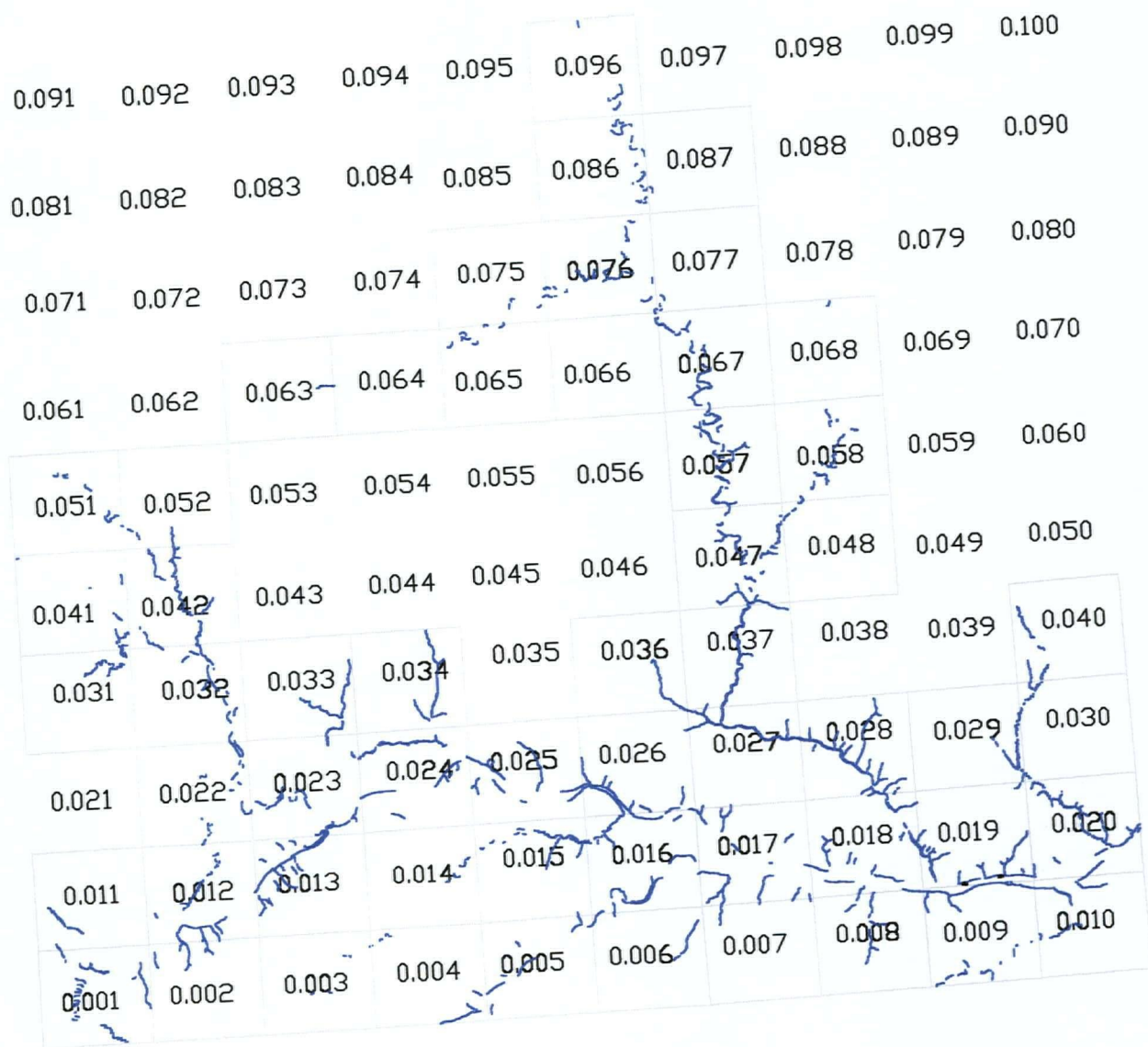
APPENDIX III

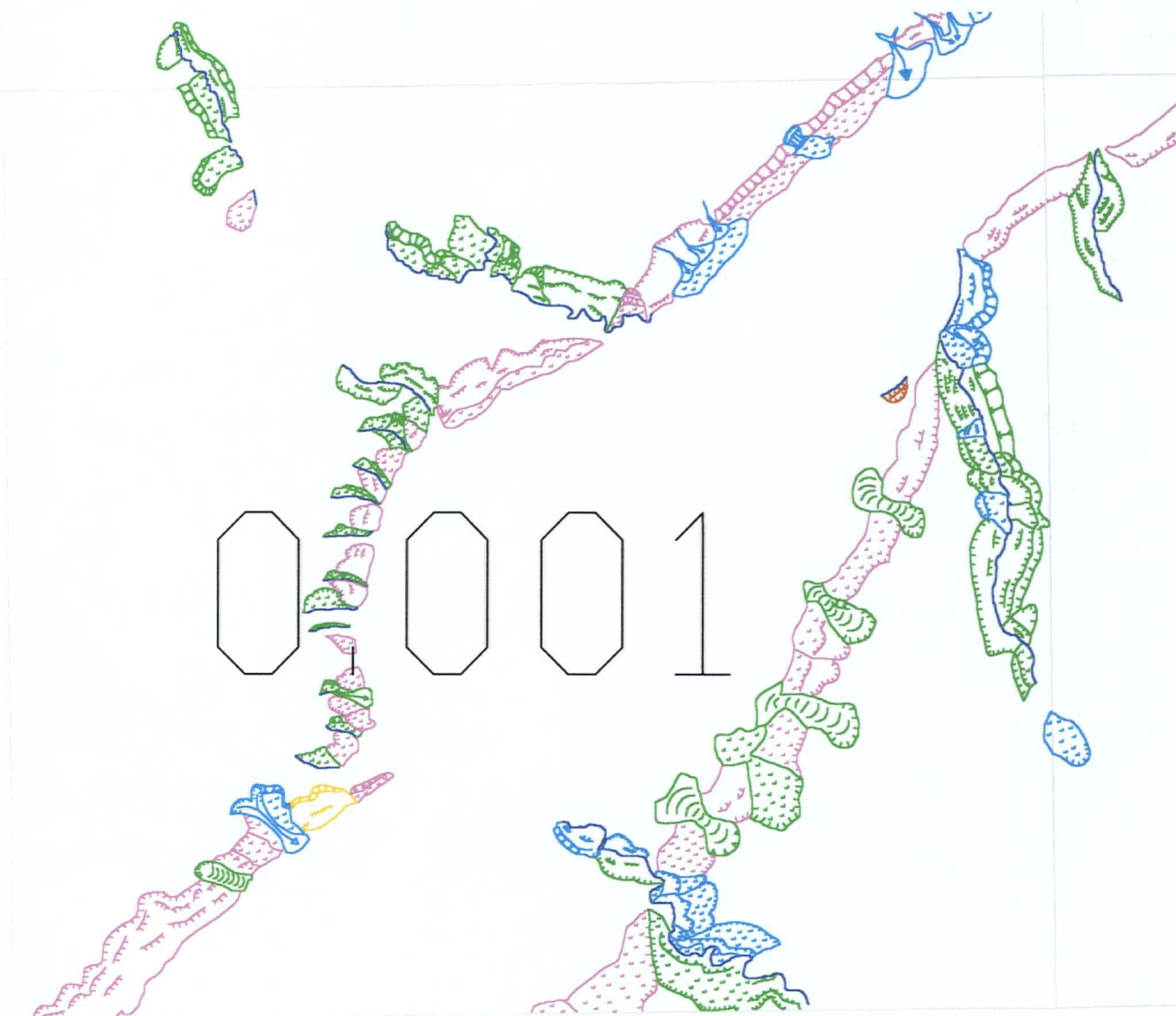
Landslide Maps


Region

Individual TRIM Sheets





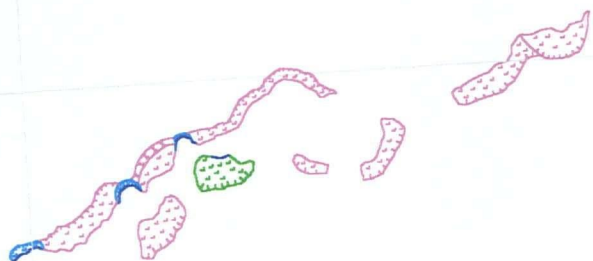




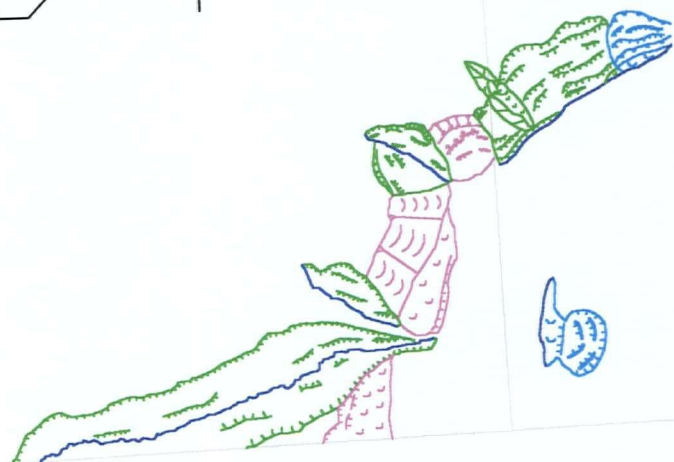
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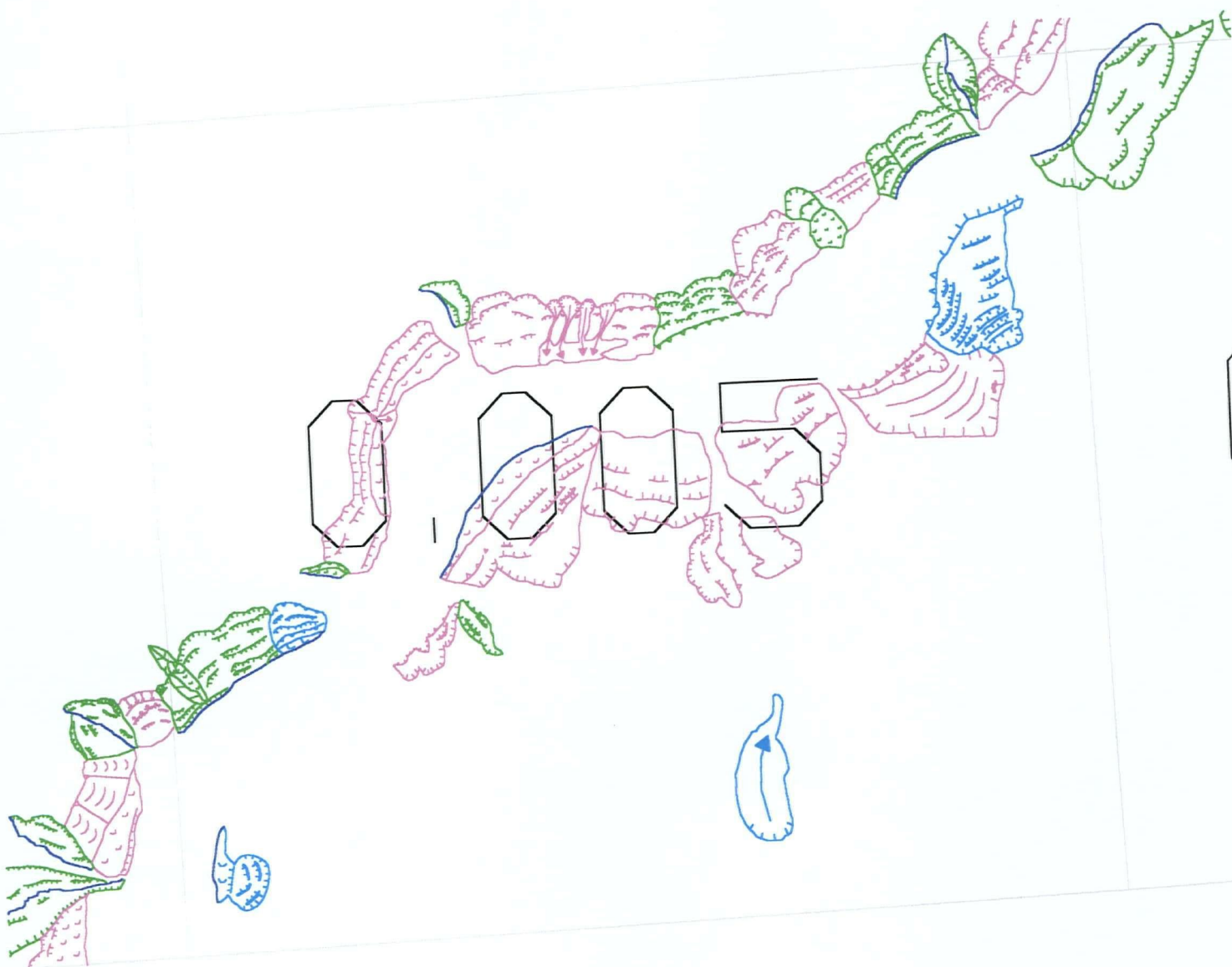
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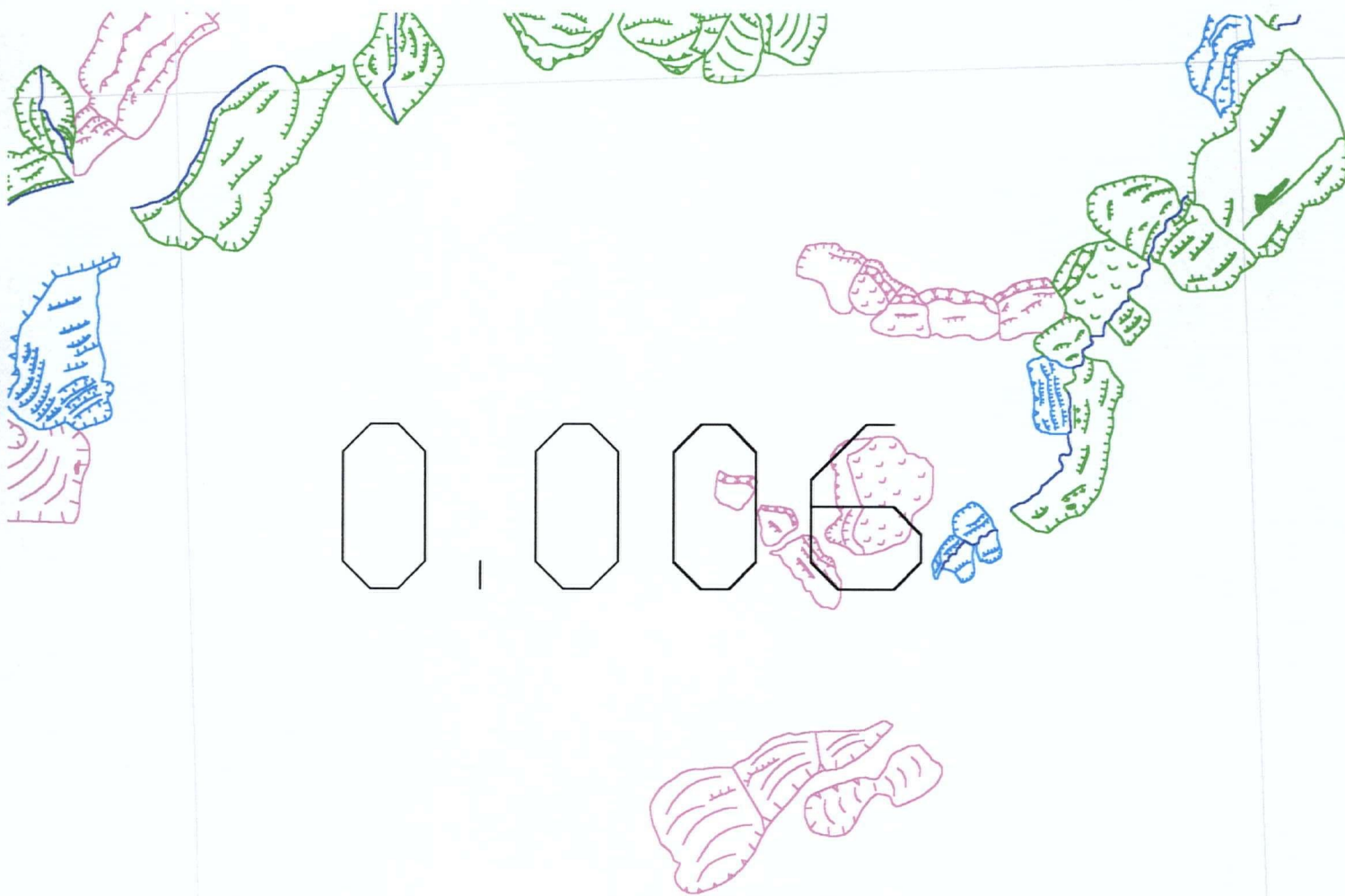


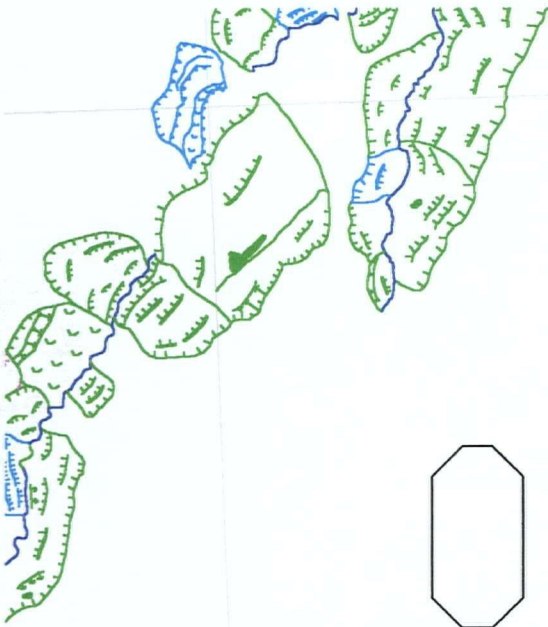


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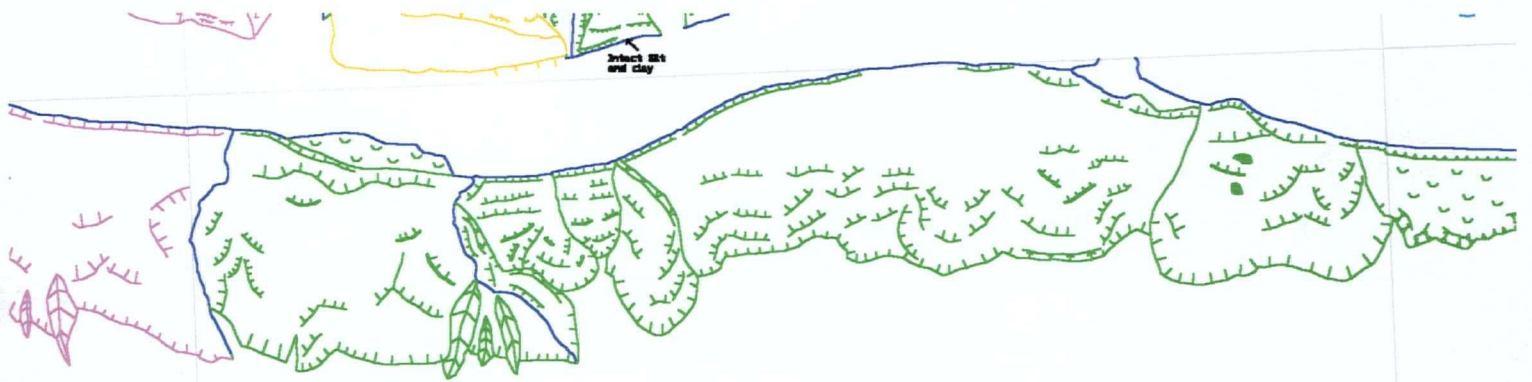




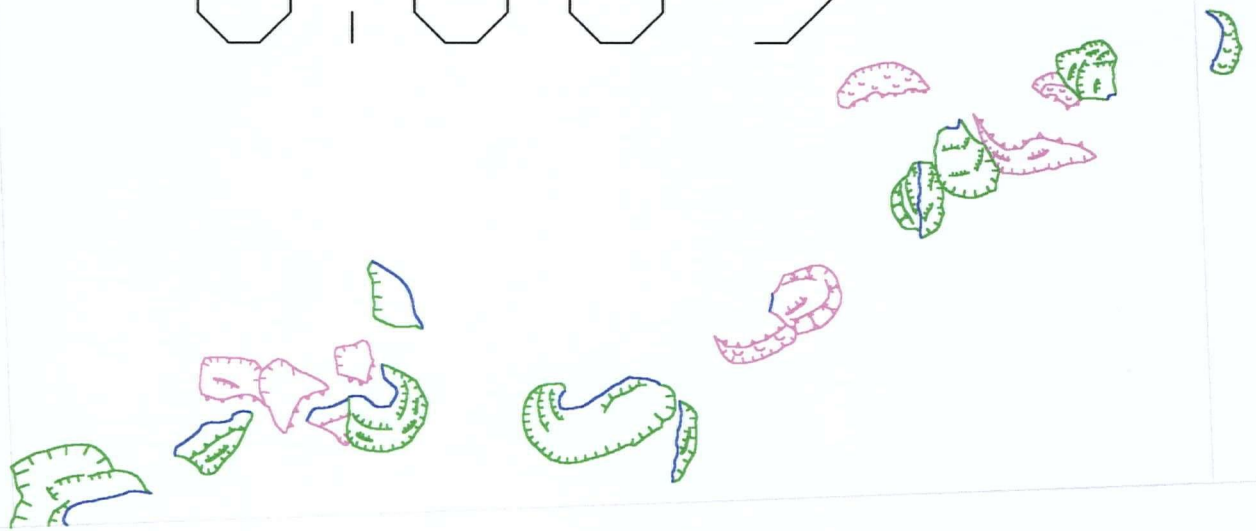


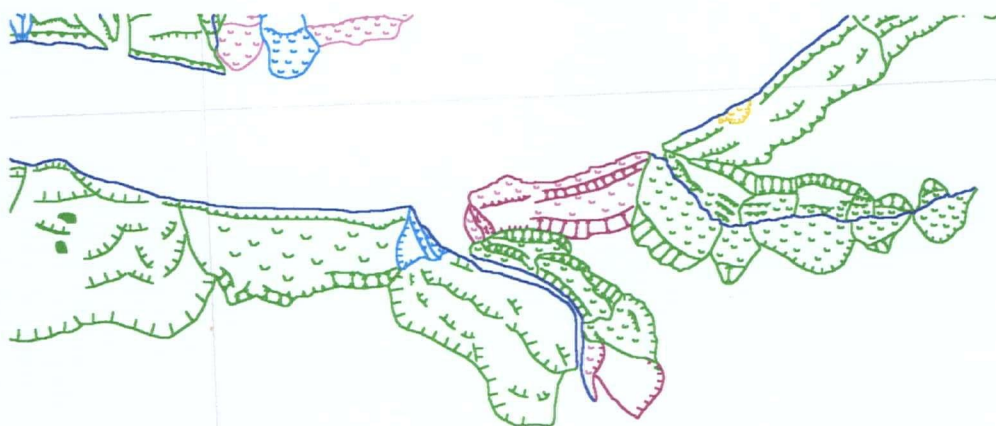
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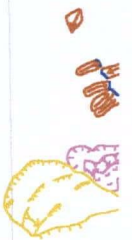
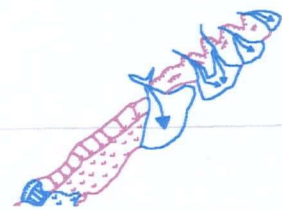
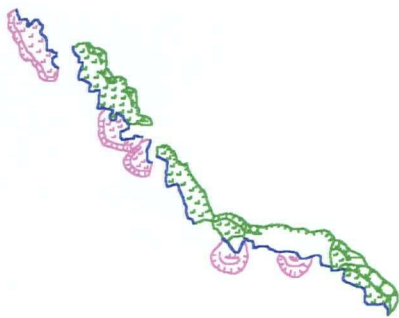




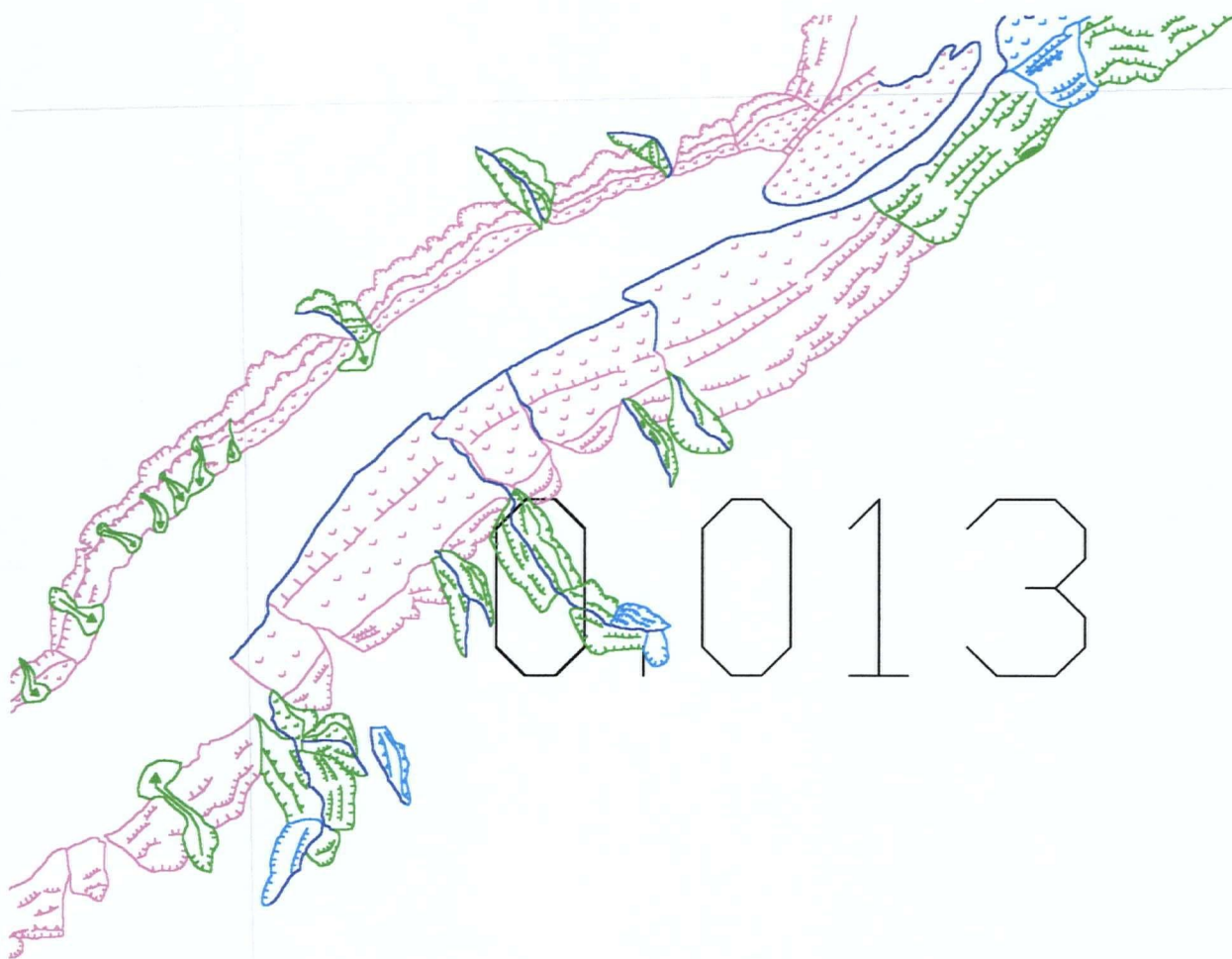
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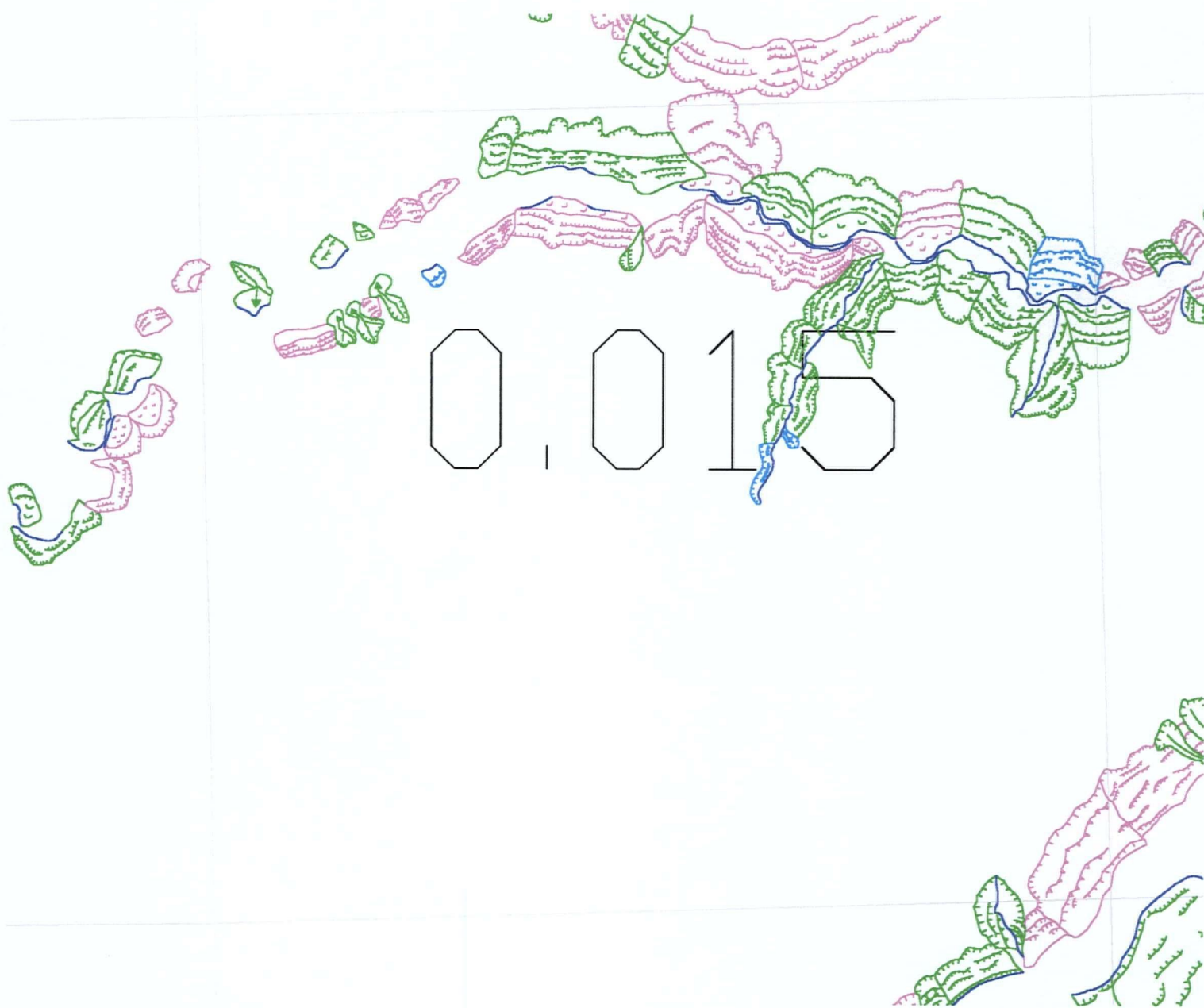


