DEVELOPMENT OF AN INNOVATIVE EARTHQUAKE-RESILIENT REINFORCED CONCRETE CORE WALL SYSTEM

by

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B.A.Sc., The University of British Columbia, 2012

A DISSERTATION SUBMITTED IN PARTIAL FULFILLMENT OF

THE REQUIREMENTS FOR THE DEGREE OF

DOCTOR OF PHILOSOPHY

in

THE FACULTY OF GRADUATE AND POSTDOCTORAL STUDIES

(Civil Engineering)

THE UNIVERSITY OF BRITISH COLUMBIA

(Vancouver)

June 2021

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DEVELOPMENT OF AN INNOVATIVE EARTHQUAKE-RESILIENT REINFORCED CONCRETE CORE WALL SYSTEM

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Abstract

This thesis introduces an earthquake-resilient reinforced concrete core wall system, named the controlled rocking outrigger core wall (CROCW). In the CROCW system, along one principal axis is a controlled outriggered rocking wall (CORW), where an outrigger is situated at the roof and the base is designed to rock. Dampers are incorporated within the outrigger and the rocking base to give CORW added energy dissipation. Along the other principal axis is the self-centering coupled wall (SCCW). In the SCCW system, self-centering friction dampers are incorporated within the coupling beams to dissipate the earthquake energy and ensure the wall will self center. In addition, the base is design to rock, to ensure a low damage response.

The CROCW, CORW and SCCW were designed using the novel equivalent energy design procedure (EEDP). New factors were developed to modify the original EEDP to account for the dynamic responses and the different hysteretic shapes for the new systems.

Using the newly developed design procedure, four prototype buildings (two different building heights and two different building sites) were designed. The uniform hazard spectra at three hazard levels were developed using probabilistic seismic hazard analysis. Detailed nonlinear models were developed and validated using available experimental data.

Nonlinear time history analysis showed the CROCW, CORW, and SCCW can meet the design objectives, where the systems have limited damage after strong earthquake shaking. Additional studies were conducted to compare the performance of the newly proposed CROCW with conventional RC core wall system. The result shows that the CROCW has superior performance compare with conventional RC core walls.

The newly developed CROCW system will be tested using a shake table to validate the behaviour. Shake table testing will take place at the multi-function shake table array facility in Shanghai China. Due to the COVID-19 pandemic, the experimental test has been temporarily delayed. This thesis presents the specimen design, construction, planned instrumentation, and the testing plan to be implemented at a later date.

Lay Summary

Rapid urbanization is creating a demand for tall building construction globally. Many of these buildings are located in high seismic zones. To improve the resiliency of high-rise buildings under earthquake loads, this thesis proposes a novel seismic force resisting system, named the controlled rocking outriggered core wall (CROCW) system. Design procedures for the CROCW have been developed and validated using detailed numerical models, and the system performance has been compared to existing tall building systems. Additionally, a large-scale testing program was developed, where a 1/6.5 scale specimen was designed and constructed. This thesis provides background study, detailed design procedure and verification of the CROCW, which engineers and other researchers can use to design CROCW in high seismic zones.

Preface

Various parts of this thesis work have been published or are under review for publication. The following summarises each publication and the specific contributions from each author.

Published contributions

I contributed significantly to the development of the Self-centring conical friction damper (SCFD) for coupled wall systems. My role in the conceptual development and organising the experimental testing program has resulted in two patents and one journal paper. In this work, H.C. Xu was responsible for conducting the experimental testing of the SCFD. T.Y. Yang contributed to the conceptual development and writing of the paper. The design of the SCFD and implementation in a new RC coupled wall system is presented in this thesis.

The following are the publications related to the SCFD:

- T.Y. Yang, H.C. Xu, L. Tobber, "Development and experimental testing of innovative selfcentering conical friction damper (SCFD)," Structural Control and Health Monitoring. 27(10). https://doi.org/10.1002/stc.2609
- T.Y. Yang, H.C. Xu, L. Tobber. Self-centering conical friction damper. US 62/884805A1, United States Patent and Trademark Office, filed 9 August 2019.
- T.Y. Yang, H.C. Xu, L. Tobber. Self-centering conical friction damper. PCT/CA2020/051092, Canadian receiving office, filed 7 August 2020.

I led the development of a large-scale testing program, which will validate two new structural systems presented in this thesis.

This experimental testing program has been described in the following conference papers:

- T. Y. Yang, L. Tobber, H.C. Xu, 2019, "Large-scale shake table testing of high-performance earthquake-resilient tall building", Proceedings of the 2019 Pacific Conference in Earthquake Engineering, New Zealand Society for Earthquake Engineering, Auckland, NZ
- L. Tobber, T. Y. Yang, P. Adebar, 2018, "Development of High-Performance Earthquake Resilient Tall Buildings", Proceedings of the 11th National Conference in Earthquake Engineering, Earthquake Engineering Research Institute, Victoria, CA, USA

In these conference papers, I was the lead investigator, responsible for all major conceptual development, design, and writing. T.Y. Yang contributed to conceptual development, design, and paper edits. H.C. Xu contributed to some detailed component design calculations and drawings. P. Adebar contributed to the initial conceptual development.

Under review contributions

I was also lead investigator in four recently submitted journal papers which cover four main contributions in this thesis. In these papers, I was responsible for developing the design procedures, numerical models, and analysis of the results of the novel Controlled rocking outriggered core wall (CROCW), Controlled Outriggered Rocking Wall (CORW), and Seismic Design of Self-centering coupled wall (SCCW) systems presented in this thesis. In these papers, T.Y. Yang contributed to conceptual development and writing of the papers. H. Xu and M.A. Sadeghi contributed to some initial concept development, preparing graphs, and conducting edits.

The following show the journal publications currently under review:

- 6. L. Tobber and T. Y. Yang, "Seismic Design of Controlled Outriggered Rocking Wall (CORW) using EEDP." (Under review)
- 7. L. Tobber, T.Y. Yang, H. Xu, and M.A. Sadeghi "Seismic Design of Self-centering coupled wall (SCCW) using EEDP." (Under review)
- 8. L. Tobber and T. Y. Yang, "Bidirectional design of earthquake resilient core wall." (Under review)
- 9. L. Tobber and T. Y. Yang, "Comparison of RC core wall systems integrating new technology to increase seismic performance." (Under review)

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List of Abbreviations

- ACI = American concrete institute
- AISC = American institute of steel construction
- ASCE = American society of civil engineers
- ASTM = American society for testing and materials
- BRB = Buckling restrained brace
- CORW = Controlled rocking outriggered wall
- CP = Collapse prevention
- CQC = complete quadratic combination
- CROCW = Controlled rocking outriggered core wall
- CRW = Controlled rocking wall
- CSA = Canadian standards association
- CTBUH = Council of tall Buildings and urban habitat
- CW = Coupled wall
- DAS = Degree of axial stiffness
- DBE = Design base earthquake
- DCW = Damped coupled wall
- DOC = Degree of coupling
- EEDP = Equivalent energy-based design procedure
- ELSDOF = Equivalent linear single degree-of-freedom
- ENLSDOF = Equivalent nonlinear single-degree-of-freedom
- EPP = Elastic perfectly plastic
- FEMA = Federal emergency management administration
- IO = Immediate occupancy
- ISDR = Inter-story drift ratio
- LDA = Linear dynamic analysis

- LED = Lead extrusion dampers
- LTHA = Linear time history analysis
- MCE = Maximum considered earthquake
- MDOF = Multi degree of freedom
- MPC = Multi point constraint
- MRSA = Modal response spectrum analysis
- NBCC = National building code of Canada
- NLTHA = Nonlinear time history analysis
- OW = Outriggered wall
- PEER = Pacific earthquake engineering research
- PGA = Peak ground acceleration
- PRESSS = Precast seismic structural system
- PreWEC = Precast wall with End Columns
- PSHA = Probabilistic seismic hazard analysis
- PSW = Plastic hinge supported wall
- PT = Post-tensioned
- PVC = Polyvinyl chloride plastic
- RC = Reinforced concrete
- RDR = Roof drift ratio
- RR = Rapid return
- RSFJ = Resilient slip friction joint
- SA = Spectral acceleration
- SCCW = Self centering coupled wall
- SCFD = Self-centering conical friction damper
- SDOF = Single degree of freedom
- SFRS = Seismic force resisting system
- SLE = Service level earthquake

SRC = Steel reinforced concrete

SW = Shear wall

UHS = Uniform hazard spectrum

UN = United Nations

VCD = Viscoelastic coupling damper

List of Symbols

 A_g = gross area of RC core wall

 $A_o =$ cross-sectional area of the exterior outrigger truss diagonals A_c = cross-sectional area of the outrigger truss top and bottom chords A_m = cross-sectional area of the interior outrigger truss diagonals $A_{o,g}$ = cross-sectional area of outrigger beam C_n = modal particitipation of coupling beam shear force of nth mode C_0 = factor to convert SDOF to MDOF displacements E_c = elastic modulus of reinfroced concrete F = base shear F_{PR} = primary fuse system base shear F_{SE} = secondary fuse system base shear F_p = design base shear at the moderate hazard $F_{p,e}$ = moderate hazard elastic base shear $F_{pt}^0 = \text{SCFD}$ initial post-tensioning force in the tendons $F_{u,e}$ = high hazard elastic base shear F_{v} = yield base shear H = total building height $I_{w,v}$ = moment of intertia of core-wall in the CORW direction $I_{w,x}$ = moment of intertia of core-wall in the SCCW direction K_s = system stiffness $L_{cb} =$ length of coupling beam

 L_d = distance from edge of wall to base damper

 L_f = length of wall flange

 $L_o =$ length of outrigger

 L_s = span between the RC wall and the mega-column

- $L_{w,y}$ = length of wall in CORW direction
- $L_{w,x}$ = length of wall in SCCW direction

 $M_{b,y}$ = controlled rocking yielding base moment

 $M_{b,Pg}$ = overturning moment resistance provided by gravity loads

 $M_{b,damp}$ = moment resistance provided by dampers

 M_t^{dy} = total overturning moment from LTHA

 M_t^{st} = total overturning moment from static load

 M_T = base moment in "tension" wall pier in SCCW

 M_C = base moment in "compression" wall pier in SCCW

 M_f = base moment demand in wall piers in SCCW

 $M_{T,c}$ = base moment capacity in "tension" wall pier in SCCW

 $M_{C,c}$ = base moment capacity in "compression" wall pier in SCCW

 P_q = gravity load at wall base

 P_0 = SCFD initial sliding force

 P_m = SCFD maximum sliding force

 P_p = Coupling force in wall pier

Q = static load distribution

 S_a = Spectral displacement

 S_{a1} = spectral acceleration at fundemental period

 S_{a2} = spectral acceleration at second mode period

 S_d = spectral displacement

 S_1 = shape function for first mode coupling beam shears

 S_2 = shape function for second mode coupling beam shears

 S_3 = shape function for third mode coupling beam shears

SN = normalised shape functions for coupling beam shears

 Sa_{SLE} = spectral acceleration at fundemental period for low hazard

 Sa_{MH} = spectral acceleration at fundemental period for moderate hazard

 Sa_{HH} = spectral acceleration at fundemental period for high hazard

 $V_{P,r}$ = roof coupling beam shear

 $V_{P,x}$ = roof coupling beam shear at a given story x

T = fundemental structural period

 $T_{n_{\alpha_{f}=0}}$ = fundemental period for system without outrigger

 T_2 = natural period for second mode

 T_3 = natural period for third mode

W = total building weight

 W_{ext} = external work

 W_{int} = internal work

b = system post yielding stiffness

 b_c = width of outrigger column

 d_o = depth of outrigger

 f_o = outrigger damper force

 f_b = base damper force

 f_{bt} = base damper force in "tension" wall

 f_{bc} = base damper force in "compression" wall

 $f_{bt,min}$ = minimum require base damper force in "tension" wall

 $f_{bc,min}$ = minimum require base damper force in "compression" wall

 f_e = specimen elastic modulus scaling factor

 f_l = specimen length scaling factor

 f_f = specimen force scaling factor

 f_M = specimen moment scaling factor

h = elevation of floor height

 $h_f = story height$

 k_o = rotational stiffness of outrigger

k_{p,nl} = stiffness of nonlinear bar in coupling beam model

 k_p = SCFD post-slip stiffness

 k_{nt} = SCF axial stiffness of the PT tendons.

 k_u = unloading stiffness of the SCFD

ktc= stiffness of tension and compression only materials

 f_c' = specified concrete strength

 I_o = moment of inertia of outrigger beam

m = building mass

 r_q = ratio between the overturning resistance from gravity($M_{b,Pq} = P_q L_w/2$)

 $t_{\rm f}$ = thickness of wall flange

 t_w = thickness of wall web

 x_{cg} = distance from the rocking toe to the wall centroid

w =story weight

 α_f = outrigger relative stiffness factor

 α_c = coupled wall relative stiffness factor

 β_o = force modification factor

 β_x = shape factor for story shear

 δ_b = deformation in base damper

 δ_o = deformation in outrigger damper

 γ_a = energy modification factor to relate the energy dissipated by the system under monotonic pushover as compared to the energy dissipated by dynamic load when the hazard level increases from SLE to DBE.

 γ_b = energy modification factor to relate the energy dissipated by the system under monotonic pushover as compared to the energy dissipated by dynamic load when the hazard level increases from DBE to MCE.

 λ_p = ratio of design DBE base shear to yield base shear ($\lambda = Fp/Fy$)
$\lambda_x =$ load distribution at a given story x

$$\mu'$$
= friction coefficient between conical surfaces

 μ = friction coefficient between the friction pad and the steel plate

 μ_p = ductility the ratio of the DBE design dispalcement to the yield displacement ($\mu_p = \Delta_P / \Delta_Y$)

v = poisson ratio

- θ_{b} = ultimate rotation at wall base
- θ_{o} = ultimate rotation at outrigger
- θ_{pr} = plastic rotation of primary fuse system

 $\theta_{critical}$ = critical angle which governs the SCFD hysteretic shape

 θ_{se} = plastic rotation of secondary fuse system

 Φ_s = ratio of the DBE to MCE intensity

- Φ_p = ratio of the SLE to DBE intensity
- $\Delta_p = \text{DBE}$ design roof displacement
- $\Delta_u = MCE$ design roof displacement
- $\Delta_{p,e} = \text{DBE}$ elastic roof displacement
- $\Delta_{u,e}$ = MCE elastic roof displacement
- Δ_{ν} = yield roof displacement

 ΔE_{E1} = incremental elastic energy from increasing intensity from SLE to DBE

 ΔE_{ND1} = incremental nonlinear dynamic energy from increasing intensity from SLE to DBE

 $\Delta E_{\rm NM1}$ = monotonic nonlinear energy from increasing intensity from SLE to DBE

 ΔE_{E2} = incremental elastic energy from increasing intensity from DBE to MCE

 ΔE_{ND2} = incremental nonlinear dynamic energy from increasing intensity from DBE to MCE

 $\Delta E_{\rm NM2}$ = monotonic nonlinear energy from increasing intensity from DBE to MCE

Acknowledgments

There are numerous people to which I am grateful for the support throughout this incredible journey. I will forever be thankful for all the relationships formed during my graduate studies and hope to one day repay all of the support I have been given.

I want to express my sincere gratitude to my supervisor and mentor, Professor Tony Yang, for his support and guidance throughout my academic studies. Professor Yang introduced me to the world of research in the second year of my undergraduate degree. Without his support early on in my career, I would never have pursued an academic career. For this, I am eternally grateful.

