

Optimal Design Of Deteriorating Timber Components Under Climate Variations

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ABSTRACT: The mechanical and physical properties of timber structures could be affected by a combination of loading, moisture content, temperature, biological activity, etc. This paper focuses on the optimal design of new timber structures subjected to fungal decay. Among the optimization methods available in the literature, this study considers a Time-Dependent Reliability Based-Design Optimization approach. This method aims at ensuring a target reliability level during the operational life by considering deterioration and the uncertainties related inherent to materials properties, models and climate. This approach is applied to design optimization of a timber truss subjected to an aggressive (very humid) French climate. The performance of the optimized solution is compared, in terms of safety, with solutions estimated from Deterministic Design Optimization and the classical Reliability-Based Design Optimization approaches. The overall results indicate that the optimized solution obtained by the Time-Dependent Reliability Based-Design Optimization approach ensures the target reliability level during the whole structural lifetime.

1. INTRODUCTION

The mechanical and physical properties of timber structures could be affected by a combination of loading, moisture content, temperature, biological activity, etc. This paper focuses on the optimal design of timber structures subjected to fungal decay. Structural optimization is widely used for searching the optimal design cost of civil engineering structures. The Deterministic Design Optimization (DDO) procedure is successfully applied for designing concrete and steel structures (Kravanja et al. 2013; Tomás and Martí 2010). Other works have used the DDO approach to optimize the design of timber trusses (Silih et al. 2005) or finger-joints under bending solicitations (Tran et al. 2014).

The DDO procedure is based on minimizing objective functions defined in terms of the structural volume or costs for structures and

subjected to geometric, stress and deflection constraints. These design conditions are considered in accordance with Eurocode 5 (NF-EN 1995 2005) in order to satisfy the requirements of both the ultimate and the serviceability limit states. In Fact, the real benefit of the DDO approach is cost reduction and effective use of structural capacity.

The partial safety factors introduced in the deterministic design are assumed to take account for uncertainties related to timber material, structural dimension and loading. These safety factors are applied in the design constraints to ensure the safety margin. These factors are calibrated for a large class of structures. Moreover, the safety margin produced with these partial safety factors is not directly linked to uncertainties. Thus, the use of these partial safety

factors in the deterministic design optimization can lead to over or under designing structures.

The Reliability-Based Design Optimization (RBDO) offers a suitable framework for the consideration of the uncertainties in the design optimization and to find the best compromise between cost reduction and safety assurance. A practical formulation of the RBDO consists of minimizing the expected cost under probabilistic constraints.

Furthermore, the long-term durability of timber structures depends on the effect of moisture that in combination with propitious temperature conditions and exposure time may deteriorate the material resistance capacity. The exposure of unprotected timber structures to humid environments increases the moisture content inside wood leading to fungal decay. This deterioration mechanism reduces the strength capacity of timber structures affecting its serviceability and safety.

Within this framework, this paper applies a Time-dependent Reliability-Based Design Optimization approach (TD-RBDO) (Aoues et al. 2009) to find the best design configuration of a roof structure under fungal decay. The TD-RBDO approach aims at searching the optimal design that minimizes the structural cost and to ensure a target reliability level during the operational life. This approach is applied to design optimization of a timber truss subjected to a very humid French climate (Nantes City). The performance of the optimized solution is compared with the optimal design estimated by the DDO and the classical RBDO approaches. The overall results show that an optimized solution obtained by the TD-RBDO method ensure the target reliability level during the whole structural lifetime.

2. DETERMINISTIC DESIGN OPTIMIZATION

The Deterministic Design Optimization aims at minimizing an objective function as the structural volume or cost subjected to geometric, stress and deflection constraints, as defined in the design codes, for instance by the Eurocode 5:

$$\begin{aligned} & \min_{\mathbf{d}=\{\mathbf{h},\mathbf{b}\}} \sum_{i=1}^{n_b} \rho L_i h_i b_i \\ & \text{subject to } \begin{cases} G_{ULS,i}(\mathbf{d}, \mathbf{x}_k, \gamma) \leq 0 \\ G_{SLS,i}(\mathbf{d}, \mathbf{x}_k, \gamma) \leq 0 \\ \psi_i(\mathbf{d}) \leq 0 \end{cases} \end{aligned} \quad (1)$$