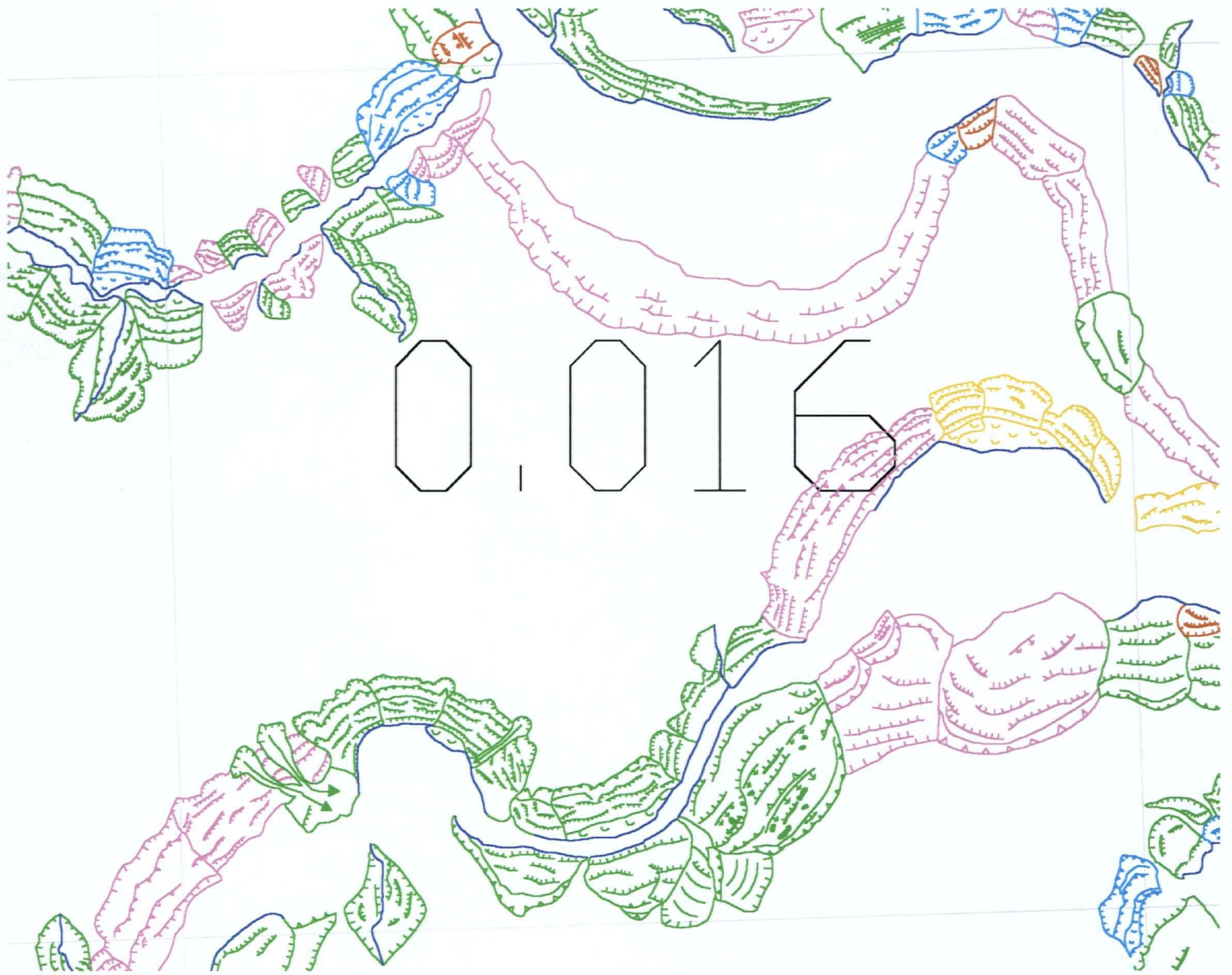




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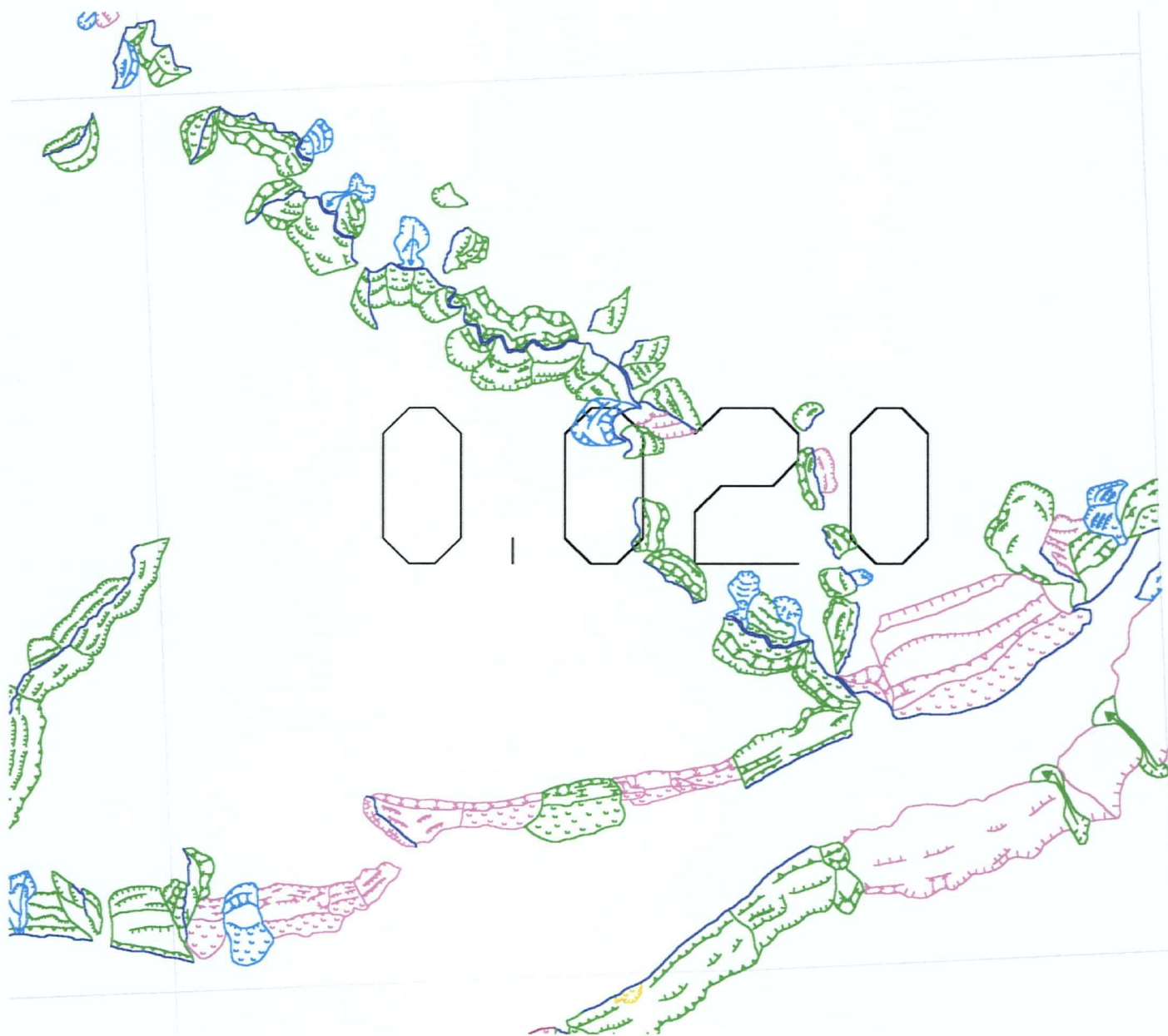
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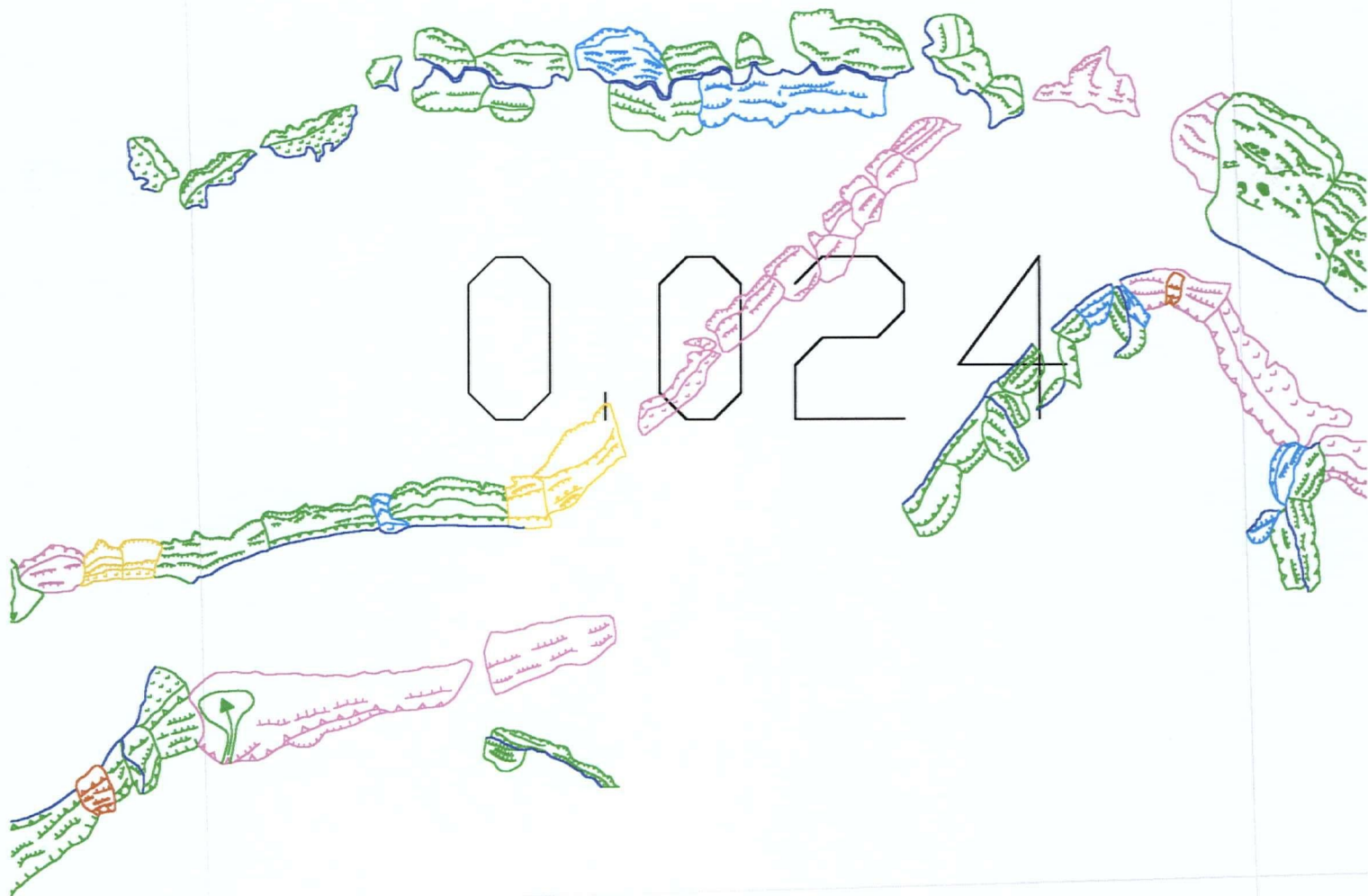




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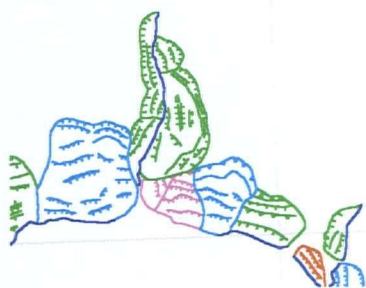
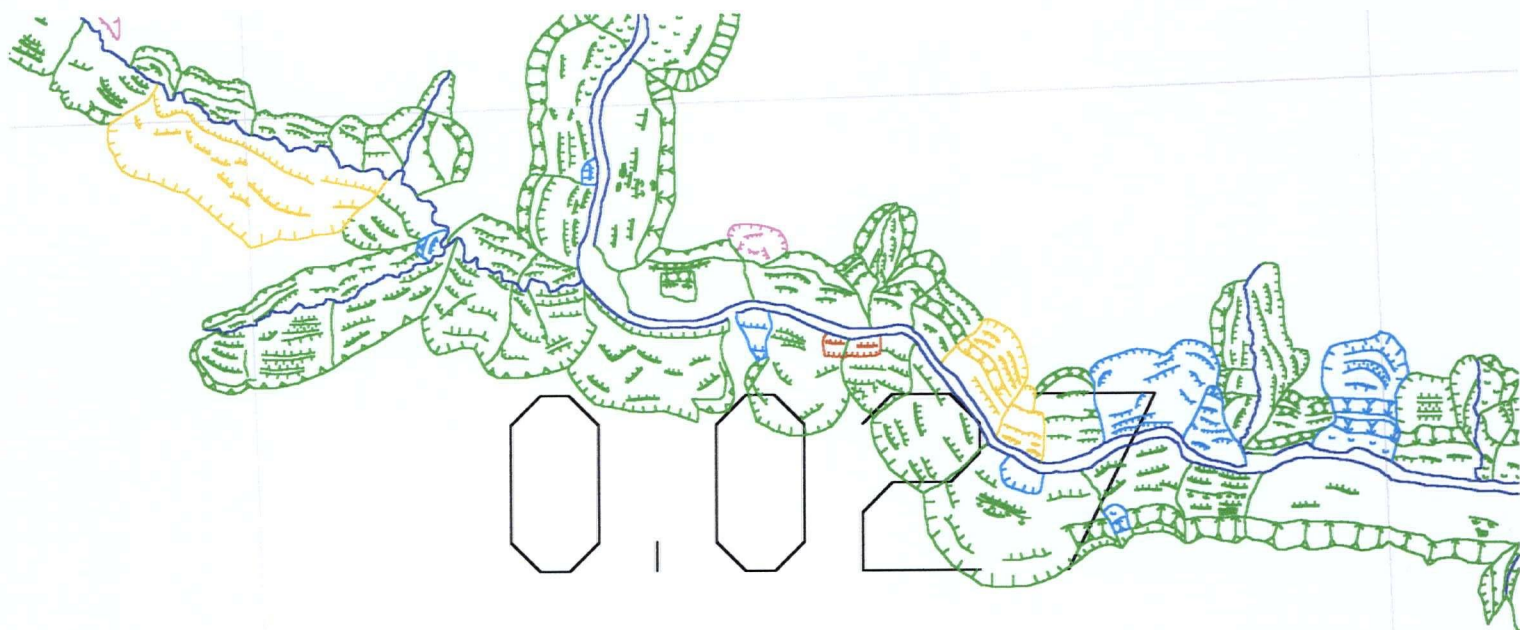


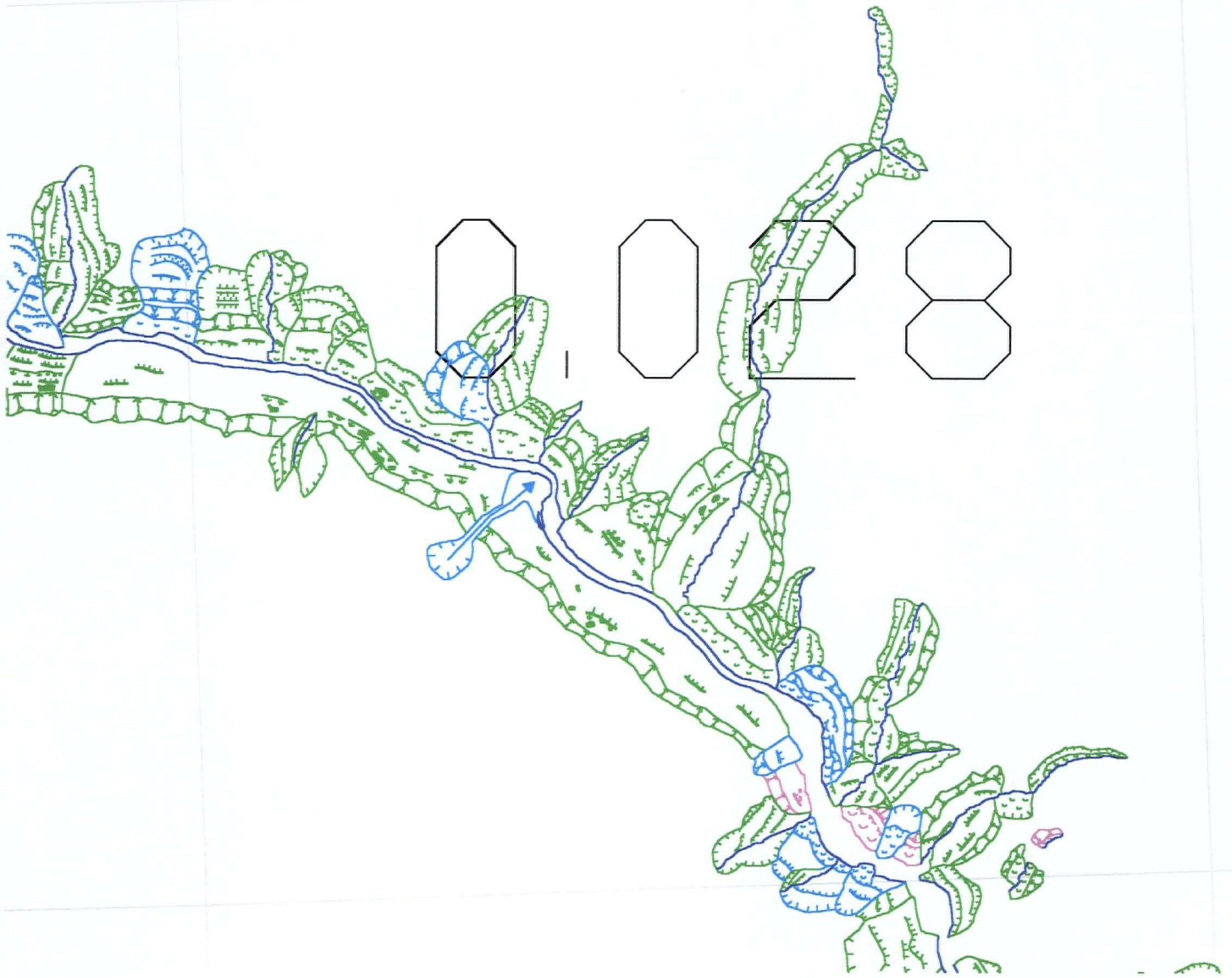




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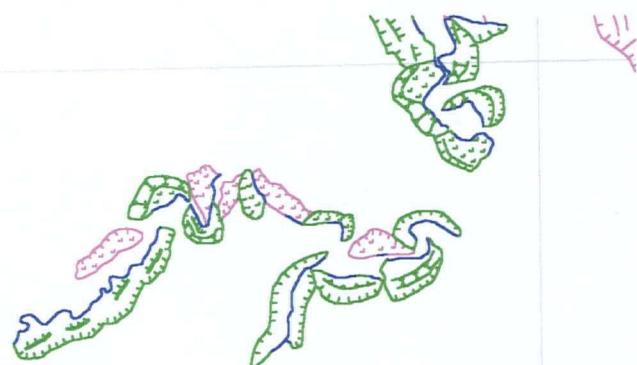




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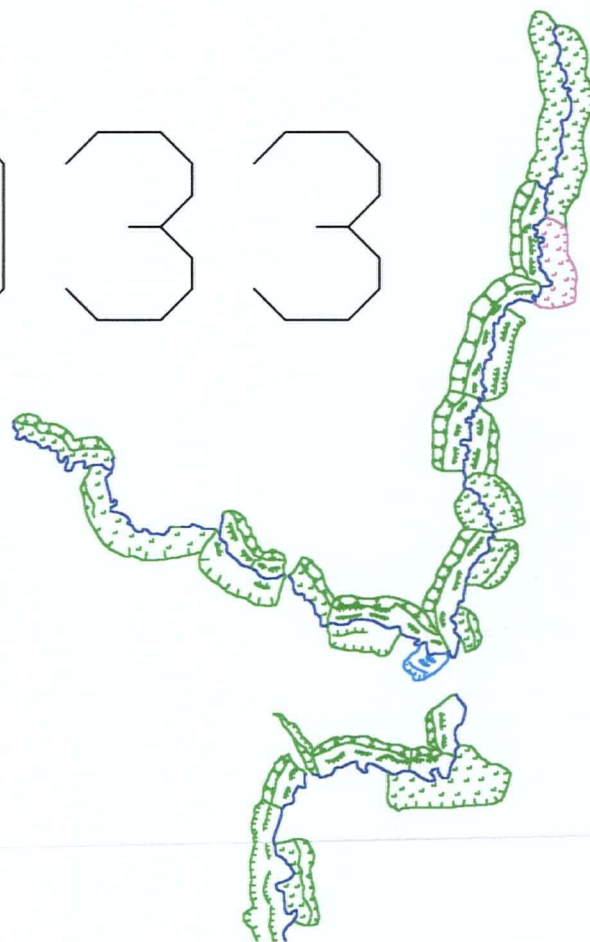
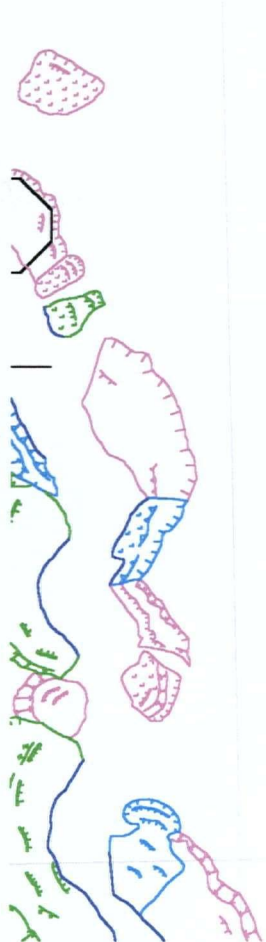
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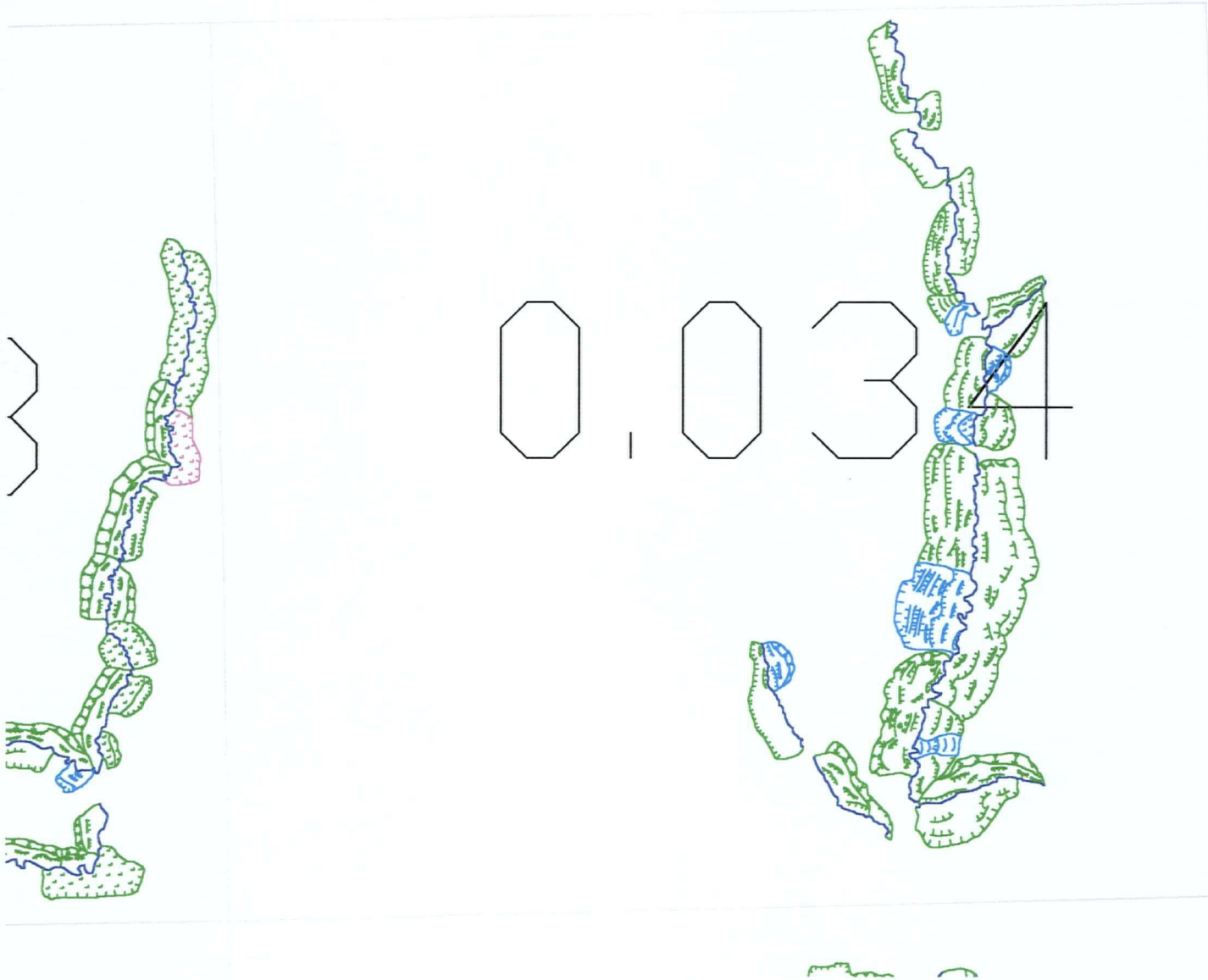


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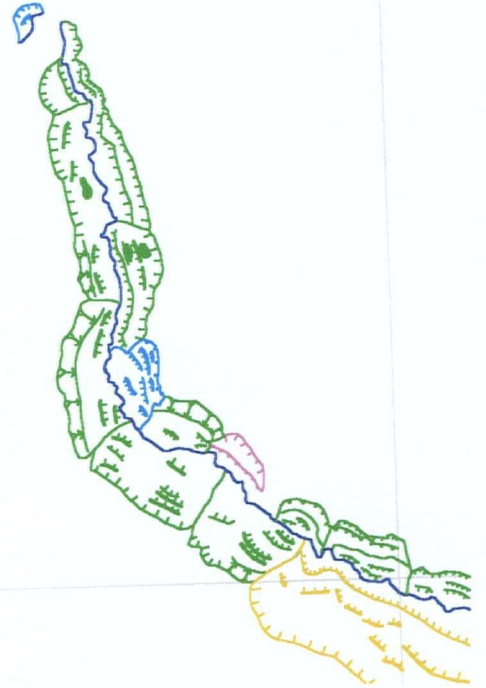


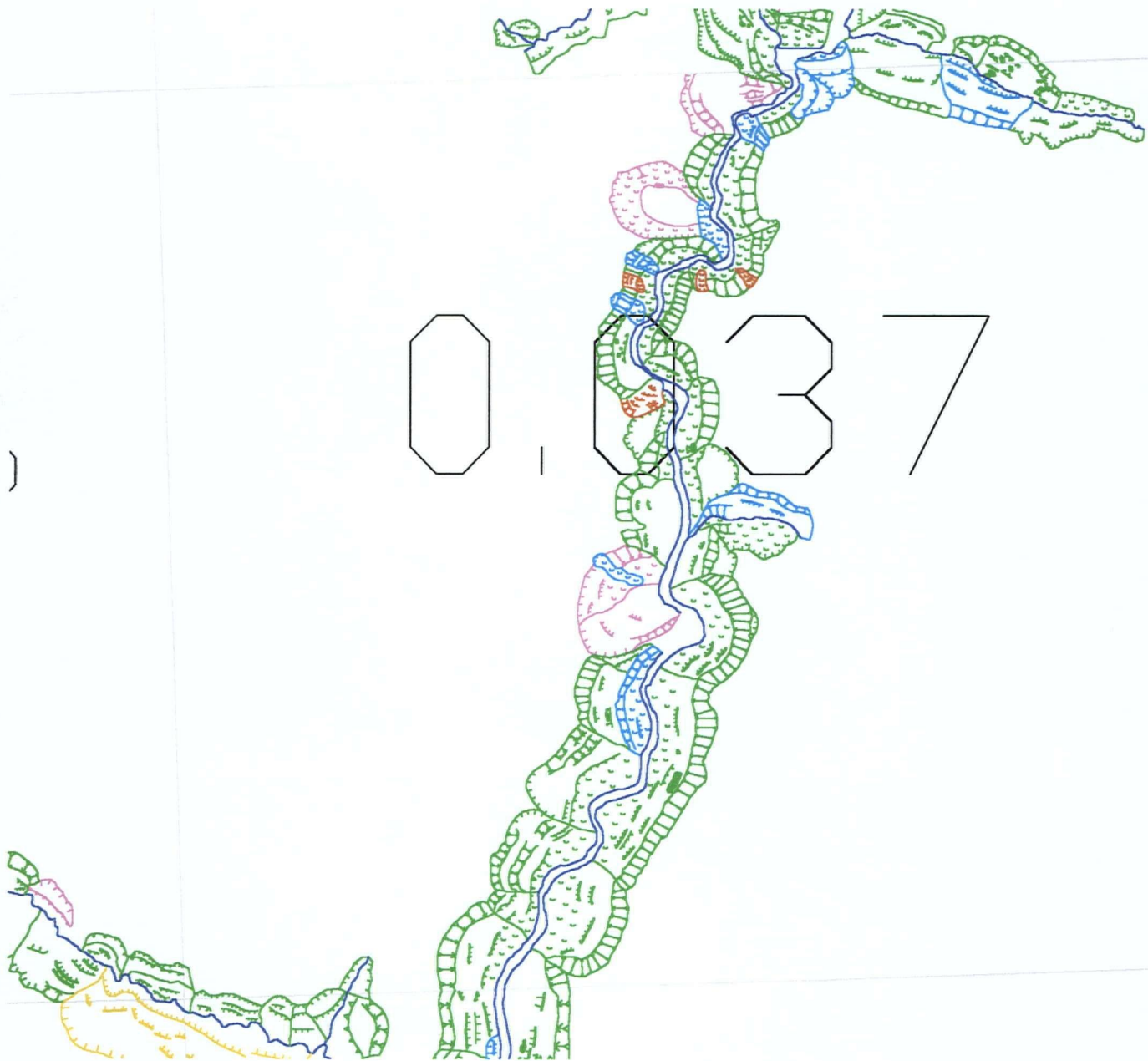
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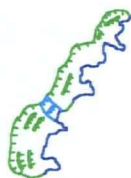
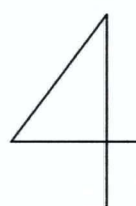




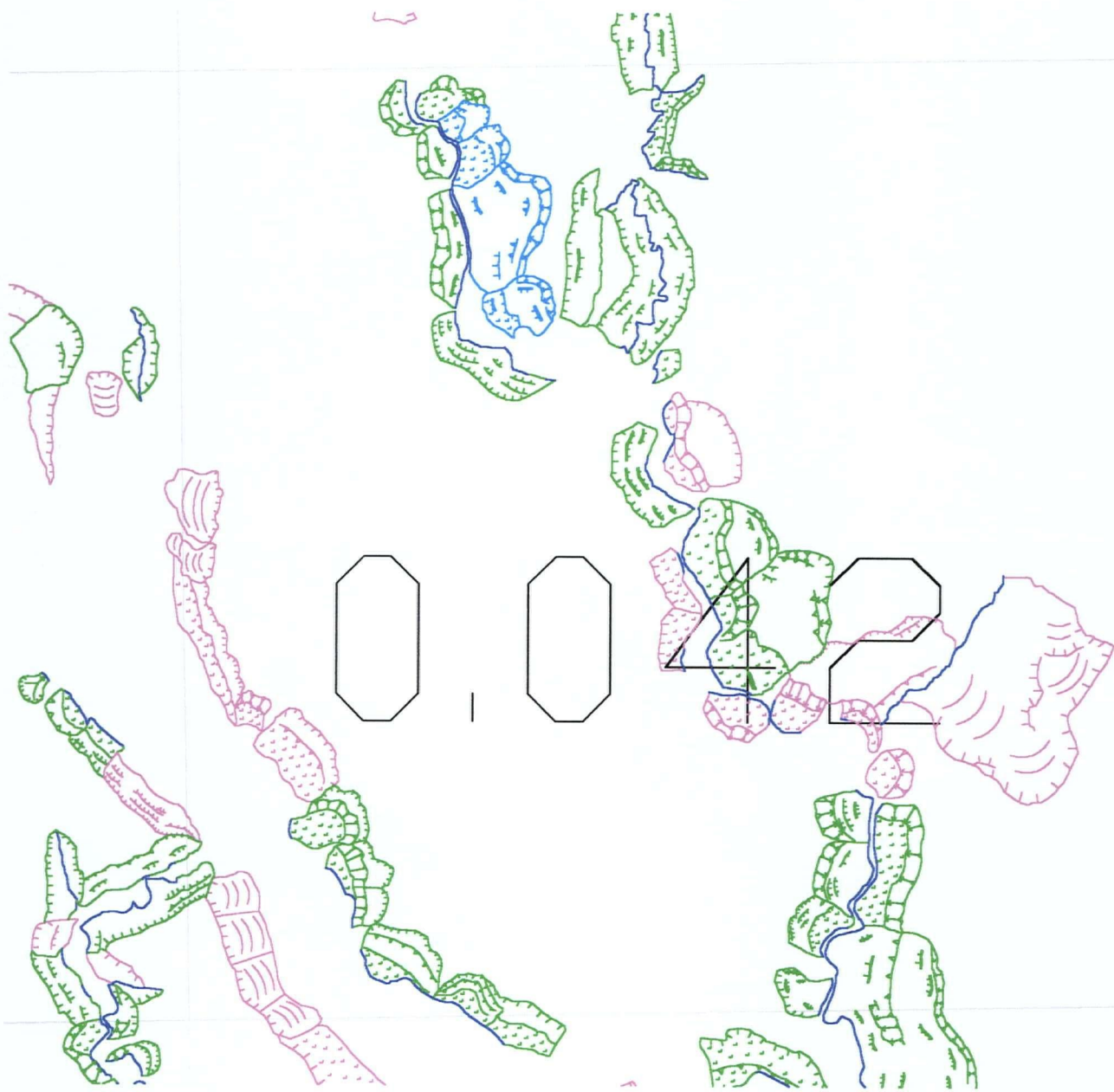
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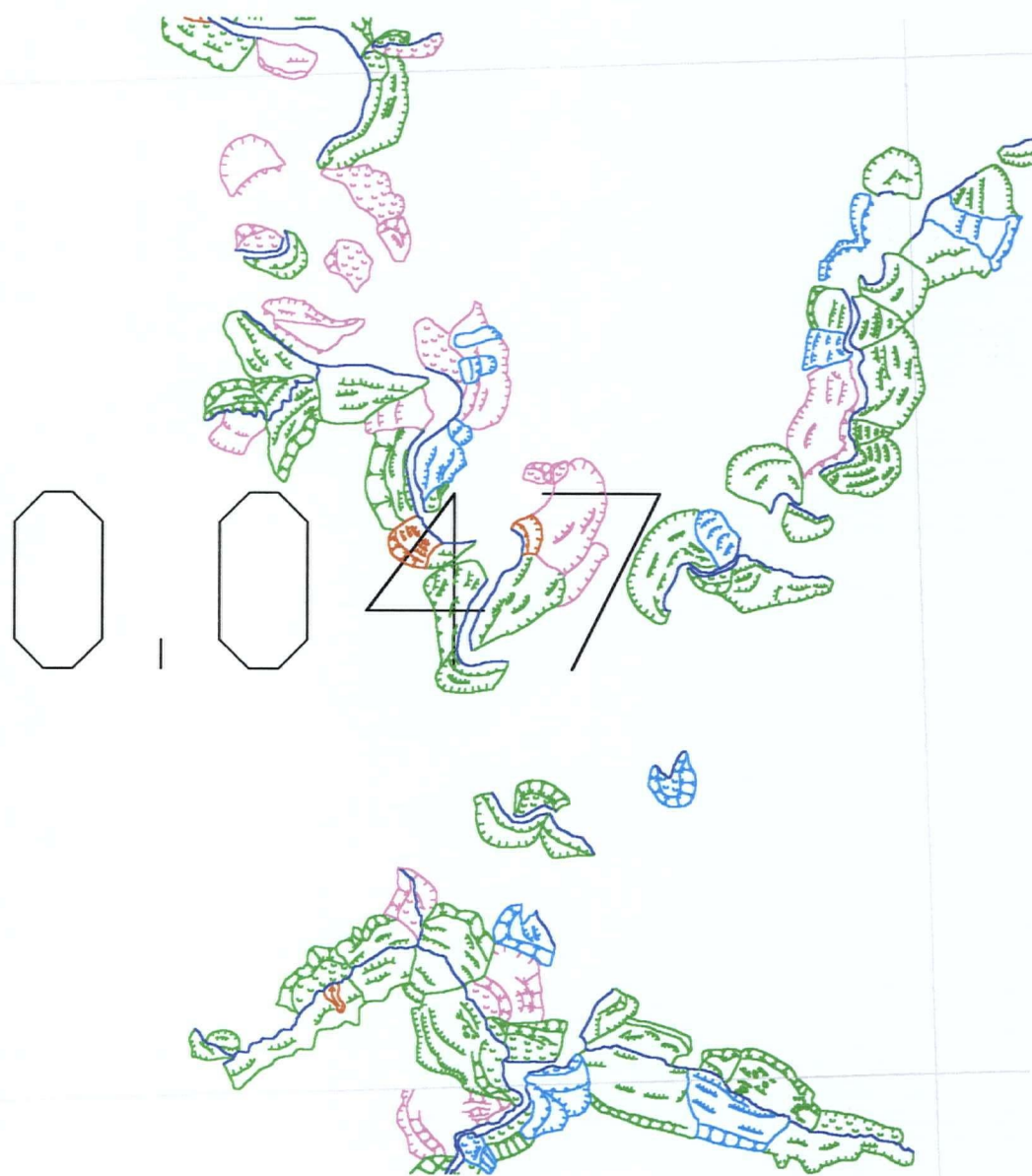


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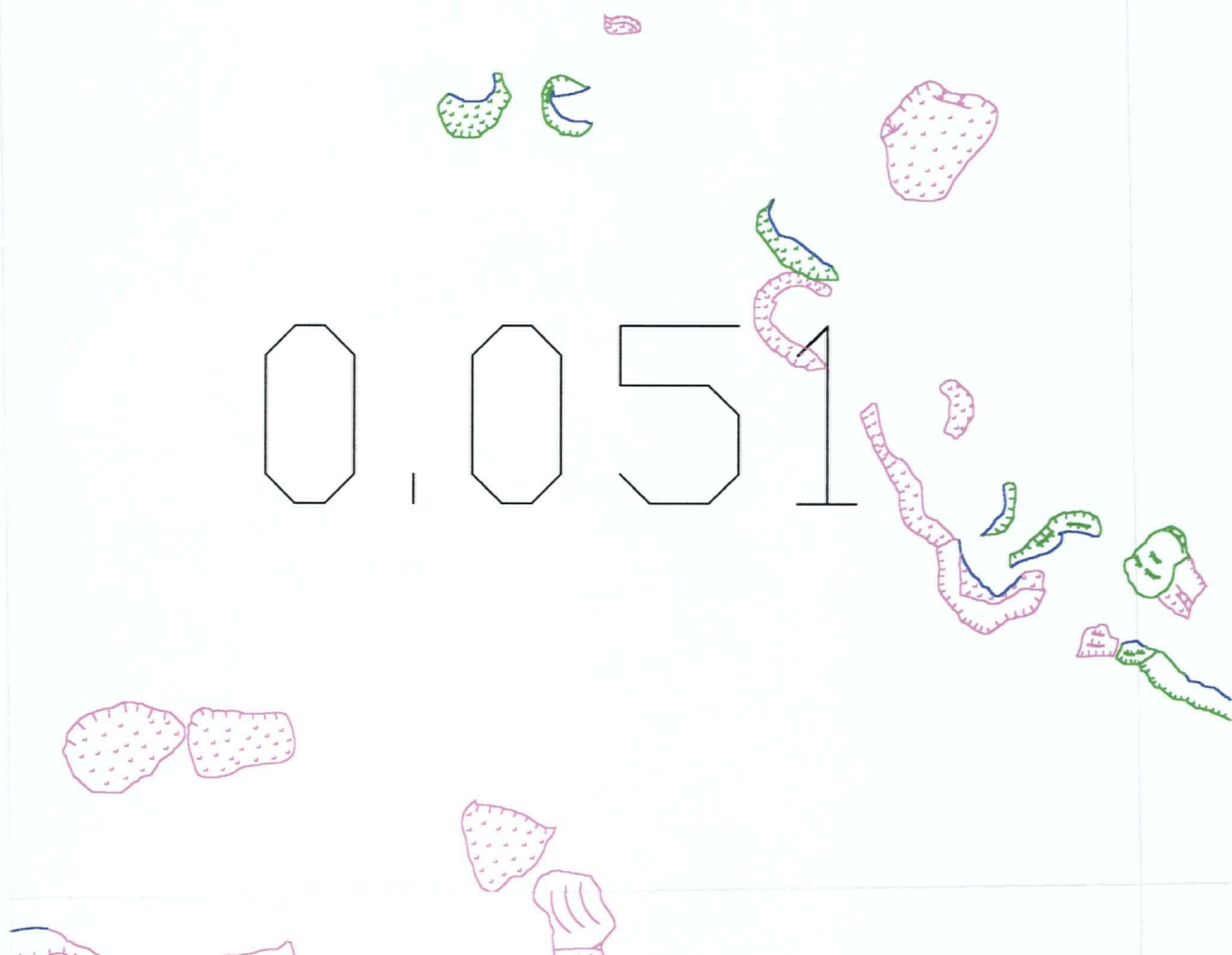