I would also like to thank my supervisory committee member, Professor Perry Adebar, for his mentorship and advice throughout the development of the experimental program described in this thesis. I would particularly like to thank Professor Adebar for hours he dedicated to supporting me on the construction site. His support was vital during this time.

I am also profoundly thankful to my supervisory committee member Professor Geoffery Rodgers for his support and guidance throughout this project. I am particularly grateful for Professor Rodgers coming to China to help me there, and for his advice in the difficult decision to leave China at the onset of the Covid-19 outbreak.

Industry advice was critical to the practicality of design and construction. For this, I would like to thank Bob Neville, principal at RJC Engineers, for his invaluable guidance. I would also like to extend my appreciation to Rob Third, from George Third and Son, for his advice and generous donations in constructing the shake table specimen. Without his extreme generosity of allowing the specimen to be built on his property and providing construction support, this project would not have progressed as far as it has.

I also want to extend my gratitude to LMS reinforcing, who generously donated the reinforcing steel and placement. Their team did a fantastic job, and I am grateful for their support. I am also appreciative of Lafarge concrete for developing and donating the concrete mix. I would also like to thank Ryan Zhang, who provided construction support, carpentry, and concrete placing.

There have been several fellow graduate students who have provided instrumental support and help throughout my graduate studies. Please know that even though you are not named here, I am forever thankful for your support. I specifically want to thank Michael Fairhurst for his help with the probabilistic seismic hazard analysis provided in this thesis. I would also like to thank Henry Xu for his tireless efforts towards constructing the specimen in Canada and China. I could not have done this without you.

Last but certainly not least, I would like to thank my family. First, I would like to thank my partner Jeremy Atkinson, for his patience and support. Not only has Jeremy provided me with personal support, but his experience as a practising engineer has been a source of great technical and

practical advice. I am incredibly grateful for my children, Amelia and Levi, born during the last few years of this PhD. They accompanied me to conferences, construction sites, and to China for testing. Throughout it all, they have shown remarkable resiliency and flexibility, for which I am very proud.

To my father, James Tobber, thank you for supporting and challenging me to think critically throughout my life. Your coaching has led to my success. Additionally, I would like to thank my father for his invaluable help and support with my children, particularly during the pandemic. To my mother, thank you for teaching me to prioritise creativity in work and life. To my siblings (Ryan, Allen, Ben, Steven, and Laura), thank you so much for being a source of love, laughter, and friendship.

Dedication

I dedicate this thesis to my family: the old, the young, and the ones to come.

Chapter 1: Introduction

1.1 Background

The rapid urbanization worldwide has created significant challenges for sustainable city growth. An estimated 6.7 billion people, approximately 68% of the global population, will live in urban areas by 2050 (UN, 2019). With increased pressure for responsible use of urban land, tall buildings are a popular solution to meet high demands for housing and commercial space. Their ability to accommodate many people makes them critical to the economic, social, and cultural fabric of our growing cities.

Much of the world's high-rise construction takes place in seismically active regions. As a result, tall buildings must be designed to resist earthquake loads. One of the most prevalent seismic force-resisting systems (SFRS) in tall buildings worldwide is the reinforced concrete (RC) shear wall. In North America, RC shear walls are often integrated within a centralized core that houses elevators and stairwells, shown in Figure 1.1 (a). These RC core wall systems are advantageous due to their high strength, stiffness, and versatility for architectural expression.

RC core walls typically consist of flanged walls (shown in Figure 1.1 (b)), connected along one principal axis using diagonally reinforced coupling beams (shown in Figure 1.1 (c)). This configuration essentially consists of two distinct lateral force resisting systems along each principal axis: the coupled wall system (East-West in Figure 1.1 (b)) and the shear wall system (North-South in Figure 1.1 (b)). Depending on regional practice and layout requirement, coupled walls may be used for the SFRS along both principal axes (more common in US construction).

The use of core walls (as opposed to planar walls) as the SFRS in tall buildings is only recently gaining popularity in cities worldwide. Cities like Vancouver, Canada have been using core walls since the 1980s, but have not had a large magnitude earthquake since the 17th century. As a result, the core system remains relatively untested in the field. However, earthquake damage has been observed within planar RC walls, and it is thought that similar damage mechanisms could occur when these walls are integrated into a core.



Figure 1.1 Reinforced concrete core wall buildings: (a) Seattle tower (Moehle, 2007); (b) Floor layout 2-flanged wall configuration; and (c) reinforcing in RC core wall (adapted from Wallace and Naish, 2009)

Past earthquakes in Chile (February 2010) and New Zealand (February 2011) - countries that utilize modern building codes - have shown RC wall systems' adequacy to prevent collapse and achieve life-safety performance. However, in some RC wall buildings, significant damage has been observed. Specifically, earthquake damage has been observed in the RC coupling beams Figure 1.2 (a) and Figure 1.2 (c) and throughout the region near the base of the wall (shown in Figure 1.2 (a) and (b)) where axial strains are generally highest due to both gravity and overturning demands.

After the February 2011 Christchurch earthquake, numerous RC walls showed singular large cracks, and further investigation revealed that some vertical bars were fractured. Figure 1.2 (c) shows one of these walls where only one crack is observed at the far end of the wall, while Figure 1.2 (d) shows the fractured reinforcing steel within the wall. In these cases, single cracks opened and induced significant strains across a relatively short unbonded gauge length on the reinforcing steel until fracture. After the earthquake shaking, the cracks closed due to the large gravity loads, making the severity of the damage difficult to identify. Additionally, the fracturing of reinforcing steel, particularly in the critical section, is a life safety issue which will require extensive repairs or total demolition.



Figure 1.2 Damage in RC wall buildings: (a) conceptual; (b) coupling beam damage from Chile, 2010 earthquake (LATBSDC, 2010); damage to tall wall Christchurch, 2011 earthquake (Buchanan et al., 2011) (d) crack at far end of wall and (d) fractured reinforcing steel

The damage exhibited in RC wall buildings may result in widespread demolition, as exemplified following the February 2011 Christchurch earthquake. The damage observed from this earthquake resulted in the demolition of over 60% of the central business district (Kim et al., 2017), an area approximately the same size as downtown Vancouver, Canada. Additionally, almost half of the total number of RC wall buildings were demolished within the central business district, despite many of the buildings meeting their design objectives (Kim et al., 2017).

The following sections describe how using innovative technology (Section 1.1.1) and improved design methods (Section 1.1.2) can increase RC core wall buildings' seismic performance and limit earthquake damage.

1.1.1 Innovative technologies

Designers may use innovative technology to enhance the performance of the RC core wall buildings. For example, damped outriggers may be used to improve performance by lowering drifts

and base-moments within an RC core wall. An outrigger system consists of stiff girders or trusses that couple the RC core wall to exterior columns. An outrigger system adds rotational stiffness to the wall, reducing lateral displacements and core moment at the wall's base.

Although outriggers have been used for half a century to reduce wind displacements, they have only recently been adapted for seismic design. For example, in 2016 a 73-story building called the Wilshire Grand (Figure 1.3) was built using a large 3-story deep outrigger which incorporate Buckling Restrained Braces (BRBs). The core was extremely slender, with a height-to-width ratio of about 33. The BRBs are yielding elements which limit the force in the outrigger, protecting the core and columns from extensive damage, as well as providing energy dissipation.



Figure 1.3 Wilshire Grand, Los Angeles, California: (a) building photo (LABJ,2020) and (b) outrigger concept (Photo courtesy of Prof. Geoffrey Rodgers from the University of Canterbury, NZ)

Coupling beam rotations are another important performance objective that must be met to limit damage and increase seismic performance. In situations where the typical coupling beams do not meet their performance objectives, designers may consider using damped coupling beams. Damped coupling beams utilize embedded steel beams within the RC wall and a central damper or sacrificial yielding element within the beam. The combination of using a sacrificial element to dissipate the earthquake energy can limit the damage to the RC wall, and add ductility to the coupling beam element.

Damped coupled walls are only now just beginning to be implemented around the world and applications of their use can be found in new high-rise construction projects in China. For example, the Sancai building located in Beijing, China, (Figure 1.4 (a)) is designed with coupled walls at

the building perimeter. This 11-story building utilises replaceable steel links as a hysteretic damper, in lieu of RC coupling beams. China is also adapting damped coupling beams in their super-tall buildings within the core walls. For example, in Beijing, the China Zun Tower (Figure 1.4 (b)) utilizes steel fuses connected to steel embedded beams in parallel with RC coupling beams.



Figure 1.4 Examples of damped coupled beam implementation: a) Sancai building adopting a hybrid coupled wall system with replaceable steel coupling beams and b) China Zun Tower (Beijing Z15 Tower) adopting replaceable steel coupling beams (Ji and Malina Hutt, 2020)

Designers can also improve performance and limit earthquake damage by reducing the vertical strains using a low-damaging wall solution, such as a controlled rocking wall. Controlled rocking is the concept of only connecting the wall vertically to the foundation with dampers. This design approach allows the wall to rock on its foundation, dissipating energy through dampers, essentially creating a damped pin at the wall's base. To the author's knowledge, controlled rocking has not been implemented in high rise RC core wall construction. However, successful implementation of these systems has been completed globally on low-rise structures.

A notable application of controlled rocking technology is the Endoscopy Consultants' Building in Christchurch, New Zealand. This low-rise building includes both controlled rocking walls and frames. The walls are coupled with U-shaped flexural plates, which are steel yielding dampers configured in a U-shape. This structure is an important example, as it survived the strong shaking and aftershocks of the February 2011 earthquakes. Figure 1.5 shows the RC walls post-earthquake. As shown, only cosmetic damage was observed on the structural elements (wall and U-dampers).



Figure 1.5 Endoscopy Consultants' Building in Christchurch, New Zealand (a) wall post-earthquake and (b) U-dampers post earthquake (Buchanan et al., 2011)

Recently, new technology, such as damped outriggers, damped coupled walls, and controlled rocking walls, has been incorporated in RC wall systems to improve seismic performance. However, damped outriggers and damped coupled walls can still have damage concentrated at the wall base. Further, the controlled rocking base has not been applied to RC core walls or used in tall building applications. Therefore, there is a need for high-performance, low-damage, RC core walls for tall building applications.

1.1.2 Alternative design methods

Presently, most modern building codes use a prescriptive based design approach and do not have provisions for the latest technology developed for improving the seismic performance of RC core walls. This design approach is intended to achieve specific performance objectives, such as meeting the life-safety requirement, at a single hazard (e.g., Maximum Considered Earthquake (MCE) hazard). These design methods are based on broad classifications of building types and occupancies, and the actual performance is not assessed. As a result, the behaviour of the building after an earthquake shaking is unknown. Hence, some buildings may have higher performance than set out by the code, while others may only meet or have lower performance, even when satisfying the basic building code clauses.

The code-based design approach in Canada for normal importance buildings (i.e., typical highrise) only considers the MCE, which corresponds to 2% probability of exceedance in 50 years, without considerations for lower hazard (more frequent) earthquakes. Since there is no requirement to check performance at different hazards, buildings may underperform - at least with regard to the general public's expectation of performance – at smaller hazards.

More recently, recognizing the issues with the code-based prescriptive design approach, some designers are adopting a performance-based design approach to understand tall buildings' seismic performance. In a performance-based design, buildings usually are required to meet strict serviceability level earthquake (SLE) requirements, in addition to the MCE required by the prescriptive design approach. While generally it is acceptable for designers to use linear elastic analysis to design at the SLE hazard, at the MCE hazard, where inelastic action is expected, designers utilize a comprehensive nonlinear modelling approach to determine the building response. These nonlinear numerical models are peer-reviewed to ensure estimates of the response are acceptable.

A challenge with the performance-based design approach is that the building's performance is unknown until the building is designed and the nonlinear modelling is complete – the nonlinear model requires a completed preliminary design to be modelled. If the performance objectives are not met, the building must be re-designed in an iterative process. This process can be particularly problematic if the design solution is to change the geometry of structural elements when other aspects of the building design are complete (i.e., the architectural layout).

The state-of-practice to design structures to meet multiple performance objectives at different shaking intensities usually requires a nonlinear analysis performance-based design approach, which is cumbersome and involves a substantial amount of design effort and a rigorous peer-review process. To simplify this rigorous process, Yang et al. (2018) developed a simple design method called Equivalent Energy Design Procedure (EEDP). EEDP utilizes an energy-balance concept to design structures to satisfy strength and displacement limitations without iteration. Using EEDP allows engineers to design structures to achieve multiple performance objectives at different seismic hazard levels. However, the original derivation of EEDP was limited to structures that are not sensitive to higher modes and have basic elastic-perfectly-plastic hysteretic behaviour. It follows that modifications are needed to apply to RC core walls in tall buildings, which have higher mode dynamic responses.

1.2 Goals and objective

This dissertation seeks to contribute to the development of high-performance low-damage tall buildings through the creation of new technology and design methods. The overarching goals of this thesis are to develop:

- 1) an innovative high-performance RC core wall system which has a low-damage response under strong earthquake shaking.
- 2) a novel design methodology to design the proposed system for different performance objectives under different shaking intensities, in a simple non-iterative format.

These goals are accomplished through the following five objectives.

- 1. Conceptually develop and propose a new high-performance RC core wall system.
- 2. Quantify the dynamic characteristics of the new high-performance RC core wall system.
- 3. Develop a novel design procedure for the new high-performance RC core wall system to achieve different performance objectives at different shaking intensities.
- 4. Assess the seismic performance of the new high-performance RC core wall system.
- 5. Develop a large-scale shake table testing program which may be used to validate the behaviour of the new high-performance RC core wall system.

1.3 Methodology

In this thesis, a novel high-performance RC core wall system is proposed, named the controlled rocking outrigger core wall (CROCW) system. The CROCW system essentially consists of two unique SFRS along each principal axis. Along one principal axis is a controlled outriggered rocking wall (CORW). In the CORW system, an outrigger is situated at the roof, and the base is designed to rock. Dampers are incorporated within the outrigger and the rocking wall-base to give the system added energy dissipation. Along the other principal axis is the self-centering coupled wall (SCCW). In this system, self-centering conical friction dampers (SCFD) are incorporated within the coupling beams to dissipate energy and self-centre the wall. In addition, the base is designed to rock to ensure a low-damage response.

The novel equivalent energy design procedure (EEDP) developed by Yang et al., (2018), is modified to design the CORW and SCCW. EEDP utilizes an energy balance concept to design

structures to satisfy strength and displacement limitations without iteration. Using EEDP allows engineers to design structures to achieve multiple performance objectives at different seismic hazard levels. However, the original derivation of EEDP was limited to structures which are not heavily influenced by higher modes and with basic elastic-plastic hysteretic behaviour. In this thesis, the EEDP was modified to account for these aspects and to design the CORW and SCCW.

Extensive linear and nonlinear dynamic analysis methods are used to develop and evaluate the proposed systems and design methods under earthquake loads at various hazards. The systems and design methods are thoroughly investigated using numerical modelling under both bidirectional and unidirectional earthquake shaking.

The newly developed CORW and SCCW systems will be shake table tested to validate the behaviour of these new systems. Shake table testing will take place at the Multi-function shake table array facility in Shanghai, China. This thesis provides details on the specimen design and construction, and the planned experimental programme, but not the final testing.

The outcome of this thesis is two new SFRS which can have a high-performance under different levels of shaking and new design methods so that engineers can practically implement these structural systems.

1.4 Thesis organisation

This thesis is organized into thirteen chapters with the following content.

Chapter 1: Introduction describes the background, motivation, thesis goals and objectives, methodology, and the thesis organisation.