where, \mathbf{d} is the vector of design variables, for rectangular cross-section \mathbf{d} is composed by the depth \mathbf{b} and the breadth \mathbf{h} of the members, L_i is the length of the i^{th} member and ρ is the timber density. $G_{ULS,i}$ and $G_{SLS,i}$ are respectively the i^{th} Ultimate Limit State (ULS) and Service Limit State (SLS). The limit state functions are defined in terms of the design variables \mathbf{d} , the characteristic values of load actions and material properties collected in the vector \mathbf{x}_k and the partial safety factors γ and ψ are the feasibility constraints (e.g. upper and lower bounds of design variables).

3. TIME-DEPENDENT RELIABILITY BASED-DESIGN OPTIMIZATION

For structural systems, The Reliability Based Design Optimization is formulated as the minimization of the cost function under reliability constraints.

$$\begin{aligned} & \min_{\mathbf{d}=\{\mathbf{h},\mathbf{b}\}} \sum_{i=1}^{n_b} \rho L_i h_i b_i \\ & \text{subject to } \begin{cases} \beta_i(\mathbf{d}, \mathbf{X}) \geq \beta_i^t \\ \psi_i(\mathbf{d}) \leq 0 \end{cases} \end{aligned} \quad (2)$$

where $\beta_i(\mathbf{d}, \mathbf{X})$ are the reliability indexes for the i^{th} ultimate or serviceability limit state G_i and β_i^t is the target reliability index for the i^{th} limit state. In the above formulation, the reliability constraints define the feasible domain, such as the reliability indexes β are kept upper than the target reliability indexes β^t . Several approaches have been recently developed to solve the above formulation in Eq. 2. Aoues and Chateauneuf (2010) have compared and discussed different

RBDO approaches regarding robustness and numerical performance. The SORA method (Du and Chen 2004) appears more robust and more accurate than the other methods. This approach is used in the study to search the best reliable design of the timber structures.

However, when degradation is considered, the time-dependent Reliability-Based Design optimization is proposed to find the optimal design by satisfying appropriate safety levels during the whole structure lifetime. The TD-RBDO is formulated as:

$$\begin{aligned} & \min_{\mathbf{d}=\{\mathbf{h},\mathbf{b}\}} \sum_{i=1}^{n_b} \rho L_i h_i b_i \\ \text{subject to } & \begin{cases} \beta_i(\mathbf{d}, \mathbf{X}, t) \geq \beta_i^{t, T_L} & \forall t \in [0, T_L] \\ \psi_i(\mathbf{d}) \leq 0 \end{cases} \end{aligned} \quad (3)$$

where, $\beta_i(\mathbf{d}, \mathbf{X}, t)$ is the reliability index at the time t taken the lifetime interval $[0, T_L]$, β_i^{t, T_L} is the target reliability index at the allowable life time T_L , depending on the target reliability related to one year reference period by the following relation (NF-EN 1990 2003):

$$\beta_{T_L}^c = \Phi^{-1}\left(\Phi\left(\beta_1^t\right)^{T_L}\right) \quad (4)$$

where β_1^t is the target reliability index for one year. The classical TD-RBDO formulation given in Eq. (3) is based on the time-dependent reliability, that aims at computing the probability of failure during the whole structure lifetime. The time dependency lies mainly in the degradation phenomena. This formulation is not suitable for real engineering structures because considerable computational effort is required, and convergence can hardly be achieved. The major drawback lies in the time-dependent reliability analysis, which requires considerable computational efforts. In this work, the Sequential Optimization and Time-Variant Reliability Analysis (SOTVRA) approach developed and implemented by Aoues et al. (2009) is used to perform the TD-RBDO

methodology. The SOTVRA approach is based on transforming the TD-RBDO problem into a sequence of equivalent deterministic design optimization sub-problems. This transformation is defined by the mean of optimal safety factors, linking the reliability requirement to the equivalent deterministic optimization. At the end of each optimization sub-problem, the reliability constraint is verified by performing the time-dependent reliability. The safety factors corresponding to the target reliability level at the initial time are calibrated by a probabilistic approach. Finally, these safety factors are provided to the following sub-problem of the equivalent deterministic optimization and so on, until convergence.