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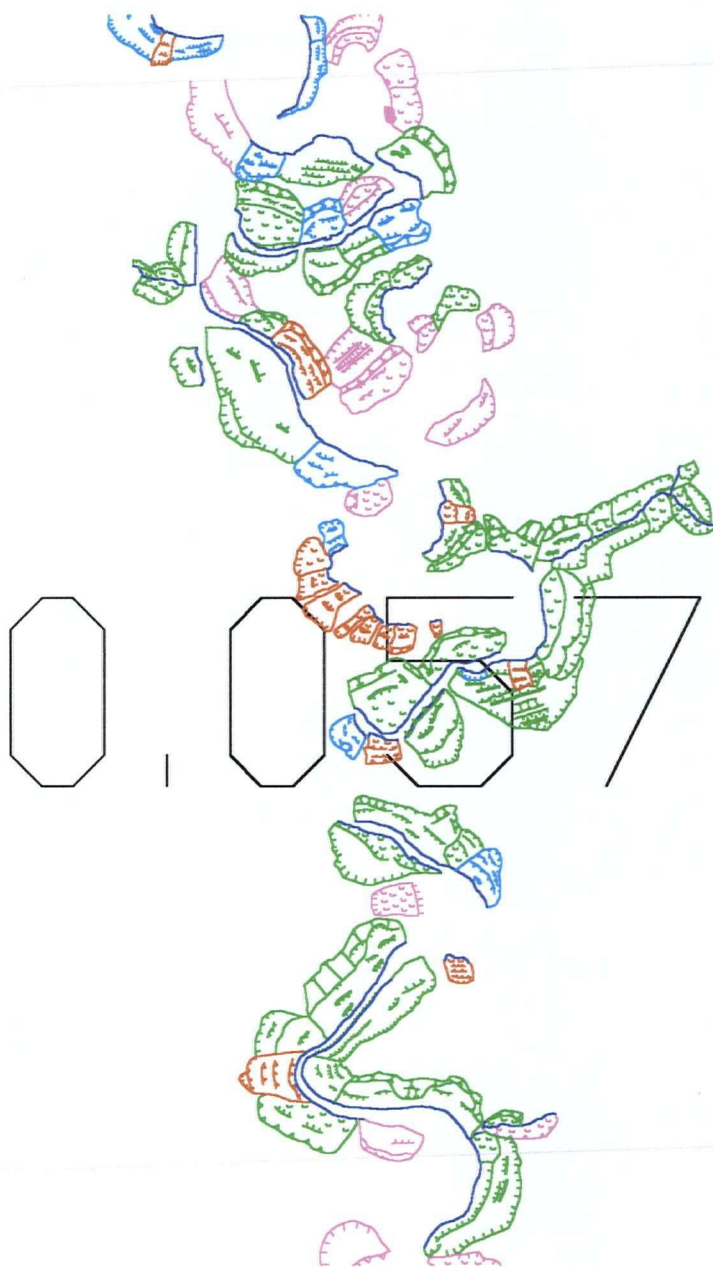


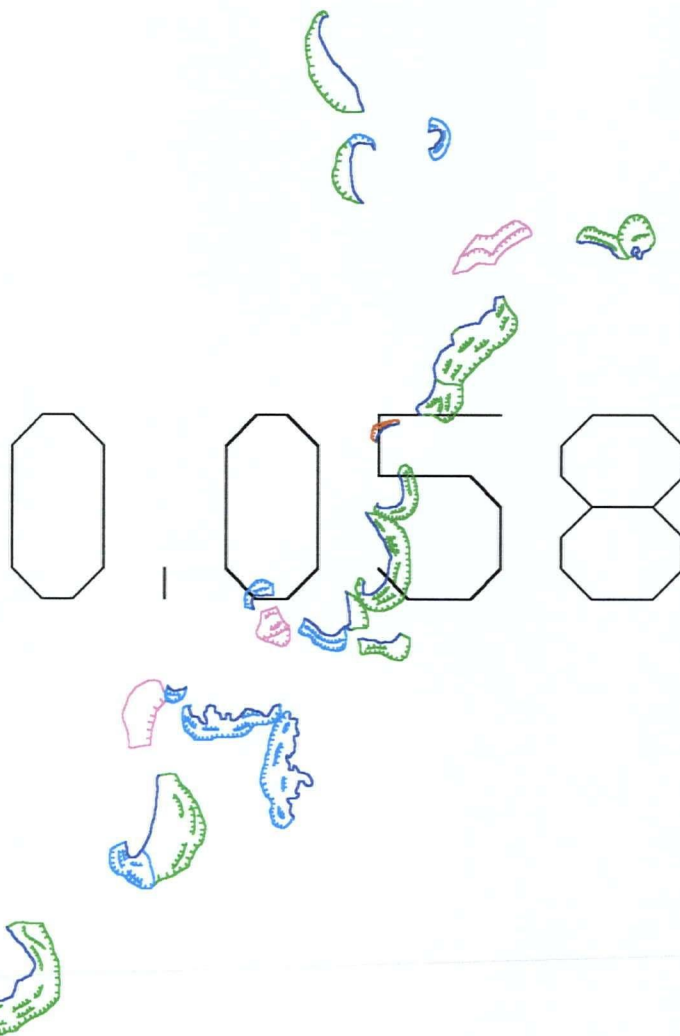
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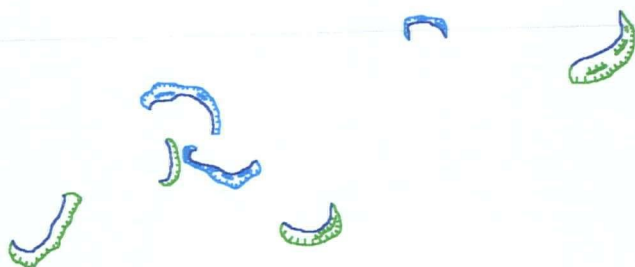
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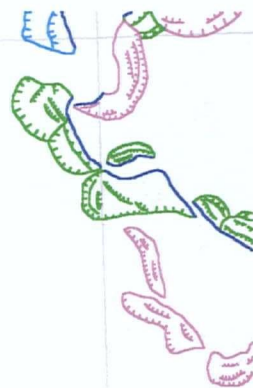


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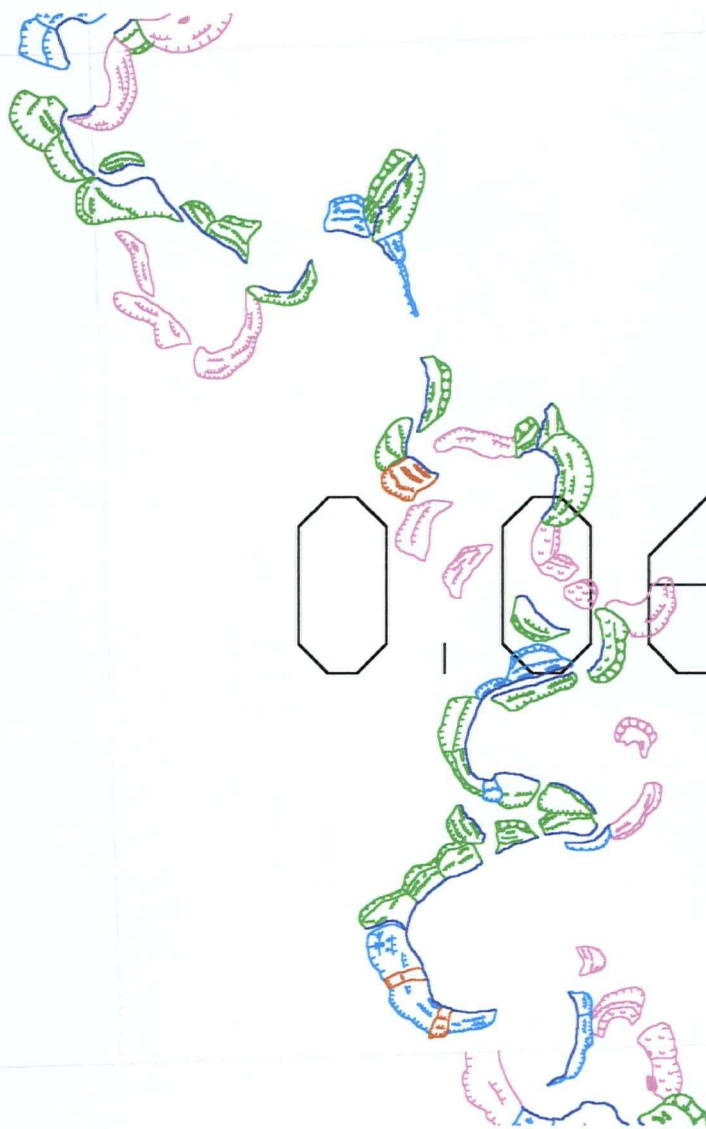




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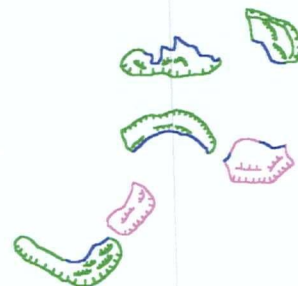


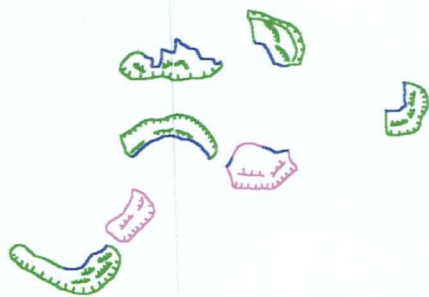
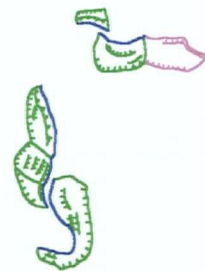
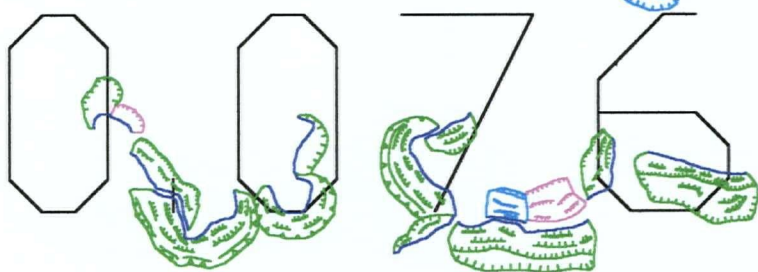
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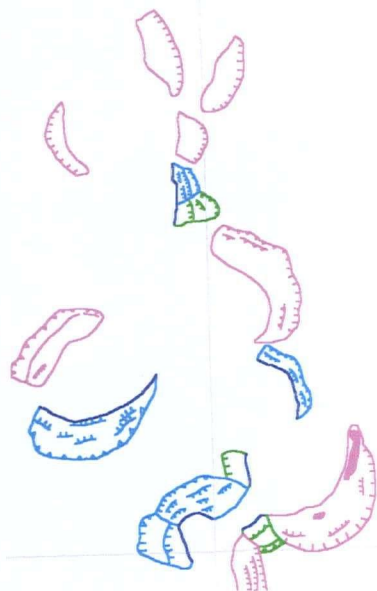
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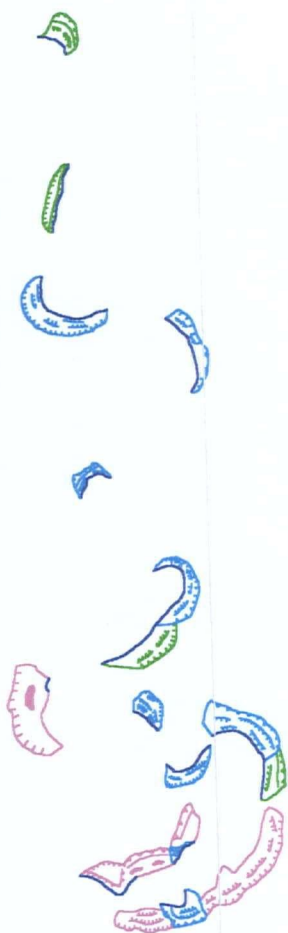
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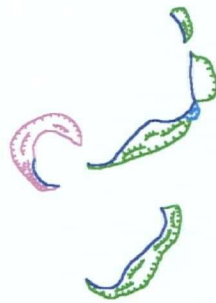


0.087





0.096



APPENDIX IV

Landslide Inventory

Peace River Division

Beatton River Division

Southern Tributaries Division

Beatton River East Division

Halfway River West Division

**Halfway River to Beatton River
Division**

Appendix IV - Landslide Inventory
 Table IV - 1 Peace River Division

Failure Name	River	Photo	Row Segment	Activity	Failure Type	Strike Aspect	Probable Base Failure Material	Length (m)	Width (m)	Area m ²	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Shape Angle	Failure Clarity	Info has mapped as	Additional Comments
PL-001	Peace	BC 86047 no 158	PL-128	3	6	212	Shale	1700	1600	2720000	Intact	No	yes	yes	complex	steep	good		Base Flat side
PL-002	Peace	BC 86047 no 131	PL-43	2	6	208	Quaternary	500	400	200000	Intact	No	yes	yes	complex	steep	good		excellent side
PL-003	Peace	BC 86047 no 131	PL-43	3	6	179	Shale	1100	1000	1100000	Intact	No	yes	yes	complex	steep	good		excellent side
PL-004	Peace	BC 86047 no 160	PL-145	5	6	177	Quaternary	500	500	250000	Intact	yes	yes	yes	single	steep	steep		
PL-005	Peace	BC 86047 no 87	PL-3	2	7	137	Quaternary	7	7	49	Intact	yes	yes	yes	single	steep	steep		
PL-006	Peace	BC 86047 no 87	PL-4	3	6	161	Shale	500	550	275000	Intact	No	yes	yes	single	steep	good		small sec scarp could be in debris
PL-007	Peace	BC 86047 no 87	PL-6	3	6	133	Shale	600	700	420000	Intact	No	yes	yes	single	steep	good		
PL-008	Peace	BC 86047 no 87	PL-6	4	6	190	Shale	700	500	350000	Intact	No	yes	yes	single	steep	steep		
PL-009	Peace	BC 86047 no 87	PL-7	3	6	129	Shale	1000	800	800000	Intact	No	yes	yes	complex	steep	steep		
PL-010	Peace	BC 86047 no 87	PL-7	4	6	129	Shale	1000	800	800000	Intact	No	yes	yes	complex	steep	steep		
PL-011	Peace	BC 86047 no 87	PL-11	3	6	175	Shale	1000	2000	2000000	Intact	No	yes	yes	complex	steep	steep		
PL-012	Peace	BC 86047 no 87	PL-11	3	6	175	Shale	1000	2000	2000000	Intact	No	yes	yes	complex	steep	steep		
PL-013	Peace	BC 86047 no 87	PL-12	3	3	174	Shale	400	1000	400000	Intact	No	yes	yes	complex	steep	steep		
PL-014	Peace	BC 86047 no 87	PL-14	3	2	190	Shale	500	1200	600000	Intact	No	yes	yes	complex	steep	steep		
PL-015	Peace	BC 86047 no 89	PL-14b	4	2	143	Shale	350	21000	7350000	Intact	No	yes	yes	complex	steep	steep		
PL-016	Peace	BC 86047 no 89	PL-17	4	6	166	Shale	400	400	160000	Intact	No	yes	yes	single	steep	steep		old slide
PL-017	Peace	BC 86047 no 89	PL-17	4	6	173	Shale	600	850	510000	Intact	yes	yes	yes	single	steep	steep		
PL-018	Peace	BC 86047 no 89	PL-17	2	6	165	Shale	1000	500	500000	Intact	yes	yes	yes	single	steep	steep		
PL-019	Peace	BC 86047 no 89	PL-17	3	6	194	Shale	750	400	300000	Intact	yes	yes	yes	single	steep	steep		
PL-020	Peace	BC 86047 no 89	PL-18	3	6	267	Shale	300	800	240000	Intact	No	yes	yes	single	steep	steep		
PL-021	Peace	BC 86047 no 89	PL-20	3	6	110	Quaternary	150	850	127500	Intact	No	yes	yes	single	steep	steep		
PL-022	Peace	BC 86047 no 89	PL-20	3	6	182	Quaternary	800	700	560000	Intact	yes	yes	yes	single	steep	steep		
PL-023	Peace	BC 86047 no 89	PL-22	3	6	172	Quaternary	800	400	320000	Intact	No	yes	yes	single	steep	steep		
PL-024	Peace	BC 86047 no 89	PL-24	2	4	172	Quaternary	800	400	320000	Intact	No	yes	yes	single	steep	steep		
PL-025	Peace	BC 86047 no 89	PL-24	3	6	191	Quaternary	700	1000	700000	Intact	No	yes	yes	single	steep	steep		year of highly mobile flows
PL-026	Peace	BC 86047 no 89	PL-24	3	6	272	Quaternary	700	1000	700000	Intact	No	yes	yes	single	steep	steep		
PL-027	Peace	BC 86047 no 89	PL-24	3	6	301	Shale	600	2000	1200000	Intact	No	yes	yes	single	steep	steep		
PL-028	Peace	BC 86047 no 89	PL-24	3	6	141	Shale	300	1400	420000	Intact	No	yes	yes	single	steep	steep		
PL-029	Peace	BC 86047 no 89	PL-24	3	6	109	Shale	600	900	540000	Intact	No	yes	yes	single	steep	steep		
PL-030	Peace	BC 86047 no 89	PL-24	3	6	196	Shale	400	350	140000	Intact	No	yes	yes	single	steep	steep		
PL-031	Peace	BC 86047 no 89	PL-25	3	6	132	Quaternary	600	1100	660000	Intact	No	yes	yes	single	steep	steep		
PL-032	Peace	BC 86047 no 89	PL-25	3	6	165	Quaternary	600	450	270000	Intact	yes	yes	yes	single	steep	steep		
PL-033	Peace	BC 86047 no 89	PL-26	3	6	238	Quaternary	550	300	165000	Intact	No	yes	yes	single	steep	steep		
PL-034	Peace	BC 86047 no 89	PL-27	3	6	238	Quaternary	450	650	292500	Intact	No	yes	yes	single	steep	steep		
PL-035	Peace	BC 86047 no 89	PL-30	3	6	155	Quaternary	500	500	250000	Intact	No	yes	yes	single	steep	steep		Part of PL-31
PL-036	Peace	BC 86047 no 89	PL-34	3	6	62	Quaternary	600	1200	720000	Intact	No	yes	yes	single	steep	steep		
PL-037	Peace	BC 86047 no 89	PL-35	3	6	156	Quaternary	600	1600	960000	Intact	yes	yes	yes	single	steep	steep		
PL-038	Peace	BC 86047 no 89	PL-35	3	6	80	Shale	350	1600	560000	Intact	yes	yes	yes	single	steep	steep		
PL-039	Peace	BC 86047 no 89	PL-35	3	6	165	Quaternary	600	700	420000	Intact	No	yes	yes	single	steep	steep		
PL-040	Peace	BC 86047 no 89	PL-35	3	6	165	Quaternary	600	700	420000	Intact	No	yes	yes	single	steep	steep		
PL-041	Peace	BC 86047 no 89	PL-35	3	6	132	Quaternary	600	1000	600000	Intact	No	yes	yes	single	steep	steep		
PL-042	Peace	BC 86047 no 89	PL-35	3	6	186	Quaternary	600	2000	1200000	Intact	No	yes	yes	single	steep	steep		
PL-043	Peace	BC 86047 no 89	PL-37	3	6	175	Quaternary	600	600	360000	Intact	No	yes	yes	single	steep	steep		
PL-044	Peace	BC 86047 no 91	PL-38	4	2	196	Quaternary	800	800	640000	Intact	yes	yes	yes	single	steep	steep		new has farm field on it can't really tell
PL-045	Peace	BC 86047 no 91	PL-39	4	6	181	Quaternary	600	600	360000	Intact	yes	yes	yes	single	steep	steep		older than above
PL-046	Peace	BC 86047 no 91	PL-40	3	4	181	Quaternary	600	300	180000	Intact	yes	yes	yes	single	steep	steep		Highly mobile flow
PL-047	Peace	BC 86047 no 91	PL-39	4	6	181	Quaternary	600	750	450000	Intact	yes	yes	yes	single	steep	steep		
PL-048	Peace	BC 86047 no 91	PL-41	3	6	206	Quaternary	600	600	360000	Intact	No	yes	yes	single	steep	steep		
PL-049	Peace	BC 86047 no 91	PL-42	3	6	198	Quaternary	700	400	280000	Intact	No	yes	yes	single	steep	steep		
PL-050	Peace	BC 86047 no 91	PL-42	3	6	198	Quaternary	700	400	280000	Intact	No	yes	yes	single	steep	steep		
PL-051	Peace	BC 86047 no 91	PL-42	3	6	198	Quaternary	700	400	280000	Intact	No	yes	yes	single	steep	steep		
PL-052	Peace	BC 86047 no 91	PL-42	3	6	187	Quaternary	450	450	202500	Intact	No	yes	yes	single	steep	steep		
PL-053	Peace	BC 86047 no 91	PL-46	3	6	158	Quaternary	600	800	480000	Intact	No	yes	yes	single	steep	steep		debris all mobile
PL-054	Peace	BC 86047 no 91	PL-47	2	6	210	Quaternary	700	1000	700000	Intact	No	yes	yes	single	steep	steep		less mobile flow
PL-055	Peace	BC 86047 no 91	PL-47	2	5	205	Quaternary	1100	300	330000	Intact	No	yes	yes	single	steep	steep		
PL-056	Peace	BC 86047 no 91	PL-47	4	6	205	Quaternary	600	700	420000	Intact	No	yes	yes	single	steep	steep		
PL-057	Peace	BC 86047 no 91	PL-49	3	3	123	Quaternary	250	600	150000	Intact	No	yes	yes	single	steep	steep		
PL-058	Peace	BC 86047 no 91	PL-51	3	6	250	Quaternary	300	600	180000	Intact	No	yes	yes	single	steep	steep		
PL-059	Peace	BC 86047 no 91	PL-53	4	3	188	Quaternary	500	500	250000	Intact	No	yes	yes	single	steep	steep		
PL-060	Peace	BC 86047 no 91	PL-53	3	6	182	Quaternary	600	1100	660000	Intact	No	yes	yes	single	steep	steep		
PL-061	Peace	BC 86047 no 93	PL-53	3	6	182	Quaternary	600	1100	660000	Intact	No	yes	yes	single	steep	steep		
PL-062	Peace	BC 86047 no 93	PL-53	3	6	255	Shale	500	1300	650000	Intact	No	yes	yes	single	steep	steep		
PL-063	Peace	BC 86047 no 93	PL-53	3	6	291	Shale	400	16000	6400000	Intact	No	yes	yes	single	steep	steep		
PL-064	Peace	BC 86047 no 93	PL-53	3	6	230	Shale	400	500	200000	Intact	yes	yes	yes	single	steep	steep		
PL-065	Peace	BC 86047 no 93	PL-53	2	6	56	Shale	600	250	150000	Intact	yes	yes	yes	single	steep	steep		good for looking at
PL-066	Peace	BC 86047 no 93	PL-53	3	6	134	Shale	600	2000	1200000	Intact	No	yes	yes	single	steep	steep		
PL-067	Peace	BC 86047 no 93	PL-54	3	6	203	Shale	700	1300	910000	Intact	No	yes	yes	single	steep	steep		
PL-068	Peace	BC 86047 no 93	PL-55	3	6	190	Shale	500	500	250000	Intact	yes	yes	yes	single	steep	steep		a 2 with 5's on the side
PL-069	Peace	BC 86047 no 93	PL-55	3	6	168	Quaternary	450	400	180000	Intact	yes	yes	yes	single	steep	steep		
PL-070	Peace	BC 86047 no 93	PL-55	2	6	225	Quaternary	800	4000	3200000	Intact	No	yes	yes	single	steep	steep		
PL-071	Peace	BC 86047 no 93	PL-59	4	6	225	Quaternary	800	4000	3200000	Intact	No	yes	yes	single	steep	steep		
PL-072	Peace	BC 86047 no 93	PL-64	3	6	305	Quaternary	300	1300	390000	Intact	No	yes	yes	single	steep	steep		old mudflow
PL-073	Peace	BC 86047 no 93	PL-64	4	6	208	Quaternary	500	600	300000	Intact	No	yes	yes	single	steep	steep		old mudflow
PL-074	Peace	BC 86047 no 93	PL-12	5	3	189	Shale	200	300	60000	No	No	yes	yes	single	steep	steep		layer bridge

[illegible]

PdL-157	Pace	BC 86047 no 190	PdL 143	5	6	175	Quaternary	600	500	300000	intact	No	yes	enclosed	no	yes	single complex	gentle steep	erect good
PdL-158	Pace	BC 86047 no 190	PdL 144	3	6	175	Quaternary	500	200	100000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-159	Pace	BC 86047 no 190	PdL 144	3	6	169	Quaternary	500	200	100000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-160	Pace	BC 86047 no 190	PdL 144/145	3	6	169	Quaternary	550	1400	710000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-161	Pace	BC 86047 no 120	PdL 145	3	6	236	Quaternary	350	450	157500	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-162	Pace	BC 86047 no 120	PdL 145	5	6	175	Quaternary	500	400	200000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-163	Pace	BC 86047 no 120	PdL 145	4	6	175	Quaternary	600	600	480000	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-164	Pace	BC 86047 no 124	PdL 145	4	6	150	Quaternary	500	1200	600000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-165	Halfway	BC 86047 no 124	H.L. 2	4	6	127	Quaternary	500	600	300000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-166	Pace	BC 86047 no 124	PdL 146	4	6	125	Quaternary	300	1200	360000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-167	Pace	BC 86047 no 124	PdL 146	4	6	106	Quaternary	500	1300	650000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-168	Pace	BC 86047 no 124	PdL 147	4	6	146	Quaternary	700	900	630000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-169	Pace	BC 86047 no 124	PdL 147	4	6	146	Quaternary	400	300	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-170	Pace	BC 86047 no 124	PdL 147	4	6	154	Quaternary	400	300	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-171	Pace	BC 86047 no 124	PdL 149/150	4	6	93	Quaternary	400	300	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-172	Pace	BC 86047 no 124	PdL 151	4	6	93	Quaternary	400	300	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-173	Pace	BC 86047 no 124	PdL 152	3	6	225	Quaternary	300	900	270000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-174	Pace	BC 86047 no 124	PdL 153	3	6	78	Quaternary	300	1100	330000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-175	Pace	BC 86047 no 124	PdL 154	4	6	143	Quaternary	500	200	1000000	no	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-176	Pace	BC 86047 no 124	PdL 155	4	6	174	Quaternary	400	250	100000	no	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-177	Pace	BC 86047 no 103	PdL 155	3	6	193	Quaternary	300	400	120000	no	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-178	Pace	BC 86047 no 103	PdL 157/158	4	6	137	Quaternary	600	3000	1500000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-179	Pace	BC 86047 no 103	PdL 157/158	4	6	105	Quaternary	500	500	260000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-180	Pace	BC 86047 no 105	PdL 159	5	6	111	Quaternary	300	2700	810000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-181	Pace	BC 86047 no 105	PdL 164	4	6	141	Quaternary	300	2700	810000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-182	Pace	BC 86047 no 107	PdL 165	4	6	144	Quaternary	400	500	200000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-183	Pace	BC 86047 no 107	PdL 165	5	6	135	Quaternary	800	900	720000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-184	Pace	BC 86047 no 107	PdL 169	4	1	140	Quaternary	200	1300	260000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-185	Pace	BC 86047 no 107	PdL 172/173	4	1	132	Quaternary	400	2400	960000	no	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-186	Pace	BC 86047 no 107	PdL 173	3	6	116	Quaternary	600	250	120000	no	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-187	Pace	BC 86047 no 60	PdL 172	4	6	131	Quaternary	400	1200	480000	intact	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-188	Pace	BC 86047 no 60	PdL 175	4	6	214	Quaternary	300	500	1000000	intact	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-189	Pace	BC 86047 no 60	PdL 175	4	6	214	Quaternary	300	500	1000000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-190	Pace	BC 86047 no 60	PdL 177/178/179	4	6	180	Quaternary	200	500	150000	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-191	Pace	BC 86047 no 60	PdL 177/178/179	3	2	105	Quaternary	200	500	100000	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-192	Pace	BC 86047 no 60	PdL 180	4	6	147	Quaternary	200	500	100000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-193	Pace	BC 86047 no 60	PdL 180	5	6	147	Quaternary	400	600	240000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-194	Pace	BC 86047 no 60	PdL 181	2	5	134	Quaternary	800	300	270000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-195	Pace	BC 86047 no 60	PdL 182/183	4	6	135	Quaternary	800	3400	2720000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-196	Pace	BC 86047 no 61	PdL 180	4	6	189	Quaternary	300	400	120000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-197	Pace	BC 86047 no 61	PdL 180	3	7	206	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-198	Pace	BC 86047 no 61	PdL 180	3	5	216	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-199	Pace	BC 86047 no 61	PdL 180	3	5	216	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-200	Pace	BC 86047 no 61	PdL 180	3	2	278	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-201	Pace	BC 86047 no 61	PdL 180	3	3	129	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-202	Pace	BC 86047 no 61	PdL 180	3	6	140	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-203	Pace	BC 86047 no 61	PdL 180	3	1	203	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-204	Pace	BC 86047 no 61	PdL 180	3	4	40	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-205	Pace	BC 86047 no 61	PdL 180	3	3	63	Quaternary	7	7	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-206	Pace	BC 86047 no 108	PdL 180	3	6	240	Shale	200	1200	240000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-207	Pace	BC 86047 no 108	PdL 180	3	6	83	Quaternary	300	700	210000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-208	Pace	BC 86047 no 108	PdL 180	3	3	63	Shale	200	9000	90000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-209	Pace	BC 86047 no 108	PdL 180	3	3	210	Shale	200	200	180000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-210	Pace	BC 86047 no 108	PdL 180	3	3	186	Shale	200	900	180000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-211	Pace	BC 86047 no 108	PdL 180	3	3	238	Shale	200	1000	300000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-212	Pace	BC 86047 no 108	PdL 180	3	3	231	Shale	300	1000	300000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-213	Pace	BC 86047 no 108	PdL 180	3	7	60	Shale	300	270000	debris	debris	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-214	Pace	BC 86047 no 108	PdL 180	3	3	48	Shale	300	800	240000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-215	Pace	BC 86047 no 108	PdL 180	4	2	7	Shale	250	400	100000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-216	Pace	BC 86047 no 107	PdL 180	3	4	130	Quaternary	200	600	120000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-217	Pace	BC 86047 no 107	PdL 180	3	6	23	Quaternary	200	200	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-218	Pace	BC 86047 no 87	PdL 180	3	6	184	Shale	200	400	80000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-219	Pace	BC 86047 no 87	PdL 180	3	6	184	Quaternary	200	400	80000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-220	Pace	BC 86047 no 87	PdL 180	3	3	210	Quaternary	200	400	80000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-221	Pace	BC 86047 no 89	PdL 20	3	6	91	Quaternary	300	200	60000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-222	Golden Ch.	BC 86047 no 89	PdL 180	3	6	140	Shale	400	350	140000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-223	Pace	BC 86047 no 91	PdL 46	3	6	283	Quaternary	600	400	240000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-224	Pace	BC 86047 no 93	PdL 63	4	6	237	Quaternary	500	300	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-225	Pace	BC 86047 no 93	PdL 63	3	5	215	Quaternary	500	250	125000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-226	Pace	BC 86047 no 93	PdL 63	4	6	217	Quaternary	400	1500	600000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-227	Pace	BC 86047 no 95	PdL 158	3	6	316	Quaternary	400	1100	440000	intact	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-228	Pace	BC 86047 no 128	PdL 113	3	6	202	Quaternary	300	2500	750000	intact	Yes	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-229	Pace	BC 86047 no 108	PdL 110	3	4	134	Quaternary	200	200	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-230	Pace	BC 86047 no 108	PdL 110	4	3	230	Quaternary	200	200	150000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-231	Pace	BC 86047 no 108	PdL 110	3	5	157	Quaternary	600	200	120000	intact	No	yes	enclosed	no	yes	single complex	erect good	erect good
PdL-232	Pace	BC 86047 no 131	PdL 58	3	6	84	Quaternary	300	300	60000	intact	No	yes	enclosed	no	yes	single complex	erect good	

342

343

[illegible]

Failure Name	Flow	Photo	Flow Segment	Activity	Failure Type	Slide	Probable Basal Failure Material	Length (m)	Width (m)	Area m ²	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Slope Angle	Feature Clarity	May Be Mapped As	Additional Comments
BL-003	Station	BC 86074 no 125	BL-94	1	7	348	Quaternary	800	1300	960000	debris	No	Yes	yes	single	steep	good		features don't show on all photos, but there is an older one there
BL-004	Station	BC 86074 no 126	BL-94	1	7	348	Quaternary	400	300	60000	debris	No	Yes	yes	single	steep	good		
BL-006	Station	BC 86047 no 153	BL-59	3	6	240	Quaternary	500	300	150000	debris	No	Yes	yes	single	steep	good		Christians scarp, large head wall with fill, intact scarp with slide plane exposed, rest of slide is fairly steep.
BL-008	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	500	300	150000	debris	No	Yes	yes	single	steep	good		With tilting trees, obviously layered
BL-009	Station	BC 86047 no 91	BL-10	3	7	6	Quaternary	800	1600	640000	debris	Yes	Yes	yes	complex	gentle	modified	BL-035a	
BL-010	Station	BC 86075 no 15	BL-66	3	6	238	shale	800	800	720000	debris	Yes	Yes	yes	complex	steep	good		on large band of free, steep scarp, in canyon, may have shallower slides at top
BL-011	Station	BC 86075 no 15	BL-68	3	6	230	shale	800	1000	400000	debris	Yes	Yes	yes	single	steep	covered		Small slide, steep head scarp, in canyon
BL-012	Station	BC 86075 no 15	BL-68	3	6	230	shale	500	1200	360000	debris	Yes	Yes	yes	single	steep	covered		Small slide, steep head scarp, in canyon
BL-013	Station	BC 86075 no 15	BL-66	3	6	238	shale	700	600	420000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-014	Station	BC 86075 no 15	BL-66	3	6	238	shale	500	800	400000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-016	Station	BC 86075 no 15	BL-77, 1/2	3	6	50	shale	500	1300	500000	debris	Yes	Yes	yes	single	steep	covered		In tributary, shallower than in canyon, south facing, small head scarp
BL-017	Station	BC 86075 no 15	BL-65	3	6	254	shale	700	700	490000	debris	Yes	Yes	yes	single	steep	covered		In tributary, shallower than in canyon, north facing
BL-018	Station	BC 86075 no 15	BL-66	3	6	238	shale	600	1300	780000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-021	Station	BC 86075 no 15	BL-72	3	6	230	shale	500	1100	550000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-022	Station	BC 86075 no 15	BL-72	3	6	230	Quaternary	400	1000	400000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-023	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	700	1200	840000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-024	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-025	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-026	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-027	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-028	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-029	Station	BC 86075 no 15	BL-78	3	6	145	Quaternary	600	1200	720000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-030	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-031	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-032	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-033	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-034	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-035	Station	BC 86047 no 153	BL-59	3	6	231	shale	650	650	422500	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-041	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-042	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-043	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-044	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-045	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-046	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-047	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-048	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-049	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-050	Station	BC 86047 no 153	BL-54	3	6	168	shale	1000	800	600000	debris	Yes	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-051	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-052	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-053	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-054	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-055	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-056	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-057	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-058	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-059	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-060	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-061	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-062	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-063	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-064	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-065	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-066	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-067	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-068	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-069	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-070	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-071	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-072	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-073	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-074	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-075	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-076	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-077	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-078	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-079	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-080	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-081	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-082	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp, smaller debris fan, more like a build up of debris instead of large event
BL-083	Station	BC 86047 no 153	BL-47	3	6	205	shale	800	800	320000	debris	No	Yes	yes	single	steep	covered		Shallow head scarp