Chapter 2: Literature review of high-performance RC wall technologies presents detailed literature review of different high performance RC wall systems and components, including energy dissipation devices (metallic yielding, friction, viscous, and viscoelastic dampers), controlled rocking walls, pin-based walls, replaceable corner walls, damped coupled walls, viscoelastic coupled walls, and outrigger systems. The merits and gaps in literature of each of these systems are presented and the need for a new high-performance RC core wall system is discussed.

Chapter 3: Mechanism and design considerations of the CROCW proposes the CORW and the SCCW as alternative systems to the RC shear wall and RC coupled wall systems, respectively.

The design objectives of the plastic mechanisms are presented in this chapter. Additionally, the unique mechanical characteristics of these systems are described, which are key in the latter chapter design and understanding of seismic response.

Chapter 4: Dynamic characteristics of proposed earthquake resilient RC core wall system presents the dynamic characteristics of the CORW and SCCW which were studied using parametric analysis. Empirical equations, developed to estimate the dynamic responses of the proposed systems, are presented in this chapter.

Chapter 5: Proposed design procedure for the proposed CROCW presents the proposed design procedure of the CORW and SCCW systems using EEDP. A step-by-step design approach for EEDP is provided. This chapter demonstrates how the empirical equations, developed in Chapter 4, are used to modify EEDP. Additionally, new energy modification factors are presented which account for the unique hysteretic shape of the CORW and SCCW systems. This chapter concludes with the detailed design of four prototype buildings.

Chapter 6: Nonlinear modelling approach presents the nonlinear modelling approach for conducting unidirectional and bidirectional nonlinear time-history analysis for the proposed CORW and SCCW systems. The experimental validation of the numerical modelling approach for each structural component is presented. Additionally, the assumptions about gravity loads, mass distribution, and damping are presented.

Chapter 7: Probabilistic seismic hazard analysis and ground motion selection and scaling describes the Probabilistic Seismic Hazard Analysis (PSHA) used to develop the uniform hazard spectra. In addition, the procedure for selecting and scaling ground motions, based on deaggregation of the hazards, is presented.

Chapter 8: Seismic performance assessment of CORW presents the unidirectional nonlinear dynamic responses of the CORW, using the four prototype buildings designed in Chapter 5, the nonlinear modelling approach from Chapter 6, and the ground motions selected in Chapter 7. The results are used to validate EEDP and study the system behaviour.

Chapter 9: Seismic performance assessment of SCCW presents the unidirectional nonlinear dynamic responses of the SCCW, using the four prototype buildings designed in Chapter 5, the

nonlinear modelling approach from Chapter 6, and the ground motions selected in Chapter 7. The results are used to validate EEDP and study the system behaviour.

Chapter 10: Bidirectional performance of CROCW system presents the bidirectional nonlinear dynamic responses of the CROCW, using the four prototype buildings designed in Chapter 5, the nonlinear modelling approach from Chapter 6, and the ground motions selected in Chapter 7. The results are used to validate EEDP and study the system behaviour.

Chapter 11: Comparison between CORW and SCCW with alternative RC core wall systems presents comparisons of the seismic performance of prototype buildings designed with alternative RC wall systems. The seismic performance of the CORW and SCCW systems are compared to three alternative designs. The comparison between important parameters, such as displacements, shear forces, and moments are presented.

Chapter 12: Design and construction of shake-table testing describes the work completed towards shake table testing a large-scale specimen designed using the CORW and SCCW systems. An overview of the testing facility, specimen scaling, design, and construction is presented. In addition to the completed work, this chapter also describes the planned work for the experimental testing program. An overview of the phases of testing, instrumentation plan, and the ground motion details is provided in this chapter. A detailed numerical model is developed of the specimen, and predicted nonlinear responses are provided.

Chapter 13: Summary of work and recommendations for future work highlights some of the key contributions in this thesis and provides several recommendations for future work.

Chapter 2: Literature review of high-performance RC wall technologies

2.1 Overview

This chapter presents a literature review on different structural systems and components which can facilitate a low-damage response (i.e., higher performance) in reinforced concrete (RC) walls. The chapter begins with a brief overview of the behaviour of RC shear walls and RC coupling beams, followed by various methods of mitigating damage and improving the performance of typical RC shear walls and RC coupled walls.

Most low-damage systems use energy dissipation devices of some type. Therefore, before introducing low-damage systems, a high-level summary of various damping devices is provided. Specifically, the basic mechanics and behaviour as well as suggestions on some of the advantages and disadvantages of metallic yielding dampers, friction dampers, and viscous dampers are provided.

Following the summary of energy dissipating devices are detailed descriptions of different innovative solutions for low damage shear walls, low damage coupled walls, and damped outrigger wall buildings. A discussion on the merits of existing literature and the knowledge gaps are identified and discussed. Overall, this chapter informs the need for the development of the new systems proposed in Chapter 3.

2.2 Reinforced concrete core walls

Reinforced Concrete (RC) core wall systems are a widely used structural system to resist lateral loads in tall buildings. In these systems, a series of RC wall segments are connected by RC coupling beams to form an interconnected wall assembly. The coupling beams significantly enhance the structure's overturning resistance, stiffness, and energy dissipation as compared to isolated wall piers. The behavior of coupled walls depends strongly on the nonlinear behavior of the coupling beams joining adjacent wall segments.

The construction technique used for the beam varies, but the most common two schemes utilize either conventional beam reinforcing, shown Figure 2.1 (a), or diagonal reinforcing, shown in

Figure 2.1 (b). Compared with conventional beams, diagonally reinforced coupling beams exhibit a more ductile behavior, whereby they can sustain large inelastic rotational demands and withstand many cycles of loading without significant degradation (e.g., Paulay and Binney, 1974; Naish et al., 2013). For this reason, diagonally reinforced coupling beams are commonly used in tall buildings in seismically active regions along the West Coast of Canada and the United States

The concept of diagonally reinforced coupling beams was first tested in 1974 by Paulay and Binney (1974). They tested four coupling beam specimens with low length-to-depth ratios (between 1 - 1.29), of which three were diagonally reinforced and one was conventionally reinforced. They were able to show that diagonally reinforced coupling beams offer superior performance to conventionally reinforced beams in terms of ductility and energy dissipation.

Figure 2.1 (a) and (b) shows the hysteretic response for conventional and diagonally reinforced coupling beams, respectively. Conventionally reinforced beams exhibit much more pronounced pinching behavior than their diagonally reinforced counterparts due to crack formation and bar slippage, causing lower stiffness at high ductility demands. The diagonally reinforced coupling beam generally exhibits much less pinching behavior due to the direct transfer of shear through the diagonal. However, the beam still exhibits softening behavior due to bar slippage at the face of the walls. The confinement around the diagonal bars contributes significantly to the in-cycle and cyclic degradation, as reaffirmed by several experimental studies (Barney et al., 1980; Galano and Vignoli, 2000; Adebar et al., 2001; Naish et al, 2013; and Lim 2016).

Different damage patterns are observed in specimens with lower length-to-depth ratios when compared to specimens with higher length-to-depth ratios. Figure 2.2 shows the damage patterns of diagonally reinforced coupling beams with (a) stocky beams (i.e., length-to-depth < 2) and (b) slender beams (i.e., length-to-depth > 2). As shown, in stocky specimens, diagonal cracking formed at the same locations as the diagonal reinforcing. The concrete spalled in a diagonal pattern, and for most specimens the ultimate failure mode was buckling of the diagonal bar after the concrete spalled away. This behavior was observed in tests with length-to-depth ratios which ranged from 1-1.5 by Paulay and Binney, 1974; Galano & Vignoli, 2000; Canbolat et al., 2005.

Cyclic loading of slender beams results in several flexural cracks are observed in addition to diagonal cracking. Strength loss was eventually observed in conjunction with spalling at the wall interface, diagonal bar buckling, and concrete crushing. This behavior was exhibited in several

slender diagonally reinforced coupling beam tests with length-to-depth ratios ranging from 2.4 to 3.3 by Adebar et al., 2001; Fortney et al., 2008; Naish et al., 2013; and Lim et al., 2016.



Figure 2.1 Reinforcing layout of RC coupling beams: (a) longitudinally reinforced (Naish et al., 2013); and (b) diagonally reinforced (Naish et al., 2013)



Figure 2.2: Cracking and failure pattern of (a) length-to-depth = 1.5, specimen P07 (Galano & Vignoli, 2000) and (b) length to depth = 2.74 (Adebar et al., 2001).

Damage may occur within the shear wall in addition to the coupling beams. The behavior of shear walls under cyclic loads have been studied extensively in the past half century (e.g., Massone & Wallace, 2004; Adebar et al., 2007; Dazio et al., 2009). Under high lateral loads, flexural cracks open, and vertical reinforcing yields over a region called the plastic hinge.

In typical design of shear walls the vertical reinforcing must stay constant over the plastic hinge region, to avoid concentrations of strain resulting in nonductile behaviour. The wall above the plastic hinge region is capacity designed to ensure a well predicted mechanism. Given the change in axial load over the height of the building, it is usually unnecessary to increase the reinforcement above the plastic hinge region. Reinforcement detailing varies between building codes, but generally, the requirement is to have sufficient ties within boundary elements, to avoid buckling of the vertical reinforcing.

The damage which occurs within the plastic deformations of the reinforcing steel and cracking of concrete is difficult to repair after the earthquake. Due to the large axial loads on walls in a core configuration, flexural cracks will close, and the level of yielding in the reinforcing steel may not be easily determined. Recognizing these difficulties, various alternative mechanisms have been proposed to enhance the performance of shear wall and coupled wall systems. Recent developments on the improved performance of shear wall and coupled wall systems are summarized in the following sections: *2.2 Energy dissipating devices, 2.3 Innovative RC shear wall systems; 2.4 Innovative RC coupled wall systems;* and *2.5 Damped outriggered RC walls.*

2.3 Energy dissipating devices

The inclusion of energy dissipation devices (or dampers) can enhance the structural performance of any building systems, including shear wall or coupled wall buildings. As dampers are such an intrinsic component to the development of earthquake resilient systems, it is worth further exploring the different damper types, behaviors, applications, and limitations.

All the methods discussed and used within this dissertation are referred to as passive damping. Passive damping, as opposed to active/semi-active, relies on the natural movement of the building to be activated, as oppose relying on computer control and/or an external power source. Generally, passive dampers are connected where the most motion will occur, maximizing energy dissipation. It is important to note that the addition of dampers often requires some degree of modification to the system. For example, adding dampers to an RC core wall using an outrigger (discussed in Section 2.6) results in both additional damping through devices incorporated within an outrigger and additional stiffness through the incorporation of the outrigger system.

Energy dissipating devices can usually be differentiated by four categories: yielding metallic dampers, friction devices, viscous fluid dampers, and viscoelastic dampers, shown in Figure 2.3. Each category has a unique set of behaviours and characteristics, which may be used for different applications, discussed in the following sections.

	Viscous fluid	Viscoelastic	Metallic	Friction
Basic construction			ADAS	-
Idealized hysteretic behaviour	eg B Displacement	bisplacement	g Displacement	Displacement
Idealized mechanical	Force Disp.	Force Disp.	Idealisation unavailable	Force Disp.
Advantages	 Activated at low displacements Minimal restoring force Simple modeling Temperature - independent properties 	 Activated at low displacements Provides restoring force Linear behaviour Simple modeling Strong long-term behaviour 	 Stable hysteretic behaviour Long-term reliability Insensitive to ambient temperatures Materials and behaviour are familiar to practicing engineers 	 Sliding interfaces may change with time (reliability concern) Strong nonlinear behaviour Permanent deformations possible if no restoring force provided.
Disadvantages	• Possible fluid leaks (long-term reliability concern).	 Limited deformation capacity Temperature and frequency dependant properties 	 Device damaged after earthquake (may require replacement) Nonlinear behaviour (may require nonlinear analysis) 	 Large energy dissipation per cycle Insensitivity to ambient temperature

Figure 2.3 Summary of damper characteristics (adapted from Symans et al., 2008)

2.3.1 Metallic yielding dampers

Metallic yielding dampers dissipate the earthquake energy through the plastic deformations of metals. Several metallic yielding dampers have been developed in the past, including buckling restrained braces (BRBs) (Iwata, Midorikawa, & Koyano, 2018; Xie, 2005) and plate yielding devices (Ma et al., 2011; Yang et al., 2019; Yang, Banjuradja, & Tobber, 2018).

Buckling restrained braces are typically loaded axially and consist of a steel yielding member encased by another material such as concrete. The encasing material prevents the yielding member from buckling in compression, which promotes a nearly symmetric tension and compression response. BRBs are loaded unidirectionally and can be incorporated within a variety of systems, such as braced frames and truss systems.

In some circumstances it is beneficial to have the device loaded in shear, like in the Linked column frame (Dusicka & Iwai, 2007) or H-frame structures (Etebarian, Yang & Tung, 2019). An example of a shear loaded device is the Honeycomb damper (Yang et al, 2019), shown in Figure 2.4 (a). This device consists of a steel plate with welded flanges and is loaded parallel to the flanges. The web consists of unique hole cut-out patterns designed to promote ductile yield mechanisms, while the flanges remain elastic to provide connection to the structural system.

Figure 2.4 (b) shows the hysteretic shape of the Honeycomb damper. Like most metallic yielding dampers, the hysteretic shape follows the behaviour of steel, where isotropic and kinematic hardening is observed. In the case of the Honeycomb damper, the force begins to degrade once the web undergoes inelastic buckling.



Figure 2.4 Honeycomb damper: (a) conceptual view and (b) hysteretic response (Yang et al., 2019)

Metallic dampers are popular due to their simplicity in design, reliability in behaviour, and low costs. A potential drawback to these devices is the coupling of stiffness and force. For example, in the design of the Honeycomb damper, a lower desired force requires a smaller thickness of the steel plate resulting in a lower initial overall stiffness of the device. A low initial stiffness device can lead to various issues regarding the system level design, including, higher structural period of the building and higher displacement demands required to engage the device.

2.3.2 Friction dampers

Friction dampers dissipate earthquake energy through sliding contact friction. In the past researchers have investigated various configurations and loading conditions for these friction devices including the Pall friction damper (Pall & Marsh, 1982), symmetric friction damper (Gregorian et al., 1992), and the Asymmetrical Friction Connection (Rodgers, 2017). Figure 2.5 (a) shows a basic configuration of a symmetric friction damper.

In most friction dampers, plates are pretensioned together, creating a normal force. The pretensioned bolts pass through slotted hole connections to permit lateral movement, which develops the sliding friction force. Friction dampers do not begin to absorb energy until the slip force is reached, resulting in a rigid-plastic hysteretic response, shown in Figure 2.5 (b). This rigid-plastic, or stick-slip, behaviour makes friction dampers superior for energy dissipation; however, it has no self-centering tendency which may result in residual displacements when the structure no longer contains enough energy to overcome the static friction force – i.e., the device may "lock up" at some residual displacement.



Figure 2.5 Slotted bolted connection: (a) Schematic and (b) hysteretic response (Gregorian et al., 1993)

To address these residual displacements, some researchers have proposed features to introduce self-centering behaviour, such as using sloped plates, in addition to providing sliding friction. Such devices include the Ring Spring Damper (Filiatrault et al., 2000), Resilient Slip Friction Joint (RSFJ) (Hashemi et al., 2017); and the Self-centering Conical Friction Damper (SCFD) (Yang et

al., 2020). Introducing a self-centering mechanism modifies the hysteresis from a rectangular shape to a flag-shaped hysteresis. For example, the RSFJ dissipates energy through the sliding friction between sloped plates. The sliding motion results in displacements both vertically and laterally. The normal force is provided with bolts which are connected in series with low axial stiffness springs. The springs allow for vertical movement and reduces the axial forces on the bolts. The vertical movement, results in an increased clamping force, which causes a post-slip stiffness in the hysteretic response. This hysteretic behaviour results in an efficient device, which has a high post-slip stiffness and a self-centering capacity.