4. MODELING TIMBER DECAY

4.1. Background

Decay models are based on in-field or in-lab measures and report results for specific wood species and locations (Brischke and Thelandersson 2014). Among the models based on in-field observations, the Timber Life empirical model was derived based on 20-year data obtained by exposing 4000 small clear timber specimens in 11 sites around Australia, augmented by more than 1500 bits of data obtained from examining existing timber constructions (Leicester et al. 2009). This study focused on Australian species. This model has been adapted and used for many researchers worldwide (de Freitas et al. 2010; Lourenço et al. 2012; Ryan et al. 2014; Sousa et al. 2014). However, it does not provide direct relationships between decay and specific climate conditions for a given zone that we would like to consider in an optimal design.

Isaksson et al. (2012) also proposed a decay model based on in field measurements on Scots pine sapwood (*Pinus sylvestris* L.) and Douglas fir heartwood (*Pseudotsuga menziesii* Franco) specimens. The specimens were subjected to real exposure conditions in 24 European test sites with different climate regimes between 2000 and 2008. The model developed by Isaksson et al.

(2012) aims at estimating decay in terms of ‘dose-response functions’. The ‘dose’ is expressed as a function of daily wood moisture content and wood temperature and the level of decay is defined according to (EN 252 1990). EN 252 (1990) proposes 5 levels of decay condition from no decay (sound) until very severe decay. These levels are useful when condition assessment is based on serviceability limit states. However, they cannot provide quantitative information about the loss of effective section that can be used for estimate the strength loss. Consequently, this model is not considered in this work.

4.2. Adopted decay model

On the basis of previous in-lab experimental studies (Viitanen 1996), Viitanen et al. (2010) developed a model for the decay growth of brown rot in pine sapwood under variant climate conditions. Such a model is divided into two processes: (i) activation process and (ii) mass loss process.

4.2.1. Activation process

A parameter α is used as a relative measure of fungi deterioration activity. α is set initially to 0. Once it reaches the limit value $\alpha=1$, the mass loss initiates. The parameter α varies with time according to:

$$\alpha(t) = \sum_{i=0}^t \Delta\alpha(i) \text{ with } \alpha(t) \in [0,1] \quad (5)$$

where

$$\Delta\alpha(i) = \begin{cases} \frac{\Delta t}{t_{crit}(RH(i),T(i))} & \text{if } T(i) > 0^\circ\text{C} \\ & \text{and } RH(i) > 95\% \\ -\frac{\Delta t}{17520} & \text{otherwise} \end{cases} \quad (6)$$

where $RH(i)$ and $T(i)$ are the i^{th} air relative humidity (in %) and temperature ($^\circ\text{C}$), respectively, Δt is the time step between two consecutive climatic records (hours), and t_{crit} (in hours) is estimated as follows:

$$t_{crit}(i) = \left[\frac{2.3T(i) + 0.035RH(i) - 0.024RH(i)T(i)}{-42.9 + 0.14T(i) + 0.45RH(i)} \right] \times 30 \times 24 \quad (7)$$

Eq. (6) shows that $\Delta\alpha(i)$ increases when $T > 0^\circ\text{C}$ and $RH > 95\%$. Under dry and cold conditions $\alpha(t)$ decreases linearly from 1 to 0 in two years (17520 hours).

4.2.2. Mass loss process

Mass loss (in % of initial weight) occurs once the fungi activation process is reached, ($\alpha(t) = 1$) and it is estimated as:

$$ML(t) = \sum_{i=0}^t \left(\frac{ML(RH(i),T(i))}{dt} \times \Delta t \times 1_{\alpha}(i) \right) \quad (8)$$

where

$$1_{\alpha}(i) = \begin{cases} 0 & \text{if } 0 \leq \alpha(i) < 1 \\ 1 & \alpha(i) = 1 \end{cases} \quad (9)$$

and

$$\frac{ML(RH(i),T(i))}{dt} = -5.96 \cdot 10^{-2} + 1.96 \cdot 10^{-4} T(i) + 6.25 \cdot 10^{-4} RH(i) \text{ [% loss/hour]} \quad (10)$$

According to eq. (6), mass loss only takes place when the temperature is above 0°C and the relative humidity is above 95%. Otherwise mass loss process is stopped.