346

348

BR-170	BR-171	BR-172	BR-173	BR-174	BR-175	BR-176	BR-177	BR-178	BR-179	BR-180	BR-181	BR-182	BR-183	BR-184	BR-185	BR-186	BR-187	BR-188	BR-189	BR-190	BR-191	BR-192	BR-193	BR-194	BR-195	BR-196	BR-197	BR-198	BR-199	BR-200	BR-201	BR-202	BR-203	BR-204	BR-205	BR-206	BR-207	BR-208	BR-209	BR-210	BR-211	BR-212	BR-213	BR-214	BR-215	BR-216	BR-217	BR-218	BR-219	BR-220	BR-221	BR-222	BR-223	BR-224	BR-225	BR-226	BR-227	BR-228	BR-229	BR-230	BR-231	BR-232	BR-233	BR-234	BR-235	BR-236	BR-237	BR-238	BR-239	BR-240	BR-241	BR-242	BR-243	BR-244	BR-245	BR-246	BR-247	BR-248	BR-249	BR-250	BR-251	BR-252	BR-253	BR-254	BR-255	BR-256	BR-257	BR-258	BR-259	BR-260	BR-261	BR-262	BR-263	BR-264	BR-265	BR-266	BR-267	BR-268	BR-269	BR-270	BR-271	BR-272	BR-273	BR-274	BR-275	BR-276	BR-277	BR-278	BR-279	BR-280	BR-281	BR-282	BR-283	BR-284	BR-285	BR-286	BR-287	BR-288	BR-289	BR-290	BR-291	BR-292	BR-293	BR-294	BR-295	BR-296	BR-297	BR-298	BR-299	BR-300	BR-301	BR-302	BR-303	BR-304	BR-305	BR-306	BR-307	BR-308	BR-309	BR-310	BR-311	BR-312	BR-313	BR-314	BR-315	BR-316	BR-317	BR-318	BR-319	BR-320	BR-321	BR-322	BR-323	BR-324	BR-325	BR-326	BR-327	BR-328	BR-329	BR-330	BR-331	BR-332	BR-333	BR-334	BR-335	BR-336	BR-337	BR-338	BR-339	BR-340	BR-341	BR-342	BR-343	BR-344	BR-345	BR-346	BR-347	BR-348	BR-349	BR-350	BR-351	BR-352	BR-353	BR-354	BR-355	BR-356	BR-357	BR-358	BR-359	BR-360	BR-361	BR-362	BR-363	BR-364	BR-365	BR-366	BR-367	BR-368	BR-369	BR-370	BR-371	BR-372	BR-373	BR-374	BR-375	BR-376	BR-377	BR-378	BR-379	BR-380	BR-381	BR-382	BR-383	BR-384	BR-385	BR-386	BR-387	BR-388	BR-389	BR-390	BR-391	BR-392	BR-393	BR-394	BR-395	BR-396	BR-397	BR-398	BR-399	BR-400	BR-401	BR-402	BR-403	BR-404	BR-405	BR-406	BR-407	BR-408	BR-409	BR-410	BR-411	BR-412	BR-413	BR-414	BR-415	BR-416	BR-417	BR-418	BR-419	BR-420	BR-421	BR-422	BR-423	BR-424	BR-425	BR-426	BR-427	BR-428	BR-429	BR-430	BR-431	BR-432	BR-433	BR-434	BR-435	BR-436	BR-437	BR-438	BR-439	BR-440	BR-441	BR-442	BR-443	BR-444	BR-445	BR-446	BR-447	BR-448	BR-449	BR-450	BR-451	BR-452	BR-453	BR-454	BR-455	BR-456	BR-457	BR-458	BR-459	BR-460	BR-461	BR-462	BR-463	BR-464	BR-465	BR-466	BR-467	BR-468	BR-469	BR-470	BR-471	BR-472	BR-473	BR-474	BR-475	BR-476	BR-477	BR-478	BR-479	BR-480	BR-481	BR-482	BR-483	BR-484	BR-485	BR-486	BR-487	BR-488	BR-489	BR-490	BR-491	BR-492	BR-493	BR-494	BR-495	BR-496	BR-497	BR-498	BR-499	BR-500	BR-501	BR-502	BR-503	BR-504	BR-505	BR-506	BR-507	BR-508	BR-509	BR-510	BR-511	BR-512	BR-513	BR-514	BR-515	BR-516	BR-517	BR-518	BR-519	BR-520	BR-521	BR-522	BR-523	BR-524	BR-525	BR-526	BR-527	BR-528	BR-529	BR-530	BR-531	BR-532	BR-533	BR-534	BR-535	BR-536	BR-537	BR-538	BR-539	BR-540	BR-541	BR-542	BR-543	BR-544	BR-545	BR-546	BR-547	BR-548	BR-549	BR-550	BR-551	BR-552	BR-553	BR-554	BR-555	BR-556	BR-557	BR-558	BR-559	BR-560	BR-561	BR-562	BR-563	BR-564	BR-565	BR-566	BR-567	BR-568	BR-569	BR-570	BR-571	BR-572	BR-573	BR-574	BR-575	BR-576	BR-577	BR-578	BR-579	BR-580	BR-581	BR-582	BR-583	BR-584	BR-585	BR-586	BR-587	BR-588	BR-589	BR-590	BR-591	BR-592	BR-593	BR-594	BR-595	BR-596	BR-597	BR-598	BR-599	BR-600	BR-601	BR-602	BR-603	BR-604	BR-605	BR-606	BR-607	BR-608	BR-609	BR-610	BR-611	BR-612	BR-613	BR-614	BR-615	BR-616	BR-617	BR-618	BR-619	BR-620	BR-621	BR-622	BR-623	BR-624	BR-625	BR-626	BR-627	BR-628	BR-629	BR-630	BR-631	BR-632	BR-633	BR-634	BR-635	BR-636	BR-637	BR-638	BR-639	BR-640	BR-641	BR-642	BR-643	BR-644	BR-645	BR-646	BR-647	BR-648	BR-649	BR-650	BR-651	BR-652	BR-653	BR-654	BR-655	BR-656	BR-657	BR-658	BR-659	BR-660	BR-661	BR-662	BR-663	BR-664	BR-665	BR-666	BR-667	BR-668	BR-669	BR-670	BR-671	BR-672	BR-673	BR-674	BR-675	BR-676	BR-677	BR-678	BR-679	BR-680	BR-681	BR-682	BR-683	BR-684	BR-685	BR-686	BR-687	BR-688	BR-689	BR-690	BR-691	BR-692	BR-693	BR-694	BR-695	BR-696	BR-697	BR-698	BR-699	BR-700	BR-701	BR-702	BR-703	BR-704	BR-705	BR-706	BR-707	BR-708	BR-709	BR-710	BR-711	BR-712	BR-713	BR-714	BR-715	BR-716	BR-717	BR-718	BR-719	BR-720	BR-721	BR-722	BR-723	BR-724	BR-725	BR-726	BR-727	BR-728	BR-729	BR-730	BR-731	BR-732	BR-733	BR-734	BR-735	BR-736	BR-737	BR-738	BR-739	BR-740	BR-741	BR-742	BR-743	BR-744	BR-745	BR-746	BR-747	BR-748	BR-749	BR-750	BR-751	BR-752	BR-753	BR-754	BR-755	BR-756	BR-757	BR-758	BR-759	BR-760	BR-761	BR-762	BR-763	BR-764	BR-765	BR-766	BR-767	BR-768	BR-769	BR-770	BR-771	BR-772	BR-773	BR-774	BR-775	BR-776	BR-777	BR-778	BR-779	BR-780	BR-781	BR-782	BR-783	BR-784	BR-785	BR-786	BR-787	BR-788	BR-789	BR-790	BR-791	BR-792	BR-793	BR-794	BR-795	BR-796	BR-797	BR-798	BR-799	BR-800	BR-801	BR-802	BR-803	BR-804	BR-805	BR-806	BR-807	BR-808	BR-809	BR-810	BR-811	BR-812	BR-813	BR-814	BR-815	BR-816	BR-817	BR-818	BR-819	BR-820	BR-821	BR-822	BR-823	BR-824	BR-825	BR-826	BR-827	BR-828	BR-829	BR-830	BR-831	BR-832	BR-833	BR-834	BR-835	BR-836	BR-837	BR-838	BR-839	BR-840	BR-841	BR-842	BR-843	BR-844	BR-845	BR-846	BR-847	BR-848	BR-849	BR-850	BR-851	BR-852	BR-853	BR-854	BR-855	BR-856	BR-857	BR-858	BR-859	BR-860	BR-861	BR-862	BR-863	BR-864	BR-865	BR-866	BR-867	BR-868	BR-869	BR-870	BR-871	BR-872	BR-873	BR-874	BR-875	BR-876	BR-877	BR-878	BR-879	BR-880	BR-881	BR-882	BR-883	BR-884	BR-885	BR-886	BR-887	BR-888	BR-889	BR-890	BR-891	BR-892	BR-893	BR-894	BR-895	BR-896	BR-897	BR-898	BR-899	BR-900	BR-901	BR-902	BR-903	BR-904	BR-905	BR-906	BR-907	BR-908	BR-909	BR-910	BR-911	BR-912	BR-913	BR-914	BR-915	BR-916	BR-917	BR-918	BR-919	BR-920	BR-921	BR-922	BR-923	BR-924	BR-925	BR-926	BR-927	BR-928	BR-929	BR-930	BR-931	BR-932	BR-933	BR-934	BR-935	BR-936	BR-937	BR-938	BR-939	BR-940	BR-941	BR-942	BR-943	BR-944	BR-945	BR-946	BR-947	BR-948	BR-949	BR-950	BR-951	BR-952	BR-953	BR-954	BR-955	BR-956	BR-957	BR-958	BR-959	BR-960	BR-961	BR-962	BR-963	BR-964	BR-965	BR-966	BR-967	BR-968	BR-969	BR-970	BR-971	BR-972	BR-973	BR-974	BR-975	BR-976	BR-977	BR-978	BR-979	BR-980	BR-981	BR-982	BR-983	BR-984	BR-985	BR-986	BR-987	BR-988	BR-989	BR-990	BR-991	BR-992	BR-993	BR-994	BR-995	BR-996	BR-997	BR-998	BR-999	BR-1000
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Appendix IV - Landslide Inventory
 Table IV - 3 Southern Tributaries

Failure Name	River	Photo	River Segment	Activity	Failure Type	Slope Aspect	Probable Base Failure Material	Length (m)	Width (m)	Area m ²	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Slope Angle	Failure Clarity	May Be Mapped As	Additional Comments
KL-001	Kiskatinaw	BC 86047 no 87	KL 1	2	2	62	Quaternary	500	500	250000	debris	YES	eroded	no	single	steep	covered		good exposure of sliding plane
KL-002	Kiskatinaw	BC 86047 no 87	KL 1/2	4	7	43	Shale	1100	2000	2200000	NO	YES	partial	yes	complex	gentle	vegetated		large vegetated area
KL-003	Kiskatinaw	BC 86047 no 81	KL 7	3	3	182	Shale	400	400	160000	NO	YES	no	yes	single	steep	eroded		small failures above canyon wall, not likely peace lake failures
KL-004	Kiskatinaw	BC 86047 no 81	KL 7	4	3	184	Quaternary	200	200	40000	NO	YES	no	yes	single	steep	eroded		
KL-005	Kiskatinaw	BC 86047 no 81	KL 10	4	3	149	Quaternary	200	600	120000	NO	YES	no	yes	single	steep	eroded		
KL-006	Kiskatinaw	BC 86047 no 81	KL 11	4	3	186	Quaternary	200	300	60000	NO	YES	no	yes	single	steep	eroded		
KL-007	Kiskatinaw	BC 86047 no 81	KL 14	4	7	286	Quaternary	200	700	140000	NO	YES	no	yes	single	steep	eroded		
KL-008	Kiskatinaw	BC 86047 no 80	KL 19	3	6	131	Shale	300	500	350000	infect	YES	no	yes	single	steep	good		
KL-009	Kiskatinaw	BC 86047 no 80	KL 20	4	3	190	Shale	300	500	150000	NO	YES	no	yes	single	steep	good		
KL-010	Kiskatinaw	BC 86047 no 80	KL 25	4	3	170	Shale	300	900	270000	NO	YES	no	yes	single	steep	good		
KL-011	Kiskatinaw	BC 86047 no 79	KL 35	4	3	130	Shale	400	400	160000	NO	YES	no	yes	single	steep	eroded		
KL-012	Kiskatinaw	BC 86047 no 79	KL 38	4	2	193	Shale	400	600	240000	debris	YES	no	yes	single	steep	eroded		
KL-013	Kiskatinaw	BC 86047 no 79	KL 37	4	3	144	Shale	500	500	250000	NO	YES	no	yes	single	steep	eroded		
KL-014	Kiskatinaw	BC 86047 no 79	KL 41	3	7	101	Shale	400	700	280000	NO	YES	no	yes	single	steep	eroded		
KL-015	Kiskatinaw	BC 86047 no 79	KL 41	3	7	101	Shale	400	700	280000	NO	YES	no	yes	single	steep	eroded		
KL-016	Kiskatinaw	BC 86047 no 79	KL 42	3	6	147	Shale	600	1300	780000	infect	NO	no	yes	complex	steep	eroded		
KL-017	Kiskatinaw	BC 86047 no 81	KL 10	3	3	155	Shale	300	500	150000	NO	NO	yes	yes	complex	steep	eroded		
KL-018	Kiskatinaw	BC 86047 no 81	KL 10	3	3	155	Shale	400	300	120000	NO	NO	yes	yes	complex	steep	eroded		
KL-019	Kiskatinaw	BC 86047 no 81	KL 11	3	3	155	Shale	300	400	120000	NO	NO	yes	yes	complex	steep	eroded		
KR-001	Kiskatinaw	BC 86047 no 87	KR 1	3	3	256	Shale	400	500	200000	no	yes	yes	yes	single	steep	eroded		
KR-002	Kiskatinaw	BC 86047 no 87	KR 2	3	6	248	Shale	500	700	350000	infect	yes	yes	yes	single	steep	eroded		
KR-003	Kiskatinaw	BC 86047 no 87	KR 2	3	3	227	Shale	400	1000	400000	no	yes	eroded	yes	complex	steep	eroded		
KR-004	Kiskatinaw	BC 86047 no 87	KR 2	3	7	265	Shale	800	600	480000	debris	yes	yes	yes	single	steep	eroded		
KR-005	Kiskatinaw	BC 86047 no 87	KR 2	4	7	308	Shale	800	1000	800000	debris	yes	yes	yes	single	steep	eroded		
KR-006	Kiskatinaw	BC 86047 no 81	KR 11	3	6	215	Shale	200	1000	200000	infect	yes	yes	yes	single	steep	eroded		
KR-007	Kiskatinaw	BC 86047 no 81	KR 9	3	6	320	Shale	500	500	250000	infect	no	no	yes	complex	steep	eroded		
KR-008	Kiskatinaw	BC 86047 no 81	KR 11	3	3	335	Shale	400	500	200000	no	no	no	yes	single	steep	eroded		
KR-009	Kiskatinaw	BC 86047 no 81	KR 11	3	3	4	Shale	100	800	80000	no	no	yes	no	single	steep	eroded		
KR-010	Kiskatinaw	BC 86047 no 81	KR 12	4	2	348	Quaternary	500	1100	550000	debris	yes	yes	yes	single	steep	eroded		
KR-011	Kiskatinaw	BC 86047 no 81	KR 13	3	3	287	Quaternary	400	600	240000	no	yes	yes	yes	single	steep	eroded		
KR-012	Kiskatinaw	BC 86047 no 80	KR 12L 1	3	7	65	Shale	300	500	150000	infect	yes	yes	yes	single	steep	eroded		
KR-013	Kiskatinaw	BC 86047 no 80	KR 16	3	2	340	Quaternary	300	1200	360000	debris	yes	yes	yes	single	steep	eroded		
KR-014	Kiskatinaw	BC 86047 no 80	KR 17	3	6	335	Shale	800	600	480000	infect	no	yes	yes	single	steep	eroded		
KR-015	Kiskatinaw	BC 86047 no 80	KR 17R 1	3	6	294	Shale	300	800	240000	infect	no	yes	yes	single	steep	eroded		
KR-016	Kiskatinaw	BC 86047 no 80	KR 20	4	2	256	Shale	700	600	420000	debris	no	yes	yes	single	steep	eroded		
KR-017	Kiskatinaw	BC 86047 no 80	KR 21	4	3	343	Shale	200	800	160000	no	yes	yes	yes	single	steep	eroded		
KR-018	Kiskatinaw	BC 86047 no 80	KR 23	3	2	282	Quaternary	200	600	180000	debris	no	yes	yes	single	steep	eroded		
KR-019	Kiskatinaw	BC 86047 no 80	KR 25/26	3	6	13	Shale	600	1600	960000	infect	yes	yes	yes	single	steep	eroded		
KR-020	Kiskatinaw	BC 86047 no 79	KR 33	3	2	338	Shale	400	800	320000	debris	yes	no	yes	single	steep	eroded		
KR-021	Kiskatinaw	BC 86047 no 79	KR 31	4	3	334	Shale	300	200	60000	no	yes	no	yes	single	steep	eroded		
KR-022	Kiskatinaw	BC 86047 no 79	KR 30	3	2	331	Shale	500	900	450000	debris	yes	no	yes	single	steep	eroded		
ML-001	Maurice	BC 86047 no 81	PR-T15L 2	2	2	29	Quaternary	500	600	300000	debris	no	yes	yes	single	steep	eroded		
ML-002	Maurice	BC 86047 no 81	PR-T15L 2	3	2	5	Quaternary	400	800	320000	debris	yes	no	yes	single	steep	eroded		
ML-003	Maurice	BC 86047 no 81	PR-T15L 3/4	3	3	21	Quaternary	500	1200	600000	debris	yes	no	yes	single	steep	eroded		
ML-004	Maurice	BC 86047 no 81	PR-T15R 2	3	2	171	Quaternary	250	800	200000	debris	no	yes	yes	single	steep	eroded		
ML-005	Maurice	BC 86047 no 81	PR-T15R 3	2	2	211	Quaternary	200	500	100000	debris	yes	yes	yes	single	steep	eroded		
ML-006	Maurice	BC 86047 no 81	PR-T15R 5	3	3	206	Quaternary	800	800	640000	no	yes	yes	yes	single	steep	eroded		
ML-007	Maurice	BC 86047 no 81	PR-T15R 5	3	3	253	Quaternary	400	1000	400000	no	yes	yes	yes	single	steep	eroded		
ML-008	Maurice	BC 86047 no 81	ML 1	2	6	145	Shale	600	1600	1280000	infect	no	yes	yes	single	steep	eroded		
ML-009	Maurice	BC 86047 no 130	ML 2	3	6	152	Shale	500	300	300000	infect	yes	eroded	yes	single	steep	eroded		
ML-010	Maurice	BC 86047 no 130	ML 5	4	6	124	Shale	400	300	120000	infect	no	eroded	yes	single	steep	eroded		
ML-011	Maurice	BC 86047 no 130	ML 6	3	3	148	Shale	300	400	120000	no	yes	eroded	yes	single	steep	eroded		
ML-012	Maurice	BC 86047 no 130	ML 6	4	3	136	Shale	400	400	160000	infect	yes	eroded	yes	single	steep	eroded		
ML-013	Maurice	BC 86047 no 130	ML 7	3	6	156	Shale	400	400	160000	infect	yes	eroded	yes	single	steep	eroded		
ML-014	Maurice	BC 86047 no 130	ML 10	4	2	183	Shale	250	300	75000	debris	yes	eroded	yes	single	steep	eroded		
ML-015	Maurice	BC 86047 no 130	ML 10	2	6	239	Shale	700	1000	700000	infect	yes	eroded	yes	single	steep	eroded		
ML-016	Maurice	BC 86047 no 129	ML 11	3	6	204	Shale	750	1200	900000	infect	yes	eroded	yes	single	steep	eroded		
ML-017	Maurice	BC 86047 no 129	ML 11	3	8	175	Shale	900	900	810000	no	yes	yes	yes	single	steep	eroded		
ML-018	Maurice	BC 86047 no 129	ML 12	3	6	168	Quaternary	600	1200	1080000	infect	yes	eroded	yes	single	steep	eroded		
ML-019	Maurice	BC 86047 no 129	ML 13	3	8	210	Quaternary	600	1000	600000	debris	yes	eroded	yes	single	steep	eroded		
ML-020	Maurice	BC 86047 no 129	ML 14	3	8	173	Quaternary	1300	1400	1820000	infect	yes	eroded	yes	single	steep	eroded		

ML-016	Moherly	BC 86047 no 128	ML 15	3	6	184	Shale	700	2200	1540000	intact	yes	yes	yes	yes	complex single	steep	good
ML-017	Moherly	BC 86047 no 128	ML 15	3	6	190	Shale	800	400	320000	intact	yes	yes	yes	yes	complex single	steep	good
ML-018	Moherly	BC 86047 no 128	ML 18	4	6	138	Shale	200	600	120000	intact	no	yes	gullied	yes	complex single	steep	old
ML-019	Moherly	BC 86047 no 128	ML 18	4	6	165	Shale	300	850	195000	intact	no	yes	yes	yes	complex single	steep	good
ML-020	Moherly	BC 86047 no 128	ML 21	3	6	139	Shale	300	300	270000	intact	no	yes	yes	yes	complex single	steep	vegetated
ML-021	Moherly	BC 86047 no 128	ML 25	3	4	184	Quaternary	800	300	240000	no	no	yes	yes	yes	complex single	steep	good
ML-022	Moherly	BC 86047 no 127	ML 25	3	4	138	Quaternary	400	500	200000	no	no	yes	yes	yes	complex single	steep	good
ML-023	Moherly	BC 86047 no 127	ML 26	4	6	148	Shale	350	400	140000	intact	no	yes	yes	yes	complex single	steep	good
ML-024	Moherly	BC 86047 no 127	ML 29	3	3	143	Shale	600	900	540000	no	no	yes	yes	yes	complex single	steep	good
ML-025	Moherly	BC 86047 no 127	ML 30	3	6	135	Shale	500	700	330000	intact	no	yes	yes	yes	complex single	steep	good
ML-027	Moherly	BC 86047 no 127	ML 36	3	6	172	Shale	600	400	240000	intact	no	yes	yes	yes	complex single	steep	good
ML-029	Moherly	BC 86047 no 127	ML 36	3	6	172	Shale	600	400	240000	no	yes	yes	yes	yes	complex single	steep	good
ML-030	Moherly	BC 86047 no 127	ML 37	4	3	138	Shale	500	600	300000	no	no	yes	yes	yes	complex single	steep	good
ML-040	Moherly	BC 86047 no 87	ML 54	2	3	168	Quaternary	200	2000	400000	no	no	yes	no	yes	complex gentle	steep	vegetated
ML-041	Moherly	BC 86047 no 87	ML 55	2	3	168	Quaternary	50	200	10000	no	no	yes	no	yes	complex single	steep	good
ML-042	Moherly	BC 86047 no 87	ML 55	3	3	140	Quaternary	300	600	180000	no	no	yes	gullied	yes	complex single	steep	vegetated
ML-043	Moherly	BC 86047 no 87	ML 55	2	3	153	Quaternary	50	250	12500	no	no	yes	yes	yes	complex single	steep	good
ML-044	Moherly	BC 86047 no 87	ML 57	2	3	175	Quaternary	50	500	25000	no	no	yes	yes	yes	complex single	steep	good
ML-045	Moherly	BC 86047 no 87	ML 60	4	3	217	Quaternary	150	400	60000	no	no	no	no	yes	complex single	steep	vegetated
ML-046	Moherly	BC 86047 no 86	ML 61	2	3	181	Quaternary	100	200	20000	no	no	eroded	yes	yes	complex single	steep	good
ML-047	Moherly	BC 86047 no 86	ML 62	2	3	212	Quaternary	100	300	15000	no	no	eroded	yes	yes	complex single	steep	good
ML-048	Moherly	BC 86047 no 86	ML 70	3	3	158	Quaternary	200	400	80000	no	no	yes	gullied	yes	complex single	steep	good
ML-049	Moherly	BC 86047 no 87	ML 55	4	3	144	Quaternary	300	1100	330000	no	no	yes	no	yes	complex gentle	steep	vegetated
ML-001	Moherly	BC 86047 no 130	MR 2	4	6	330	shale	300	700	210000	intact	no	no	yes	yes	complex	steep	vegetated
ML-002	Moherly	BC 86047 no 130	MR 3	4	6	326	shale	400	800	320000	intact	no	yes	yes	yes	complex single	steep	vegetated
ML-003	Moherly	BC 86047 no 130	MR 3	2	6	298	shale	600	300	180000	intact	no	no	no	no	complex single	steep	vegetated
ML-004	Moherly	BC 86047 no 130	MR 4	3	6	324	shale	500	700	300000	intact	no	no	no	no	complex single	steep	vegetated
ML-005	Moherly	BC 86047 no 130	MR 4	3	6	325	shale	500	800	350000	intact	no	no	no	no	complex single	steep	vegetated
ML-006	Moherly	BC 86047 no 130	MR-T1R 1	2	6	253	shale	500	500	250000	intact	no	yes	yes	yes	complex single	steep	good
ML-007	Moherly	BC 86047 no 130	MR-T1R 1	3	6	225	shale	500	1000	500000	intact	no	yes	yes	yes	complex	steep	good
ML-008	Moherly	BC 86047 no 130	MR-T1L 3	3	6	37	shale	300	800	240000	intact	no	no	no	no	complex gentle	steep	vegetated
ML-010	Moherly	BC 86047 no 130	MR 5	4	6	321	shale	400	900	360000	intact	yes	partial	yes	yes	complex	steep	vegetated
ML-011	Moherly	BC 86047 no 130	MR 6	3	6	302	shale	400	300	120000	intact	no	yes	yes	yes	complex	steep	old
ML-013	Moherly	BC 86047 no 130	MR 8	4	6	343	shale	500	500	250000	intact	no	yes	yes	yes	complex	steep	old
ML-014	Moherly	BC 86047 no 130	MR 11	4	6	12	shale	900	810000	intact	no	yes	yes	yes	yes	complex single	steep	vegetated
ML-015	Moherly	BC 86047 no 128	MR 12	3	6	314	Quaternary	600	1400	840000	no	yes	no	no	yes	complex	gentle	vegetated
ML-016	Moherly	BC 86047 no 128	MR 13	3	6	78	Quaternary	400	1100	440000	no	yes	no	no	no	complex	gentle	vegetated
ML-017	Moherly	BC 86047 no 128	MR 14	3	6	38	shale	800	1000	800000	intact	no	no	no	no	complex single	steep	vegetated
ML-018	Moherly	BC 86047 no 128	MR 15	3	6	18	shale	850	750	637500	intact	yes	no	no	no	complex	steep	vegetated
ML-019	Moherly	BC 86047 no 128	MR 16	3	6	359	shale	550	800	440000	intact	no	yes	no	no	complex single	steep	vegetated
ML-020	Moherly	BC 86047 no 128	MR-T2R 1	3	6	323	shale	600	800	480000	intact	no	yes	yes	yes	complex single	steep	vegetated
ML-021	Moherly	BC 86047 no 128	MR-T2R 2	3	6	261	shale	500	300	150000	intact	no	no	no	no	complex single	steep	vegetated
ML-022	Moherly	BC 86047 no 128	MR-T2L 1	3	6	110	shale	600	800	540000	intact	no	yes	yes	yes	complex single	steep	good
ML-023	Moherly	BC 86047 no 128	MR-T2L 1	3	6	114	shale	300	400	120000	intact	no	yes	yes	yes	complex single	steep	good
ML-024	Moherly	BC 86047 no 128	MR 17	4	6	45	shale	400	400	160000	intact	no	yes	yes	yes	complex single	steep	good
ML-025	Moherly	BC 86047 no 128	MR 17	4	6	24	shale	800	2000	1600000	intact	no	yes	yes	yes	complex	steep	good
ML-026	Moherly	BC 86047 no 128	MR 18	4	6	343	shale	400	800	320000	intact	no	yes	yes	yes	complex single	steep	good
ML-027	Moherly	BC 86047 no 128	MR 21	4	6	8	shale	600	1800	1080000	intact	no	yes	yes	yes	complex single	steep	vegetated
ML-028	Moherly	BC 86047 no 128	MR 22	4	6	332	shale	400	850	340000	intact	yes	yes	yes	yes	complex	steep	covered
ML-029	Moherly	BC 86047 no 128	MR 25	2	6	323	shale	250	300	75000	intact	no	yes	yes	yes	complex single	steep	good
ML-030	Moherly	BC 86047 no 128	MR 26	4	6	330	shale	400	300	90000	intact	no	yes	yes	yes	complex single	steep	good
ML-031	Moherly	BC 86047 no 128	MR 28	4	6	340	shale	400	300	320000	intact	no	yes	yes	yes	complex	steep	vegetated
ML-033	Moherly	BC 86047 no 127	MR 30	4	6	338	shale	600	1200	720000	intact	no	no	yes	yes	complex single	steep	covered
ML-034	Moherly	BC 86047 no 127	MR 31	4	3	321	shale	500	600	300000	no	no	yes	yes	yes	complex single	steep	covered
ML-035	Moherly	BC 86047 no 127	MR 32	4	3	314	shale	600	600	360000	no	no	yes	yes	yes	complex single	steep	covered
ML-036	Moherly	BC 86047 no 127	MR 33	4	6	288	shale	400	800	320000	intact	no	yes	yes	yes	complex single	steep	covered
ML-037	Moherly	BC 86047 no 99	MR 34	3	6	332	shale	350	1300	455000	intact	no	yes	yes	yes	complex single	steep	covered
ML-038	Moherly	BC 86047 no 100	MR 37	4	6	337	shale	500	400	200000	intact	no	yes	yes	yes	complex single	steep	good
ML-039	Moherly	BC 86047 no 100	MR 39/40	4	3	310	shale	500	1400	700000	intact	no	yes	no	yes	complex	gentle	vegetated
ML-040	Moherly	BC 86047 no 87	MR 47	4	3	348	Quaternary	400	1600	840000	no	no	yes	no	yes	complex	gentle	vegetated
ML-041	Moherly	BC 86047 no 87	MR 47	4	3	323	Quaternary	300	30000	300000	no	no	yes	no	yes	complex single	gentle	vegetated
ML-042	Moherly	BC 86047 no 87	MR 48	3	3	318	Quaternary	300	700	210000	no	no	no	no	yes	complex single	gentle	vegetated
ML-043	Moherly	BC 86047 no 87	MR 50	3	7	555	Quaternary	400	700	280000	no	no	yes	no	yes	complex single	gentle	vegetated
ML-044	Moherly	BC 86047 no 87	MR 51	3	7	329	Quaternary	500	800	400000	debris	no	no	yes	no	complex single	gentle	vegetated
ML-045	Moherly	BC 86047 no 86	MR 67	4	3	231	Quaternary	300	600	180000	no	no	no	yes	yes	complex single	gentle	vegetated
ML-046	Moherly	BC 86047 no 86	MR 71	2	3	28	Quaternary	100	400	40000	no	no	no	no	yes	complex	gentle	good
ML-047	Moherly	BC 86047 no 100	MR 35	4	3	31	Quaternary	200	8000	80000	no	no	yes	yes	yes	complex single	gentle	vegetated
ML-048	Moherly	BC 86047 no 128	MR 28	3	4	332	Quaternary	500	200	100000	no	yes	yes	yes	yes	complex	gentle	vegetated
ML-049	Moherly	BC 86047 no 128	MR 28	3	4	316	Quaternary	700	200	140000	no	yes	yes	yes	yes	complex single	gentle	vegetated
ML-050	Moherly	BC 86047 no 128	MR 28	3	4	317	Quaternary	500	250	125000	no	yes	yes	yes	yes	complex single	gentle	vegetated