Overall, friction dampers are advantageous due to the high energy dissipation, and nearly rigid initial stiffness. The high initial stiffness may be particularly important for flexible structures under serviceability conditions, as the device will not be activated and initiate sliding until a specified threshold load is reached, which can help keep structural movements within acceptable limits. However, a disadvantage to friction dampers is the difficulty in preparing and maintaining a predictable sliding surface, especially over a 50 year, or more, design life.

Many proposed devices (Gregorian et al., 1993, and Tremblay, 1993) exhibit a noticeable peak in the hysteretic response before sliding initiates, due to the difference in static and dynamic friction coefficients. These peaks need to be well-defined to apply capacity design principles to the rest of the system. It has also been observed that some devices utilizing pretensioned bolts exhibit a concentration of heat generated around the bolt holes, which may degrade the sliding surfaces and change the hysteresis over time (Tremblay, 1993). There is also little research on the long-term behaviour of such devices. For reliability, they may require some level of maintenance and inspections, and special lubricants and oils to ensure the device will perform as expected. Practical issues such as fireproofing, corrosion protection, and inspection access may further complicate the maintainability of the devices.

2.3.3 Fluid viscous dampers

Fluid viscous dampers dissipate energy through the movement of viscous fluids. As a means of energy dissipation devices for earthquakes, fluid viscous dampers have been extensively studied by multiple researchers (e.g., Landi et al., 2014, and Reinhorn et al., 1995). A basic configuration of a viscous damper is shown in Figure 2.6 (a) and the hysteretic response is shown in Figure 2.6 (b). Viscous dampers dissipate energy through the movement of high viscosity fluids in or around

the moving piston. The movement of the piston head results in a pressure differential between chambers, forcing the fluid through orifices and expending energy.



Figure 2.6 Typical viscous damper (Constantinou et al., 1992)

The mechanism at work is velocity-dependant which may have implications for conventional design procedures (e.g., the design cannot be easily displacement based). Like the friction damper, the reliability of the device needs to be considered, with leaks or loss of fluid containment being of primary concern. It is worth noting, fluid damper inter-device variability is low, however, uncertainty in in-service velocity needs to be considered in overstrength factors. This is particularly important in capacity design, where the ultimate force of the damper must be accurately predicted to ensure ductile response.

2.3.4 Viscoelastic dampers

Viscoelastic dampers dissipate earthquake energy through the deformation of viscous elastic materials. Figure 2.8 (a) shows the basic configuration of Viscoelastic dampers. Viscoelastic dampers often consist of layers of viscoelastic material rigidly bonded to metal plates (Montgomery & Christopoulos, 2015). Like fluid viscous dampers, viscoelastic dampers also have velocity dependence, but the hysteresis occurs at an incline related to the elastic stiffness. The hysteretic behaviour of the viscoelastic damper is essentially a linear spring in parallel with viscous damper (Figure 2.7 (b)).

Viscoelastic dampers are advantageous due to high energy dissipation, and an initial stiffness which helps in recentering the device. However, like viscous dampers, viscoelastic dampers have a velocity dependence, which should be considered in the design procedure. The properties of the viscoelastic material may also make them sensitive to temperature changes. Experimental testing of each device at temperature and frequencies of interest should be conducted prior to implementation into the structure.



Figure 2.7 Viscoelastic material: (a) basic configuration and (b) hysteretic response (Montgomery & Christopoulos, 2015)

2.4 Innovative RC shear wall systems

Researchers have investigated various methods of incorporating energy dissipation devices with RC shear walls to improve the performance. The following are some of the recent developments on innovative, low-damage RC shear wall systems.

2.4.1.1 Controlled rocking

The yielding mechanisms of typical fixed based RC walls is effective in dissipating energy, however, the damage exhibited through excessive yielding of reinforcing steel may be difficult to repair after strong earthquake shaking. To address this damage, researchers have investigated controlled rocking, which is the concept of intentionally designing the structure to be vertically separated from the foundation, such that it can freely move vertically during earthquake shaking. This movement protects the structural system from undergoing excessive strains and damage. Due to high axial loads, usually provided by PT tendons, the wall only initiates rocking at large lateral loads. Prior to the initiation of rocking the wall behaves like a fixed based wall.

PRESSS (Precast Seismic Structural System) is a US-Japan seismic research program and includes a series of experimental tests to study the behaviour of precast rocking walls (Priestley, 1991). As part of this program, Kurama et al. (1999) presented research on unbonded post-tensioned jointed precast rocking walls. This system consists of multiple precast wall panels that are connected to the foundation using unbonded post-tensioned tendons. The post-tensioned tendons provide the restoring force required for self-centering and a small level of energy dissipation through yielding at large drifts. Spiral confined reinforcing was placed within the rocking toe to mitigate damage due to pounding. Experimental testing showed excellent self-centering hysteretic behaviour with repairable damage at the rocking toe. The yielding of post-tensioned tendons limited the lateral force resulting in minimal damage in the rocking toes. A potential drawback of this system is the low energy dissipation, which can result in high displacements.

Kurama (2000) introduced joint rocking walls that included supplemental viscous dampers to dissipate energy. In their study, linear viscous fluid dampers are mounted diagonally in-plane with the walls. Nonlinear analysis demonstrated that supplemental damping results in lower drifts and accelerations compared with systems without supplemental damping. Restrepo and Rahman (2007) proposed a rocking wall system that incorporated mild steel yielding dampers. These dampers are cast in the wall and foundation. This system demonstrated excellent energy dissipation. However, extensive damage was observed on the rocking toes. Additionally, the dampers' location makes them difficult to repair or replace after a strong earthquake shaking.

Marriott et al. (2008) tested controlled rocking walls with externally mounted dampers on a shake table. They studied four unique wall designs with different damping configurations: 1) no added damping; 2) four viscous dampers; 3) four viscous dampers and two mild steel dampers; 4) two mild steel dampers. Each wall was post-tensioned such that the hysteretic backbone of each specimen was similar. Externally mounted steel plates mitigated the damage in the rocking toe. Figure 2.8 shows an example of the tested specimen. The dampers mounted near the ends of the wall, shown in Figure 2.8 (a) and Figure 2.8 (b), are the viscous dampers, while the dampers mounted in the center of the wall are the mild steel dampers.

The hysteretic response of the combined viscous and mild steel dampers, shown in Figure 2.8 (d), shows higher energy dissipation than mild steel dampers alone, shown in Figure 2.8 (c) when cycled at 0.5hz. However, when dynamically testing the walls under near-field and far-field

motions, the results showed that the specimen used only mild steel dampers had lower peak displacements under lower intensity motions. In comparison, the combined viscous-mild steel damper specimen demonstrated the lowest displacements under high-intensity earthquakes. This response is due to the velocity dependence of the viscous dampers, where the lower shaking intensities result in lower overall effectiveness of the viscous damper devices. All wall specimens demonstrated low damage and negligible residual deformations. However, their study was limited to low-rise planar walls, without consideration for tall building applications.



Figure 2.8 Marriott et al. (2008) wall specimens (a) left: Unit 2 (4 viscous dampers); (b) centre: Unit 3 (4 viscous dampers and 2 TCY mild steel dampers); (c) top right: experimental response of the specimen with two mild steel dampers: (d) bottom right: experimental response of specimen with four viscous dampers and two mild steel dampers.

Sritharan et al. (2015) introduced a controlled rocking Precast Wall with End Columns (PreWEC). O-connectors, which are low-yield mild steel connectors, are mounted between the rocking wall and the end columns to dissipate the earthquake energy. Figure 2.9 shows the concept of the PreWEC system. Two specimens were constructed and experimentally tested: 1) conventional RC wall and 2) PreWEC. The result showed that the PreWEC demonstrated superior performance when compared with the conventional design in terms of residual displacements and repairable damage.

Twigden, et al. (2017) conducted a series of static cyclic tests on controlled rocking systems, including two single rocking systems and two PreWEC systems. The cyclic response of the walls using different configurations of hysteretic damping was systematically studied. The results showed that the use of O-connectors within the PReWEC system resulting in higher capacity and energy dissipation. However, the PreWEC system has higher residual displacements when compared to the controlled rocking system. This study was later expanded in by Twigden and Henry (2019), who conducted shake table testing on two PreWEC specimens and one controlled rocking specimen. The results confirmed the superior energy dissipation of the PreWEC system. An important observation from this study was the dynamic loading showed significantly lower residual drifts in the PreWEC than were observed under static cyclic loading. However, their study was limited to low-rise planar walls, without consideration for tall building applications.



Figure 2.9 The PreWEC system including different configuration options (Sritharan et al., 2015)

2.4.1.2 Controlled rocking with tall building applications

Most research on controlled rocking walls has been limited to low-rise buildings. However, there is growing interest in applying controlled rocking mechanisms to taller buildings. For example, Wiebe and Christopoulos (2009) suggested the use of multiple rocking walls over the height of the

building. They numerically investigated the seismic response of buildings from 4 stories to 20stories using simple lump hinge modeling techniques. Their results show that including multiple rocking points throughout the building height reduces higher mode forces. This result was later verified in 2015 by Khanmohammadi and Heydari, who used more detailed distributed plasticity modeling of the controlled rocking system.

In 2015, Lu and Panagiotou studied the use of a dual system consisting of base-isolated and a controlled rocking core wall system, shown in Figure 2.10. They studied a 20-story building designed with three different systems: a) new dual system, the controlled rocking base, and a fixed base system. Their results indicated the dual system exhibited low-damage response, the controlled rocking system demonstrated minimal damage, and the fix-base structure experience significant damage at the maximum considered earthquake. However, their study was limited in the number of buildings studied, and detailed design procedures were not examined.



Figure 2.10 Concept of dual isolated building by Lu and Panagiotou (2015)

Tong and Christopoulos (2020) conceptualized a new dual mechanism for transferring moments and shears to the foundation. This new system, called MechRV3D transfers the overturning moments through a damped controlled rocking mechanism. The shear mechanism is a series of bracing components that allow for yielding and shear ductility. This innovative system is shown in Figure 2.11. Tong and Christopoulos (2020) show that this system could potentially reduce both shear and moment demand within the core wall. Additionally, the tall building system is essentially damage-free after strong earthquake shaking. However, their study was limited to one prototype building, and further investigations are required to develop design guidance on this novel system.



Figure 2.11 Idealized configuration of the MechRV3D system by Tong and Christopoulos (2020)

2.4.1.3 Pin-connected walls

In addition to controlled rocking, other systems have been developed as alternatives to traditional plastic hinging in RC walls. In 2012, Qu et al. proposed a RC wall with a pin support at the base connected to moment frames to retrofit existing structures. Steel shear links are situated between the shear wall and the frame to dissipated energy. Similarly, in 2017, Wada et al. developed a system which consisted of a planar wall connected to a pin at the center of the base. Either end of the wall is connected to buckling restrained braces, which dissipate the earthquake energy through the yielding of steel. Figure 2.12 shows the tested specimen. Their results showed that the damped

pin could effectively transfer the shear forces while providing a stable, rapidly repairable flexural mechanism for energy dissipation.

Although effective for planar walls, these pin-connected walls can not be applied to RC core wall buildings. In RC core wall buildings, flanged walls are connected in at least one direction with coupled beams. The wall is expected high lateral loads along each axis. If the PSW were used in either the flanges or the web within an RC core wall, it would act as vertical restraint under bidirectional loading and damage the pin or the RC walls.



Figure 2.12 Plastic-hinge-supported wall (PSW) specimens (units: mm): (a) PSW-1: plastic hinge with gear-shaped support and (b), PSW-2: plastic hinge with pin support (Wada et al., 2017)

2.4.1.4 Replaceable cornered walls

Liu and Jiang (2017) proposed a new type of shear wall, which essentially replaces the corner of the wall with steel yielding dampers. The reduced section of the wall is strengthened in shear using

steel plates. Figure 2.13 (a) shows the concept recommended by Liu and Jiang (2017). Although this system is effective in limiting damage, the walls may have some residual deformations post-disaster.

To mitigate residual displacements, Xu et al., 2018 proposed using a self-centering damper called ring springs as the energy dissipating component, shown in Figure 2.13 (b). The ring springs help in re-centering the wall after strong earthquake shaking.

Although the replaceable corner component shear walls have proven effective in reducing damage in planar shear wall systems, in their present configurations, these systems could not be integrated into a centralized core wall system. If the replaceable corner component were used in either the flanges or the web within an RC core wall, the fixed portions would act as vertical restraint under bidirectional loading and could be damaged.



Figure 2.13 Replaceable corner component shear walls: (a) concept by Liu and Jiang, 2017 and (b) selfcentering concept by Xu et al., 2018.

2.5 Innovative RC coupled wall systems

Researchers have investigated various methods of incorporating energy dissipation devices with RC coupled walls to improve the performance. The following are some of the recent developments on innovative, low-damage RC coupled wall systems.

2.5.1.1 Steel-reinforced concrete coupling beams

Steel-reinforced concrete (SRC) coupling beams, sometimes called Composite coupling beams, refer to the use of a structural steel section embedded into the boundary zones of reinforced concrete walls. These beams resist earthquake loads through the yielding of the structural steel section. Although not necessarily a low-damage component on its own, many of the damped coupling beams discussed later in the chapter utilise embedded steel beams as an important portion of the structural system. Hence, it is important to note the behavior of the SRC beams.

In the SRC beams, coupling forces are transferred to the wall through bearing of the embedded beam. The advantage of SRC beams over diagonally reinforced coupling beams is the energy dissipating capacity and ductility of the steel sections, while allowing for a small depth. Figure 2.14 shows an example SRC beam under construction in Vancouver house, Canada.



Figure 2.14 Embedded steel-reinforced concrete beams (Poh, 2020)

The behavior of the SRC beams is well established, with research extending back to 1980 (Markakis & Mitchell, 1980; Aktan & Bertero, 1984; Gong & Shahrooz, 2001; Harries et al., 1993; Motter et al., 2017) and in 2010, AISC released seismic provisions for design special walls and ordinary walls with SRC beams.

This literature describes how the performance of SRC beams is highly dependent on the embedment length. SRC beams with improper embedment length results in gaps forming in the RC wall under cyclic loading. The outcome is a highly pinched hysteresis, resulting in reduced

energy dissipation (Motter et al., 2017). Figure 2.15 shows the comparison between two W310x143 SRC beams, where SRC1 (shown in Figure 2.15 (a)) has an embedment length of 32in and SRC2 (shown in Figure 2.15 (b)) has an embedment length of 24in. As shown, the longer embedment length shows superior performance in terms of both ductility and energy dissipation.

It is important to note that although the SRC beams make use of the excellent energy dissipation of structural steel sections, the stiffness of the SRC beams reported by Motter et al., 2017 demonstrated that the primary sources of deformations prior to yielding occurred due to slip and extension from the embedment regions and the overall construction of shear and flexural deformations from the steel section was relatively small.