5. NUMERICAL EXAMPLE

5.1. Problem description

Building roofs are usually made of timber trusses. The design of timber structures requires the verification of a certain number of rules resulting from the codes of practice, such as Eurocode 5 (NF-EN 1995 2005). Where, these rules should satisfy given requirements related to their ultimate of capacity in ultimate limit state (ULS) and their deformation in service limit state (SLS).

In practice, roof trusses are made and composed of wood members connected by steel plates. Generally, the timber joints are considered completely flexible (free rotations

hinges in the connections of the timber members). However, this situation is rarely found in real structures, due to eccentricity and geometrical imperfection of the timber members. A rigid joints hypothesis can be considered as an intermediate solution with respect to the flexible model. The best model of timber connections is to consider the semi-rigid law behavior by using springs or contact elements (Riahi et al. 2011).

The numerical application is carried out for the roof truss described in Figure 1. The depth b and breadths $\{h_1, h_2, h_3\}$ of the cross-section of the truss members are optimized by accounting for material and loading uncertainties. Table 1 gives the statistical parameters for the truss parameters, loading and material properties. It is assumed that all random variables follow lognormal distributions (Köhler et al. 2007).

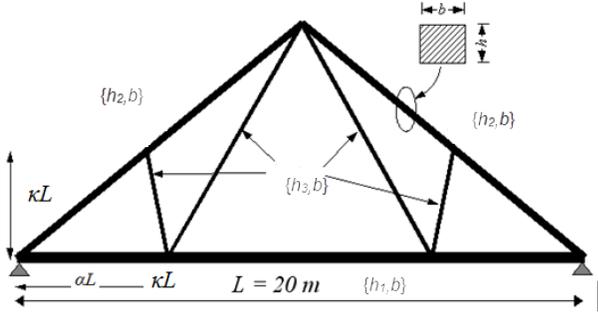


Figure 1: roof truss.

For plane timber truss, the ultimate and serviceability limit state functions for each i^{th} member are defined as:

$$\begin{aligned} \sigma_{m,d} / f_{m,d} + \sigma_{t,0,d} / f_{t,0,d} &\leq 1 \quad \text{in tension,} \\ \left(\frac{\sigma_{m,d}}{k_{crit} f_{m,d}} \right)^2 + \frac{\sigma_{c,0,d}}{k_{c,z} f_{c,0,d}} &\leq 1 \quad \text{in compression,} \\ \tau_d / (k_v f_{v,d}) &\leq 1, \quad \delta_{inst} / \delta_{inst,lim} \leq 1 \quad (SLS), \\ \delta_{fin} / (\delta_{fin,lim}) &\leq 1 \quad (SLS) \end{aligned} \quad (11)$$

where, $\sigma_{m,d}$, $\sigma_{t,0,d}$, and $\sigma_{c,0,d}$ and are respectively the design values of bending stress, tensile stress along the grain and compressive stress along the grain. $f_{m,d}$, $f_{t,0,d}$, and $f_{c,0,d}$ are respectively the design values of bending strength, tensile

strength and the compressive strength along the grain. k_{crit} and $k_{c,z}$ are respectively factors which take into account the reduced bending strength due to lateral buckling and compressive strength due to buckling about the y and z axes in accordance with Eurocode 5. τ_d is the design value of the shear stress and $f_{v,d}$ is the design value of shear strength and k_v is the reduction factor of the shear strength at the notched member. The strength design values are defined by :

$$f_d = k_{mod} \times f_k / \gamma_m \quad (12)$$

where f_d and f_k are respectively the design and the characteristic values of the strength, k_{mod} is the modification factor, which takes into account the effect of the duration of the load and the moisture content, γ_m is the partial safety factor for a material property. In the deterministic design the values for k_{mod} and γ_m are taken from Eurocode 5.