MR-051	Mobility	BC 86047 no 121	MR-20	3	3	57	Quaternary	400	350	140000	no	yes	no	yes	no	yes	single	sleep	vegetated																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
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PR-019	Pine	BC 86047 no 95	PR 7	3	7	352	shale	?	?	no	no	yes	no	complex	gentle	vegetated	
PR-020	Pine	BC 86047 no 95	PR 7	3	8	5	shale	1200	550	inlet	no	no	no	complex	steep	good	
PR-021	Pine	BC 86047 no 95	PR 7	3	8	344	shale	1700	2000	3400000	inlet	no	yes	single	steep	good	
PR-022	Pine	BC 86047 no 95	PR 7	4	6	345	shale	1600	1400	2240000	inlet	yes	yes	complex	steep	vegetated	
PR-023	Pine	BC 86047 no 95	PR 8	3	6	307	shale	1700	2400	4680000	inlet	yes	yes	single	steep	good	
PR-024	Pine	BC 86047 no 96	PR 8	3	5	281	Quaternary	1000	600	560000	debris	no	no	complex	gentle	vegetated	
PR-025	Pine	BC 86047 no 96	PR 10	3	5	27	Quaternary	1000	600	500000	debris	yes	no	complex	steep	vegetated	
PR-026	Pine	BC 86047 no 96	PR 11	3	6	350	shale	900	1400	1260000	inlet	yes	yes	complex	steep	vegetated	
PR-027	Pine	BC 86047 no 96	PR 12/13	3	7	335	Quaternary	500	600	300000	inlet	yes	yes	complex	steep	vegetated	PR-10
PR-028	Pine	BC 86047 no 96	PR 14	3	6	10	shale	900	1300	1170000	inlet	yes	yes	complex	steep	good	
PR-029	Pine	BC 86047 no 96	PR 17	3	3	290	shale	600	1300	720000	debris	yes	no	complex	gentle	vegetated	PR-30b
PR-030	Pine	BC 86047 no 96	PR 18/17	3	2	53	shale	500	1000	500000	debris	yes	no	complex	gentle	vegetated	
PR-031	Pine	BC 86047 no 96	PR 21/22	3	6	309	shale	1000	2200	2200000	inlet	yes	no	complex	gentle	vegetated	
PR-032	Pine	BC 86047 no 71	PR 22	3	6	338	shale	900	700	630000	inlet	no	no	complex	gentle	vegetated	fully recessed
PR-033	Pine	BC 86047 no 71	PR 25	2	7	330	shale	1700	1200	2040000	inlet	no	no	complex	gentle	vegetated	fully recessed
PR-034	Pine	BC 86047 no 71	PR 25	4	7	334	Quaternary	1500	1300	1800000	debris	yes	yes	single	gentle	vegetated	with plaster benches
PR-035	Pine	BC 86047 no 71	PR 27/30	4	7	327	Quaternary	1000	1800	1600000	debris	yes	no	complex	steep	vegetated	like flintrock
PR-036	Pine	BC 86047 no 71	PR 23/24	4	7	13	Quaternary	1200	1800	1920000	inlet	no	no	complex	steep	vegetated	
PR-037	Pine	BC 86047 no 70	PR 34	3	2	55	Quaternary	300	900	270000	debris	yes	no	complex	gentle	vegetated	
PR-038	Pine	BC 86047 no 70	PR 35	4	1	318	Quaternary	300	1300	360000	debris	yes	yes	complex	steep	vegetated	
PR-042	Pine	BC 86047 no 69	PR 42	2	2	282	shale	700	800	500000	debris	yes	yes	single	steep	good	
PR-043	Pine	BC 86047 no 71	PR 28	3	7	321	shale	1600	2300	3600000	no	no	no	single	gentle	vegetated	
PR-044	Pine	BC 86047 no 70	PR 31/32	4	7	314	Quaternary	1500	2200	3300000	no	yes	yes	single	gentle	vegetated	
PR-047	Pine	BC 86047 no 69	PR-T1L 6	2	4	10	Quaternary	1400	600	840000	inlet	no	no	complex	steep	vegetated	
PR-048	Pine	BC 86047 no 69	PR-T1L 3	4	7	58	Quaternary	500	1200	600000	inlet	yes	yes	complex	steep	vegetated	
PR-049	Pine	BC 86047 no 69	PR-T1R 3	4	7	261	Quaternary	500	800	400000	no	yes	yes	complex	steep	vegetated	
SCL-001	Scirpus Ch	BC 86047 no 72	SCL 8	2	2	108	Shale	?	?	900000	debris	yes	yes	complex	steep	good	
SCL-002	Scirpus Ch	BC 86047 no 72	SCL 12	3	2	117	Shale	1000	900	720000	inlet	yes	yes	single	steep	vegetated	
SCL-003	Scirpus Ch	BC 86047 no 72	SCL 13	3	6	121	Shale	800	900	480000	inlet	yes	yes	single	steep	good	
SCL-004	Scirpus Ch	BC 86047 no 72	SCL 14	4	6	162	Shale	500	600	300000	inlet	yes	yes	single	steep	good	
SCL-005	Scirpus Ch	BC 86047 no 72	SCL 14	3	6	148	Shale	500	600	300000	inlet	yes	yes	single	steep	good	
SCL-006	Scirpus Ch	BC 86047 no 72	SCL 14	4	2	178	Shale	500	800	400000	inlet	yes	yes	single	steep	good	
SCL-007	Scirpus Ch	BC 86047 no 72	SCL 14	4	6	183	Shale	500	800	300000	inlet	yes	yes	single	steep	good	
SCL-008	Scirpus Ch	BC 86047 no 72	SCL 14	4	3	219	Shale	550	500	275000	no	yes	yes	single	steep	good	
SCL-009	Scirpus Ch	BC 86047 no 72	SCL 14	4	5	9	Quaternary	800	700	560000	no	yes	yes	single	steep	good	
SCL-010	Scirpus Ch	BC 86047 no 72	SCL 16	2	6	90	Shale	500	800	450000	inlet	no	yes	single	steep	good	
SCL-012	Scirpus Ch	BC 86047 no 72	SCL 17	4	3	69	Shale	1200	1300	1560000	no	yes	yes	single	steep	good	
SCL-013	Scirpus Ch	BC 86047 no 72	SCL 17	2	6	146	Shale	400	400	160000	inlet	yes	no	single	steep	good	
SCL-014	Scirpus Ch	BC 86047 no 72	SCL 17	2	6	128	Shale	200	500	100000	inlet	no	yes	single	steep	good	
SCL-015	Scirpus Ch	BC 86047 no 72	SCL 18	4	6	215	Shale	400	900	360000	inlet	no	yes	single	steep	good	
SCL-016	Scirpus Ch	BC 86047 no 72	SCL 18	4	2	218	Shale	500	400	200000	debris	no	yes	single	steep	good	
SCL-017	Scirpus Ch	BC 86047 no 72	SCL 18	4	3	201	Shale	350	400	140000	no	yes	yes	single	steep	good	
SCL-001	Scirpus Ch	BC 86047 no 72	SCL 5a	3	7	320	Shale	1400	1800	2520000	debris	no	yes	single	steep	good	
SCL-002	Scirpus Ch	BC 86047 no 72	SCL 8	3	2	310	Shale	1000	1200	1200000	debris	no	yes	single	steep	good	
SCL-003	Scirpus Ch	BC 86047 no 72	SCL 7	2	3	319	Shale	400	400	180000	debris	no	yes	single	steep	good	
SCL-004	Scirpus Ch	BC 86047 no 72	SCL 7	3	2	298	Shale	500	300	150000	debris	no	yes	single	steep	good	
SCL-006	Scirpus Ch	BC 86047 no 72	SCL 8	2	3	337	Quaternary	300	300	90000	no	yes	yes	single	steep	good	
SCL-007	Scirpus Ch	BC 86047 no 72	SCL 8	2	3	334	Quaternary	350	400	140000	no	yes	yes	single	steep	good	
SCL-008	Scirpus Ch	BC 86047 no 72	SCL 9	4	4	63	Quaternary	1800	500	800000	inlet	yes	no	single	steep	good	
SCL-009	Scirpus Ch	BC 86047 no 72	SCL 9	3	6	299	Shale	1000	950	950000	inlet	yes	no	single	steep	good	
SCL-010	Scirpus Ch	BC 86047 no 72	SCL-T1R 1	3	6	290	Shale	900	1400	1260000	inlet	no	yes	single	steep	good	
SCL-011	Scirpus Ch	BC 86047 no 72	SCL-T1R 2	3	6	270	Shale	300	700	210000	debris	no	yes	single	steep	good	
SCL-012	Scirpus Ch	BC 86047 no 72	SCL-T1L 1	3	6	61	Shale	500	800	300000	inlet	no	yes	single	steep	good	
SCL-013	Scirpus Ch	BC 86047 no 72	SCL-T1L 1	2	2	98	Shale	500	500	250000	debris	no	yes	single	steep	good	
SCL-014	Scirpus Ch	BC 86047 no 72	SCL-T1L 1	3	6	85	Shale	400	650	290000	inlet	no	yes	single	steep	good	
SCL-015	Scirpus Ch	BC 86047 no 72	SCL 9	4	5	325	Quaternary	900	3000	2700000	inlet	no	yes	complex	steep	good	
Total										151617500	m2						
										1516175	km2						

Appendix IV - Landslide Inventory
Table IV - 4 Beaton River East

Failure Name	River	Photo	River Segment	Activity	Failure Type	Probable Basal Failure Material	Length (m)	Width (m)	Area m ²	Slope Aspect	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Slope Angle	Feature Clarity	May Be Mapped As	Additional Comments
AL-001	Alces	BC 86047 no 87	AL-T1R 1	3	2	Shale	500	800	400000	125	debris	yes	no	yes	complex	steep	muted		2 parts, a 3 and a 2
AL-002	Alces	BC 86047 no 87	AL-T1R 2	3	2	Shale	400	400	160000	75	debris	yes	yes	yes	single	gentle	covered		
AL-003	Alces	BC 86047 no 87	AL-T1L 1	2	3	Shale	200	200	40000	232	no	yes	eroded	yes	single	steep	good		
AL-004	Alces	BC 86047 no 87	AL-T1L 2	3	2	Shale	300	400	120000	235	debris	no	yes	yes	single	steep	covered		
AL-005	Alces	BC 86047 no 87	AL-T1R 4	3	2	Shale	200	700	140000	82	debris	no	yes	gullied	complex	steep	covered		
AL-006	Alces	BC 86047 no 87	AL-T1R 4	3	3	Shale	200	400	80000	137	no	no	yes	yes	single	steep	covered		
AL-007	Alces	BC 86047 no 87	AL 5	3	3	Shale	400	600	240000	265	no	yes	no	gullied	complex	steep	good		
AL-008	Alces	BC 86047 no 87	AL 10	3	2	Shale	400	500	200000	277	debris	yes	yes	yes	single	steep	muted		
AL-009	Alces	BC 86047 no 87	AL-T2L 1	3	3	Shale	400	500	200000	294	no	yes	yes	gullied	single	steep	covered		
AL-010	Alces	BC 86047 no 87	AL-T2R 2	3	3	Shale	400	300	120000	107	no	yes	no	yes	single	gentle	good		
AL-011	Alces	BC 86047 no 87	AL 15T16	2	4	Quaternary	600	400	240000	183	no	yes	yes	yes	single	gentle	good		
AL-012	Alces	BC 86047 no 87	AL 18	2	4	Quaternary	700	100	70000	253	no	yes	yes	yes	complex	gentle	good		
AL-013	Alces	BC 86047 no 138	AL 19	2	3	Quaternary	500	300	150000	238	no	yes	yes	gullied	complex	steep	covered		
AL-014	Alces	BC 86047 no 138	AL 23	3	6	Shale	400	600	240000	233	intact	no	yes	yes	single	steep	good		
AL-015	Alces	BC 86047 no 138	AL 24	3	3	Shale	600	1300	780000	268	no	no	yes	yes	complex	steep	covered		
AL-016	Alces	BC 86047 no 138	AL 25	2	6	Shale	600	500	300000	274	intact	yes	yes	yes	single	steep	good		
AL-017	Alces	BC 86047 no 138	AL 25	3	6	Shale	400	400	160000	273	intact	no	no	no	complex	steep	covered		
AL-018	Alces	BC 86047 no 138	AL 25	2	2	Shale	400	500	200000	280	debris	no	yes	yes	single	steep	good		
AL-019	Alces	BC 86047 no 138	AL 26	3	3	Quaternary	400	500	200000	279	no	yes	yes	no	single	gentle	vegetated		
AL-020	Alces	BC 86047 no 138	AL 29	3	6	Shale	400	400	160000	295	intact	yes	yes	yes	single	gentle	vegetated		
AL-021	Alces	BC 86047 no 147	AL 32	3	6	Shale	200	600	120000	320	intact	no	no	no	single	gentle	vegetated		
AL-022	Alces	BC 86047 no 147	AL 32	3	6	Shale	300	500	150000	263	intact	no	no	no	single	gentle	vegetated		
AL-023	Alces	BC 86047 no 147	AL 33	3	3	Quaternary	400	600	240000	304	no	no	no	no	complex	gentle	vegetated		
AL-024	Alces	BC 86047 no 147	AL 34	3	2	Quaternary	400	1100	440000	287	debris	no	no	no	single	gentle	vegetated		
AL-025	Alces	BC 86047 no 87	AL 3	2	2	Quaternary	600	200	120000	321	debris	yes	yes	gullied	single	steep	muted		
AL-026	Alces	BC 86047 no 87	AL 4	4	6	Shale	400	800	320000	215	intact	yes	no	yes	complex	steep	muted		
AL-027	Alces	BC 86047 no 87	AL 6	3	6	Shale	400	400	160000	273	intact	yes	no	yes	single	steep	vegetated		
AL-028	Alces	BC 86047 no 87	AL 8	3	6	Shale	600	600	360000	284	intact	yes	no	gullied	single	steep	vegetated		
AL-029	Alces	BC 86047 no 87	AL 9	4	6	Shale	500	300	150000	278	intact	yes	no	no	single	steep	vegetated		
AL-030	Alces	BC 86047 no 87	AL 18	3	6	Shale	400	400	200000	259	intact	no	yes	yes	single	steep	muted		
AL-031	Alces	BC 86047 no 87	AL 12	4	6	Shale	500	1500	750000	329	intact	yes	partial	yes	complex	steep	muted		
AL-032	Alces	BC 86047 no 87	AL 9	3	6	Shale	500	300	150000	239	intact	yes	no	yes	single	steep	muted		
AL-033	Alces	BC 86047 no 87	AL 4	3	5	Quaternary	600	300	180000	195	no	yes	yes	yes	complex	steep	covered		
AL-034	Alces	BC 86047 no 87	AL 4	3	6	Shale	400	600	240000	203	intact	no	no	no	complex	steep	vegetated		
AL-035	Alces	BC 86047 no 87	AL-T1L 5	4	3	Shale	200	600	120000	258	no	yes	yes	yes	single	gentle	vegetated		
AL-036	Alces	BC 86047 no 87	AL 1	4	1	Shale	400	600	240000	199	no	no	yes	yes	single	steep	good	AL-0	
AL-037	Alces	BC 86047 no 147	AL 34	3	6	Quaternary	400	1300	520000	162	intact	no	yes	no	complex	steep	vegetated	AL-25	
AL-038	Alces	BC 86047 no 147	AL 34	4	3	Quaternary	200	600	120000	311	no	no	no	no	complex	gentle	vegetated	AL-26	
AL-039	Alces	BC 86047 no 87	AL 5	3	6	Shale	400	400	160000	202	intact	no	no	yes	single	gentle	vegetated	AL-31	
AR-001	Alces	BC 86047 no 87	AR 1	3	3	Quaternary	400	1000	400000	53	no	yes	no	no	complex	gentle	vegetated		
AR-002	Alces	BC 86047 no 87	AR 2	3	3	Quaternary	500	1100	550000	24	no	yes	yes	yes	complex	gentle	vegetated		
AR-003	Alces	BC 86047 no 87	AR 5	2	6	Shale	500	600	300000	93	intact	no	no	no	complex	steep	covered		
AR-004	Alces	BC 86047 no 87	AR 7	3	2	Shale	400	600	240000	13	debris	yes	no	no	complex	steep	vegetated		
AR-005	Alces	BC 86047 no 87	AR 8	3	3	Shale	400	500	200000	41	no	yes	no	no	complex	steep	vegetated		
AR-006	Alces	BC 86047 no 87	AR 9	3	3	Shale	400	400	160000	69	no	yes	no	yes	single	steep	good		
AR-007	Alces	BC 86047 no 87	AR 10	3	3	Shale	400	300	120000	31	no	yes	no	no	single	gentle	vegetated		
AR-008	Alces	BC 86047 no 87	AR 10	3	6	Shale	500	350	175000	3	intact	no	no	yes	single	steep	vegetated		

AR-009	Alces	BC 86047 no 87	AR 14	4	6	3	Shale	600	1000	600000	49	infact	no	no	no	no	gullied	no	complex	single	gentle	steep	vegetated	good
AR-010	Alces	BC 86047 no 138	AR 17	3	3	3	Shale	300	700	210000	153	no	no	yes	yes	yes	yes	yes	single	single	gentle	steep	vegetated	good
AR-011	Alces	BC 86047 no 138	AR 19	3	3	3	Shale	200	700	140000	288	no	no	no	no	no	yes	yes	single	single	gentle	steep	vegetated	good
AR-012	Alces	BC 86047 no 138	AR 20	3	3	3	Shale	250	750	187500	109	no	no	no	no	no	yes	yes	single	single	steep	steep	covered	good
AR-013	Alces	BC 86047 no 138	AR 22	3	3	3	Shale	400	400	160000	48	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	covered	good
AR-014	Alces	BC 86047 no 138	AR 22	3	3	3	Shale	400	350	140000	38	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	covered	good
AR-015	Alces	BC 86047 no 138	AR 22	3	3	3	Shale	200	100	20000	54	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	covered	good
AR-016	Alces	BC 86047 no 138	AR-T1R 1	2	3	3	Shale	300	600	180000	289	no	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	covered	good
AR-017	Alces	BC 86047 no 138	AR-T1R 2	3	3	3	Quaternary	300	600	180000	127	no	no	yes	yes	yes	yes	yes	complex	complex	gentle	gentle	covered	good
AR-018	Alces	BC 86047 no 138	AR-T1L 3	3	3	3	Quaternary	200	500	100000	145	no	no	yes	yes	yes	yes	yes	complex	complex	gentle	gentle	covered	good
AR-019	Alces	BC 86047 no 138	AR 23	2	4	4	Quaternary	600	100	60000	49	no	no	yes	yes	yes	yes	yes	single	single	gentle	gentle	good	good
AR-020	Alces	BC 86047 no 138	AR 23	3	6	6	Shale	300	600	180000	40	infact	no	no	no	no	no	no	complex	complex	steep	steep	covered	good
AR-021	Alces	BC 86047 no 138	AR-T2R 1	3	6	6	Shale	500	1800	900000	43	infact	no	no	no	no	no	no	complex	complex	steep	steep	vegetated	good
AR-022	Alces	BC 86047 no 138	AR-T2R 4	3	6	6	Shale	300	400	120000	82	infact	no	no	no	no	no	no	complex	complex	steep	steep	vegetated	good
AR-023	Alces	BC 86047 no 138	AR-T2L 6	3	3	3	Shale	200	400	80000	220	no	no	no	no	no	gullied	no	complex	complex	steep	steep	muted	good
AR-024	Alces	BC 86047 no 138	AR-T2L 4	3	3	3	Shale	300	500	150000	234	no	no	no	no	no	yes	yes	complex	complex	steep	steep	muted	good
AR-025	Alces	BC 86047 no 138	AR-T2L 1	3	2	2	Shale	200	300	60000	203	debris	no	no	no	no	yes	yes	single	single	steep	steep	muted	good
AR-026	Alces	BC 86047 no 138	AR-T2L 1	3	3	3	Shale	200	400	80000	232	no	no	no	no	no	gullied	no	single	single	steep	steep	muted	good
AR-027	Alces	BC 86047 no 138	AR-T2L 1	3	3	3	Shale	300	400	120000	221	no	no	no	no	no	yes	yes	single	single	steep	steep	good	good
AR-028	Alces	BC 86047 no 138	AR-T2L 3	3	3	3	Shale	100	300	30000	231	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	good	good
AR-029	Alces	BC 86047 no 138	AR 26	2	6	6	Shale	300	800	240000	103	infact	no	no	no	no	yes	yes	complex	complex	steep	steep	good	good
AR-030	Alces	BC 86047 no 138	AR 26	3	3	3	Quaternary	200	500	100000	120	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	good	good
AR-031	Alces	BC 86047 no 138	AR 24	3	6	6	Shale	500	2600	1300000	91	infact	no	no	yes	yes	gullied	no	complex	complex	steep	steep	vegetated	good
AR-032	Alces	BC 86047 no 147	AR 31	4	3	3	Quaternary	300	600	180000	123	no	no	no	no	no	yes	yes	single	single	gentle	gentle	vegetated	good
AR-033	Alces	BC 86047 no 147	AR 32	5	2	2	Quaternary	300	600	180000	126	debris	no	no	yes	yes	yes	yes	single	single	steep	steep	modified	good
AR-034	Alces	BC 86047 no 147	AR 35	3	6	6	Quaternary	200	700	140000	152	infact	no	yes	yes	yes	yes	yes	single	single	gentle	gentle	vegetated	good
AR-035	Alces	BC 86047 no 147	AR 36	4	3	3	Quaternary	200	700	140000	121	no	no	yes	yes	yes	no	no	single	single	gentle	gentle	vegetated	good
AR-036	Alces	BC 86047 no 147	AR-T3R 1	3	2	2	Quaternary	200	600	120000	36	debris	no	no	no	no	no	no	single	single	gentle	gentle	vegetated	good
AR-037	Alces	BC 86047 no 147	AR 44	3	3	3	Quaternary	200	1200	240000	55	no	no	no	no	no	yes	yes	complex	complex	gentle	gentle	vegetated	good
AR-038	Alces	BC 86047 no 147	AR-T3R 1	3	2	2	Quaternary	400	600	240000	108	debris	no	no	no	no	yes	yes	single	single	gentle	gentle	vegetated	good
AR-039	Alces	BC 86047 no 147	AR-T3R 2	3	2	2	Quaternary	200	600	120000	110	debris	no	no	no	no	yes	yes	single	single	gentle	gentle	vegetated	good
AR-040	Alces	BC 86047 no 147	AR-T3R 2	3	3	3	Quaternary	200	600	120000	77	no	no	no	no	no	yes	yes	single	single	steep	steep	covered	good
AR-041	Alces	BC 86047 no 147	AR-T3R 3	3	3	3	Quaternary	100	1000	100000	77	no	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	covered	good
AR-042	Alces	BC 86047 no 147	AR 37	5	3	3	Quaternary	500	800	400000	168	no	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	muted	good
AR-043	Alces	BC 86047 no 147	AR 43	3	3	3	Quaternary	300	800	240000	7	no	no	no	no	no	no	no	single	single	gentle	gentle	vegetated	good
AR-044	Alces	BC 86047 no 147	AR 38	3	2	2	Quaternary	400	800	320000	98	debris	no	no	no	no	no	no	complex	complex	gentle	gentle	vegetated	good
AR-045	Alces	BC 86047 no 147	AR 39	2	2	2	Quaternary	200	150	30000	140	debris	no	yes	yes	yes	yes	yes	single	single	steep	steep	good	good
AR-046	Alces	BC 86047 no 147	AR 3940	3	2	2	Quaternary	300	1200	360000	138	debris	no	no	yes	yes	yes	yes	complex	complex	gentle	gentle	good	good
AR-047	Alces	BC 86047 no 10	AR 45/46	2	2	2	Quaternary	300	2800	840000	69	debris	no	no	yes	yes	yes	yes	complex	complex	gentle	gentle	vegetated	good
AR-048	Alces	BC 86047 no 87	AR 6	3	2	2	Shale	600	800	480000	61	debris	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	muted	good
AR-049	Alces	BC 86047 no 87	AR 7	3	6	6	Shale	400	600	240000	24	infact	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	muted	good
AR-050	Alces	BC 86047 no 138	AR 16	3	6	6	Shale	500	500	250000	257	infact	no	no	yes	yes	yes	yes	single	single	gentle	gentle	vegetated	good
AR-051	Alces	BC 86047 no 138	AR 18	3	6	6	Shale	400	1000	400000	32	infact	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	vegetated	good
AR-052	Alces	BC 86047 no 147	AR 30	3	3	3	Quaternary	300	500	150000	81	no	no	yes	yes	yes	yes	yes	single	single	gentle	gentle	vegetated	good
AR-053	Alces	BC 86047 no 147	AR 32	4	3	3	Quaternary	300	600	180000	36	no	no	no	no	no	yes	yes	single	single	gentle	gentle	vegetated	good
AR-054	Alces	BC 86047 no 147	AR-T3L 2	3	3	3	Quaternary	200	900	180000	18	infact	no	no	yes	yes	yes	yes	complex	complex	steep	steep	vegetated	good
AR-055	Alces	BC 86047 no 138	AR 10	3	6	6	Shale	600	400	240000	175	infact	no	no	yes	yes	yes	yes	single	single	steep	steep	covered	good
DL-001	Doig	BC 86075 no 62	DL 3	3	3	3	Shale	300	500	150000	356	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	good	good
DL-002	Doig	BC 86075 no 62	DL 8	3	3	3	Shale	250	450	112500	355	infact	no	no	no	no	yes	yes	single	single	gentle	gentle	good	good
DL-003	Doig	BC 86075 no 62	DL 8	3	6	6	Shale	400	1100	440000	345	infact	no	yes	yes	yes	yes	yes	single	single	steep	steep	vegetated	good
DL-004	Doig	BC 86075 no 62	DL 13	3	3	3	Shale	400	500	200000	18	no	no	yes	yes	yes	yes	yes	single	single	steep	steep	covered	good
DL-005	Doig	BC 86075 no 62	DL 15	3	6	6	Shale	600	900	540000	271	infact	no	no	no	no	no	no	single	single	steep	steep	vegetated	good
DL-006	Doig	BC 86075 no 62	DL 15	3	6	6	Shale	500	500	250000	273	infact	no	yes	yes	yes	yes	yes	single	single	steep	steep	vegetated	good
DL-007	Doig	BC 86075 no 62	DL 16	3	6	6	Shale	700	600	420000	313	infact	no	yes	yes	yes	yes	yes	complex	complex	steep	steep	vegetated	good

Unit	Stratigraphic Unit	Age	Depth (m)	Soil Type	Vegetation	Soil Profile	Soil Description	Soil Color	Soil Texture	Soil Structure	Soil pH	Soil Moisture	Soil Temperature	Soil Bulk Density	Soil Porosity	Soil Permeability	Soil Stability	Soil Erosion	Soil Compaction	Soil Contamination	Soil Use	Soil Notes		
DL-008	Doig	BC 86075 no 62	DL 16	Shale	1000	400	400000	118	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-009	Doig	BC 86075 no 62	DL 16	Shale	700	600	420000	137	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-010	Doig	BC 86075 no 62	DL 18	Shale	400	1500	600000	143	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-011	Siphon Ck	BC 86074 no 255	DL-T11.1	Quaternary	300	1000	300000	3	debris	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-012	Doig	BC 86074 no 255	DL 24	Shale	300	400	120000	21	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-013	Doig	BC 86074 no 255	DL 24	Shale	500	1000	500000	290	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-014	Doig	BC 86074 no 255	DL 2627	Quaternary	350	700	245000	299	debris	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-015	Doig	BC 86074 no 255	DL 27	Shale	100	200	20000	14	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-016	Doig	BC 86074 no 255	DL 29	Quaternary	400	400	160000	339	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-017	Doig	BC 86074 no 255	DL 31	Shale	150	300	45000	10	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-018	Doig	BC 86074 no 255	DL 32	Quaternary	200	400	80000	260	no	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-019	Doig	BC 86074 no 255	DL 35	Quaternary	200	600	120000	312	intact	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-020	Doig	BC 86074 no 255	DL 36	Quaternary	400	400	160000	265	intact	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-021	Doig	BC 86074 no 255	DL 1	Quaternary	400	1000	400000	318	intact	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-022	Doig	BC 86074 no 255	OR 2	Quaternary	400	400	160000	164	debris	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-023	Doig	BC 86074 no 255	OR 1	Quaternary	150	500	75000	201	intact	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-024	Doig	BC 86074 no 255	DL 37	Quaternary	200	1000	200000	196	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-025	Doig	BC 86074 no 255	DL 38	Quaternary	100	400	40000	280	debris	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DL-026	Doig	BC 86074 no 255	DL 33	Quaternary	200	1200	240000	310	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-029	Doig	BC 86075 no 62	DL 14	Shale	500	800	400000	315	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-028	Siphon Ck	BC 86074 no 255	DL-T11.1	Quaternary	300	1200	360000	98	debris	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-027	La Guarite	BC 86074 no 255	DL-T11.2	Shale	200	500	100000	357	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DL-032	Doig	BC 86075 no 62	DL 17	Quaternary	350	600	280000	151	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-001	Doig	BC 86075 no 62	DR 6	Shale	500	1000	500000	145	debris	yes	no	no	no	no	no	no	no	no	no	no	no	no	no	
DR-002	Doig	BC 86075 no 62	DR 6	Shale	600	300	180000	147	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-003	Doig	BC 86075 no 62	DR 7	Shale	600	600	480000	150	intact	yes	eroded	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-004	Doig	BC 86075 no 62	DR 8	Shale	500	1200	600000	97	intact	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DR-005	Doig	BC 86075 no 62	DR 9	Shale	400	400	160000	70	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-006	Doig	BC 86075 no 62	DR 9	Shale	500	500	250000	101	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-008	Doig	BC 86075 no 62	DR 12	Shale	500	600	300000	174	intact	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-010	Doig	BC 86074 no 255	DR 30	Shale	200	300	60000	175	intact	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-011	Doig	BC 86074 no 255	DR 37	Quaternary	100	300	30000	150	debris	no	eroded	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-012	Doig	BC 86074 no 255	DR 38	Quaternary	200	600	120000	21	no	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DR-013	Doig	BC 86074 no 255	DR 39	Quaternary	100	200	20000	177	no	no	eroded	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
DR-014	Doig	BC 86074 no 255	DR 41	Quaternary	300	1200	360000	60	no	no	no	no	no	no	no	no	no	no	no	no	no	no	no	
DR-015	Doig	BC 86075 no 62	DR 10	Quaternary	100	100000	130	no	yes	yes	no	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	
Total																			33.01	km2	33010000		m2	

Appendix IV - Landslide Inventory
Table IV - 5 Halfway River West

Failure Name	River	Photo	River Segment	Activity	Failure Type	Slite Aspect	Probable Basal Failure Material	Length (m)	Width (m)	Area m2	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Slope Angle	Feature Clarity	May Be Mapped As	Additional Comments
CL-001	Cameron	BC 86075 no 28	CL 12	3	6	260	Shale	1200	2000	2400000	Intact	no	yes	yes	complex	gentle	vegetated		
CL-002	Cameron	BC 86075 no 28	CL 34	3	6	275	Shale	500	1400	700000	Intact	no	yes	no	complex	gentle	vegetated		
CL-003	Cameron	BC 86075 no 28	CL 45	3	5	301	Quaternary	1600	2200	3520000	Intact	no	yes	no	complex	gentle	vegetated		
CL-004	Cameron	BC 86075 no 28	CL 5	4	3	212	Shale	300	600	180000	no	yes	yes	yes	single	steep	good		
CL-005	Cameron	BC 86075 no 28	CL 4	4	3	274	Shale	500	600	300000	no	yes	yes	yes	single	steep	good		
CL-006	Cameron	BC 86075 no 28	CL 5	4	3	210	Shale	600	600	360000	no	yes	yes	yes	single	steep	vegetated		
CL-007	Cameron	BC 86075 no 50	CL 7	3	3	228	Shale	1800	1200	2160000	no	yes	yes	yes	complex	steep	vegetated		
CL-008	Cameron	BC 86075 no 50	CL 8	3	2	258	Shale	1100	1000	1100000	debris	yes	yes	yes	complex	steep	vegetated		
CL-009	Cameron	BC 86075 no 50	CL 9	4	3	250	Shale	900	1200	1080000	no	yes	yes	yes	complex	gentle	vegetated		
CL-010	Cameron	BC 86075 no 50	CL-T1L 1	3	3	255	Shale	300	400	120000	no	yes	no	no	single	gentle	vegetated		
CL-011	Cameron	BC 86075 no 50	CL-T1L 1	3	6	271	Quaternary	400	2400	960000	Intact	no	yes	no	complex	steep	vegetated		
CL-012	Cameron	BC 86075 no 50	CL-T1R 1	3	6	95	Quaternary	500	2000	1000000	Intact	no	no	yes	complex	steep	vegetated		
CL-013	Cameron	BC 86075 no 50	CL-T1L 3	3	3	202	Shale	150	600	90000	no	no	yes	yes	single	steep	good		
CL-014	Cameron	BC 86075 no 50	CL-T1L 3	3	6	273	Quaternary	300	900	270000	Intact	no	yes	yes	complex	steep	vegetated		
CL-015	Cameron	BC 86075 no 50	CL-T1L 4	3	2	270	Quaternary	400	1000	400000	debris	no	yes	no	complex	gentle	vegetated		
CL-016	Cameron	BC 86075 no 50	CL-T1R 3	3	2	96	Quaternary	400	1200	480000	debris	no	yes	no	complex	gentle	vegetated		
CL-017	Cameron	BC 86075 no 50	CL-T1R 3	3	3	83	Quaternary	400	1000	400000	no	no	yes	no	complex	gentle	vegetated		
CL-018	Cameron	BC 86075 no 50	CL 11	2	2	213	Shale	800	800	640000	debris	yes	yes	yes	single	steep	good		
CL-019	Cameron	BC 86075 no 50	CL 12	2	6	279	Shale	1200	1200	1440000	Intact	yes	yes	yes	complex	steep	good		
CL-020	Cameron	BC 86075 no 50	CL 12	2	6	242	Shale	700	500	350000	Intact	yes	yes	yes	single	steep	good		
CL-021	Cameron	BC 86075 no 50	CL 13	2	3	238	Quaternary	600	400	240000	no	yes	yes	yes	single	steep	good		
CL-022	Cameron	BC 86075 no 50	CL 13	3	3	229	Shale	600	400	240000	no	no	yes	yes	single	steep	good		
CL-023	Cameron	BC 86075 no 152	CL 15	4	3	218	Quaternary	500	1600	800000	no	no	yes	yes	complex	steep	old		
CL-024	Cameron	BC 86075 no 152	CL 16	4	6	243	Quaternary	500	300	150000	Intact	yes	eroded	no	single	steep	good		
CL-025	Cameron	BC 86075 no 152	CL 16	3	2	226	Quaternary	900	500	450000	debris	yes	yes	yes	single	steep	vegetated		
CL-026	Cameron	BC 86075 no 152	CL 17	2	2	170	Shale	200	1100	220000	debris	no	yes	yes	single	steep	vegetated		
CL-027	Cameron	BC 86075 no 152	CL 19	3	3	289	Shale	150	700	105000	no	no	no	no	single	gentle	vegetated		
CL-028	Cameron	BC 86075 no 153	CL 20	4	3	256	Quaternary	300	600	180000	no	no	no	no	single	gentle	vegetated		
CL-029	Cameron	BC 86075 no 153	CL 22	4	5	185	Quaternary	1300	1200	1560000	no	no	yes	no	complex	gentle	vegetated		
CL-030	Cameron	BC 86075 no 153	CL 26	4	3	192	Quaternary	200	400	80000	no	no	yes	yes	single	steep	good		
CL-031	Cameron	BC 86075 no 28	CL 3	3	4	277	Quaternary	1600	1200	1920000	no	no	yes	no	single	gentle	good		
CL-032	Cameron	BC 86075 no 28	CL 4	4	6	149	Shale	800	1400	1120000	Intact	no	yes	yes	single	steep	good		
CL-033	Cameron	BC 86075 no 28	CL 4	4	2	196	Shale	700	500	350000	debris	no	yes	yes	single	gentle	good		
CL-034	Cameron	BC 86075 no 152	CL 14	4	3	256	Quaternary	400	1200	480000	no	no	yes	no	complex	gentle	vegetated	CL 32	
CR-001	Cameron	BC 86075 no 28	CR 1	3	6	133	Quaternary	500	600	300000	Intact	no	yes	yes	single	steep	good		
CR-002	Cameron	BC 86075 no 28	CR 2	3	6	107	Quaternary	700	800	560000	Intact	no	yes	yes	single	steep	good		
CR-003	Cameron	BC 86075 no 28	CR 3	3	6	82	Quaternary	700	800	420000	debris	no	yes	yes	single	steep	vegetated		
CR-004	Cameron	BC 86075 no 28	CR 5	3	3	40	Shale	500	800	400000	no	no	yes	yes	single	steep	ruined		
CR-005	Cameron	BC 86075 no 50	CR 7	4	3	105	Shale	300	700	210000	no	no	no	no	single	gentle	vegetated		
CR-006	Cameron	BC 86075 no 50	CR 8	4	3	73	Shale	300	800	240000	no	no	yes	yes	single	steep	vegetated		
CR-007	Cameron	BC 86075 no 50	CR 11	3	2	55	Quaternary	500	1200	600000	debris	no	no	no	complex	gentle	vegetated		
CR-008	Cameron	BC 86075 no 50	CR 16	3	2	85	Quaternary	400	1300	520000	debris	no	yes	gullied	complex	steep	vegetated		
CR-009	Cameron	BC 86075 no 50	CR 16	3	3	74	Shale	500	600	300000	no	yes	yes	no	single	steep	vegetated		
CR-010	Cameron	BC 86075 no 50	CR 17	3	3	57	Shale	300	700	210000	no	no	yes	gullied	single	steep	vegetated		
CR-011	Cameron	BC 86075 no 152	CR 20	3	6	62	Shale	400	800	320000	Intact	no	yes	gullied	single	steep	vegetated		
CR-012	Cameron	BC 86075 no 152	CR 21	3	3	33	Shale	300	1400	420000	no	no	yes	gullied	complex	gentle	vegetated		
CR-013	Cameron	BC 86075 no 152	CR 21	3	2	10	Quaternary	250	300	75000	debris	no	eroded	yes	single	steep	vegetated		
CR-014	Cameron	BC 86075 no 152	CR 22	4	2	6	Quaternary	400	400	160000	debris	no	yes	no	single	steep	vegetated		
CR-015	Cameron	BC 86075 no 152	CR 23	4	6	29	Quaternary	500	1300	650000	Intact	no	no	no	complex	gentle	vegetated		
CR-016	Cameron	BC 86075 no 153	CR 26	4	3	49	Quaternary	300	1100	330000	no	no	eroded	no	complex	gentle	vegetated		
CR-017	Cameron	BC 86075 no 153	CR 28	3	2	72	Quaternary	200	700	140000	debris	no	eroded	no	complex	gentle	vegetated		