Figure 2.15 SRC beams from Motter et al. (2017): (a) specimen SRC1 (embedment length = 32") and (b) specimen SRC2 reduced embedment length (embedment length = 24")

2.5.1.2 Fused coupling beams

Although diagonally reinforced and SRC beams are efficient energy-dissipating components; strong earthquake shaking may still result in damage at the wall-beam interface and yielding in the wall boundary reinforcing. To mitigate this damage, researchers investigate the Fused coupled walls, which is essentially an RC core wall which utilizes dampers in place of RC coupling beams.

Fortney et al. (2007) proposed a replaceable fused steel coupling beam. In this coupling beam design, a central steel beam section, called a fuse, absorbs all the inelastic deformation and damage. Connecting the fuse to embedded steel sections through slip-critical bolted connections facilitates efficient replacement after an earthquake, leading to a more resilient coupled wall system. Chung et al. (2009) proposed a similar system, using sliding friction dampers as a fuse. Nonlinear time

history analysis demonstrated that using the friction damper resulted in reduced roof displacements and overall damage.

Ahn et al. (2013) conducted large-scale quasi-static tests on a conventional longitudinally reinforced coupling beam, a coupling beam with a yielding metallic damper fuse, and a coupling beam with a friction damper fuse. The yielding metallic damper dissipates energy through the yielding of plates in parallel. The plates are cut in a specific shape to ensure that the plates endure constant curvature during bending, maximizing energy dissipation. The friction dampers dissipate energy through the sliding of two plates, which are bolted together with slotted holes. The tests showed that the coupling beams with dampers exhibited a more stable hysteretic shape, high energy dissipation, and minimal damage to the walls compared with the conventional RC beams.

Ji et al. (2016) conducted a series of quasi-static tests to study the behaviour of fused coupling beams and evaluate various end connection plates. Their tests demonstrated the high energy dissipation of the fused coupling beam and rapid repairability. In 2017, Ji et al expanded this study to include the RC slab within the component test. In this study, four types of slabs were examined: 1) a composite slab which incorporated steel studs to achieve composite action between the coupling beam and the slab; 2) a bearing slab where the RC slab was bearing on the steel coupling beam but had no mechanical connection; 3) an isolated slab where there was a gap between the coupling beam and the slab; and 4) a slotted slab where slots are inserted within the slab to allow the movement of the coupling beam. Their experimental studies concluded that the isolated floor slabs provided superior performance and limited damage when compared with the other slab connections.

Substituting the conventional RC coupling beam with rapid repairable energy dissipation devices is an effective tool in mitigating the damage within the coupling beam and the beam-wall connection. However, the use of these devices, while maintaining a fixed concrete base, can result in significant damage in the wall. For example, Cheng et al. (2017) tested a low yielding point steel damper device connected to embedded steel beams and compared it to the fixed based wall with diagonally reinforced coupling beams. The result showed more moderate damage in the wall at the coupling beam connection; however, extensive damage was observed in both wall specimens. As shown in Figure 2.16 extensive damage can be observed in the base of the wall for both systems: (a) damped coupled wall and (b) conventional coupled wall.



Figure 2.16 Coupled wall testing by Cheng et al. (2017): (a) diagonally reinforced coupled wall and (b) low yield point steel coupling beams

2.5.1.3 Viscoelastic coupling damper

Christopoulos & Montgomery (2013) proposed a viscoelastic coupling damper (VCD) to enhance the coupled wall system's performance subjected to wind and moderate seismic load, shown in Figure 2.17. This device combines viscoelastic materials in addition to steel yielding beams. The VCD provides an additional velocity-dependent viscous damping to the building and increases the damping in all modes of vibration. Analysis of the system response under wind and moderate seismic load showed that using the VCD resulted in a reduction in the maximum displacement, acceleration, base shear, and base moment. Additionally, for large earthquakes, VCD is designed to yield, hence the wall is protected from damage at the coupling beam locations.



Figure 2.17 Viscoelastic coupling damper (VCD) concept from Christopoulos & Montgomery (2013): (a) reinforced concrete coupled walls in a 58-story tower in Toronto; (b) VCD; (c) exaggerated deformed shape of coupled walls; (d) exaggerated viscoelastic deformed shape; (e) exaggerated viscoelastic-plastic deformed shape; and (f) design hysteresis envelopes. VE, viscoelastic; MCE, maximum credible earthquake; SLE, service-level earthquake.
MacKay-Lyons et al. (2018) studied the nonlinear performance of tall buildings designed with VCDs using nonlinear analysis software Perform3D (Computers and Structures, 2018). Their studies showed that VCDs resulted in lower accelerations under service level earthquakes and lower interstorey drifts and base shears at moderate earthquakes, when compared to conventional RC wall buildings. Additionally, the added viscous damping provided by the viscoelastic materials in the VCD allowed for reduced wind demands. Overall, their study demonstrated that adding damping in leu of conventional RC results in improved seismic and wind performance.

Similar to fused coupled walls, coupled walls which have VCDs, are still at risk from damage near the base of the wall. Therefore, further investigations are needed to develop low damage coupled wall solutions.

2.6 Damped outriggered RC walls

An outrigger system consists of stiff girders or trusses that couple the RC core wall to exterior columns, as shown in Figure 2.18. An outrigger system adds a rotational stiffness to the RC core wall, reducing lateral displacements and core moment at the base of the wall. Outriggers can be oriented in either the shear wall direction (Figure 2.18 (a)), the coupled wall direction (Figure 2.18 (b)), or both directions in a core wall. Although outriggers have been used for half a century to reduce wind displacements, they have only recently been adapted for seismic design.

Smith and Willford (2007) proposed the use of viscous dampers in outrigger systems for tall buildings. Their study showed that the damped outrigger virtually eliminated the dynamic effects of wind, leading to 30% reduction in the quantity of concrete required and a 2% increase in floor area compared with a conventional building.

Zhou et al. (2016) compared the behaviour of a conventional buckling brace with a BRB as the energy dissipating element in an outrigger using a numerical and experimental study. Additionally, they studied the use of BRBs in high-rise buildings using nonlinear time history analysis. Their results showed that BRB elements can increase the damping on the building by 0.5% and the BRBs protects the structure from severe damage under high-intensity earthquake shaking.

Huang and Takeuchi (2017) used complex eigenvalue analysis to optimize the outrigger's location for a single-damped-outrigger building. Their results indicated that the optimal outrigger location typically falls between 50%-80% of the overall building height. They conclude that the outrigger

system's governing response is mostly controlled by the first mode shape, first-mode damping ratio, and first-mode period.



Figure 2.18 Damped outrigger system concept: (a) damped outriggered shear wall; and (b) damped outriggered coupled wall

Lin et al. (2018) investigated the influence of outrigger elevation, outrigger truss flexural stiffness, BRB axial stiffness, and perimeter column axial stiffness on roof drift and acceleration. They studied this using spectral analysis and nonlinear time history analysis. Their study indicated that the outrigger's optimum location is at 60-80% of the height of the building. According to their research, an optimal outrigger location could reduce the wall's rotation by 22%-42%, depending on the height of the building.

Lin et al. (2018) studied the use of an improved viscously damped outrigger. Their outrigger includes a top horizontal chord member on the outrigger truss preventing a break between the perimeter column and the outrigger, allowing the outrigger to maintain some degree of stiffness. They showed that structures that used this new outrigger system showed a larger damping ratio, smaller drifts, and lesser damage.

Morales-Beltran et al. (2018) investigated using multiple viscously damped outriggers. Their study showed that it is economical to use a viscously damped outrigger at 70% and 50% the building height rather than using a single damped outrigger.

Xing et al. (2018) studied using outriggers damped with both viscous dampers and BRBs. The objective of their study was to determine the optimal combination of viscous and BRB dampers. They studied the combination of viscous dampers and BRBs using small scale shake table testing and finite element analysis. The results showed that when using only two energy dissipation outriggers, combining viscous and BRB dampers provided superior performance. However, the performance decreases when more energy dissipation outriggers are used. Their studies showed that it was more beneficial to include the BRB at taller heights than the viscous dampers.

Overall, the current literature shows that the use of outriggers in tall buildings is an effective way to increase the overall performance of the structure. Generally, the optimal location for an outrigger with hysteretic dampers is at 60%-80% of the building height. However, the outrigger is often located at the roof for ease of construction and architectural purposes.

Although the outrigger system is proven to be effective in limiting roof displacements, all current literature assumes a fixed core base, where the core wall ultimately undergoes damage under strong earthquake shaking. The combination of low-damage rocking walls with a damped outrigger system has not yet been investigated.

2.7 Summary and conclusions

In this chapter, a state-of-art review of different high-performance RC wall systems was presented. This chapter demonstrated how, although several other low-damage solutions have been developed for RC walls, few tall building applications have been studied. Further, the few studies on tall buildings have been limited to a small sample size of buildings. Additionally, although some of the latest technology shows increased seismic performance, damage still occurs in wall's base and at the coupling beam wall interface. It follows that there is a gap in knowledge in the development of high-performance, low damage tall RC core wall systems. The following chapter (Chapter 3) presents a new high-performance RC wall system's conceptual development to address this gap in knowledge.

Chapter 3: Mechanism and design considerations of the CROCW

3.1 Overview

In this chapter, a novel high-performance RC core wall system is presented. This system, named the controlled rocking outrigger core wall (CROCW), essentially consists of two independent systems: the controlled outriggered rocking wall (CORW) and the self-centering coupled wall (SCCW). In this chapter, the plastic mechanisms of each system are discussed, and the critical characteristics of each system are described. Specifically, this chapter describes the behaviour of the outrigger system, the mechanics of controlled rocking wall base, and the mechanics and hysteretic behavior of the SCFDs. An essential outcome of this chapter is the derivation of the relative stiffness factor, α_f . This factor is used throughout this thesis (particularly in Chapter 4 and Chapter 5) to analyse the dynamic behaviour and the development of the design procedures for this system.

3.2 Introduction

The literature review in Chapter 2 showed the earthquake-resilient systems' effectiveness to reduce damage on low-rise RC planar walls. However, there has been minimal investigations on low-damage high-rise RC core wall buildings. For example, the experimental studies on controlled rocking systems have been limited to unidirectional loading on low-rise planar walls. Additionally, some of the mechanisms of other low damage plastic hinge walls, such as the Pin shear wall (PSW), are limited to unidirectional and would not function in the bidirectional bending mechanisms required of core walls. Damped coupled walls have demonstrated lower amounts of damage when compared with conventional coupled walls. However, the damage is still observed within the plastic hinge region of the base of the wall. Similarly, damped outrigger systems have been shown to have higher performance and limit damage than RC core walls. However, the damage was still observed within the plastic hinge region. For the aforementioned reasons there is a need for a robust earthquake-resilient tall RC core wall system which can provide limited damage in earthquakes.

This chapter proposes a novel earthquake-resilient RC core wall system that is low-damage under strong earthquake shaking. Two new systems are proposed along each axis of a centralized core wall to improve the coupled wall and shear wall systems' performance. The new system was developed in consultation with engineering practitioners familiar with designing tall buildings across Canada and the USA. Early discussions with these practitioners made evident the necessity of including the RC core wall as part of the SFRS to make the new system more adaptable to the industry. By keeping the RC core wall, these innovative buildings can use similar architectural plans as conventional buildings and the local workforce is trained to build most of the structure, with little need for specialization or re-training.

Figure 3.1 shows the proposed earthquake-resilient RC core wall's conceptual view, called the controlled rocking outrigger core wall (CROCW) system. The newly proposed system is "dual-fused," where two unique energy dissipation mechanisms (primary fuse system and secondary fuse system) act to resist earthquake shaking. In the CROCW system, along one principal axis, the Controlled outriggered rocking wall (CORW) is proposed, a novel system where the primary fuse system is a damped outrigger located at the top of an RC core wall, and the secondary fuse system is a damped controlled rocking base. Along the other principal axis, the Self-centering coupled wall (SCCW) is proposed, where novel self-centring coupling beams which utilize the self-centring conical friction dampers (SCFD), developed by Yang et al. 2020, are used as the primary fuse system. Dampers within the outrigger and the controlled rocking base use unidirectional friction dampers to uncoupling the damper's stiffness and yield force and combining the CORW and the SCCW results in an interdependent earthquake-resilient core wall system that can resist lateral loads bidirectionally.



Figure 3.1 Conceptual drawing of earthquake-resilient core wall building

The proposed CROCW system utilises the conventional RC core wall, with three additional new technologies: (1) novel self-centering coupled beams (described in Section 3.3); (2) damped outrigger system (described in Section 3.4); and (3) controlled rocking base (described in Section 3.5). The following sections describe the unique energy dissipation mechanisms and some of the practical considerations in the development of the proposed CROCW system.

3.3 Self-centering coupling beams

The SCCW direction consists of self-centering coupling beams. The self-centering coupling beams are comprised of two embedded steel beams connected with Self centering conical friction dampers (SCFDs), developed by Yang et al. in 2020. Figure 3.2 shows the conceptual view of the self-centering coupling beam. In this configuration, the SCFDs are designed to dissipate the earthquake energy through sliding friction, while the embedded steel beams are designed to remain elastic.



Figure 3.2 Self-centering coupling beam (image courtesy of Hamza Chaudhry)

The SCFD was explicitly developed for the SCCW with significant contributions from the author of this dissertation, however, the primary investigation of the SCFD was conducted as part of Henry Xu's thesis work (Xu, 2020).

Figure 3.3 shows the assembly view of the SCFD. As shown, the SCFD consists of an outer clamp plate A, an outer clamp plate B, and the inner plate. In the configurations presented, end plates are added at the end of clamp plate A and clamp plate B, which can be used to connect the structures. To prevent the relative in-plane rotational movement between the outer clamp plate A and outer clamp plate B, a stopper is added. In the SCFD, the energy is dissipated through sliding friction on both the cone surfaces, and the sliding of the Inner plate and the Outer clamping plate B.



Figure 3.3 Exploded view of SCFD (Yang et al., 2020)

The mechanism of the SCFD is shown in Figure 3.4. As shown, the SCFD is permitted to move in all in-plane directions. The inner and outer plates are confined by a male cone and a female cone.

The SCFD will always self-center due to PT tendons forcing the inner and outer plates back to original position, after any translational movement. The configuration of the SCFD results in two sliding surfaces (between the inner and outer plates), which may be tuned to have different friction coefficients to obtain a desired hysteretic response.



Figure 3.4 SCFD having all-direction motion (from Yang et al., 2020)

Figure 3.5 shows the hysteretic behaviour of the SCFD. The SCFD is designed to slide after the shear force exceeds the initial sliding force, P_0 . After that, the SCFD is designed to move with a post-slip stiffness, k_p . When the motion reverses the direction, the SCFD is expected to drop in force to the unloading force P_u . The SCFD is designed to move back to the initial position with the unloading stiffness until it reaches zero displacement with the residual force, P_r .



Figure 3.5 Damper movement and the corresponding hysteretic behaviour (Yang et al., 2020)

The derivation of the key hysteretic parameters can be found in Yang et al., 2020, the following is a short summary of these parameters. The initial sliding force, P_0 , can be calculated using Equation 3.1. The maximum sliding force, P_m , is shown in Equation 3.2. The post sliding stiffness, k_p , is shown in Equation 3.3.

$$P_0 = F_{pt}^0 \cdot B1$$
 Equation 3.1

$$P_m = \left(F_{pt}^0 + k_{pt} \cdot \Delta_m \cdot tan\theta_c\right) \cdot B1$$
 Equation 3.2

$$k_p = \frac{P_m - P_0}{\Delta_m} = k_{pt} \cdot tan\theta_c \cdot B1$$
 Equation 3.3

where F_{pt}^0 is the initial post-tensioning force in the tendons; B1 relates the cone angle (θ_c) , the friction coefficient between the friction pad and the steel plate (μ_f) , and the friction coefficient between conical surfaces (μ') , $B1 = \left(\frac{tan\theta + \mu'}{1 - \mu' \cdot tan\theta_c} + \mu_f\right)$; Δ_m is the maximum displacement of the SCFD; and k_{pt} is the axial stiffness of the PT tendons.