Eq. (11) considers the following stress criteria: i) tension and bending: where the tension is parallel to the grain; ii) compression: where members are checked for compressive strength as well as for buckling; iii) shear: for all the truss members; and iv) deformation: corresponding to the serviceability state functions, where δ_{inst} and δ_{fin} are respectively the instantaneous deflection and the final deflection composed with the instantaneous and creep deflections. $\delta_{inst,lim}$ and $\delta_{fin,lim}$ are respectively the limit values for instantaneous and final deflections, taken respectively to $L_i/300$ and $L_i/200$ in mm for the i^{th} component.

To find the optimal design that minimizes the structural volume of the roof truss, three optimization methods are applied:

- The DDO method on the basis of the safety factors prescribed by the Eurocode 5.
- The RBDO method using the SORA approach without considering degradation model.
- The TD-RBDO method using the SOTVRA approach considering degradation model.

Table 1: Statistical parameters for materials and loads.

Name parameter	x_k	x_m	COV
f_m (MPa)	24	33.9	0.2
f_c (MPa)	21	29.66	0.2
f_t (MPa)	14	19.77	0.2
$f_{c,90}$ (MPa)	2.5	3.53	0.2
f_v (MPa)	4	5.65	0.2
E (GPa)	10.8	11	0.2
Permanent load (kN/m ²)	620	466.52	0.2
Snow (kN/m ²)	1193	798.81	0.3
Wind (kN/m ²)	1320	883.53	0.36

The target reliability index for one year for the ultimate limit state is set to 3.8 and for serviceability limit state is set to 2.9 (NF-EN 1990 2003). In the TD-RBDO method, the target reliability for allowable lifetime T_L fixed to 30 years is estimated with Eq. (4). For all the optimal solutions found by these methods, a time-dependent reliability analysis considering the decay model is performed. The time-dependent reliability analysis is based on the design equation established by Eq. 11. However, the partial safety factors are neutralized (Brites et al. 2013)

Timber decay is assessed by the deterioration model presented in section 4 that depends mainly on specific environmental conditions of a given place. It allows for estimating the mass loss during the time. For illustrative purposes, the roof was exposed to a very humid environment corresponding to the city of Nantes (France). Nantes is close to the Atlantic Ocean and has a temperate oceanic climate with annual mean temperature and relative humidity of 12.7 °C and 81%, respectively. Hourly variations of temperature and relative humidity for 30 years [1980-2010] were used in the example.

5.2. Results

Tables 2 and 3 present the optimal solutions corresponding to different optimization methods considering flexible and rigid joints, respectively. It is observed that the optimized volume obtained by TD-RBDO doubles the

values calculated by the other approaches. This is expected because DDO and RBDO did not include directly the deterioration process. Small volumes are found when the design considers rigid joints.

Table 2: Design optimization results for flexible joints.

	DDO	RBDO	TD-RBDO
b (mm)	166.89	175.22	240.30
h_1 (mm)	235.81	248.80	375.32
h_2 (mm)	333.78	350.44	480.60
h_3 (mm)	222.52	233.62	320.40
Volume (cm ³)	1182.65	1305.4	2524.11

Table 3: Design optimization results for rigid joints.

	DDO	RBDO	TD-RBDO
b (mm)	162.24	143.80	236.27
h_1 (mm)	216.24	191.74	315.03
h_2 (mm)	324.50	287.61	472.55
h_3 (mm)	216.33	191.74	315.03
Volume (cm ³)	1100.12	864.26	2333.04

Figure 2 depicts the time-dependent reliability indexes of the optimal design of the roof truss given by the DDO approach. The initial reliability index at $t = 0$ of the ultimate limit state satisfies the target reliability index of one year. However, the initial reliability index at $t = 0$ of the serviceability limit state is not checked. In other words, the use of the partial safety factors in the design optimization cannot guarantee the target reliability. When timber degradation is considered, the reliability indexes of the serviceability and ultimate limit state fell suddenly, where the serviceability reliability index reaches the target after 8 years when the flexible joints are considered. This is explained by the fact that the structure is less rigid with flexible joints increasing the critical deflections.

Figure 3 shows the time-dependent reliability profiles of the RBDO solution, where this optimal solution is estimated without the decay model and by considering only the target reliability of one year. Figure 3 indicates that the reliability indexes for ultimate and serviceability

limit states at the initial time are checked regarding the target values for one year (3.8 for ULS and 2.9 for SLS). However, considering the timber decay, the reliability indexes decrease quickly and the serviceability and ultimate target reliability indexes are reached after 13 and 9 years respectively for the rigid joints.