CR-018	Cameron	BC 86075 no 153	CR 29	3	7	347	Quaternary	400	800	320000	no	yes	no	no	no	single	gentle	vegetated
CR-019	Cameron	BC 86075 no 28	CR 1	3	7	83	Quaternary	600	1800	360000	no	no	yes	yes	no	single	gentle	good
CR-020	Cameron	BC 86075 no 153	CR 25	4	3	30	Quaternary	200	360000	no	no	no	yes	no	complex	gentle	vegetated	
CR-021	Cameron	BC 86075 no 153	CR 24	4	3	69	Quaternary	300	1700	510000	no	no	no	no	complex	gentle	vegetated	
CR-022	Cameron	BC 86075 no 50	CR 9/10	3	6	60	Quaternary	400	1500	600000	intact	no	yes	no	single	steep	vegetated	
FCL-001	Farell Ck	BC 86047 no 122	FCL 9	4	3	96	Quaternary	100	300	30000	no	no	no	no	complex	gentle	vegetated	
FCL-002	Farell Ck	BC 86047 no 122	FCL 3	4	8	246	Quaternary	600	300	180000	no	yes	yes	yes	complex	single	muted	
FCL-003	Farell Ck	BC 86047 no 122	FCL 6	3	6	102	Shale	300	300	90000	intact	yes	yes	yes	single	steep	muted	
FCL-004	Farell Ck	BC 86047 no 122	FCL 18	2	3	240	Quaternary	100	500	50000	no	no	yes	no	single	gentle	muted	
FCL-005	Farell Ck	BC 86047 no 122	FCL 4	3	8	265	Quaternary	400	800	320000	no	yes	yes	yes	complex	gentle	good	
FCL-006	Farell Ck	BC 86047 no 122	FCL 2	3	8	293	Quaternary	500	1200	600000	no	yes	yes	yes	complex	gentle	vegetated	
FCL-007	Farell Ck	BC 86047 no 122	FCL 2	4	8	322	Quaternary	500	2400	1200000	no	yes	yes	gullied	complex	steep	vegetated	
FCL-008	Farell Ck	BC 86047 no 122	FCL 10	3	3	306	Shale	400	500	200000	no	yes	yes	yes	complex	steep	vegetated	
FCL-009	Farell Ck	BC 86047 no 122	FCL 11	3	5	267	Quaternary	350	300	105000	no	yes	yes	no	single	steep	vegetated	
FCL-010	Farell Ck	BC 86047 no 122	FCL 12	3	5	268	Quaternary	500	200	100000	no	yes	yes	no	single	gentle	vegetated	
FCL-011	Farell Ck	BC 86047 no 122	FCL 2	4	6	297	Shale	700	800	560000	intact	yes	yes	yes	complex	steep	vegetated	
FCL-012	Farell Ck	BC 86047 no 106	FCL 1	3	6	298	Shale	600	500	300000	intact	yes	yes	yes	complex	steep	good	
FGR-001	Farell Ck	BC 86047 no 122	FGR 5	3	6	110	Shale	500	400	200000	intact	no	yes	yes	single	steep	muted	
FGR-002	Farell Ck	BC 86047 no 122	FGR 7	4	7	124	Shale	400	1300	520000	debris	no	yes	yes	complex	steep	muted	
FGR-003	Farell Ck	BC 86047 no 122	FGR 9	4	8	130	Quaternary	600	400	240000	no	yes	yes	yes	complex	steep	good	
FGR-004	Farell Ck	BC 86047 no 122	FGR 10	2	8	65	Quaternary	300	300	90000	no	yes	yes	yes	complex	steep	good	
FGR-005	Farell Ck	BC 86047 no 122	FGR 12	3	3	117	Shale	400	900	360000	no	no	yes	gullied	single	steep	muted	
FGR-006	Farell Ck	BC 86047 no 122	FGR 12	4	3	99	Shale	500	500	250000	no	no	yes	yes	single	steep	vegetated	
FGR-007	Farell Ck	BC 86047 no 122	FGR 14	4	3	94	Quaternary	250	400	100000	no	no	yes	yes	single	steep	muted	
FGR-008	Farell Ck	BC 86047 no 122	FGR 15	2	3	125	Quaternary	200	800	160000	no	no	no	yes	single	gentle	steep	
FGR-009	Farell Ck	BC 86047 no 122	FGR 16	2	3	111	Quaternary	250	800	200000	no	no	no	no	single	gentle	muted	
FGR-010	Farell Ck	BC 86047 no 122	FGR 17	2	3	125	Quaternary	250	600	150000	no	no	no	no	single	gentle	good	
FGR-011	Farell Ck	BC 86047 no 122	FGR17L 1	4	3	159	Quaternary	200	600	120000	no	no	yes	yes	single	steep	vegetated	
FGR-012	Farell Ck	BC 86047 no 106	FGR 2	2	2	111	Quaternary	700	700	490000	debris	yes	yes	yes	single	gentle	good	
FGR-013	Farell Ck	BC 86047 no 122	FGR 2	3	6	125	Shale	800	600	480000	intact	no	yes	yes	single	steep	vegetated	
FGR-014	Farell Ck	BC 86047 no 122	FGR 2	4	2	136	Quaternary	600	800	360000	debris	no	yes	yes	single	steep	vegetated	
FGR-015	Farell Ck	BC 86047 no 122	FGR17L 1	3	6	196	Shale	400	600	240000	intact	no	yes	yes	single	steep	good	
FGR-016	Farell Ck	BC 86047 no 122	FGR 12	3	3	64	Shale	500	400	200000	no	no	yes	no	single	steep	muted	
FGR-017	Farell Ck	BC 86047 no 122	FGR 9	3	6	261	Quaternary	400	300	120000	intact	no	yes	yes	single	gentle	good	
FGR-018	Farell Ck	BC 86047 no 106	FGR 5	3	3	181	Quaternary	150	400	60000	no	yes	yes	yes	single	gentle	good	
GBL-001	Ground Birch	BC 86075 no 30	GBL 1	3	3	153	Quaternary	200	1800	360000	debris	no	yes	yes	complex	gentle	vegetated	
GBL-002	Ground Birch	BC 86075 no 30	GBL 2/3	3	6	253	Quaternary	300	800	270000	intact	no	yes	no	complex	gentle	vegetated	
GBL-003	Ground Birch	BC 86075 no 30	GBL 4	4	6	278	Quaternary	400	500	200000	intact	no	yes	no	single	gentle	vegetated	
GBL-004	Ground Birch	BC 86075 no 30	GBL 5	3	2	87	Shale	500	800	400000	debris	no	yes	yes	single	steep	vegetated	
GBL-005	Ground Birch	BC 86075 no 30	GBL 7	3	1	142	Shale	100	500	50000	no	no	no	yes	single	steep	vegetated	
GBL-006	Ground Birch	BC 86075 no 30	GBL 9	3	3	205	Shale	100	4000	40000	no	no	no	yes	single	steep	good	
GBL-007	Ground Birch	BC 86075 no 30	GBL 10	4	3	210	Shale	200	700	140000	intact	no	yes	no	complex	steep	vegetated	
GBL-008	Ground Birch	BC 86075 no 30	GBL 14	3	3	155	Shale	300	600	180000	no	no	yes	yes	single	steep	vegetated	
GBL-009	Ground Birch	BC 86075 no 30	GBL 15	3	7	143	Shale	?	?	?	no	no	no	no	complex	gentle	vegetated	
GBL-010	Ground Birch	BC 86075 no 30	GBL 4	4	6	57	Quaternary	350	900	315000	intact	no	57	no	complex	steep	vegetated	
GBL-011	Ground Birch	BC 86075 no 30	GBL 3	3	3	81	Shale	400	1100	440000	debris	no	yes	yes	single	gentle	vegetated	
GBL-012	Ground Birch	BC 86075 no 30	GBL 5	3	3	56	Shale	400	500	200000	no	no	yes	yes	single	gentle	vegetated	
GBL-013	Ground Birch	BC 86075 no 30	?	4	3	175	Quaternary	300	600	180000	no	no	yes	yes	single	gentle	vegetated	
GBL-014	Ground Birch	BC 86075 no 30	?	4	3	118	Quaternary	400	400	160000	no	no	yes	no	single	gentle	vegetated	
GBR-001	Ground Birch	BC 86075 no 30	GBR 1	3	5	335	Quaternary	200	1600	320000	debris	no	yes	no	complex	gentle	vegetated	
GBR-002	Ground Birch	BC 86075 no 30	GBR 2	3	3	270	Quaternary	400	1100	440000	no	yes	no	no	single	gentle	vegetated	
GBR-003	Ground Birch	BC 86075 no 30	GBR 5	3	6	325	Shale	400	700	280000	intact	yes	yes	gullied	single	steep	vegetated	
GBR-004	Ground Birch	BC 86075 no 30	GBR 6	3	7	293	Quaternary	150	1000	150000	debris	no	yes	no	complex	gentle	vegetated	

GBR-005	Ground Birch	BC 86075 no 30	GBR 10	3	3	101	Quaternary	100	1200	120000	debris	no	yes	no	no	no	complex	gentle	vegetated		
GBR-006	Ground Birch	BC 86075 no 30	GBR 11	3	3	60	Quaternary	300	500	150000	intact	yes	yes	yes	yes	yes	single	gentle	vegetated		
GBR-007	Ground Birch	BC 86075 no 30	GBR 12	3	3	1	Quaternary	200	400	80000	intact	yes	yes	yes	yes	yes	single	steep	good		
GBR-008	Ground Birch	BC 86075 no 30	GBR 14	2	2	330	Quaternary	300	2000	600000	debris	no	yes	no	yes	yes	complex	gentle	good		
GBR-009	Ground Birch	BC 86075 no 30	GBR 2	4	5	42	Quaternary	300	700	210000	debris	no	yes	no	yes	no	single	gentle	vegetated		
GBR-010	Ground Birch	BC 86075 no 30	GBR 11	3	3	349	Quaternary	250	300	75000	no	yes	yes	yes	no	no	single	gentle	vegetated		
GBR-011	Ground Birch	BC 86075 no 30	?	4	3	327	Quaternary	300	300	60000	no	yes	yes	yes	no	no	single	gentle	vegetated		
GBR-012	Ground Birch	BC 86075 no 30	?	3	6	7	Shale	300	700	210000	intact	yes	yes	yes	yes	no	single	steep	good		
GBR-013	Ground Birch	BC 86075 no 30	?	3	6	248	Shale	300	600	180000	intact	yes	yes	yes	yes	no	single	steep	good		
HL-001	Halfway	BC 86047 no 124	HL 3	4	6	191	Quaternary	500	500	250000	intact	yes	yes	yes	yes	yes	single	steep	good		
HL-002	Halfway	BC 86047 no 124	HL 3	4	6	188	Quaternary	400	400	160000	intact	yes	yes	yes	yes	yes	single	steep	good		
HL-003	Halfway	BC 86047 no 124	HL 3	3	6	205	Quaternary	400	400	160000	intact	yes	yes	yes	yes	yes	single	steep	muted		
HL-004	Halfway	BC 86047 no 124	HL 4	4	6	247	Quaternary	350	400	140000	intact	yes	yes	yes	yes	yes	single	steep	muted		
HL-005	Halfway	BC 86075 no 28	HL 25	3	3	225	Quaternary	200	400	80000	no	yes	yes	yes	yes	yes	single	gentle	vegetated	HL-54(2nd Rev)	
HL-006	Halfway	BC 86047 no 124	HL 6	3	6	212	Quaternary	600	600	360000	intact	no	yes	yes	yes	yes	complex	steep	good		
HL-007	Halfway	BC 86047 no 124	HL 6	3	2	166	Quaternary	400	1200	480000	debris	no	yes	yes	yes	yes	complex	steep	old		
HL-008	Halfway	BC 86047 no 124	HL 9	4	3	189	Quaternary	200	200	40000	no	yes	yes	yes	yes	yes	single	steep	muted		
HL-009	Halfway	BC 86047 no 124	HL 10	3	6	140	Quaternary	1100	1600	1760000	intact	yes	yes	yes	yes	yes	complex	steep	vegetated		
HL-010	Halfway	BC 86047 no 124	HL 11	4	6	166	Quaternary	700	1400	980000	intact	no	yes	yes	yes	yes	complex	steep	vegetated		
HL-011	Halfway	BC 86047 no 123	HL 11	3	6	162	Quaternary	1500	900	1350000	intact	yes	yes	yes	yes	yes	single	gentle	good		
HL-012	Halfway	BC 86047 no 123	HL 12	4	5	179	Quaternary	200	800	160000	no	yes	yes	yes	yes	yes	complex	steep	vegetated		
HL-013	Halfway	BC 86047 no 123	HL 14	3	6	222	Shale	500	800	400000	intact	yes	yes	yes	yes	yes	single	steep	muted		
HL-014	Halfway	BC 86047 no 123	HL 15	4	1	298	Shale	400	800	360000	no	yes	yes	yes	yes	yes	single	steep	vegetated		
HL-015	Halfway	BC 86047 no 162	HL 16	3	7	293	Shale	500	650	325000	intact	yes	yes	yes	yes	yes	single	steep	covered		
HL-016	Halfway	BC 86047 no 162	HL 16	4	3	275	Shale	400	400	160000	no	yes	yes	yes	yes	yes	single	steep	vegetated		
HL-017	Halfway	BC 86047 no 162	HL 17	4	2	254	Shale	1000	2000	2000000	debris	no	yes	yes	yes	yes	complex	steep	good		
HL-018	Halfway	BC 86047 no 162	HL 17	2	8	223	Quaternary	800	500	400000	debris	yes	yes	yes	yes	yes	single	gentle	good		
HL-019	Halfway	BC 86047 no 162	HL 19	3	8	298	Quaternary	600	600	360000	debris	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-020	Halfway	BC 86047 no 162	HL 19	4	8	241	Quaternary	300	900	270000	no	yes	yes	yes	yes	yes	complex	gentle	muted		
HL-021	Halfway	BC 86047 no 162	HL 20	2	3	289	Shale	400	1000	400000	no	yes	yes	yes	yes	yes	complex	steep	muted		
HL-022	Halfway	BC 86047 no 162	HL 20	3	6	242	Shale	900	1700	1530000	debris	no	yes	yes	yes	yes	complex	steep	muted		
HL-023	Halfway	BC 86047 no 162	HL 21	3	8	245	Quaternary	600	200	120000	intact	yes	yes	yes	yes	yes	single	gentle	good		
HL-024	Halfway	BC 86047 no 162	HL 21	4	3	243	Quaternary	600	300	180000	no	yes	yes	yes	yes	yes	single	steep	muted		
HL-025	Halfway	BC 86075 no 28	HL 21	4	3	241	Quaternary	600	1000	600000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-026	Halfway	BC 86075 no 28	HL 22	3	6	242	Shale	800	2000	1600000	intact	no	yes	yes	yes	yes	complex	steep	good		
HL-027	Halfway	BC 86075 no 28	HL 23	3	3	232	Quaternary	500	400	200000	no	yes	yes	yes	yes	yes	single	gentle	good		
HL-028	Halfway	BC 86075 no 28	HL 21	2	3	248	Quaternary	600	250	150000	no	yes	yes	yes	yes	yes	single	steep	good		
HL-029	Halfway	BC 86075 no 28	HL 21	4	3	220	Quaternary	600	750	450000	no	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-030	Halfway	BC 86075 no 28	HL 21	4	3	249	Quaternary	600	1000	600000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-031	Halfway	BC 86075 no 28	HL 22	3	6	242	Shale	800	2000	1600000	intact	no	yes	yes	yes	yes	complex	steep	good		
HL-032	Halfway	BC 86075 no 28	HL 23	3	3	200	Quaternary	400	900	360000	no	yes	yes	yes	yes	yes	complex	steep	vegetated		
HL-033	Halfway	BC 86075 no 28	HL 23	3	3	160	Shale	550	400	220000	no	yes	yes	yes	yes	yes	single	steep	vegetated		
HL-034	Halfway	BC 86075 no 28	HL 24	3	3	202	Shale	500	800	400000	no	yes	yes	yes	yes	yes	complex	steep	vegetated		
HL-035	Halfway	BC 86075 no 28	HL 24	3	6	206	Quaternary	500	450	225000	intact	yes	yes	yes	yes	yes	complex	steep	vegetated		
HL-036	Halfway	BC 86075 no 28	HL 24	3	6	213	Quaternary	600	1000	600000	intact	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-037	Halfway	BC 86075 no 28	HL 25	3	3	253	Quaternary	900	700	630000	no	yes	yes	yes	yes	yes	single	gentle	good		
HL-038	Halfway	BC 86075 no 28	HL 26	4	6	224	Quaternary	700	1000	700000	intact	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-039	Halfway	BC 86075 no 30	HL 26	4	3	191	Quaternary	350	300	105000	no	yes	yes	yes	yes	yes	single	steep	muted		
HL-040	Halfway	BC 86075 no 50	HL 28	4	5	104	Quaternary	500	400	200000	no	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-041	Halfway	BC 86075 no 50	HL 28	4	5	95	Quaternary	600	700	420000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-042	Halfway	BC 86075 no 50	HL 18	3	3	95	Quaternary	200	600	120000	no	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-043	Halfway	BC 86075 no 50	HL 29	3	3	265	Quaternary	300	1000	300000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-044	Halfway	BC 86075 no 50	HL 29	4	3	250	Quaternary	800	800	640000	debris	no	yes	yes	yes	yes	complex	gentle	vegetated		
HL-045	Halfway	BC 86075 no 50	HL 28	4	3	173	Quaternary	500	400	200000	no	yes	yes	yes	yes	yes	single	gentle	vegetated		
HL-046	Halfway	BC 86075 no 49	HL 31	4	3	210	Quaternary	500	1600	800000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-047	Halfway	BC 86075 no 49	HL 31	4	6	215	Quaternary	600	1400	840000	intact	no	yes	yes	yes	yes	single	steep	muted		
HL-048	Halfway	BC 86075 no 49	HL 32	4	5	89	Quaternary	600	2300	1380000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-049	Halfway	BC 86075 no 49	HL 32	4	5	199	Quaternary	900	900	810000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-050	Halfway	BC 86075 no 49	HL 32	4	5	89	Quaternary	600	2300	1380000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		
HL-051	Halfway	BC 86075 no 49	HL 32	4	5	199	Quaternary	900	900	810000	no	yes	yes	yes	yes	yes	complex	gentle	vegetated		

	HL-052	Hallway	BC 86075 no 154	HL 34	4	5	179	Quaternary	700	1200	840000	no debris	no	yes	yes	complex	gentle	vegetated	
	HL-053	Hallway	BC 86075 no 154	HL 35	3	5	163	Quaternary	900	1300	1170000	intact	no	yes	yes	single	steep	good	
	HL-054	Hallway	BC 86047 no 124	HR 6	4	6	190	Quaternary	500	350	175000	intact	no	yes	yes	single	steep	good	
	HL-055	Hallway	BC 86047 no 124	HL 25	4	3	267	Quaternary	600	900	540000	no	yes	no	yes	single	gentle	vegetated	
	HL-056	Hallway	BC 86047 no 124	HL 27 <td>4</td> <td>3</td> <td>249</td> <td>Quaternary</td> <td>350</td> <td>1500</td> <td>525000</td> <td>no</td> <td>yes</td> <td>yes</td> <td>yes</td> <td>complex</td> <td>gentle</td> <td>vegetated</td> <td></td>	4	3	249	Quaternary	350	1500	525000	no	yes	yes	yes	complex	gentle	vegetated	
	HL-057	Hallway	BC 86075 no 50	HL 29	4	3	234	Quaternary	400	1400	560000	no	yes	yes	no	complex	gentle	vegetated	
	HL-058	Hallway	BC 86075 no 50	HL 17	3	3	231	Quaternary	150	900	135000	no	no	yes	no	single	gentle	vegetated	
	HR-001	Hallway	BC 86047 no 124	HR 2	4	6	56	Quaternary	400	1300	520000	intact	no	yes	yes	complex	steep	vegetated	
	HR-002	Hallway	BC 86047 no 124	HR 3	2	4	72	Quaternary	600	300	180000	no	yes	yes	yes	single	steep	good	
	HR-003	Hallway	BC 86047 no 124	HR 3	3	6	60	Quaternary	500	1000	500000	intact	no	yes	gullied	complex	steep	vegetated	
	HR-004	Hallway	BC 86047 no 124	HR 4	4	6	12	Quaternary	600	600	360000	intact	no	yes	yes	single	steep	vegetated	
	HR-005	Hallway	BC 86047 no 124	HR 5	3	3	30	Quaternary	700	700	490000	no	yes	yes	no	single	gentle	vegetated	
	HR-006	Hallway	BC 86047 no 124	HR 6	3	3	78	Quaternary	600	450	270000	no	no	yes	yes	complex	steep	vegetated	
	HR-007	Hallway	BC 86047 no 124	HR 7	4	6	331	Quaternary	1000	1200	1200000	intact	yes	yes	no	complex	gentle	vegetated	
	HR-008	Hallway	BC 86047 no 124	HR 8	3	6	3	Quaternary	900	1400	1120000	intact	yes	yes	gullied	complex	steep	vegetated	
	HR-009	Hallway	BC 86047 no 123	HR 9/9	4	6	356	Quaternary	800	1400	1120000	intact	yes	yes	yes	complex	steep	vegetated	
	HR-010	Hallway	BC 86047 no 123	HR 10	3	2	308	Quaternary	550	900	495000	debris	yes	yes	yes	single	steep	vegetated	
	HR-011	Hallway	BC 86047 no 123	HR 10	2	2	19	Shale	900	700	630000	no	yes	yes	yes	single	gentle	good	upper 8's
	HR-012	Hallway	BC 86047 no 123	HR 10	3	6	51	Shale	1600	400	640000	no	no	enoded	yes	complex	gentle	vegetated	
	HR-013	Hallway	BC 86047 no 123	HR 11 <td>3</td> <td>6</td> <td>39</td> <td>Shale</td> <td>700</td> <td>1000</td> <td>700000</td> <td>debris</td> <td>no</td> <td>yes</td> <td>yes</td> <td>single</td> <td>steep</td> <td>vegetated</td> <td></td>	3	6	39	Shale	700	1000	700000	debris	no	yes	yes	single	steep	vegetated	
	HR-014	Hallway	BC 86047 no 123	HR 13 <td>3</td> <td>2</td> <td>149</td> <td>Shale</td> <td>600</td> <td>500</td> <td>300000</td> <td>debris</td> <td>no</td> <td>yes</td> <td>yes</td> <td>single</td> <td>steep</td> <td>good</td> <td></td>	3	2	149	Shale	600	500	300000	debris	no	yes	yes	single	steep	good	
	HR-015	Hallway	BC 86047 no 123	HR 14 <td>3</td> <td>2</td> <td>138</td> <td>Shale</td> <td>700</td> <td>1300</td> <td>910000</td> <td>debris</td> <td>no</td> <td>yes</td> <td>yes</td> <td>single</td> <td>steep</td> <td>good</td> <td></td>	3	2	138	Shale	700	1300	910000	debris	no	yes	yes	single	steep	good	
	HR-016	Hallway	BC 86047 no 162	HR 15	4	2	112	Shale	700	1200	840000	no	no	yes	gullied	single	steep	covered	
	HR-017	Hallway	BC 86047 no 162	HR 16/17	3	2	81	Shale	1000	4000	4000000	debris	no	partial	yes	complex	steep	vegetated	poding, long so high translational
	HR-018	Hallway	BC 86047 no 162	HR 18	3	2	104	Shale	800	500	400000	debris	yes	yes	yes	single	steep	vegetated	
	HR-019	Hallway	BC 86047 no 162	HR 19	3	2	84	Shale	900	2400	2160000	debris	no	no	yes	single	steep	vegetated	
	HR-020	Hallway	BC 86047 no 162	HR 20	2	3	62	Shale	400	1000	400000	no	yes	no	yes	single	steep	covered	
	HR-021	Hallway	BC 86075 no 28	HR 22	3	3	60	Shale	500	900	450000	no	yes	enoded	no	single	steep	muted	
	HR-022	Hallway	BC 86075 no 28	HR 22	3	3	50	Shale	500	500	250000	no	yes	enoded	yes	single	steep	muted	
	HR-023	Hallway	BC 86075 no 28	HR-TTR 1	3	7	310	Quaternary	1500	400	600000	no	no	yes	yes	complex	steep	vegetated	
	HR-024	Hallway	BC 86075 no 28	HR-TTL 1	3	3	130	Shale	1400	300	420000	debris	no	yes	yes	complex	steep	vegetated	
	HR-025	Hallway	BC 86075 no 28	HR 23	3	3	80	Shale	400	800	320000	no	yes	enoded	yes	single	steep	good	
	HR-026	Hallway	BC 86075 no 28	HR 23	3	4	76	Quaternary	700	400	280000	no	yes	enoded	no	single	gentle	vegetated	
	HR-027	Hallway	BC 86075 no 28	HR 25	4	2	40	Quaternary	700	700	480000	debris	yes	yes	yes	single	steep	vegetated	
	HR-028	Hallway	BC 86075 no 28	HR 25	4	3	14	Quaternary	600	500	300000	no	yes	yes	no	complex	steep	vegetated	
	HR-029	Hallway	BC 86075 no 28	HR 25/26	4	3	349	Quaternary	800	2000	1600000	no	yes	yes	yes	complex	steep	vegetated	
	HR-030	Hallway	BC 86075 no 28	HR 27	4	3	34	Quaternary	5000	2000	10000000	no	yes	no	no	complex	steep	vegetated	
	HR-031	Hallway	BC 86075 no 28	HR 27	3	5	23	Quaternary	800	200	160000	no	yes	yes	no	single	gentle	vegetated	
	HR-032	Hallway	BC 86075 no 30	HR 29	4	5	65	Quaternary	600	2400	1440000	no	yes	yes	no	complex	gentle	vegetated	
	HR-033	Hallway	BC 86075 no 30	HR 30	3	6	42	Shale	200	1200	240000	intact	no	yes	yes	complex	steep	muted	
	HR-034	Hallway	BC 86075 no 30	HR 30	3	6	43	Shale	400	500	200000	intact	yes	yes	yes	single	steep	covered	
	HR-035	Hallway	BC 86075 no 50	HR 30	3	3	55	Shale	400	600	240000	no	no	yes	yes	complex	steep	vegetated	
	HR-036	Hallway	BC 86075 no 50	HR 30	3	4	56	Quaternary	500	300	150000	no	yes	yes	no	single	gentle	vegetated	
	HR-037	Hallway	BC 86075 no 50	HR 34	4	3	83	Quaternary	400	1500	600000	no	no	no	no	complex	steep	vegetated	
	HR-038	Hallway	BC 86075 no 49	HR 34	4	3	29	Quaternary	400	1200	480000	no	no	no	yes	complex	steep	vegetated	
	HR-039	Hallway	BC 86075 no 49	HR 35	4	3	356	Quaternary	500	1000	500000	no	no	no	no	complex	steep	vegetated	
	HR-040	Hallway	BC 86075 no 49	HR 35	4	3	20	Quaternary	600	600	360000	no	no	no	no	complex	steep	vegetated	
	HR-041	Hallway	BC 86075 no 49	HR 36	4	3	2	Quaternary	400	900	380000	no	yes	yes	no	single	gentle	vegetated	
	HR-042	Hallway	BC 86047 no 124	HR 5	3	6	41	Quaternary	300	400	120000	intact	no	yes	yes	single	steep	covered	
	HR-043	Hallway	BC 86075 no 28	HR 28	4	3	50	Quaternary	600	1600	960000	no	yes	yes	no	complex	gentle	vegetated	
	HR-044	Hallway	BC 86075 no 49	HR 35	4	3	15	Quaternary	300	1400	420000	no	yes	no	no	complex	gentle	vegetated	
	HR-045	Hallway	BC 86075 no 49	HR 35	4	3	79	Quaternary	200	400	80000	no	yes	no	no	complex	gentle	vegetated	
	HR-046	Hallway	BC 86075 no 49	HR 37	4	3	104	Quaternary	200	1000	200000	no	yes	no	no	complex	gentle	vegetated	
	HR-047	Hallway	BC 86075 no 49	HR 39/04/1	4	3	1	Quaternary	300	3200	960000	no	yes	no	no	complex	gentle	vegetated	
	HR-048	Hallway	BC 86075 no 28	HR 24	4	3	38	Quaternary	600	1200	720000	no	no	yes	yes	complex	gentle	vegetated	
									117.05	192	116790000	m2	Total						
										Area	116.79	km2							

Appendix IV - Landslide Inventory
Table IV - 6 Halfway River to Beaton River

Failure Name	River	Photo	River Segment	Activity	Failure Type	Probable Beral Failure Material	Slide Aspect	Length (m)	Width (m)	Area m ²	Secondary Scarps	Over Scarp	Debris Apron	Defined Head Scarp	Single or Complex	Shape Angle	Feature Clarity	May Be Mapped As	Additional Comments
BBL-001	Blueberry	BC 86074 no 234	BBL 7	3	6	Quaternary	326	300	500	150000	intact	yes	no	yes	single	steep	good		
BBL-002	Blueberry	BC 86074 no 234	BBL 34	3	6	Quaternary	167	400	500	200000	intact	yes	yes	no	single	gentle	good		
BBL-003	Blueberry	BC 86074 no 234	BBL 4	3	6	Quaternary	117	300	600	180000	intact	no	yes	no	single	gentle	good		
BBL-004	Blueberry	BC 86074 no 234	BBL 5	3	6	Quaternary	193	300	400	120000	intact	no	yes	no	single	steep	good		
BBL-005	Blueberry	BC 86074 no 234	BBL 11/12	3	6	Quaternary	222	200	1000	200000	intact	no	yes	yes	complex	steep	good		
BBL-006	Blueberry	BC 86074 no 234	BBL 12	3	3	Quaternary	172	400	400	160000	no	no	no	no	single	gentle	vegetated		
BBL-007	Unbach ok	BC 86074 no 234	BBL-12L 1	3	2	Quaternary	257	400	700	280000	debris	no	yes	no	single	gentle	vegetated		
BBL-008	Unbach ok	BC 86074 no 234	BBL-12R 2	3	2	Quaternary	5	300	1000	300000	debris	no	yes	yes	complex	gentle	vegetated		
BBL-009	Blueberry	BC 86074 no 236	BBL 20	2	2	Quaternary	129	300	600	180000	debris	no	yes	yes	single	steep	good		
BBL-010	Blueberry	BC 86074 no 236	BBL 30	2	2	Quaternary	152	200	1000	200000	debris	no	yes	no	single	gentle	good		
BBL-011	Blueberry	BC 86075 no 144	BBL 58	3	3	Quaternary	194	300	600	180000	debris	no	yes	no	single	gentle	good		
BBL-012	Blueberry	BC 86075 no 144	BBL 57	3	3	Quaternary	169	200	700	140000	no	no	yes	no	single	gentle	good		
BBL-013	Blueberry	BC 86074 no 236	BBL 26	2	2	Quaternary	317	100	400	40000	debris	no	yes	yes	single	gentle	good		
BBL-014	Blueberry	BC 86075 no 142	BBL 68	4	2	Quaternary	193	100	600	60000	debris	no	yes	no	single	gentle	vegetated		
BBL-015	Blueberry	BC 86074 no 236	BBL 12	3	2	Quaternary	226	150	250	37500	no	no	no	no	single	gentle	vegetated		
BBL-016	Blueberry	BC 86074 no 236	BBL 17	3	2	Quaternary	98	200	600	120000	debris	yes	yes	yes	single	steep	good	BBL 14	
BBL-017	Blueberry	BC 86074 no 236	BBL 17	3	2	Quaternary	170	200	600	120000	debris	yes	yes	yes	single	steep	good	BBL 15	
BBR-001	Blueberry	BC 86074 no 237	BBR 31	3	3	Quaternary	342	100	700	70000	no	yes	yes	yes	complex	gentle	good		
BBR-002	Blueberry	BC 86074 no 237	BBR 32	2	2	Quaternary	100	100	80	8000	debris	no	yes	yes	single	steep	good		
BBR-003	Blueberry	BC 86074 no 237	BBR 5	3	7	Quaternary	359	400	1400	560000	intact	no	yes	no	complex	steep	vegetated		
BBR-004	Blueberry	BC 86074 no 234	BBR 6	3	6	Quaternary	147	200	600	120000	intact	no	yes	yes	single	steep	good		
BBR-005	Blueberry	BC 86074 no 234	BBR 7	3	6	Quaternary	47	400	1000	400000	intact	no	yes	yes	complex	gentle	vegetated	BBR 17	
BBR-006	Blueberry	BC 86074 no 235	BBR 3	3	7	Quaternary	22	500	1200	600000	intact	yes	yes	yes	single	steep	good		
BBR-007	Blueberry	BC 86074 no 234	BBR 10	3	3	Quaternary	280	300	600	180000	no	yes	yes	yes	single	steep	good		
BBR-008	Blueberry	BC 86074 no 234	BBR 11	3	6	Quaternary	336	300	240000	intact	no	no	yes	yes	complex	steep	good		
BBR-009	Blueberry	BC 86074 no 234	BBR 12	3	6	Quaternary	5	500	1200	600000	intact	no	yes	yes	complex	steep	good		
BBR-010	Blueberry	BC 86074 no 234	BBR 15	3	2	Quaternary	320	300	500	150000	debris	no	yes	yes	single	steep	good		
BBR-011	Blueberry	BC 86074 no 234	BBR 17	4	6	Quaternary	350	500	600	300000	intact	no	yes	no	single	gentle	vegetated		
BBR-012	Blueberry	BC 86074 no 236	BBR 19	3	7	Quaternary	308	300	600	180000	no	no	no	no	single	steep	good		
BBR-013	Blueberry	BC 86074 no 236	BBR 20	3	2	Quaternary	338	400	1100	440000	debris	no	yes	yes	complex	steep	good		
BBR-014	Blueberry	BC 86074 no 236	BBR 26	3	2	Quaternary	320	300	800	240000	debris	no	no	no	single	gentle	vegetated		
BBR-015	Blueberry	BC 86074 no 237	BBR 34	3	3	Quaternary	321	150	1000	150000	no	yes	yes	yes	single	gentle	vegetated		
BBR-016	Blueberry	BC 86074 no 237	BBR 33	3	3	Quaternary	278	100	450	45000	no	yes	yes	yes	single	gentle	vegetated		
CdL-001	Red Ok	BC 86047 no 159	RdL 2	3	6	Shale	245	500	500	250000	intact	no	yes	yes	single	steep	mulled		
CdL-002	Red Ok	BC 86047 no 159	RdL 2	3	6	Shale	269	600	500	300000	intact	no	yes	yes	single	steep	mulled		
CdL-003	Red Ok	BC 86047 no 159	RdL-T1L 1	3	6	Shale	340	600	1300	780000	intact	no	yes	yes	complex	gentle	vegetated		
CdL-004	Red Ok	BC 86047 no 159	RdL-T1R 1	3	7	Shale	187	400	1400	560000	debris	no	yes	yes	complex	steep	good		
CdL-005	Red Ok	BC 86047 no 159	RdL 6	3	6	Shale	283	500	1200	600000	intact	no	yes	no	complex	steep	covered		
CdL-006	Red Ok	BC 86047 no 159	RdL 6	3	6	Shale	222	600	3200	2560000	intact	no	yes	no	complex	gentle	vegetated		
CdL-007	Red Ok	BC 86047 no 159	RdL 6	3	6	Shale	109	800	1100	880000	intact	yes	no	yes	complex	gentle	vegetated		
CdL-008	Red Ok	BC 86047 no 159	RdL 5	3	7	Shale	102	500	1600	800000	debris	no	yes	yes	complex	steep	good		
CdL-009	Red Ok	BC 86047 no 159	RdL 5	3	2	Shale	228	400	900	360000	debris	no	yes	yes	single	steep	good		
CdL-010	Red Ok	BC 86047 no 159	RdL-T1L 1	3	2	Shale	245	300	600	180000	debris	no	yes	yes	single	steep	good		
CdL-011	Red Ok	BC 86047 no 159	RdL-T1L 2	2	2	Shale	195	550	1300	715000	intact	yes	yes	yes	complex	steep	good		
CdL-012	Cache Ok	BC 86047 no 159	CdL 6	3	6	Quaternary	182	400	600	240000	intact	yes	yes	yes	complex	steep	good		
CdL-013	Cache Ok	BC 86047 no 159	CdL 6	3	6	Quaternary	175	600	1000	600000	intact	yes	yes	yes	complex	steep	good		
CdL-014	Cache Ok	BC 86047 no 159	CdL 9	2	6	Quaternary	185	500	1100	550000	intact	yes	yes	yes	complex	steep	good		
CdL-015	Cache Ok	BC 86047 no 159	CdL 9	3	6	Quaternary	196	400	600	240000	intact	no	yes	yes	complex	steep	vegetated		
CdL-016	Cache Ok	BC 86047 no 159	CdL 13	3	6	Quaternary	130	400	400	160000	intact	no	yes	yes	single	steep	vegetated		
CdL-017	Cache Ok	BC 86047 no 159	CdL 18	3	3	Shale	157	400	1000	400000	no	no	yes	yes	complex	steep	vegetated		
CdL-018	Cache Ok	BC 86047 no 159	CdL 13	3	6	Quaternary	235	500	700	350000	intact	no	no	no	complex	gentle	vegetated		
CdL-019	Red Ok	BC 86047 no 159	RdL 6	3	6	Quaternary	300	300	600	360000	intact	no	no	no	complex	steep	vegetated		
CdL-020	Red Ok	BC 86047 no 159	RdL 7	3	6	Quaternary	99	500	900	450000	intact	no	yes	yes	complex	gentle	vegetated		
CdL-021	Red Ok	BC 86047 no 159	RdL 7	3	6	Shale	121	350	1200	420000	debris	no	yes	yes	complex	steep	vegetated		
CdL-022	N Cache Ok	BC 86047 no 159	CdL-T1L 34	4	2	Quaternary	201	300	1200	360000	debris	no	yes	yes	complex	gentle	good		
CdL-023	Cache Ok	BC 86047 no 159	CdL-30	3	2	Quaternary	289	?	?	?	debris	no	yes	yes	complex	gentle	good		
CdL-024	N Cache Ok	BC 86047 no 159	CdL-T1L 7	3	7	Shale	103	400	800	320000	debris	no	yes	yes	single	steep	good		
CdL-025	N Cache Ok	BC 86047 no 159	CdL-T1R 2	3	2	Quaternary	116	350	1400	490000	debris	no	yes	yes	single	gentle	good		
CdL-026	N Cache Ok	BC 86047 no 159	CdL-T1R 3	3	2	Quaternary	95	300	1000	300000	debris	no	yes	yes	single	gentle	good		
CdL-027	N Cache Ok	BC 86047 no 159	CdL-T1R 4	3	2	Quaternary	222	300	600	180000	no	no	yes	yes	complex	steep	good		
CdL-028	Cache Ok	BC 86047 no 159	CdL 31	3	3	Quaternary	189	300	350	105000	debris	no	yes	yes	complex	steep	good		
CdL-029	Cache Ok	BC 86047 no 159	CdL 32	3	2	Quaternary					debris	no	yes	yes	complex	steep	good		