The unloading stiffness of the SCFD can be calculated based on the geometry and is shown in Equation 3.4.

$$k_u = k_{pt} \cdot tan\theta_c \cdot B2$$
 Equation 3.4

where
$$B2 = \left(\frac{\tan\theta_c - \mu'}{1 + \mu' \cdot \tan\theta_c} - \mu_f\right)$$
.

Variables F_{pt} , k_{pt} , and $tan\theta$ must always positive, therefore, B2 will govern the sign of the unloading force P_u , the residual force P_r and the unloading stiffness k_u . More specifically, if B2 is positive, P_u , P_r , and k_u must be positive. Similarly, if B2 is negative, P_u , P_r , and k_u must be negative. The sign of B2 depends on $\theta_{critical}$, which is a function of μ and μ' , and is shown in Equation 3.5. A slope of $\theta > \theta_{critical}$, will result in a positive B2 while a slope of $\theta_c < \theta_{critical}$, results in negative B2. In cases where the slope $\theta_c = \theta_{critical}$, B2 will be zero.

$$\theta_{critical} = \operatorname{atan}\left(\frac{\mu + \mu'}{1 - \mu \cdot \mu'}\right)$$
Equation 3.5

Figure 3.6 shows the different SCFD hysteresis types. In Figure 3.6 (a), (b), and (c) correspond to B2= positive, B2= zero, and B2= negative, respectively. Under each of these three cases the SCFD can center itself.



Figure 3.6 Three different types of hysteresis (a) Type I (b) Type II (c) Type III

3.4 Outrigger system

The outrigger system consists of mega-columns, outrigger dampers, and outrigger trusses or beams (Figure 3.7 (a)). The outrigger system provides the CORW direction with both added stiffness and added energy dissipation. The dampers within the outrigger system use symmetric friction dampers, which have an elastic-perfectly-plastic (EPP) hysteretic behaviour. The outrigger mega-column and beams are designed elastically; hence, the outrigger system has an EPP behaviour.

Mega-columns and outrigger dampers primarily resist axial loads, having a low shear resistance, and can therefore be idealized as a rotational spring (Figure 3.7 (b)). Figure 3.7 (c) shows the response of the equivalent rotational spring, where the spring rotational stiffness is k_0 and the yield moment is $M_{o,y}$.

The yielding of the outrigger dampers, which are assumed to have symmetric elastic-plastic behaviour, provides the only nonlinearity in the outrigger system. The remaining outrigger columns and the beams are capacity-designed to remain elastic. Hence, the yield moment of the equivalent spring consists of the damper yield force $(f_{o,y})$ multiplied by the length of the outrigger truss or beam (L_o) , the result is given in Equation 3.6.

$$M_{o,y} = f_{o,y}L_o$$
 Equation 3.6

The rotational spring properties can be calculated by assuming the outrigger beam behaves as a beam connected to two springs in series (Figure 3.7 (a)). The first spring in series has the stiffness of the damper (k_d) while the other represents the stiffness of the column (k_c) . The equivalent rotational stiffness (k_o) of an outrigger beam is calculated using Timoshenko Beam Theory and is shown in Equation 3.7.

$$k_o = \left[\frac{2L_s}{G_o A_v L_o} + \frac{{L_s}^2}{3E_o I_o L_o} + \frac{2}{k_{com} {L_o}^2}\right]^{-1}$$
 Equation 3.7

where k_{com} is the axial stiffness of the column (k_c) and the damper (k_d) in series $(k_{com} = (1/k_c + 1/k_d)^{-1}$; I_o is the moment of inertia of the outrigger beam; L_s is span between the wall and the mega-column; v is Poisson ratio of the outrigger material; E_o is the elastic modulus of the outrigger material; and L_o is the length of the outrigger beam. In these calculations, the equivalent beam is assumed to have a rectangular cross section with a shear area of $A_v = \frac{5}{6}A_{o,g}$ and a shear modulus of $G = \frac{E_o}{2(1+v)}$; where $A_{o,g}$ is the gross area of outrigger beam.

It may not always be efficient to use a beam as an outrigger system. For instance, steel beams are limited in the sizes availability, and it would be difficult to obtain the high stiffness required by outrigger systems. As an alternative, designers may use RC beams, however, the RC beams add additional weight, and utilize a significant amount of space. As such, large steel outrigger trusses may be used.



Figure 3.7 Behaviour of the outrigger system in the CORW: (a) CORW system; (b) equivalent CORW system; (c) force-deformation of equivalent outrigger spring.

In some cases, the wall may extend into the truss. This is common when the outrigger is located near mid-height of the building. In this case, the determination of the rotational stiffness assumes that wall is rigid, relative to the truss. Figure 3.8 shows the configuration where the wall extends into the truss system. Equation 3.8 shows the derived equation for a truss system with a rigid wall.

$$k_o = \left(\frac{38L_s}{3E_sA_c (\tan^2\theta_t)L_o^2} + \frac{6d_o}{A_oE_s(\sin^3\theta_t)L_o^2} + \frac{2d_o}{A_oE_sL_o^2} + \frac{2}{k_{com}L_o^2}\right)^{-1}$$
 Equation 3.8

where E_s is the elastic modulus and A_o , A_c , θ_t , L_s , d_o and L_o are shown in Figure 3.8.

In Figure 3.8 A_o = area of the diagonal vertical truss, Ac = area of the top and bottom chords, Lw = length of the wall, and Ls = span from the wall edge to the centerline of the outrigger column, d_o = outrigger depth, Lo = total length of the truss, and θ_t = angle of inclination of the inside diagonal truss.



Figure 3.8 Outrigger roof truss configuration

3.4.1 Static behaviour of outrigger system

The addition of rotational stiffness provided by the outrigger system is advantageous as it provides moment resistance to the system as well as rotational stiffness, reducing drifts and displacements. The advantages of the added stiffness from outrigger systems can be exemplified by applying an inverted triangular static loading triangular loading distribution on a Euler-Bernoulli beam (representing the RC core wall) with a rotational spring (representing the outrigger). Using this simple model, the ratio of the outrigger moment (Mo) to the total overturning moment (Mt) can be derived and is shown in Equation 3.9.

$$M_o/M_t = \frac{(a^3 - 6a + 8)}{8} \alpha_f$$
 Equation 3.9

where M_o is the moment in the outrigger; M_t is the total overturning moment; *a* is the ratio of the elevation of the outrigger to the total elevation of the wall; and α_f is a relative stiffness parameter, which is a unitless parameter between 0-1, shown in Equation 3.10. For a cantilever wall with no outrigger, $\alpha_f = 0$. Contrastingly, for an infinitely rigid outrigger, $\alpha_f = 1$.

$$\alpha_{\rm f} = \left(\frac{1}{\frac{E_c I_w}{ak_o H} + 1}\right)$$
 Equation 3.10

where E_c is the elastic modulus of the RC wall; I_w is the moment of inertia of the core wall; H is the total height of the wall; and k_o is the rotational stiffness of the outrigger system described previously.

Figure 3.9 (a) shows the ratio of outrigger moment to total overturning moment for different relative stiffness factors, α_f , and outrigger locations using Equation 3.9. As shown, higher outrigger stiffness results in more significant outrigger moment resistance. Additionally, the lower the outrigger is situated on the building, the greater the resistance is provided by the outrigger.

The ratio of the outrigger building roof displacement compared with the roof displacement of a system without an outrigger ($\Delta_{\alpha_f=0}$) is shown in Equation 3.11. As shown, for this theoretical model, this ratio is entirely dependent on a and α_f .

$$\Delta/\Delta_{\alpha_{\rm f}=0} = 1 - \frac{5}{22} a\alpha_{\rm f}(2-a)$$
 Equation 3.11

Figure 3.9 (b) graphically shows Equation 3.11 under different *a* and α_f . As shown, the outrigger is most effective when located near the top of the building, where a rigid outrigger ($\alpha_f = 1$) gives a 70% reduction in roof displacements. Additionally, the result shows that outrigger stiffness is more sensitive at locations high in the building. Hence, there is more benefit to adding stiffness to the outrigger near the top of the building rather than at near the bottom for this model. It is

important to note that these are theoretical relations under static loads, and the behavior of the outrigger system will depend on the dynamic characteristics. The dynamic characteristics are analysed in more detail in *Chapter 4: Dynamic characteristics of proposed earthquake resilient RC core wall system* of this dissertation.



Figure 3.9 Ratio of elastic static response of outrigger systems comparing the (a) outrigger moment to the total overturning moment and (b) the roof displacement compared to the displacement of a system without an outrigger (i.e., distribution of outrigger $\alpha_f = 0$)

3.5 Controlled rocking base

The controlled rocking base was selected as the CROCW's secondary fuse system to ensure a lowdamage response under high intensity, bidirectional earthquake shaking. Figure 3.10 shows the conceptual view of the controlled rocking base, exemplified on a RC core wall. It is important to note that experimental studies at the time of writing this dissertation are limited to planar walls, and therefore, there are no studies on flanged walls.

Typically, research has shown that controlled rocking bases can have a good performance if the design consists of a wall-base damper, bedding layer, base armouring, and a shear key. In this study, the wall-base dampers utilise symmetric friction dampers. Friction dampers are ideally suited to the controlled rocking base, as the rigid stiffness of the dampers allow them to be near instantly engaged at the onset of rocking.

A layer of high-strength grout placed between the foundation and the rocking wall is called a bedding layer. Research has shown that bedding layers provide a flat surface makes the rocking behavior consistent and predictable, while not including bedding layers results in severe damage at the underneath contacting surface (Henry, 2011).



Figure 3.10 Conceptual view of controlled rocking base (courtesy of Tianyang Qiao)

Base armouring is used to protect the wall from local damage, due to concentration of compression stress on the area which the wall contacts the foundation. There are various methods to include base armouring. For example, Marriott et al. (2008) proposed and tested a rocking wall system which included confining steel channels which were welded reinforcing bars are specially designed to protect rocking toes, shown in Figure 3.11. This system resulted in minimum damage after many cycles of testing.



Figure 3.11 Base armouring specimen example (Marriott et al. (2008))

Ensuring sufficient shear restraints is also a key component of controlled rocking base design. For many controlled rocking applications, the expected sliding friction resistance is sufficient However, additional mechanical shear restraints may be provided, such as a shear key may be provided.

Figure 3.12 (a) shows the controlled rocking base in the CORW system, and Figure 3.12 (b) shows the global base moment-rotation (i.e., $M_b - \theta_b$) relationship of the controlled rocking base and the local force-deformation (i.e., $f_b - \delta_b$) relationship of the base dampers at different stages of cyclic loading. As shown in Figure 3.12, the yielding base moment $(M_{b,y})$ is a combination of the restoring force from axial load (P_g) and the yielding force of the dampers $(f_{b,y})$. The yielding base moment $(M_{b,y})$ can be estimated using Equation 3.12.

$$M_{b,v} = f_b(L_w + 2L_d) + P_g L_w/2$$
Equation 3.12

where $f_{b,y}$ is the yield force of the base damper; L_w is the length of the wall; L_d is the length from the edge of the wall to the damper; and P_g is the axial load in the wall.

As shown in Figure 3.12, the initial loading portion of the force-deformation relation of the controlled rocking base is governed by the wall's elastic behaviour (Segment 1, which is between the origin and point A). After Point A, the wall rocks, and the base dampers are engaged (Segment 2, which is between points A and B), and the global stiffness is controlled by a combination of the base dampers and the wall in series. If the base damper can be approximated as a rigid-perfectly plastic behaviour, then the global stiffness between Points A and B can be approximated using the elastic stiffness of the wall. However, if the damper has elastic-plastic behavior, the stiffness between point A and B will represent the elastic stiffness of the wall and the elastic stiffness of the damper in series.

After Point B, the base damper will start to yield, and the force-deformation of the controlled rocking base system plateaus (Segment 3, which is between points B and C). At Point C the loading is reversed, and the force-deformation of the controlled rocking base follows the same stiffness as in Segment 2. At point D, the damper has unloaded, such that it has zero force, but residual deformation. Between points D and E, this vertical displacement in the wall is reduced to zero, through forcing the damper into compression. At Point E the dampers have reached yielding in

compression (Segment 5, which is between points E and F), at which point the force in the controlled rocking base develops a plateau. Once the wall closes the gap from rocking, the force-deformation response follows the stiffness of the wall (Segment 1).



Figure 3.12 Behaviour of the controlled rocking base: (a) CORW system and (b) Controlled rocking base behaviour

The behaviour of controlled rocking in the SCCW direction is complex when considering the interdependence of the coupling beam forces and the wall base. The yielding of coupling beams under lateral loads induces a substantial uplift force in one of wall piers, referred to as the "tension wall", and an equal opposite compression force in the other wall pier, referred to as the "compression wall".

The application of these tension and compression forces results in different rocking capacities at the base of each wall pier. Figure 3.13 shows an example free-body diagram of forces at the base of a SCCW wall. From the free-body diagram, the moment is derived and shown in Equation 3.13 and Equation 3.14 for tension wall pier moment ($M_{T,c}$) and compression wall pier moment ($M_{C,c}$), respectively. In Figure 3.13, B_w is the width of the wall pier; x_{cg} is the distance from the rocking toe to the wall centroid; and P_P is the contribution from the coupling beams; fdt is the damper force

in the "tension wall"; f_{dc} is the damper force in the "compression" wall, and P_g is the gravity force of the wall pier

$$M_{T,c} = f_b B_w / 2 + (P_g - P_p)(B_w - x_{cg})$$
 Equation 3.13

$$M_{C,c} = (P_g + P_p)(x_{cg}) + f_b B_w/2$$
Equation 3.14

where P_g is the gravity load in one wall pier; P_P represents the coupling axial force.

In a static pushover analysis P_p would simply be the summation of the forces within all coupling beams. However, it is well established that coupling beam shears have a high mode response which results in many of the coupling beams shearing in different directions along the height of the wall. As a result, the axial force (P_p) can be lower than the summation of the yield force. These dynamic characteristics are further analysed in *Chapter 4: Dynamic characteristics of proposed earthquake resilient RC core wall system* of this dissertation.



Figure 3.13 Rocking mechanism free-body diagram.

3.6 Summary

This chapter proposed two new low-damage SFRSs for tall building application: the controlled outriggered rocking wall (CORW) and the self-centring coupled wall (SCCW). Combined, these

systems form a third system called the controlled rocking outrigger core wall (CROCW) system. These systems are "dual-fused" and have a primary fuse system and a secondary fuse system.

The CORW primary fuse system is a damped outrigger, and its secondary fuse system is a damped controlled rocking base. This chapter presented equations to relate the dampers to the wall moment and estimate the outrigger system's stiffness. Additionally, this chapter presented the controlled rocking base's hysteretic behaviour and equations that relate the wall base moment to the damper forces.

The SCCW system's primary fuse system consists of novel self-centring coupling beams called Self-centering conical friction dampers (SCFDs), and its secondary fuse system is a damped controlled rocking base. This chapter presented the behaviour of the SCFD and equations to estimate the hysteretic response. Additionally, this chapter presented some of the challenges of designing a controlled rocking coupled wall system. Equations that relate the wall base moment to the damper forces were presented.