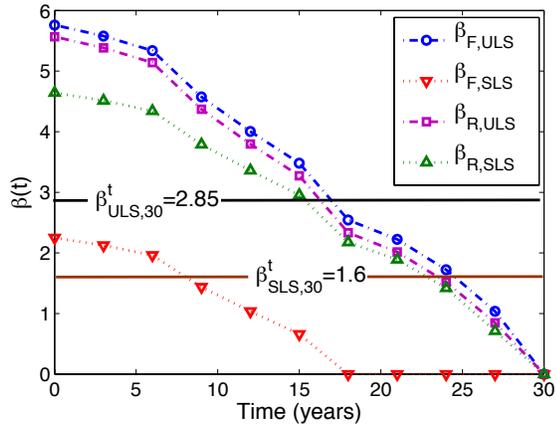


Figure 2: Time-dependent reliability index of DDO solution for flexible (F) and rigid (R) joints.

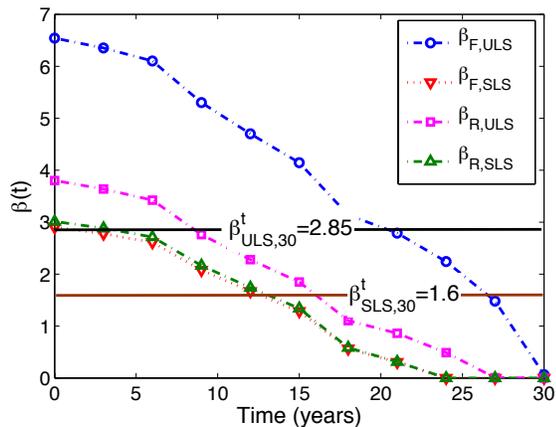


Figure 3: Time-dependent reliability index of RBDO solution for flexible (F) and rigid (R) joints.

Figure 4 indicates that the TD-RBDO method combined with the SOTVRA approach give the optimal design that satisfies the serviceability and reliability requirements during the 30 years lifetime. However, the proposed design solution is more expensive. The optimal volume is about 2.7 times larger than the RBDO volume (Tables 2 and 3). This large volume ensures the reliability and serviceability requirements under a very humid exposure that

accelerates timber decay. Different results could be obtained for other climates. Besides, if the failure cost is considered in the RBDO problem, the total cost of the TD-RBDO may be lower. For all cases, the target reliability is reached early when rigid joints are considered because these joints generate additional bending moments in the truss.

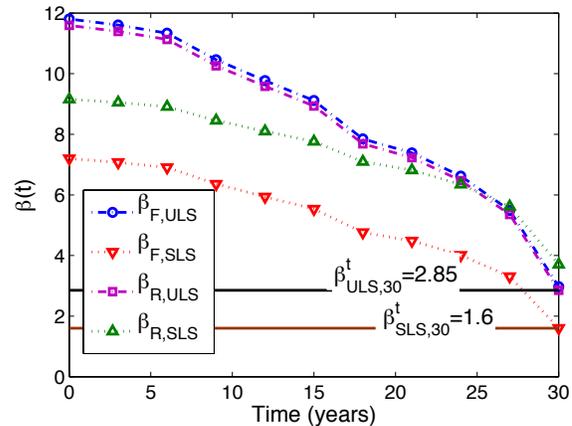


Figure 4: Time-dependent reliability index of TD-RBDO solution for flexible (F) and rigid (R) joints.

6. CONCLUSIONS

This study focused on the design optimization of timber truss structures by accounting for uncertainties, climate variations, and serviceability and safety constraints. The preliminary results indicate that the TD-RBDO solution ensures the serviceability and reliability requirements during the whole lifecycle. However, the optimized solution needs a large material volume (larger construction costs) in comparison with DDO and RBDO for the environmental exposure. Further work in this area will focus on: (1) the consideration of climate uncertainties, (2) the design optimization for other locations, and (3) the consideration of real costs (including the failure cost).

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