APPENDIX V

Landslide Inventory Analysis Results

Appendix V - Inventory Analysis Results
Table V - 1 Activity (Divisions)

Sheet	Total Slides	Activity =1		Activity =2		Activity =3		Activity =4		Activity =5	
Peace River	465	9	1.9%	54	11.6%	283	60.9%	107	23.0%	12	2.6%
Beaton River	407	17	4.2%	62	15.2%	248	60.9%	77	18.9%	3	0.7%
Southern Tributaries	255	3	1.2%	29	11.4%	140	54.9%	78	30.6%	5	2.0%
Beaton River - East	137	1	0.7%	23	16.8%	97	70.8%	14	10.2%	2	1.5%
Halfway River - West	216	0	0.0%	19	8.8%	109	50.5%	88	40.7%	0	0.0%
Halfway River to Beaton River	130	0	0.0%	20	15.4%	105	80.8%	4	3.1%	1	0.8%
Total	1610	30	1.9%	207	12.9%	982	61.0%	368	22.9%	23	1.4%

Appendix V - Inventory Analysis Results
 Table V - 2 Landslide Type (Divisions)

Sheet	Total Slides	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8	
Peace River	465	2	0.5%	53	13.0%	105	25.8%	2	0.5%	1	0.2%	216	53.1%	24	5.9%	4	1.0%
Beaton River	407	3	0.6%	43	9.2%	83	17.8%	14	3.0%	12	2.6%	279	60.0%	29	6.2%	2	0.4%
Southern Tributaries	255	1	0.4%	32	12.5%	62	24.3%	9	3.5%	6	2.4%	118	46.3%	22	8.6%	5	2.0%
Beaton River - East	137	2	0.9%	27	12.5%	97	44.9%	4	1.9%	16	7.4%	53	24.5%	7	3.2%	10	4.6%
Halfway River - West	216	1	0.8%	32	24.6%	27	20.8%	1	0.8%	2	1.5%	47	36.2%	20	15.4%	0	0.0%
Halfway River to Beaton River	130	1	0.7%	23	16.8%	49	35.8%	3	2.2%	1	0.7%	56	40.9%	3	2.2%	1	0.7%
Total	1610	10	0.6%	210	13.0%	423	26.3%	33	2.0%	38	2.4%	769	47.8%	105	6.5%	22	1.4%

Appendix V - Inventory Analysis Results
Table V - 3 Landslide Distribution (Divisions)

Sheet	Total Slides	Percentage of Total	Total Kilometers Measured	Percent km's	Slide / Km ratio	Area Covered Km ²	Percent Area	Km ² / Slide Ratio
Peace River	465	28.9%	256	20.30%	1.82	258.92	33.8%	0.56
Beaumont River	407	25.3%	254	20.14%	1.60	151.98	19.9%	0.37
Southern Tributaries	255	15.8%	200	15.86%	1.28	151.62	19.8%	0.59
Beaumont River East	137	8.5%	194	15.38%	0.71	33.01	4.3%	0.24
Halfway River West	216	13.4%	263	20.86%	0.82	116.79	15.3%	0.54
Halfway River to Beaumont River	130	8.1%	94	7.45%	1.38	53.13	6.9%	0.41
Total	1610	100.00%	1261	100.00%	1.28	765.45	100.0%	0.48

Appendix V - Inventory Analysis Results
Table V - 4 Basal Failure Plane Material (Divisions)

Sheet		Total Slides	Shale		Quaternary	
Peace River	465		184	39.6%	281	60.4%
Beaton River	407		142	34.9%	265	65.1%
Southern Tributaries	255		164	64.3%	91	35.7%
Beaton River East	137		83	60.6%	54	39.4%
Halfway River West	216		74	34.3%	142	65.7%
Halfway River to Beaton River	130		56	43.1%	74	56.9%
Total		1610	703	43.7%	907	56.3%

Appendix V - Inventory Analysis Results
Table V - 5 Slide Aspect (Divisions)

Sheet	Total Slides	0-30		30-60		60-90		90-120		120-150		150-180	
Peace River	465	34	7.3%	27	5.8%	29	6.2%	36	7.7%	44	9.5%	49	10.5%
Beaton River	407	28	6.9%	43	10.6%	31	7.6%	34	8.4%	40	9.8%	35	8.6%
Southern Tributaries	255	22	8.6%	10	3.9%	11	4.3%	15	5.9%	36	14.1%	34	13.3%
Beaton River East	137	12	8.8%	15	10.9%	10	7.3%	13	9.5%	16	11.7%	10	7.3%
Halfway River West	216	17	7.9%	25	11.6%	23	10.6%	21	9.7%	14	6.5%	14	6.5%
Halfway River to Beaton River	130	12	9.2%	8	6.2%	9	6.9%	14	10.8%	6	4.6%	14	10.8%
Total	1610	125	7.8%	128	8.0%	113	7.0%	133	8.3%	156	9.7%	156	9.7%

Sheet	Total Slides	180-210		210-240		240-270		270-300		300-330		330-360	
Peace River	465	54	11.6%	49	10.5%	34	7.3%	31	6.7%	51	11.0%	27	5.8%
Beaton River	407	30	7.4%	42	10.3%	41	10.1%	29	7.1%	35	8.6%	19	4.7%
Southern Tributaries	255	22	8.6%	16	6.3%	8	3.1%	13	5.1%	34	13.3%	34	13.3%
Beaton River East	137	8	5.8%	11	8.0%	9	6.6%	18	13.1%	10	7.3%	5	3.6%
Halfway River West	216	19	8.8%	19	8.8%	32	14.8%	17	7.9%	8	3.7%	7	3.2%
Halfway River to Beaton River	130	18	13.8%	7	5.4%	17	13.1%	9	6.9%	7	5.4%	9	6.9%
Total	1610	151	9.4%	144	8.9%	141	8.8%	117	7.3%	145	9.0%	101	6.3%

Appendix V - Inventory Analysis Results

Table V - 6 Generalized Slide Aspect (Divisions)

Sheet	Total Slides	North Facing (330 - 30)		East Facing (60 - 120)		South Facing (150 - 210)		West Facing (240 - 300)	
Peace River	465	61	13.1%	43	10.6%	31	7.6%	34	8.4%
Beaton River	407	47	11.5%	27	5.8%	29	6.2%	36	7.7%
Southern Tributaries	255	56	22.0%	10	3.9%	11	4.3%	15	5.9%
Beaton River East	137	17	12.4%	8	6.2%	9	6.9%	14	10.8%
Halfway River West	216	24	11.1%	15	10.9%	10	7.3%	13	9.5%
Halfway River to Beaton River	130	21	16.2%	25	11.6%	23	10.6%	21	9.7%
Total	1610	226	14.0%	128	8.0%	113	7.0%	133	8.3%

Appendix V - Inventory Analysis Results
Table V - 7 Activity - Area (Divisions)

Sheet	Total Area (km ²)	Activity 1 - Km ²		Activity 2 - Km ²		Activity 3 - Km ²		Activity 4 - Km ²		Activity 5 - Km ²	
Peace River	258.92	1.0	0.4%	19.9	7.7%	134.4	51.9%	98.4	38.0%	5.2	2.0%
Beaton River	151.98	4.3	2.8%	19.1	12.6%	104.6	68.9%	22.8	15.0%	1.1	0.7%
Southern Tributaries	151.62	0.9	0.6%	9.8	6.4%	86.0	56.7%	52.6	34.7%	2.4	1.6%
Beaton River East	33.01	0.0	0.1%	4.3	13.0%	23.8	72.1%	4.3	13.0%	0.6	1.8%
Halfway River West	116.79	0.0	0.0%	7.0	6.0%	54.1	46.3%	55.7	47.7%	0.0	0.0%
Halfway River to Beaton River	53.13	0.0	0.0%	4.6	8.6%	45.5	85.6%	1.1	2.0%	2.0	3.8%
Total	765.45	6.3	0.8%	64.6	8.4%	448.4	58.6%	235.0	30.7%	11.2	1.5%

Appendix V - Inventory Analysis Results
Table V - 8 Landslide Type - Area (Divisions)

Sheet	Total Area Km ²	Type 1 - Km ²		Type 2 - Km ²		Type 3 - Km ²		Type 4 - Km ²		Type 5 - Km ²		Type 6 - Km ²		Type 7 - Km ²		Type 8 - Km ²	
Peace River	258.92	1.22	0.5%	13.24	5.1%	22.79	8.8%	3.98	1.5%	4.18	1.6%	173.28	66.9%	39.15	15.1%	1.09	0.42%
Beaton River	151.98	0.38	0.3%	21.75	14.3%	23.02	15.1%	0.95	0.6%	0.27	0.2%	84.59	55.7%	20.14	13.2%	0.88	0.58%
Southern Tributaries	151.62	0.36	0.2%	13.86	9.1%	18.16	12.0%	4.10	2.7%	5.58	3.7%	79.01	52.1%	26.04	17.2%	4.51	2.97%
Beaton River East	33.01	0.24	0.7%	5.50	16.7%	8.87	26.9%	0.37	1.1%	0.18	0.5%	16.85	51.0%	0.91	2.7%	0.10	0.30%
Halfway River West	116.79	0.41	0.4%	19.69	16.9%	45.11	38.6%	2.53	2.2%	12.60	10.8%	30.41	26.0%	2.28	1.9%	3.78	3.24%
Halfway River to Beaton River	53.13	0.24	0.5%	7.81	14.7%	6.47	12.2%	0.12	0.2%	0.25	0.5%	21.64	40.7%	16.60	31.2%	0.00	0.00%
Total	764.3	2.85	0.4%	81.85	10.7%	124.4	16.3%	12.05	1.6%	23.05	3.0%	405.8	53.1%	105.1	13.8%	10.36	1.36%

Appendix V - Inventory Analysis Results
 Table V-9 Activity - Landslide Type

Appendix V - Inventory Analysis Results
 Activity - Landslide Type

Sheet	Activity = 1									
	Total	Type 1		Type 2		Type 3		Type 4		Type 5
Peace River	9	0	0.0%	2	22.2%	1	11.1%	0	0.0%	0
Beaumont River	17	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0
Southern Tributaries	3	0	0.0%	1	33.3%	0	0.0%	0	0.0%	0
Beaumont River - East	1	0	0.0%	1	100.0%	0	0.0%	0	0.0%	0
Halfway River - West	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0
Halfway River to Beaumont River	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0
Total	30	0	0.0%	4	13.3%	1	3.3%	0	0.0%	0

Sheet	Activity = 2									
	Total	Type 1		Type 2		Type 3		Type 4		Type 5
Peace River	54	0	0.0%	10	18.5%	12	22.2%	2	3.7%	3
Beaumont River	62	0	0.0%	12	19.4%	9	14.5%	1	1.6%	0
Southern Tributaries	29	0	0.0%	9	31.0%	8	27.6%	1	3.4%	0
Beaumont River - East	23	0	0.0%	4	17.4%	4	17.4%	3	13.0%	0
Halfway River - West	19	0	0.0%	5	26.3%	9	47.4%	1	5.3%	0
Halfway River to Beaumont River	20	1	5.0%	9	45.0%	1	5.0%	0	0.0%	2
Total	207	1	0.5%	49	23.7%	43	20.8%	8	3.9%	5

Sheet	Activity = 3									
	Total	Type 1		Type 2		Type 3		Type 4		Type 5
Peace River	263	1	0.4%	23	8.1%	55	19.4%	12	4.2%	5
Beaumont River	248	0	0.0%	31	12.5%	66	26.6%	1	0.4%	1
Southern Tributaries	140	0	0.0%	16	11.4%	29	20.7%	7	5.0%	3
Beaumont River - East	97	0	0.0%	17	17.5%	39	40.2%	0	0.0%	1
Halfway River - West	109	1	0.9%	16	14.7%	39	35.8%	3	2.8%	5
Halfway River to Beaumont River	105	0	0.0%	21	20.0%	25	23.8%	1	1.0%	0
Total	982	2	0.2%	124	12.6%	253	25.8%	24	2.4%	15

Appendix V - Inventory Analysis Result
Activity - Landslide Type

Sheet	Activity =4																
	Total	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8	
Peace River	107	2	1.9%	8	7.5%	13	12.1%	0	0.0%	3	2.8%	75	70.1%	6	5.6%	0	0.0%
Beaumont River	77	2	2.6%	10	13.0%	30	39.0%	0	0.0%	0	0.0%	30	39.0%	4	5.2%	1	1.3%
Southern Tributaries	78	1	1.3%	6	7.7%	25	32.1%	1	1.3%	2	2.6%	33	42.3%	10	12.8%	0	0.0%
Beaumont River - East	14	1	7.1%	0	0.0%	5	35.7%	0	0.0%	0	0.0%	7	50.0%	1	7.1%	0	0.0%
Halfway River - West	88	1	1.1%	6	6.8%	49	55.7%	0	0.0%	11	12.5%	16	18.2%	1	1.1%	4	4.5%
Halfway River to Beaumont River	4	0	0.0%	2	50.0%	1	25.0%	0	0.0%	0	0.0%	1	25.0%	0	0.0%	0	0.0%
Total	368	7	1.9%	32	8.7%	123	33.4%	1	0.3%	16	4.3%	162	44.0%	22	6.0%	5	1.4%

Sheet	Activity =5																
	Total	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8	
Peace River	12	0	0.0%	0	0.0%	2	16.7%	0	0.0%	1	8.3%	9	75.0%	0	0.0%	0	0.0%
Beaeton River	3	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	3	100.0%	0	0.0%	0	0.0%
Southern Tributaries	5	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	5	100.0%	0	0.0%	0	0.0%
Beaeton River - East	2	0	0.0%	1	50.0%	1	50.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Halfway River - West	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Halfway River to Beaeton River	1	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Total	23	0	0.0%	1	4.3%	3	13.0%	0	0.0%	1	4.3%	17	73.9%	1	4.3%	0	0.0%

Appendix V - Inventory Analysis Results

Sheet	Activity =1						Activity =2						Activity =3						Activity =4						Activity =5					
	Shale		Quaternary		Total		Shale		Quaternary		Total		Shale		Quaternary		Total		Shale		Quaternary		Total		Shale		Quaternary		Total	
Peace River	9	4	44.4%	5	55.6%	54	26	48.1%	28	51.9%	283	116	41.0%	167	59.0%	107	35	32.7%	72	67.3%	12	3	25.0%	9	75.0%					
Beaton River	17	3	17.6%	14	82.4%	62	23	37.1%	39	62.9%	248	96	38.7%	152	61.3%	77	23	29.3%	54	70.1%	3	2	66.7%	1	33.3%					
Southern Tributaries	3	1	33.3%	2	66.7%	29	13	44.8%	16	55.2%	140	93	66.4%	47	33.6%	78	53	67.9%	25	32.1%	5	4	80.0%	1	20.0%					
Beaton River - East	1	0	0.0%	1	100.0%	23	12	52.2%	11	47.8%	97	63	64.9%	34	35.1%	14	8	57.1%	6	42.9%	2	0	0.0%	2	100.0%					
Halfway River - West	0	0	0.0%	0	0.0%	19	7	36.8%	12	63.2%	109	49	45.0%	60	55.0%	88	18	20.5%	70	79.5%	0	0	0.0%	0	0.0%					
Halfway River to Beaton River	0	0	0.0%	0	0.0%	20	9	45.0%	11	55.0%	105	46	43.8%	59	56.2%	4	0	0.0%	4	100.0%	1	1	100.0%	0	0.0%					
Total	30	8	26.7%	22	73.3%	207	90	43.5%	117	56.5%	982	463	47.1%	519	52.9%	368	137	37.2%	231	62.8%	23	10	43.5%	13	56.5%					

Appendix V - Inventory Analysis Results
Table V - 11 Activity - Aspect

Sheet	Activity 1										Activity 2									
	Total	North (330-30)		(East) 60-120		(South) 150-210		West (240-300)		Total	North (330-30)		(East) 60-120		(South) 150-210		West (240-300)			
Peace River	9	3	33.3%	2	22.2%	3	33.3%	2	22.2%	54	4	7.4%	11	20.4%	11	20.4%	10	18.5%		
Beaeton River	17	4	23.5%	2	11.8%	0	0.0%	2	11.8%	62	11	17.7%	9	14.5%	12	19.4%	8	12.9%		
Southern Tributaries	3	2	66.7%	0	0.0%	0	0.0%	0	0.0%	29	6	20.7%	6	20.7%	5	17.2%	3	10.3%		
Beaeton River - East	1	0	0.0%	0	0.0%	0	0.0%	0	0.0%	23	4	17.4%	5	21.7%	3	13.0%	5	21.7%		
Halfway River - West	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	19	1	5.3%	5	26.3%	1	5.3%	4	21.1%		
Halfway River to Beaeton River	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	20	2	10.0%	2	10.0%	5	25.0%	4	20.0%		
Total	30	9	30.0%	4	13.3%	3	10.0%	4	13.3%	207	28	13.5%	38	18.4%	37	17.9%	34	16.4%		

Sheet	Activity 3										Activity 4									
	Total	North (330-30)		(East) 60-120		(South) 150-210		West (240-300)		Total	North (330-30)		(East) 60-120		(South) 150-210		West (240-300)			
Peace River	283	30	10.6%	47	16.6%	61	21.6%	45	15.9%	107	21	19.6%	4	3.7%	26	24.3%	8	7.5%		
Beaumont River	248	32	12.9%	40	16.1%	42	16.9%	45	18.1%	77	3	3.9%	16	20.8%	10	13.0%	16	20.8%		
Southern Tributaries	140	28	20.0%	16	11.4%	33	23.6%	14	10.0%	78	19	24.4%	4	5.1%	15	19.2%	4	5.1%		
Beaumont River - East	97	12	12.4%	17	17.5%	13	13.4%	18	18.6%	14	1	7.1%	1	7.1%	1	7.1%	4	28.6%		
Halfway River - West	109	9	8.3%	24	22.0%	14	12.8%	25	22.9%	88	14	15.9%	15	17.0%	18	20.5%	20	22.7%		
Halfway River to Beaumont River	105	17	16.2%	21	20.0%	26	24.8%	21	20.0%	4	1	25.0%	0	0.0%	1	25.0%	1	25.0%		
Total	982	128	13.0%	165	16.8%	189	19.2%	168	17.1%	368	59	16.0%	40	10.9%	71	19.3%	53	14.4%		

Sheet	Activity 5									
	Total	North (330-30)		(East) 60-120		(South) 150-210		West (240-300)		
Peace River	12	1	8.3%	1	8.3%	0	0.0%	5	41.7%	
Beaumont River	3	0	0.0%	0	0.0%	2	66.7%	0	0.0%	
Southern Tributaries	5	0	0.0%	0	3.0%	3	60.0%	0	0.0%	
Beaumont River - East	2	0	0.0%	0	0.0%	1	50.0%	0	0.0%	
Halfway River - West	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	
Halfway River to Beaumont River	1	1	100.0%	0	0.0%	0	0.0%	0	0.0%	
Total	23	2	8.7%	1	4.3%	6	26.1%	5	21.7%	

Appendix V - Inventory Analysis Results
Table V - 12 Landslide Type - Activity

Sheet	Landslide Type 1										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	3	0	0.0%	0	0.0%	1	33.3%	2	66.7%	0	0.0%
Beaton River	2	0	0.0%	0	0.0%	0	0.0%	2	100.0%	0	0.0%
Southern Tributaries	1	0	0.0%	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Beaton River - East	1	0	0.0%	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Halfway River - West	2	0	0.0%	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Halfway River to Beaton River	1	0	0.0%	1	100.0%	0	0.0%	0	0.0%	0	0.0%
Total	10	0	0.0%	1	10.0%	2	20.0%	7	70.0%	0	0.0%

Sheet	Landslide Type 2										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	43	2	4.7%	10	23.3%	23	53.5%	8	18.6%		
Beaton River	53	0	0.0%	12	22.6%	31	58.5%	10	18.9%	0	0.0%
Southern Tributaries	32	1	3.1%	9	28.1%	16	50.0%	6	18.8%	0	0.0%
Beaton River - East	23	1	4.3%	4	17.4%	17	73.9%	0	0.0%	1	4.3%
Halfway River - West	27	0	0.0%	5	18.5%	16	59.3%	6	22.2%	0	0.0%
Halfway River to Beaton River	32	0	0.0%	9	28.1%	21	65.6%	2	6.3%	0	0.0%
Total	210	4	1.9%	49	23.3%	124	59.0%	32	15.2%	1	0.5%

Sheet	Landslide Type 3										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	83	1	1.2%	12	14.5%	55	66.3%	13	15.7%		
Beaton River	105	0	0.0%	9	8.6%	66	62.9%	30	28.6%	2	2.4%
Southern Tributaries	62	0	0.0%	8	12.9%	29	46.8%	25	40.3%	0	0.0%
Beaton River - East	49	0	0.0%	4	8.2%	39	79.6%	5	10.2%	0	0.0%
Halfway River - West	97	0	0.0%	9	9.3%	39	40.2%	49	50.5%	1	2.0%
Halfway River to Beaton River	27	0	0.0%	1	3.7%	25	92.6%	1	3.7%	0	0.0%
Total	423	1	0.2%	43	10.2%	253	59.8%	123	29.1%	3	0.7%

Sheet	Landslide Type 4										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	14	0	0.0%	2	14.3%	12	85.7%				
Beaton River	2	0	0.0%	1	50.0%	1	50.0%	0	0.0%	0	0.0%
Southern Tributaries	9	0	0.0%	1	11.1%	7	77.8%	0	0.0%	0	0.0%
Beaton River - East	3	0	0.0%	3	100.0%	0	0.0%	1	11.1%	0	0.0%
Halfway River - West	4	0	0.0%	1	25.0%	0	0.0%	0	0.0%	0	0.0%
Halfway River to Beaton River	1	0	0.0%	1	25.0%	3	75.0%	0	0.0%	0	0.0%
				0	0.0%	1	100.0%	0	0.0%	0	0.0%
Total	33	0	0.0%	8	24.2%	24	72.7%	1	3.0%	0	0.0%

Sheet	Landslide Type 5										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	12	0	0.0%	3	25.0%	5	41.7%	3	25.0%	1	8.3%
Beaton River	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%	0	0.0%
Southern Tributaries	6	1	16.7%	0	0.0%	3	50.0%	2	33.3%	0	0.0%
Beaton River - East	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%	0	0.0%
Halfway River - West	16	0	0.0%	0	0.0%	5	31.3%	11	68.8%	0	0.0%
Halfway River to Beaton River	2	0	0.0%	2	100.0%	0	0.0%	0	0.0%	0	0.0%
Total	38	1	2.6%	5	13.2%	15	39.5%	16	42.1%	1	2.6%

Sheet	Landslide Type 6										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	279	5	1.8%	25	9.0%	165	59.1%	75	26.9%	9	3.2%
Beaton River	216	16	7.4%	37	17.1%	130	60.2%	30	13.9%	3	1.4%
Southern Tributaries	118	1	0.8%	10	8.5%	69	58.5%	33	28.0%	5	4.2%
Beaton River - East	56	0	0.0%	9	16.1%	40	71.4%	7	12.5%	0	0.0%
Halfway River - West	53	0	0.0%	2	3.8%	35	66.0%	16	30.2%	0	0.0%
Halfway River to Beaton River	47	0	0.0%	6	12.8%	40	85.1%	1	2.1%	0	0.0%
Total	769	22	2.9%	89	11.6%	479	62.3%	162	21.1%	17	2.2%

Sheet	Landslide Type 7										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	29	1	3.4%	2	6.9%	20	69.0%	6	20.7%	0	0.0%
Beaton River	24	1	4.2%	2	8.3%	17	70.8%	4	16.7%	0	0.0%
Southern Tributaries	22	0	0.0%	1	4.5%	11	50.0%	10	45.5%	0	0.0%
Beaton River - East	3	0	0.0%	2	66.7%	0	0.0%	1	33.3%	0	0.0%
Halfway River - West	7	0	0.0%	0	0.0%	6	85.7%	1	14.3%	0	0.0%
Halfway River to Beaton River	20	0	0.0%	1	5.0%	18	90.0%	0	0.0%	1	5.0%
Total	105	2	1.9%	8	7.6%	72	68.6%	22	21.0%	1	1.0%

Sheet	Landslide Type 8										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	2	0	0.0%	0	0.0%	2	100.0%	0	0.0%	0	0.0%
Beaton River	4	0	0.0%	1	25.0%	2	50.0%	1	25.0%	0	0.0%
Southern Tributaries	5	0	0.0%	0	0.0%	5	100.0%	0	0.0%	0	0.0%
Beaton River - East	1	0	0.0%	1	100.0%	0	0.0%	0	0.0%	0	0.0%
Halfway River - West	10	0	0.0%	2	20.0%	4	40.0%	4	40.0%	0	0.0%
Halfway River to Beaton River	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Total	22	0	0.0%	4	18.2%	13	59.1%	5	22.7%	0	0.0%

Appendix V - Inventory Analysis Results
 Table V-13 Type - Material

Sheet	Type = 1				Type = 2				Type = 3				Type = 4			
	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total
Peace River	3	0	0.0%	3	43	17	39.5%	26	83	23	27.7%	60	14	0	0.0%	14
Beaton River	2	2	100.0%	2	53	23	43.4%	30	105	25	23.8%	80	2	0	0.0%	82
Southern Tributaries	1	0	0.0%	1	32	18	56.3%	14	62	28	45.2%	34	9	0	0.0%	43
Beaton River - East	1	1	100.0%	1	23	10	43.5%	13	49	26	53.1%	23	3	0	0.0%	26
Halfway River - West	2	2	100.0%	2	27	13	48.1%	14	97	36	37.1%	61	4	0	0.0%	65
Halfway River to Beaton River	1	1	100.0%	1	32	6	18.8%	26	27	11	40.7%	16	1	0	0.0%	17
Total	10	6	60.0%	4	210	87	41.4%	123	423	149	35.2%	274	33	0	0.0%	367

Sheet	Type = 5				Type = 6				Type = 7				Type = 8			
	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total	Total	Shale	Quaternary	Total
Peace River	12	0	0.0%	12	279	121	43.4%	158	28	23	79.3%	6	2	0	0.0%	8
Beaton River	1	0	0.0%	1	216	82	38.0%	134	24	13	54.2%	11	4	0	0.0%	15
Southern Tributaries	6	0	0.0%	6	118	107	90.7%	11	22	11	50.0%	11	5	0	0.0%	16
Beaton River - East	1	0	0.0%	1	56	46	82.1%	10	3	0	0.0%	3	1	0	0.0%	4
Halfway River - West	16	0	0.0%	16	53	20	37.7%	33	7	3	42.9%	4	1	0	0.0%	5
Halfway River to Beaton River	2	0	0.0%	2	47	23	48.9%	24	20	15	75.0%	5	0	0	0.0%	25
Total	38	0	0.0%	38	769	399	51.9%	370	105	65	61.9%	40	22	0	0.0%	67

Appendix V - Inventory Analysis Results
Table V -14 Landslide Type - Aspect

Sheet	Landslide Type 1								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Beaton River	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Southern Tributaries	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Beaton River - East	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Halfway River - West	1	0	0.0%	0	0.0%	0	0.0%	1	100.0%
Halfway River to Beaton River	1	1	100.0%	0	0.0%	0	0.0%	0	0.0%
Total	5	1	20.0%	0	0.0%	3	60.0%	1	20.0%

Sheet	Landslide Type 2								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	27	6	22.2%	4	14.8%	7	25.9%		
Beaton River	36	5	13.9%	8	22.2%	14	38.9%	10	37.0%
Southern Tributaries	24	7	29.2%	7	29.2%	5	20.8%	9	25.0%
Beaton River - East	18	1	5.6%	7	38.9%	2	11.1%	5	20.8%
Halfway River - West	18	3	16.7%	9	50.0%	3	16.7%	8	44.4%
Halfway River to Beaton River	23	3	13.0%	6	26.1%	3	16.7%	3	16.7%
						8	34.8%	6	26.1%
Total	146	25	17.1%	41	28.1%	39	26.7%	41	28.1%

Sheet	Landslide Type 3								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	56	4	7.1%	19	33.9%	19	33.9%		
Beaton River	72	9	12.5%	18	25.0%	22	30.6%	14	25.0%
Southern Tributaries	37	9	24.3%	3	8.1%	21	56.8%	23	31.9%
Beaton River - East	26	7	26.9%	7	26.9%	4	15.4%	4	10.8%
Halfway River - West	68	12	17.6%	21	30.9%	16	23.5%	8	30.8%
Halfway River to Beaton River	20	4	20.0%	4	20.0%	6	30.0%	19	27.9%
								6	30.0%
Total	279	45	16.1%	72	25.8%	88	31.5%	74	26.5%

Sheet	Landslide Type 4								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	10	3	30.0%	0	0.0%	6	60.0%	1	10.0%
Beaton River	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Southern Tributaries	5	2	40.0%	1	20.0%	2	40.0%	0	0.0%
Beaton River - East	2	0	0.0%	0	0.0%	1	50.0%	1	50.0%
Halfway River - West	3	0	0.0%	2	66.7%	0	0.0%	1	33.3%
Halfway River to Beaton River	1	0	0.0%	1	100.0%	0	0.0%	0	0.0%
Total	21	5	23.8%	4	19.0%	9	42.9%	3	14.3%

Sheet	Landslide Type 5								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	3	1	33.3%	0	0.0%	1	33.3%	1	33.3%
Beaton River	1	0	0.0%	1	100.0%	0	0.0%	0	0.0%
Southern Tributaries	3	2	66.7%	0	3.0%	0	0.0%	1	33.3%
Beaton River - East	1	0	0.0%	0	0.0%	1	100.0%	0	0.0%
Halfway River - West	14	2	0.0%	4	0.0%	6	0.0%	2	0.0%
Halfway River to Beaton River	1	0	0.0%	0	0.0%	0	0.0%	1	100.0%
Total	23	5	21.7%	5	21.7%	8	34.8%	5	21.7%

Sheet	Landslide Type 6								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	177	36	20.3%	40	22.6%	65	36.7%	36	20.3%
Beaton River	128	27	21.1%	35	27.3%	30	23.4%	36	28.1%
Southern Tributaries	73	28	38.4%	12	16.4%	25	34.2%	8	11.0%
Beaton River - East	38	8	21.1%	8	21.1%	9	23.7%	13	34.2%
Halfway River - West	35	6	17.1%	6	17.1%	8	22.9%	15	42.9%
Halfway River to Beaton River	41	6	14.6%	7	17.1%	16	39.0%	12	29.3%
Total	492	111	22.6%	108	22.0%	153	31.1%	120	24.4%

Sheet	Landslide Type 7								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	16	8	50.0%	1	6.3%	4	25.0%	3	18.8%
Beaton River	16	9	56.3%	5	31.3%	1	6.3%	1	6.3%
Southern Tributaries	12	7	58.3%	2	16.7%	0	0.0%	3	25.0%
Beaton River - East	3	1	33.3%	1	33.3%	0	0.0%	1	33.3%
Halfway River - West	4	1	25.0%	1	25.0%	0	0.0%	2	50.0%
Halfway River to Beaton River	15	7	46.7%	5	33.3%	2	13.3%	1	6.7%
Total	66	33	50.0%	15	22.7%	7	10.6%	11	16.7%

Sheet	Landslide Type 8								
	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	2	1	50.0%	1	50.0%	0	0.0%	0	0.0%
Beaton River	1	0	0.0%	0	0.0%	0	0.0%	1	100.0%
Southern Tributaries	4	0	0.0%	1	25.0%	3	75.0%	0	0.0%
Beaton River - East	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Halfway River - West	7	0	0.0%	1	14.3%	0	0.0%	6	85.7%
Halfway River to Beaton River	0	0	0.0%	0	0.0%	0	0.0%	0	0.0%
Total	14	1	7.1%	3	21.4%	3	21.4%	7	50.0%

Appendix V - Inventory Analysis Results
Table V -15 Material - Activity

Sheet	Material	Total	Activity 1	Activity 2	Activity 3	Activity 4	Activity 5
Peace River	Shale	184	2.2%	14.1%	63.0%	19.0%	1.6%
	Quaternary	281	1.8%	10.0%	59.4%	25.6%	3.2%
Beaton River	Shale	142	2.0%	15.6%	65.3%	15.6%	1.4%
	Quaternary	265	5.4%	0.2%	58.5%	20.8%	0.4%
Southern Tributaries	Shale	164	0.6%	7.9%	56.7%	32.3%	2.4%
	Quaternary	91	2.2%	17.6%	51.6%	27.5%	1.1%
Beaton River East	Shale	83	0.0%	14.5%	75.9%	9.6%	0.0%
	Quaternary	54	1.9%	20.4%	63.0%	11.1%	3.7%
Halfway River West	Shale	74	0.0%	9.5%	66.2%	24.3%	0.0%
	Quaternary	142	0.0%	8.5%	42.3%	49.3%	0.0%
Halfway River to Beaton River	Shale	56	0.0%	16.1%	82.1%	0.0%	1.8%
	Quaternary	74	0.0%	14.9%	79.7%	5.4%	0.0%
Total	Shale	703	1.1%	12.8%	65.9%	19.5%	1.4%
	Quaternary	907	2.4%	13.0%	57.5%	25.6%	1.4%