Using a controlled rocking base as the secondary fuse mechanisms in the CORW and SCCW allows this system to function efficiently under bidirectional loading. This chapter's outcome is two unique systems that may be integrated into a centralised core wall to create a third unique system called the CROCW—as a low-damage solution to RC core walls.

This chapter presented plastic mechanisms and some high-level design considerations of the CROCW, CORW and SCCW systems. In the following chapter (Chapter 4), the CROCW, CORW and SCCW systems' dynamic characteristics are explored, and empirical design equations for estimating the dynamic responses are presented.

Chapter 4: Dynamic characteristics of proposed earthquake resilient RC core wall system

4.1 Overview

In this chapter, the CORW and SCCW systems' key dynamic characteristics were identified using modal response spectrum analysis. This chapter proposes new empirical equations, developed based on parametric analysis, which can estimate the dynamic response of the CORW and SCCW systems. These equations and factors are later used to modify the EEDP developed in Chapter 5.

4.2 Introduction

Understanding the dynamic characteristics of the proposed CROCW systems is critical in developing the design procedures later described in Chapter 5. In this chapter, the dynamic characteristics were studied using Linear dynamic analysis (LDA). The trends observed in LDA give insights into the seismic response of multi-degree-of-freedom (MDOF) structures. LDA is often used in performance-based design to estimate the response of structures expected to undergo minimal inelastic behaviour, such as assessing the SLE hazard (LATBSDC, 2020). Additionally, building codes have long since permitted the use of LDA to estimate seismic design demands on structures, even though these structures are expected to experience extensive inelastic behaviour (NBCC 2015 and ASCE, 2016). For these reasons, LDA, specifically, Modal response spectrum analysis (MRSA), was used in this chapter to study the seismic response of the proposed CROCW system.

MRSA was used to estimate the proposed earthquake-resilient RC core wall system's dynamic response. The MRSA was programmed using MATLAB (2016) and OpenSees (PEER, 2000). A total of 1920 archetype buildings with unique building geometry were developed. The archetype buildings were analysed using two different design spectra (Vancouver and Montreal) to understand the effect of spectral shape on the dynamic response. Empirical design equations were developed. The results showed that the recommended equations could estimate the dynamic response reasonably well.

4.3 Archetype development for parametric analysis

The archetype buildings, shown in Figure 4.1, are composed of two symmetric C-shaped walls which have an outrigger in the shear wall direction (along the X-axis) and coupled along the other (along the Y-axis). Figure 4.1 (a), (b), and (c) shows the generic X-axis elevation view, Y-axis elevation view, and the plan view of the archetype buildings, respectively. In all analyses, the thickness of the wall in the coupled direction (t_f) is assumed to be 1.3 times the thickness of the wall in the shear wall direction (t_w). The RC elastic modulus, E_c , is assumed to be 30 GPa for all buildings. A variety of parameters were studied, including: the story height (h_f), number of floors (n_f), length of coupling beam (L_{cb}), length of core in y-axis ($L_{w,y}$), length of core in x-axis direction ($L_{w,x}$), web wall thickness (t_w), flange wall thickness (t_f), and story mass (m_f). Additionally, a characteristic property unique to the CORW, called the relative stiffness factor (α_c), ranged between 0-1 were developed. Table 4.1 shows the parameters included in this study.

The relative stiffness parameter, α_f , shown in Equation 3.10 below is a unitless parameter which defines the relative stiffness of the outrigger and the RC core wall in the CORW system. The relative stiffness parameter, α_f , is a function of the rotational stiffness of the outrigger (k_o defined in Chapter 3), the elastic modulus of the concrete (E_c), the gross moment of inertia of the wall (I_{w,y}) and the height of the wall (H). The relative stiffness factor differentiates the CORW from a cantilever wall system: a cantilever wall without an outrigger has a relative stiffness factor of $\alpha_f = 0$, while an infinitely rigid outrigger has a relative stiffness factor of $\alpha_f = 1$. The value of α_f does not influence the behaviour of the SCCW.

$$\alpha_{\rm f} = \left(\frac{1}{\frac{E_c I_{w,y}}{k_o H} + 1}\right)$$
 from Equation 3.10

The coupling beam relative stiffness factor, α_c , shown in shown in Equation 4.1, is a unitless parameter which relates the stiffness of the coupling beams to the RC core wall stiffness in the SCCW system. A value of 1 indicates that the coupling beams are infinitely rigid, while a value of $\alpha_c = 0$ indicates that the coupling beams do not provide any stiffness. Essentially, a value of $\alpha_c = 1$ indicates the two C-shaped walls act like a single cross section in bending, while a value of $\alpha_c = 0$

indicates that the two C-shaped walls act as individual cantilever walls in bending. The value of α_c does not influence the behaviour of the CORW.

$$\alpha_c = \frac{1}{1 + \frac{E_c A_w}{H \sum_{x=1}^{n_f} k_{cb,x}}}$$
Equation 4.1

where $k_{cb,x}$ is the shear stiffness of the coupling beam at a given floor, x.

Each archetype is analyzed for two different sites (downtown Vancouver and downtown Montreal) in Canada. The spectra are taken from the National Building Code of Canada (NBCC, 2015). These spectra were selected to provide sufficient variability in the spectra shape. Figure 4.2 shows the two response spectra normalized to the maximum spectral acceleration (max (Sa) is 0.85g and 0.45g for Vancouver and Montreal, respectively) to demonstrate the variation in spectra shape. As shown, the two spectra give suitable variations in spectral shape, where the Montreal site has a steeper drop in spectral accelerations for natural periods greater than 0.1s, when compared with Vancouver.

Systematically varying these properties, assuming the CORW and SCCW directions are analysed as individual models, results in 1920 unique building models, noting that α_f and α_c are independent of the direction, and considering the two different site spectrum results in 3840 individual MRSA.



Figure 4.1 Parameters studied for earthquake-resilient core wall: (a) CORW elevation view; (b) SCCW elevation view; and (c) plan view.

Property	Variable	Value	Units
Number of stories	n _f	15, 30, 45	
Floor height	hs	3, 4	m
Mass per floor	m _f	600, 800	tonne
SCCW wall length	L _{w,y}	8, 12	m
CORW wall length	L _{w,x}	8, 12	m
Length of coupling beam	LCB	1.5, 2.5	m
Thickness of wall web	t _w	0.6, 1	m
Coupling beam stiffness parameter	α _c	0.5, 0.6, 0.7, 0.8, 0.9	
Relative stiffness parameter	$\alpha_{\rm f}$	0.1, 0.3, 0.5, 0.7, 0.9	

Table 4.1 Summary of the prototype models used for the higher mode effect study

Note: Total number of models = $N_{dis}xN_{nf}x N_{hs}x N_{mf}x N_{Lw,x}x N_{Lw,x}x N_{Lw,x}x N_{tw}x N_{\alpha} = 2 x 3 x 2 x 2 x 2 x 2 x 2 x 5 = 1920$



Figure 4.2 Normalized uniform hazard spectrums for Montreal and Vancouver. In this figure Sa is the spectral acceleration and T is the structural period.

4.4 Numerical models

The structural models were developed using OpenSees (PEER, 2000). For computational efficiency, the models CORW and SCCW directions are analyzed independently using 2-dimensional models. Figure 4.3 (a) and Figure 4.3 (b) shows the modeling approach for the CORW and the SCCW, respectively. Masses are considered equally distributed at each story level. The RC core walls are assumed to have a constant cross section over the building height, as a result, the inter-story stiffness is a constant value for every story. The RC core walls are modeled using *BeamColumn* elements. The coupling beams are modelled using elastic shear-springs with a stiffness, k_{cb}, centered on rigid beam elements. The outrigger is modeled using an elastic spring

with a rotational stiffness, k_0 . The controlled rocking base is assumed rigid until yielding is initiated, hence, in these models, the base is assumed fixed.



Figure 4.3 Modeling approach for higher mode effects for (a) CORW and (b) SCCW.

4.5 Linear dynamic analysis of CORW system

Eigenvalue analysis was conducted on each of the 960 unique prototype building geometry in the CORW direction and MRSA was conducted on each of the two building sites, resulting in 1920 analysis in the CORW direction. The following sections described the dynamic response of the CORW system and empirical equations used to estimate this response, the values from the analysis are tabulated in Appendix A.

4.5.1 Natural periods of vibration (T₁, T₂, T₃)

Equation 4.2, Equation 4.3, and Equation 4.4 shows empirical equations for the first, second and third modal periods of the CORW, respectively. Figure 4.4 shows comparison of the results obtained from the MRSA vs. the predicted equation. The result shows the equations provided can predict the structural period very well. In general, as α_f increases, the period decreases. But the maximum reduction is only 37%, which implies that increasing stiffness of the outrigger has a limited effect on the structural period.

$$T_1 \approx (1 - 0.37\alpha_f) T_{1_{\alpha_f=0}}$$
 Equation 4.2

$$T_{2} \approx \frac{\left(1 - 0.37\alpha_{f}\right)}{6} T_{1\alpha_{f}=0}$$
Equation 4.3
$$T_{3} \approx \frac{\left(1 - 0.37\alpha_{f}\right)}{16} T_{1\alpha_{f}=0}$$
Equation 4.4

where α_f is the relative stiffness factor for the outrigger and $T_{1_{\alpha_f=0}}$ is the natural period of the system without an outrigger (i.e., $\alpha_f = 0$) and is estimated using Equation 4.5.

$$T_{1_{\alpha_f=0}} \approx 1.86 \sqrt{\frac{WH^3}{gE_c I_{w,y}}}$$
 Equation 4.5

where W is the total weight; H is the wall height; E_c is the modulus of elasticity of RC; g is the acceleration due to gravity (g = 9.81m/s²) and $I_{w,y}$ is Moment of inertia of the core wall



Figure 4.4 Natural frequencies using recommended equation (960 archetype models)

4.5.2 Modal mass participation

The modal mass participation factors for the first three modes are shown in the Figure 4.5. The mode mass participation is between 60-85%, 16%-20%, and 5%-9% for the first, second, and third modes, respectively.

Empirical equations for the first, second, and third modal mass participation is shown in Equation 4.6, Equation 4.7, Equation 4.8, respectively. As shown in Figure 4.5, the modal mass participation factor is dependent on α_f , where higher α_f results in increased mass participation in the first mode

and decreased in the second and third modes. Hence a more rigid outrigger will result in a more significant first mode response.

$$Massparticipation(Mode1) \approx 62 + 9\alpha_f[\%]$$
 Equation 4.6

Massparticipation(Mode 2)
$$\approx 20 - 6\alpha_f[\%]$$
 Equation 4.7

Massparticipation(Mode 3)
$$\approx 7 - 2\alpha_f[\%]$$
 Equation 4.8



Figure 4.5 Modal mass participation factors for CORW (960 archetype models)

4.5.3 Comparison of SDOF displacement to MDOF displacements (C₀)

The C_0 factor relates the roof displacement of the MDOF system to the SDOF system. Equation 4.9 shows the C_0 factor obtained from the period data. In general, as α_f increases, the C_0 decreases. Figure 4.6 shows the comparison of the C_0 value obtained from MRSA and the Equation 4.9. The result shows Equation 4.9 can predict C_0 well.

Equation 4.9



Figure 4.6 Ratio of the multi-degree-of freedom (MDOF) roof displacements to the single-degree-offreedom (SDOF)

4.5.4 Comparison of outrigger rotation and RDR (C_{θ})

Figure 4.7 shows the C_{θ} value, which is obtained from normalising the peak outrigger rotation (θ_{o}) to the peak roof drift ratio (RDR). The RDR is the ratio of the roof displacement (Δ_{r}) and the building height (H). RDR is often used in designs for estimating the deformation demands on structures and components. As shown, C_{θ} is highly dependent on α_{f} , where higher α_{f} results in a low C_{θ} . Based on these trends an empirical relation, shown in Equation 4.11, was developed. Equation 4.11 was developed to use for design purposes, as a result, the equation intentionally overestimates the response. As shown in Figure 4.7, the proposed Equation 4.11 gives a reasonable estimate of the model response trend.

$$C_{\theta} = \frac{\theta_o}{\text{RDR}} = 2 - 1.5\alpha_f \le 1.5$$
 Equation 4.10

where θ_o is the outrigger rotation and RDR is the roof drift ratio.



Figure 4.7 Ratio of the outrigger rotation (θ_o) and the roof drift ratio (RDR)

4.5.5 Comparison of SDOF shear to MDOF shear (M_v)

In this section the base shear from MRSA was compared with the base shear determined by multiplying the first mode spectral acceleration with the weight of the building (W). This ratio is referred to, M_v , by the National building code of Canada (NBCC, 2015). It was observed that M_v was highly dependent on spectral shape. Figure 4.8 shows the relation between Mv and the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). As shown in the figure, M_v is less than one when the spectral acceleration ratio is low (i.e., flatter spectral shape) and the M_v ratio increases in value with higher spectral acceleration ratios (i.e., steeper spectral shape).

Based on the above-mentioned trends, the following empirical relation, shown in Equation 4.11, was developed to estimate the higher mode shear response. Equation 4.11 is intended to assist with design, as a result it is conservatively limited to a minimum value of 1.

$$M_{v} = 0.3 \frac{S_{a}(T_{2})}{S_{a}(T_{1})} - 0.2 \ge 1$$
 Equation 4.11

where $S_a(T_2)$ is the spectral acceleration at the second mode period and $S_a(T_1)$ is the first mode period.



Figure 4.8 M_v ratio for CORW wall systems

4.5.6 Comparison of first mode vs the MRSA response

The MRSA responses were determined by combining the first 4 modes (over 90% modal mass participation). Figure 4.9 (a), (b), and (c) compares the ratio of first mode response to the combined response with spectral shape for roof displacements, overturning moment, and base shear, respectively.

The spectral shape is represented by the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$) of second mode spectral acceleration, Sa(T2), to first mode spectral accelerations, Sa(T1). A more constant spectral shape would be represented by a ratio which approaches unity (i.e., $S_a(T_2)/S_a(T_1) \rightarrow 1$), while a spectral shape which has a steeper slope is represented by a high ratio (i.e., $S_a(T_2)/S_a(T_1) \rightarrow \infty$).

Figure 4.9 (a) shows the ratio of the peak displacement from the first mode (Δ_{mode1}) to the peak displacement from MRSA (Δ_{MRSA}) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The result shows that the peak roof displacement is entirely dominated by the first mode response for all spectral shape ratios. Design spectrums often have low spectral displacements at low period, as a result, the low periods from higher modes result in smaller contributions to roof displacement. The spectral shape combined with the dominate modal mass participation of the first mode drives the observed first mode displacement response.

Figure 4.9 (b) shows the ratio of the peak moment from the first mode (M_{mode1}) to the peak moment from MRSA (M_{MRSA}) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The

result demonstrates that moment is highly influenced by the first mode response for spectral shape ratios less than 5. As the spectral shape ratio increases, the overturning moment has a more significant higher mode response, where 10%-30% of the responses are from higher modes. The Montreal site has a more significant higher mode response than the Vancouver site. This difference can be attributed to the Montreal design spectrum having a steeper spectral acceleration slope, resulting in a higher ratio of low period acceleration to high period acceleration.

Figure 4.9 (c) shows the ratio of the peak shear from the first mode (V_{mode1}) to the peak shear from MRSA (V_{MRSA}) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The result shows that the shear response is strongly governed by higher modes for spectral shape ratios greater than 1. For spectral ratios greater than 5, over 50% of the response is governed by higher modes. Like overturning moment, base shear has a more a more significant higher moment response for the Montreal spectrum.