Appendix V - Inventory Analysis Results
Table V -16 Material - Landslide Type

Sheet	Material	Total	Type 1	Type 2	Type 3	Type 4	Type 5	Type 6	Type 7	Type 8
Peace River	Shale	184	0.0%	9.2%	12.5%	0.0%	0.0%	65.8%	12.5%	0.0%
	Quaternary	281	1.1%	9.3%	21.4%	5.0%	4.3%	56.2%	2.1%	0.7%
Beaton River	Shale	142	1.4%	16.2%	16.9%	0.0%	0.0%	56.3%	9.2%	0.0%
	Quaternary	265	0.0%	11.6%	30.6%	0.8%	0.4%	50.8%	4.3%	1.6%
Southern Tributaries	Shale	164	0.0%	11.0%	17.1%	0.0%	0.0%	65.2%	6.7%	0.0%
	Quaternary	91	1.1%	15.4%	37.4%	9.9%	6.6%	12.1%	12.1%	5.5%
Beaton River East	Shale	83	1.2%	12.0%	31.3%	0.0%	0.0%	55.4%	0.0%	0.0%
	Quaternary	54	0.0%	24.1%	42.6%	5.6%	1.9%	18.5%	5.6%	1.9%
Halfway River West	Shale	74	2.7%	17.6%	48.6%	0.0%	0.0%	27.0%	4.1%	0.0%
	Quaternary	142	0.0%	9.9%	43.0%	2.8%	11.3%	23.2%	2.8%	7.0%
Halfway River to Beaton River	Shale	56	1.8%	10.7%	19.6%	0.0%	0.0%	41.1%	26.8%	0.0%
	Quaternary	74	0.0%	35.1%	21.6%	1.4%	2.7%	32.4%	6.8%	0.0%
Total	Shale	703	0.9%	12.4%	21.1%	0.0%	0.0%	56.5%	9.2%	0.0%
	Quaternary	907	0.4%	13.6%	30.3%	3.7%	4.2%	40.7%	4.4%	2.4%

Appendix V - Inventory Analysis Results
Table V - 17 Material - Aspect

Sheet	Material	Total	North Facing (330-30)		East Facing (60-120)		South Facing (150-210)		West Facing (240-300)	
Peace River	Shale	184	28	15.2%	23	12.50%	38	20.7%	29	15.8%
	Quaternary	281	31	11.0%	42	14.95%	65	23.1%	36	12.8%
Beatton River	Shale	142	13	9.2%	24	16.90%	22	15.5%	30	21.1%
	Quaternary	265	34	12.8%	41	15.47%	43	16.2%	40	15.1%
Southern Tributaries	Shale	164	35	21.3%	16	9.76%	37	22.6%	14	8.5%
	Quaternary	91	20	22.0%	10	10.99%	19	20.9%	7	7.7%
Beatton River East	Shale	83	11	13.3%	14	16.87%	8	9.6%	20	24.1%
	Quaternary	54	6	11.1%	9	16.67%	10	18.5%	9	16.7%
Halfway River West	Shale	74	2	2.7%	17	22.97%	10	13.5%	19	25.7%
	Quaternary	142	22	15.5%	27	19.01%	23	16.2%	30	21.1%
Halfway River to Beatton River	Shale	56	8	14.3%	11	19.64%	14	25.0%	14	25.0%
	Quaternary	74	13	17.6%	12	16.22%	18	24.3%	12	16.2%
Total	Shale	703	97	13.8%	105	14.94%	129	18.3%	126	17.9%
	Quaternary	907	126	13.9%	141	15.55%	178	19.6%	134	14.8%

Appendix V - Inventory Analysis Results
Table V - 18 Aspect - Activity

Sheet	North Facing (330-30)										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	59	3	5.1%	4	6.8%	30	50.8%	21	35.6%	1	1.7%
Beaton River	50	4	8.0%	11	22.0%	32	64.0%	3	6.0%	0	0.0%
Southern Tributaries	55	2	3.6%	6	10.9%	28	50.9%	19	34.5%	0	0.0%
Beaton River - East	17	0	0.0%	4	23.5%	12	70.6%	1	5.9%	0	0.0%
Halfway River - West	24	0	0.0%	1	4.2%	9	37.5%	14	58.3%	0	0.0%
Halfway River to Beaton River	21	0	0.0%	2	9.5%	17	81.0%	1	4.8%	1	4.8%
Total	226	9	4.0%	28	12.4%	128	56.6%	59	26.1%	2	0.9%

Sheet	East Facing (60-120)										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	65	2	3.1%	11	16.9%	47	72.3%	4	6.2%	1	1.5%
Beaton River	67	2	3.0%	9	13.4%	40	59.7%	16	23.9%	0	0.0%
Southern Tributaries	26	0	0.0%	6	23.1%	16	61.5%	4	15.4%	0	0.0%
Beaton River - East	23	0	0.0%	5	21.7%	17	73.9%	1	4.3%	0	0.0%
Halfway River - West	44	0	0.0%	5	11.4%	24	54.5%	15	34.1%	0	0.0%
Halfway River to Beaton River	23	0	0.0%	2	8.7%	21	91.3%	0	0.0%	0	0.0%
Total	248	4	1.6%	38	15.3%	165	66.5%	40	16.1%	1	0.4%

Sheet	South Facing (150-210)										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	101	3	3.0%	11	10.9%	61	60.4%	26	25.7%		
Beaton River	66	0	0.0%	12	18.2%	42	63.6%	10	15.2%	0	0.0%
Southern Tributaries	56	0	0.0%	5	8.9%	33	58.9%	15	26.8%	2	3.0%
Beaton River - East	18	0	0.0%	3	16.7%	13	72.2%	1	5.6%	3	5.4%
Halfway River - West	33	0	0.0%	1	3.0%	14	42.4%	1	5.6%	1	5.6%
Halfway River to Beaton River	32	0	0.0%	5	15.6%	26	81.3%	18	54.5%	0	0.0%
								1	3.1%	0	0.0%
Total	306	3	1.0%	37	12.1%	189	61.8%	71	23.2%	6	2.0%

Sheet	West Facing (240-300)										
	Total	Activity 1		Activity 2		Activity 3		Activity 4		Activity 5	
Peace River	70	2	2.9%	10	14.3%	45	64.3%	8	11.4%	5	7.1%
Beaton River	71	2	2.8%	8	11.3%	45	63.4%	16	22.5%	0	0.0%
Southern Tributaries	21	0	0.0%	3	14.3%	14	66.7%	4	19.0%	0	0.0%
Beaton River - East	27	0	0.0%	5	18.5%	18	66.7%	4	14.8%	0	0.0%
Halfway River - West	49	0	0.0%	4	8.2%	25	51.0%	20	40.8%	0	0.0%
Halfway River to Beaton River	26	0	0.0%	4	15.4%	21	80.8%	1	3.8%	0	0.0%
Total	264	4	1.5%	34	12.9%	168	63.6%	53	20.1%	5	1.9%

Appendix V - Inventory Analysis Results
 Table V - 19 Aspect - Landslide Type

Sheet	North Facing (330 - 30)															
	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8	
	Total															

Sheet	East Facing (60 - 120)																	
	Type 1		Type 2			Type 3		Type 4			Type 5		Type 6		Type 7		Type 8	
	Total																	
	66	0	0.0%	4	6.1%	19	28.8%	0	0.0%	1	1.5%	40	60.6%	1	1.5%	1	1.5%	
	67	0	0.0%	8	11.9%	18	26.9%	0	0.0%	1	1.5%	35	52.2%	5	7.5%	0	0.0%	
	26	0	0.0%	7	26.9%	3	11.5%	1	3.8%	0	0.0%	12	46.2%	2	7.7%	1	3.8%	
	23	0	0.0%	7	30.4%	7	30.4%	0	0.0%	0	0.0%	8	34.8%	1	4.3%	0	0.0%	
	44	0	0.0%	9	20.5%	21	47.7%	2	4.5%	4	9.1%	6	13.6%	1	2.3%	1	2.3%	
	62	0	0.0%	6	9.7%	4	6.5%	1	1.6%	4	6.5%	7	11.3%	5	8.1%	35	56.5%	
	288	0	0.0%	41	14.2%	72	25.0%	4	1.4%	10	3.5%	108	37.5%	15	5.2%	38	13.2%	

Sheet	South Facing (150 - 210)																
	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8		
	Total																
Peace River	103	1	1.0%	7	6.8%	19	18.4%	6	5.8%	1	1.0%	65	63.1%	4	3.9%	0	0.0%
	65	1	1.5%	14	21.5%	22	33.8%	0	0.0%	0	0.0%	27	41.5%	1	1.5%	0	0.0%
	58	1	1.7%	5	8.6%	21	36.2%	2	3.4%	1	1.7%	25	43.1%	0	0.0%	3	5.2%
	18	1	5.6%	2	11.1%	4	22.2%	1	5.6%	1	5.6%	9	50.0%	0	0.0%	0	0.0%
	35	0	0.0%	3	8.6%	16	45.7%	2	5.7%	6	17.1%	8	22.9%	0	0.0%	0	0.0%
Halfway River to Beaton River	38	0	0.0%	8	21.1%	6	15.8%	0	0.0%	6	15.8%	16	42.1%	2	5.3%	0	0.0%
Total	317	4	1.3%	39	12.3%	88	27.8%	11	3.5%	15	4.7%	150	47.3%	7	2.2%	3	0.9%

Sheet	West Facing (240 - 300)																
	Type 1		Type 2		Type 3		Type 4		Type 5		Type 6		Type 7		Type 8		
	Total																
Peace River Beaton River Southern Tributaries Beaton River - East	65	0	0.0%	10	15.4%	14	21.5%	1	1.5%	1	1.5%	36	55.4%	3	4.6%	0	0.0%
	71	0	0.0%	9	12.7%	23	32.4%	0	0.0%	0	0.0%	36	50.7%	1	1.4%	2	2.8%
	22	0	0.0%	5	22.7%	4	18.2%	1	4.5%	1	4.5%	8	36.4%	3	13.6%	0	0.0%
	31	0	0.0%	8	25.8%	8	25.8%	1	3.2%	0	0.0%	13	41.9%	1	3.2%	0	0.0%
Halfway River - West Halfway River to Beaton River	49	1	2.0%	3	6.1%	19	38.8%	1	2.0%	2	4.1%	15	30.6%	2	4.1%	6	12.2%
	28	1	3.6%	6	21.4%	6	21.4%	1	3.6%	1	3.6%	12	42.9%	1	3.6%	0	0.0%
Total	266	2	0.8%	41	15.4%	74	27.8%	5	1.9%	5	1.9%	120	45.1%	11	4.1%	8	3.0%

Appendix V - Inventory Analysis Results
Table V - 20 Aspect - Material

Sheet	North Facing (330 - 30)				East Facing (60 - 120)					
	Total	Shale		Quaternary	Total	Shale		Quaternary		
Peace River	61	28	45.9%	31	50.8%	65	23	35.4%	42	64.6%
Beaton River	47	13	27.7%	34	72.3%	65	24	36.9%	41	63.1%
Southern Tributaries	56	35	62.5%	20	35.7%	26	16	61.5%	10	38.5%
Beaton River - East	17	11	64.7%	6	35.3%	23	14	60.9%	9	39.1%
Halfway River - West	24	2	8.3%	22	91.7%	44	17	38.6%	27	61.4%
Halfway River to Beaton River	21	8	38.1%	13	61.9%	23	11	47.8%	12	52.2%
Total	226	97	42.9%	126	55.8%	246	105	42.7%	141	57.3%

Sheet	South Facing (150 - 210)			West Facing (240 - 300)						
	Total	Shale	Quaternary	Total	Shale	Quaternary				
Peace River	103	38	36.9%	65	63.1%	29	44.6%	36	55.4%	
Beaton River	65	22	33.8%	43	66.2%	30	42.9%	40	57.1%	
Southern Tributaries	56	37	66.1%	19	33.9%	14	66.7%	7	33.3%	
Beaton River - East	18	8	44.4%	10	55.6%	20	74.1%	9	33.3%	
Halfway River - West	33	10	30.3%	23	69.7%	19	38.8%	30	61.2%	
Halfway River to Beaton River	32	14	43.8%	18	56.3%	14	53.8%	12	46.2%	
Total	307	129	42.0%	178	58.0%	258	126	48.8%	134	51.9%

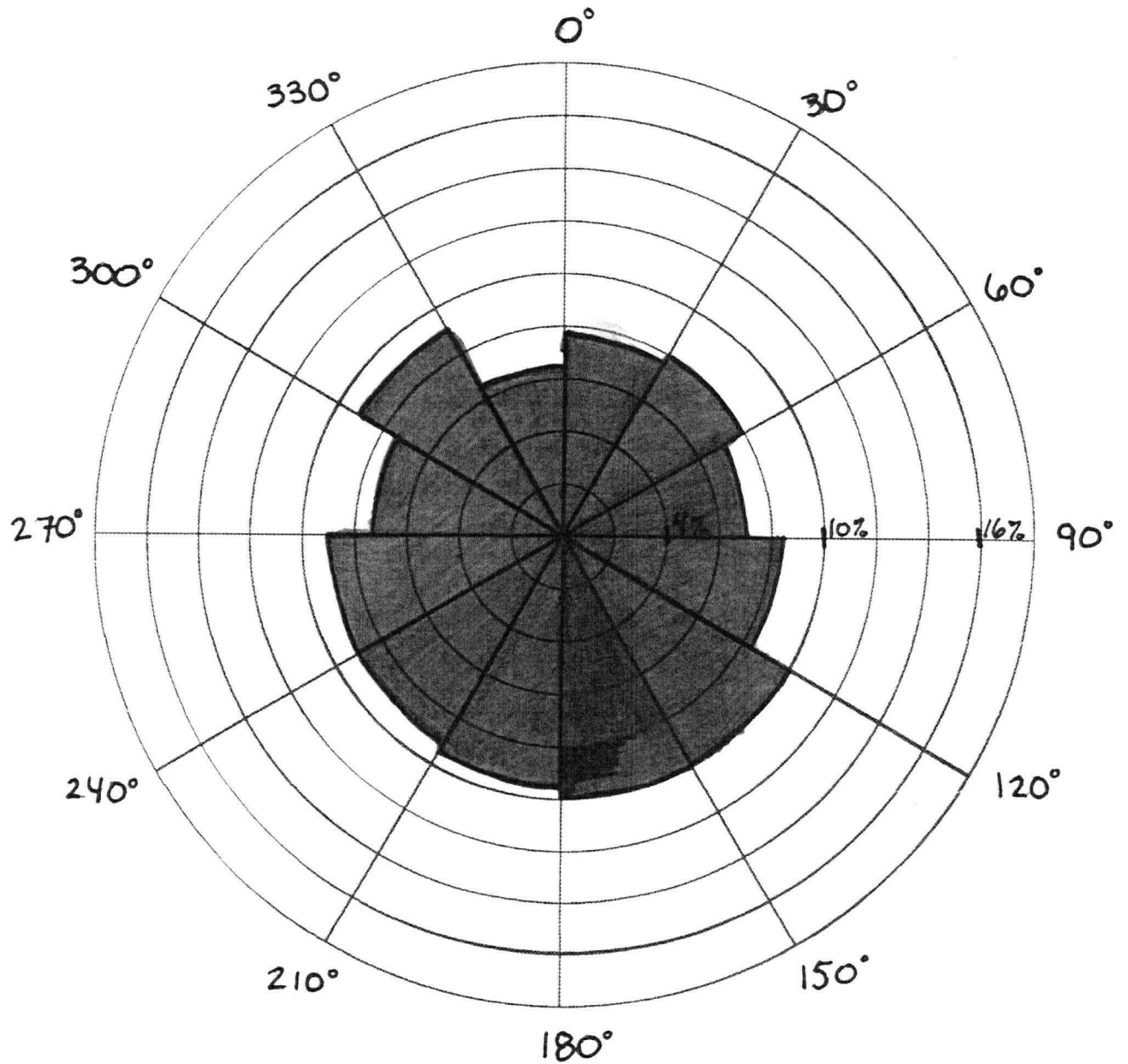
APPENDIX VI

Slide Aspect Rosette Diagrams

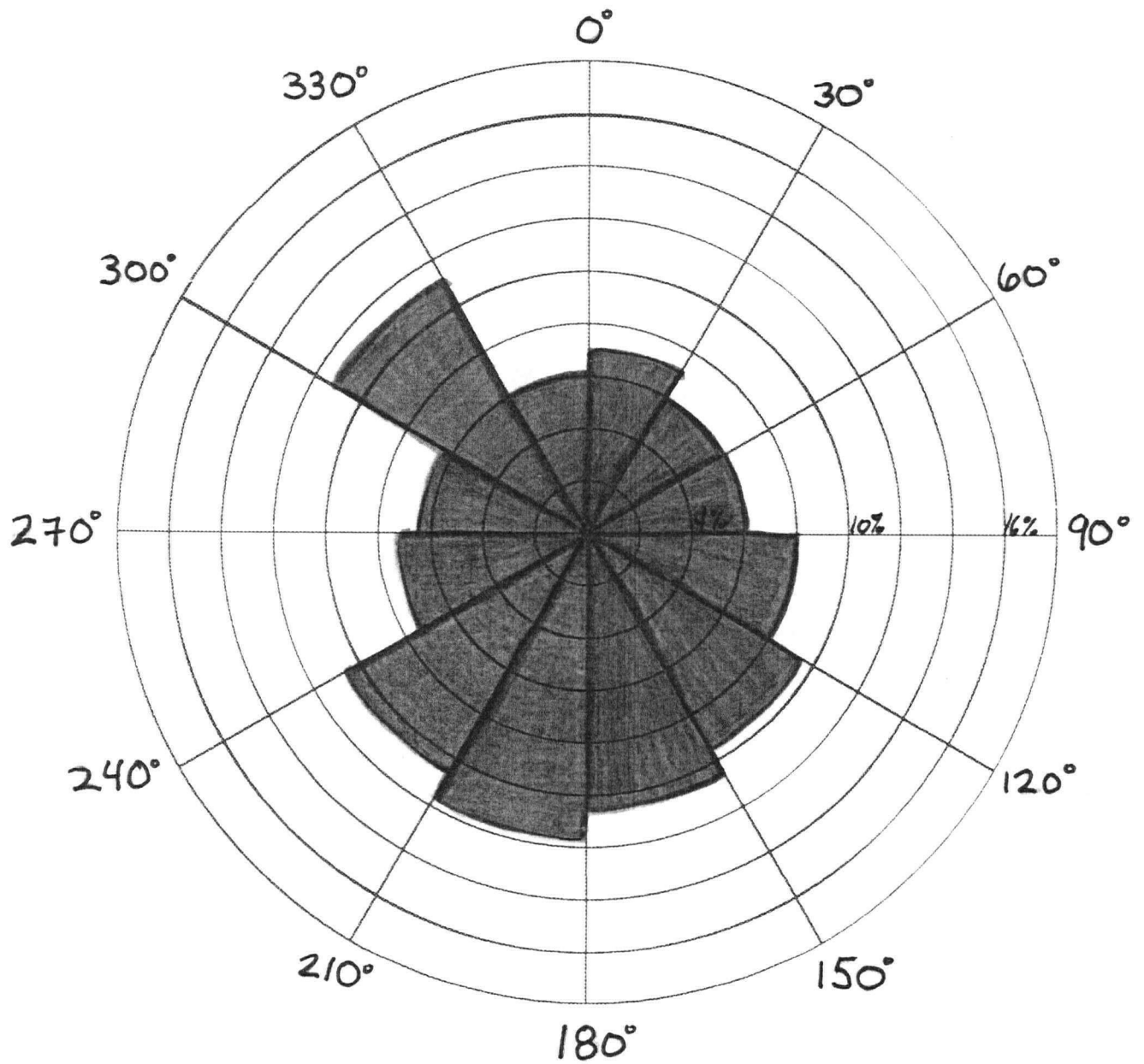
Charlie Lake Mapsheet (94A)

Landslide Aspect (2%)

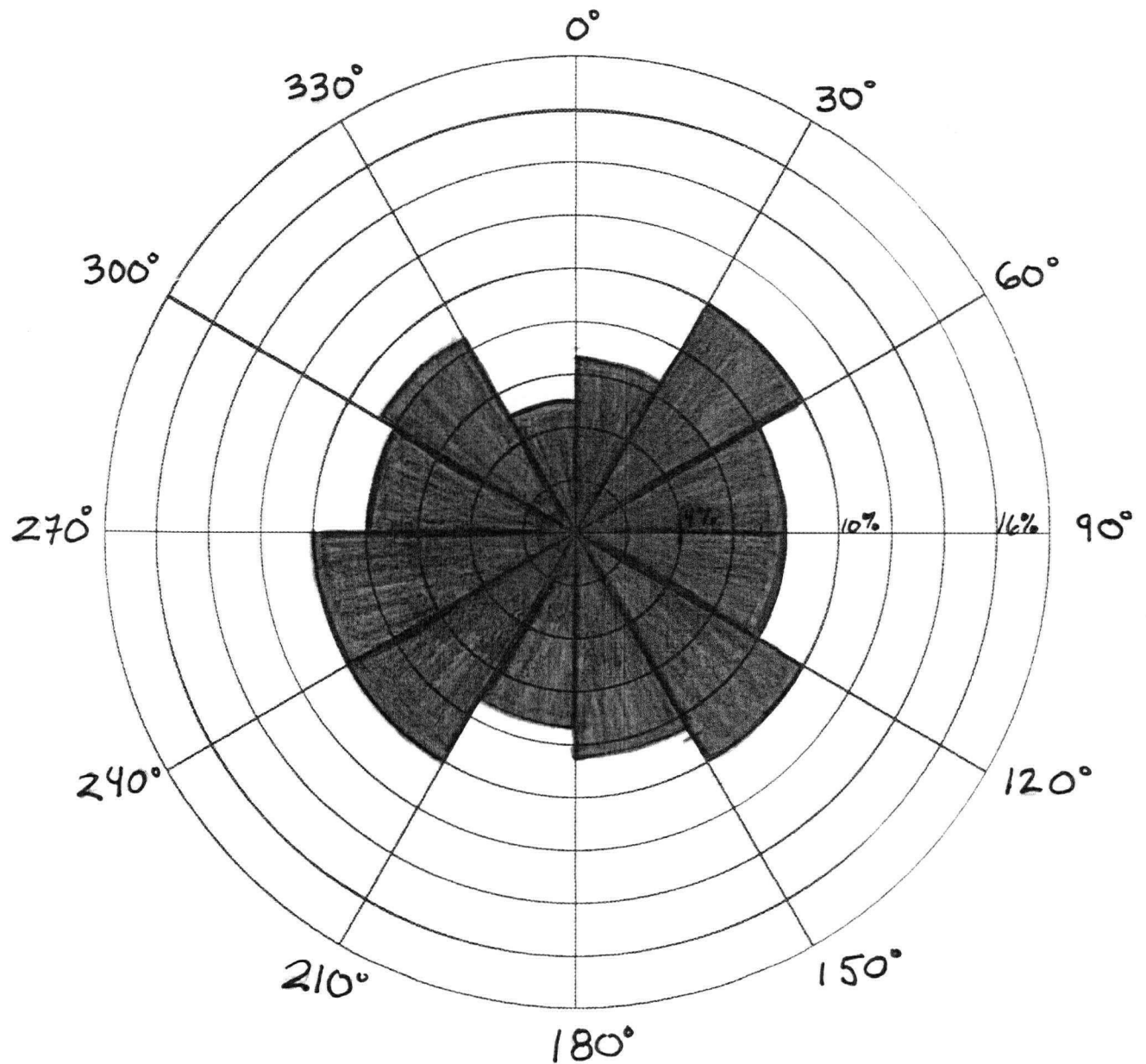
1610 Failures



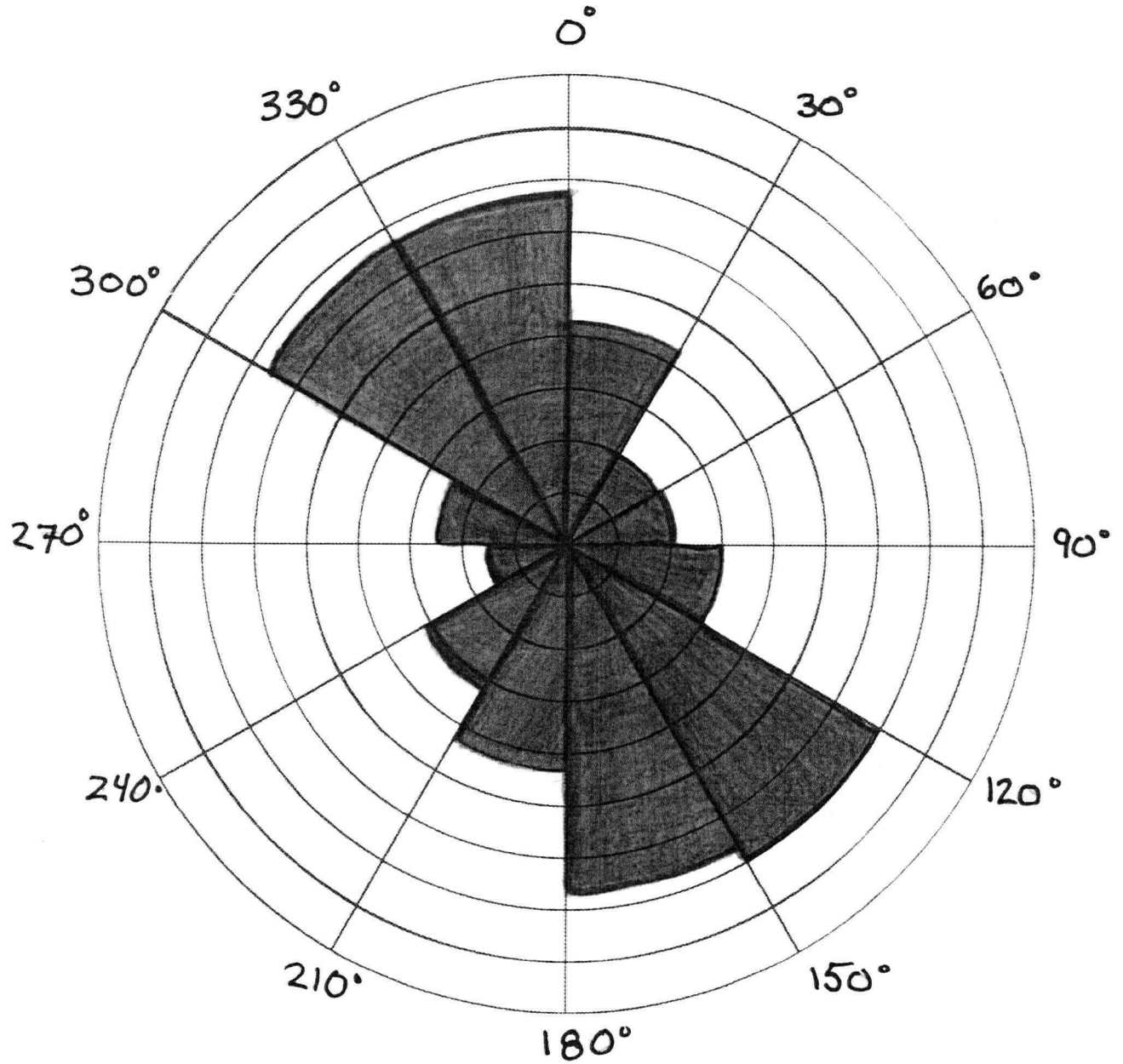
Peace River Division
Landslide Aspect (2% incr.)
465 Failures



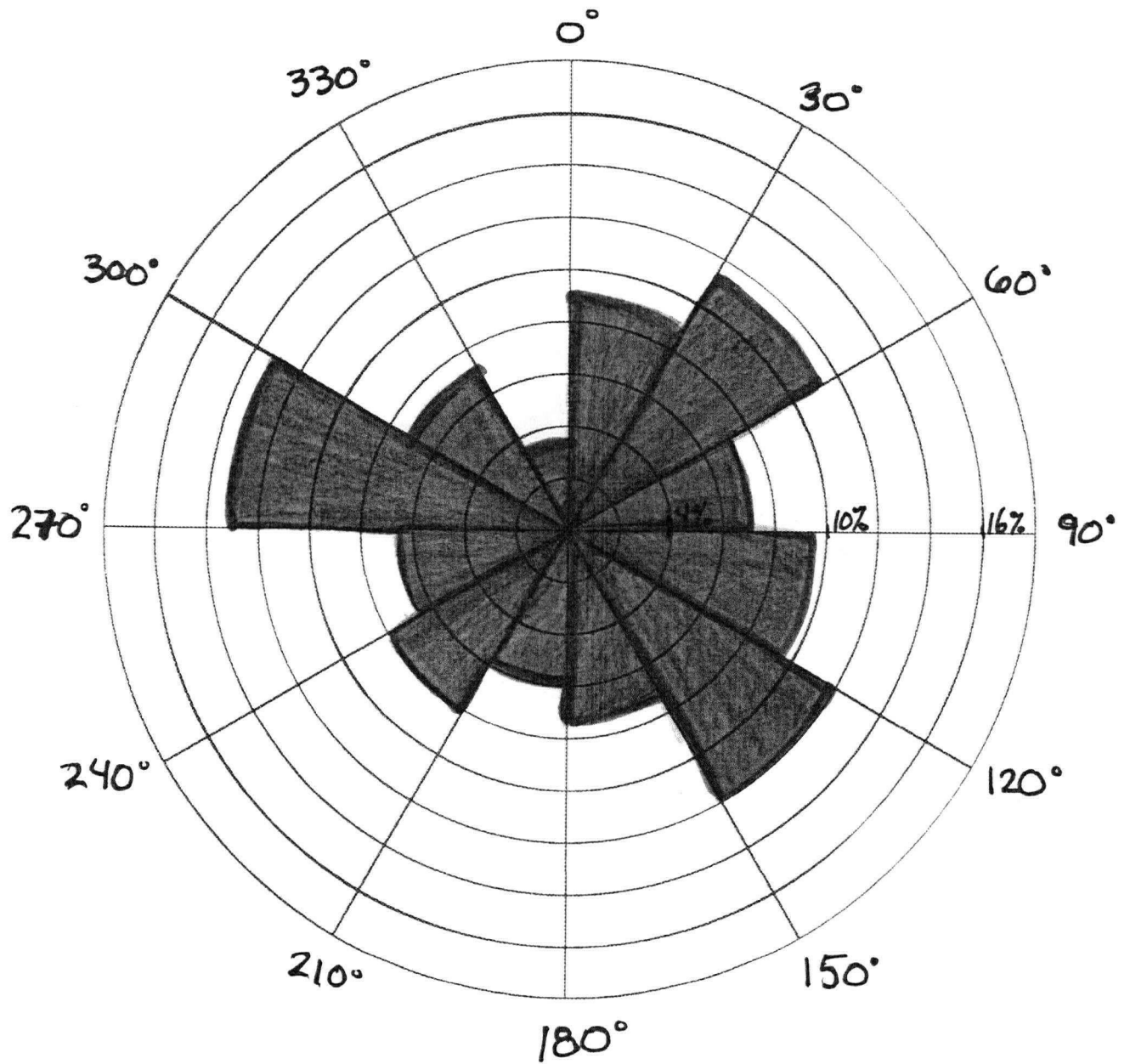
Beaumont River Division
Landslide Aspect (2% incr.)
407 Failures



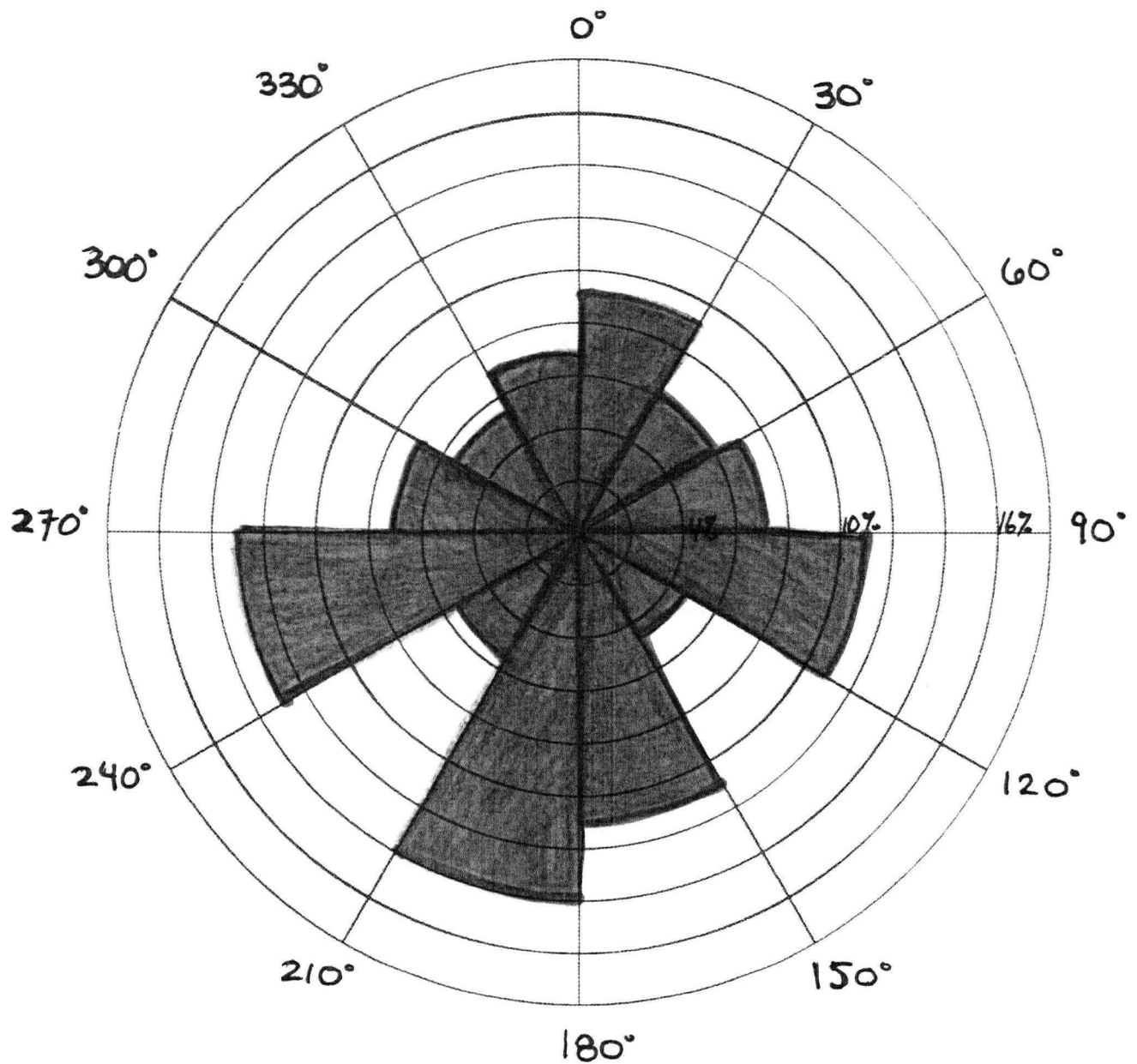
SOUTHERN TRIBUTARIES - Division
Landslide Aspect (2% incr.)
255 Failures



Beaton River - East Division
Landslide Aspect (2% inc.)
137 Failures



Halfway River to Beaton River Division
Landslide Aspect (2% incr.)
130 Failures



Halfway River - West Division
Landslide Aspect (2% incr.)
216 Failures