Figure 4.9 Comparison of first mode contribution to combined modes for (a) displacements, (b) moments, and (c) shears.

4.5.7 Static load distribution, λ

In many design procedures, it is desirable to apply a static load to approximate the dynamic behaviour. The appropriate static force distribution is dependent on the objective of the design. For the CORW system, the objectives are to design the outrigger and the controlled rocking base to yield and to control displacements. The following section compares a simple loading distribution, an inverted triangular distribution, to the dynamic loads and evaluates the applicability of using this loading distribution to design the CORW.

In any static load distribution, the base shear (V) is distributed over the building height. The applied static floor at any story x, is given in Equation 4.12.

$$F_{\rm x} = \lambda_{\rm x} V$$
 Equation 4.12

In Equation 4.12, V is the base shear and λ_x represents the distribution pattern where the sum is equal to 1. For an inverted triangular distribution with equal mass at each floor, λ_x , can be calculated using Equation 4.13.

$$\lambda_{\rm x} = \frac{w_{\rm x} h_{\rm x}}{\sum_{i=1}^{n_f} w_i h_i}$$
 Equation 4.13

where h_x is the story elevation at story x; w_x is the xth story weight; and n_f is the total number of stories.

Figure 4.10 and Figure 4.11 shows the comparison of displaced shape between the MRSA and inverted triangular distribution, separated by design spectrum and period for $\alpha_f = 0.1$ and $\alpha_f = 0.9$, respectively. As shown in the figures, there is negligible differences between the responses for all cases. Hence, the triangular distribution can predict the displaced shape response well.

Figure 4.12 and Figure 4.13 shows the comparison of moments between the MRSA and inverted triangular distribution, separated by design spectrum and period for $\alpha_f = 0.1$ and $\alpha_f = 0.9$, respectively.

As shown in the figures, low periods tend to have similar moment response between MRSA and inverted triangular distribution. However, as periods increase higher modes begin to govern through the wall height. The MRSA results show higher moment amplitudes are observed at elevations greater than 50% for $\alpha_f = 0.1$, and between 40%-60% of building heights for $\alpha_f = 0.9$. The higher value of α_f results in a closer match between MRSA and inverted triangular distributions.

Inverted triangular distributions are representative of first mode behaviour. Therefore, the close match between the models and the static distribution displacements, shown in Figure 4.10 and Figure 4.11, can be attributed to the first mode dominance of displacements. Contrastingly, the large variations in moments, shown in Figure 4.12 and Figure 4.13, are attributed to higher mode

contributions. This is exemplified using an example archetype building responses, shown in Figure 4.14. In this figure, the displacements from each mode, prior to combination using the complete quadratic combination (CQC) of the modes, are shown in Figure 4.14 (a); the ratio of each mode displacement to the MRSA displacements are shown in Figure 4.14 (b); the moments from each mode, prior to combination using CQC, are shown in Figure 4.14 (c); and the ratios of each mode displacement to the MRSA displacements are shown in Figure 4.14 (d). As shown, the first mode dominates the displaced shape behaviour over most of the building height, with only minor variations (within 15%) at lower levels. On the other hand, moments have a significant contribution from the second and third modes over the building height. However, the base moment and the top moments are dominated by the first mode.



Figure 4.10 Displaced shape comparison of MRSA to inverted triangular distribution for $\alpha_f = 0.1$ (192 archetypes, 384 analysis)



Figure 4.11 Displaced shape comparison of MRSA to inverted triangular distribution for $\alpha_f = 0.9$ (192 archetypes, 384 analysis)



Figure 4.12 Wall moment comparison of MRSA to inverted triangular distribution for $\alpha_f = 0.1$ (192 archetypes, 384 MRSA analysis)



Figure 4.13 Wall moment comparison of MRSA to inverted triangular distribution for $\alpha_f = 0.9$ (192 archetypes, 384 MRSA analysis)



Figure 4.14 Example archetype response (Located in Montreal, T1 = 3s, α_f = 0.9): (a) displacements normalised to peak response from MRSA (Δ_{max}); (b) ratio of individual modal displacement ($\Delta_{mode,n}$) to MRSA displacement (Δ_{MRSA}); (c) ratio of moment normalised to peak moment response from MSRA (M_{max}); and (d) ratio of individual modal moment ($M_{mode,n}$) to MRSA moment (M_{MRSA});

From Figure 4.12 and Figure 4.13 it is observed that the ratio between the top and bottom moments are similar for the inverted triangular distribution and MRSA. Figure 4.15 shows the ratio of the outrigger moment to the total overturning moment for both MRSA and inverted triangular distribution. In this plot the ratio is solved theoretically using a Euler-Bernoulli beam. The ratio of a triangular distribution is shown in Equation 4.14. As shown in Figure 4.15, Equation 4.14 gives a reasonable estimate of this ratio.



$$\beta_m = \frac{M_o}{M_t} = \frac{3}{8}\alpha_f$$
 Equation 4.14

Figure 4.15 Ratio of outrigger moment to total moment (960 archetypes, 1920 MRSA)

Overall, key design parameters, such as displacements and flexural yielding (controlled rocking base and yielding outrigger), are captured well using an inverted triangular distribution. Hence, an inverted triangular distribution is a suitable static load distribution to predict these key design parameters.

4.5.8 Comparison of dynamic moment to static moment (β_0)

In this section, the dynamic force factor (β_o) of CORW defined as the ratio of the dynamic overturning moment (M_t^{dy}) , determined using MRSA, to the static overturning moment (M_t^{st}) , determined using an inverted triangular distribution, is developed. The static moment is determined by summing the moments at the base of the CORW using the SDOF base shear (V = $W * S_{a1}$) distributed in an inverted triangular pattern over the height of the building.

The analytical results are presented in the Equation 4.15. The result shows that β_o has a strong dependency on $S_a(T_2)/S_a(T_1)$. Eq. (31) is recommended to the dynamic moment. As shown in Figure 4.16, Equation 4.15 gives a reasonable estimate of the dynamic moment factor.



$$\beta_o = \frac{M_t^{dy}}{M_t^{st}} = 0.05 \frac{S_{a2}}{S_{a1}} + 0.5 \ge 0.75$$
 Equation 4.15

Figure 4.16 Comparison of total overturning moment using LTHA and static analysis

4.6 Linear dynamic analysis of SCCW system

Eigenvalue analysis was conducted on each of the 960 unique SCCW archetype buildings, and MRSA was conducted for each of the two building sites, resulting in 1920 analysis in the SCCW direction. The following sections describe the linear dynamic analysis of the SCCW system empirical equations to estimate these responses.

4.6.1 Natural periods of vibration (T_1, T_2, T_3)

Based on the eigen-value analysis, Equation 4.16 was developed to estimate the period of the SCCW. In Equation 4.16, the flexibility of the coupling beams is accounted for through an effective moment of inertia (I_{Te}) . The effective moment of inertia is determined using parallel axis theorem, as shown in Equation 4.17 for two equally sized C-shaped walls. In Equation 4.17 an
effective wall area, $A_{w,eff}$, is used to account for the increased flexibility due to coupling beam stiffness and is estimated using Equation 4.18.

$$T_1 \approx 1.85 \sqrt{\frac{WH^3}{gE_cI_{Te}}}$$
 Equation 4.16

$$I_{Te} \approx 0.5 \alpha_2 A_w L_c^2 + 2I_w$$
 Equation 4.17

$$A_{w,eff} \approx \alpha_2 A_w$$
 Equation 4.18

where L_c is the distance between centroids of each wall pier; I_w is the gross moment of inertia of an individual wall pier; and α_2 is a reduction factor.

In Equation 4.18, the reduction factor α_2 was determined through back-calculation of the natural periods from the eigenvalue analysis for each archetype. Figure 4.17 shows α_2 as a function of building aspect ratio (H/Lc). As shown, α_2 increases for greater values of aspect ratio and relative stiffness factors (α_c). Hence, the slenderer core wall and more rigid the coupling beams are, the more the core behaviour resembles a cantilever wall. Based on these results, Equation 4.19 was developed to estimate this behaviour. As shown in Figure 4.17, Equation 4.19 reasonably estimates this effective area reduction factor.



$$\alpha_2 = \left(0.06\frac{H}{L_c} + 0.3\right)\alpha_c \le \alpha_c^2$$

Equation 4.19

Figure 4.17 Effective area of coupled wall

Once the fundamental period is determined, Equation 4.20 and Equation 4.21 are proposed to estimate the second and third mode periods, respectively. Figure 4.18 compares the periods determined using the proposed design equations with the results of the eigen-value analysis from the models. As shown, the proposed relations estimate the modal periods well, with most of the estimates being within 5% of the model periods and all the estimated periods are within 25% of the equations for all archetypes,



Figure 4.18 Comparison of periods estimated using Equation 4.16, Equation 4.20, and Equation 4.21

4.6.2 Modal mass participation

The modal mass participation factors for the first three modes are shown in Figure 4.19. The modal mass participation is between 63-72%, 15%-20%, and 4%-6% for the first, second, and third mode, respectively. Empirical equations for the first, second, and third modal mass participation is shown in Equation 4.25, Equation 4.26, and Equation 4.27, respectively. As shown in Figure 4.19, the modal mass participation factor is dependent on α_2 , where higher α_2 results in decreased mass participation in the first mode and slightly increased in the second and third modes.

Mass participation(Mode 1) $\approx 72 - 5\alpha_{c2}$ [%]	Equation 4.22
Mass participation(Mode 2) $\approx 11 + 10\alpha_{c2}$ [%]	Equation 4.23
Mass participation(Mode 3) $pprox 5 - lpha_{c2}[\%]$	Equation 4.24



Figure 4.19 Modal mass participation of SCCW parameter study models

4.6.3 Comparison of SDOF displacement to MDOF displacements (C_0)

The C_0 factor relates the roof displacement of the MDOF system to the SDOF system. Figure 4.20 shows the C_0 value obtained from normalizing the peak roof displacement (Δ_E) of the MDOF system from MRSA with the spectral displacement from the design spectrum. From this analysis, a reasonable estimate for the SCCW is $C_0 = 1.48$ and is applicable for both Vancouver and Montreal spectrums.



Figure 4.20 Ratio of the multi-degree-of freedom (MDOF) roof displacements to the single-degree-offreedom (SDOF) displacement (960 archetypes, 1920 analysis)

4.6.4 Comparison of SDOF shear to MDOF shear (M_v)

To estimate the higher mode shear response, the ratio of base shear from MRSA was compared with the base shear determined by multiplying the spectral acceleration with the weight of the building. This ratio is referred to as M_v by the National building code of Canada (NBCC, 2015).

Figure 4.21 shows the relation between M_v and the ratio of the spectral acceleration at the second mode period ($S_a(T_2)$) to the spectral acceleration at the first mode period ($S_a(T_1)$). As shown, M_v is less than one when the spectral acceleration ratio is low (i.e., flatter spectral shape) and the M_v ratio increases in value with higher spectral acceleration ratios (i.e., steeper spectral shape). Based on the above-mentioned trends, the following empirical relation (Equation 4.25), was developed to estimate the higher mode shear response. This equation is intended to assist with design, as a result it is conservatively limited to a minimum value of 1.



 $M_{v} = 0.26 \frac{S_{a}(T_{2})}{S_{a}(T_{1})} + 0.1 \ge 1$ Equation 4.25

Figure 4.21 M_v ratio for SCCW wall systems (960 archetypes, 1920 analysis)

4.6.5 Degree of coupling, DOC

Coupled wall systems are often distinguished by their degree-of-coupling (DOC). The DOC is the ratio of the overturning moment resisted by the axial force couple (P_pL_c) to the total overturning

moment (M_p) , shown in Equation 4.26. The DOC helps engineers understand how significant the coupling forces in the wall are. For example, a DOC equal to 0 would indicate that the walls are two individual cantilever walls. On the other hand, a DOC equal to 1 would indicate that the walls behave as if there is a pin at the base, i.e., moment is equal to zero.

$$DOC = \frac{P_p L_c}{2M_p + P_p L_c}$$
Equation 4.26

where L_c is the distance between the centroid of the wall piers.

Although a useful characteristic parameter, the DOC cannot be defined prior to numerical analysis. Therefore, a new characteristic parameter, called the degree of axial stiffness (DAS), which relates the axial couple stiffness to the total flexural stiffness of the core-wall is proposed. The DAS for a symmetric core consisting of two C-shaped walls is shown in Equation 4.27.

$$DOC = 0.85 \frac{\alpha_2 A_w L_c^2}{2I_{Te}}$$
 Equation 4.27

The result of parametric MRSA analysis, as presented in Figure 4.22, shows that the DOC can be estimated as 85% of the DAS as presented in Equation 4.28. The results show that 95% of the analysis had DOC within 5% of the estimated value. Hence, Equation 4.28 can be used to predict the value of DOC effectively.

DOC (est.) = 0.85DAS



Figure 4.22 Comparison of Equation 4.28 with models (960 archetypes, 1920 analysis)

Equation 4.28

4.6.6 Comparison of first mode to combined MRSA response

MRSA was conducted on all 960 archetypes for two building sites, resulting in 1920 analysis. The responses were determined by combining the first 4 modes (over 90% modal mass participation). Figure 4.23 shows the comparison between spectral shape and the ratio of first mode response to the combination of the first 4 modes. The spectral shape is represented by the ratio of second mode spectral acceleration to first mode spectral accelerations.

Figure 4.23 (a) shows the ratio of the peak displacement from the first mode (Δ_{mode1}) to the peak displacement from MRSA (Δ_{MRSA}) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). As shown in Figure 4.23 (a), the peak roof displacement is almost entirely dominated by the first mode response for all spectral shape ratios. The spectral shape combined with the dominant modal mass participation of the first mode drives the first mode displacement response. Similar to the CORW, the design spectra often have low spectral displacements at low period, as a result, the low periods from higher modes have a lower contribution to the response.

Figure 4.23 (b) shows the ratio of the peak core moment from the first mode ($M_{t,mode1}$) to the peak core moment from MRSA ($M_{t,MRSA}$) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). compares the ratio of the total overturning moment from the first mode to the combined overturning moment from all the modes with the spectral shape. Figure 4.23 (b) demonstrates that moment is highly influenced by the first mode response, where the first mode accounts for 80%-100% of the response. As the spectral shape ratio increases, the overturning moment has a more significant higher mode response.

Figure 4.23 (c) shows the ratio of the peak shear from the first mode (V_{mode1}) to the peak shear from MRSA (V_{MRSA}) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). As shown, the shear response is strongly governed by higher modes for spectral shape ratios greater than 1. For spectral ratios greater than 5, over 50% of the response is governed by higher modes.

Figure 4.23 (d) shows the ratio of the peak coupling beam shear from the first mode ($V_{CB,mode1}$) to the peak coupling beam shear from MRSA ($V_{CB,MRSA}$) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The result shows that the coupling beam shear response is strongly governed by higher modes, for spectral shape ratio greater than 8, the first mode can account for as little as 18% of the total response.

Figure 4.23 (e) shows the ratio of the peak wall pier axial load from the first mode ($P_{pier,mode1}$) to the peak pier axial load from MRSA ($P_{pier,MRSA}$) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The result shows that, similar to the total overturning moment, the wall axial load is strongly governed by the first mode, where the first mode accounts for 86-100% of the total response.

Figure 4.23 (f) shows the ratio of the peak wall pier moment from the first mode ($M_{pier,mode1}$) to the peak pier moment from MRSA ($M_{pier,MRSA}$) compared with the spectral shape ratio (i.e., $S_a(T_2)/S_a(T_1)$). The result shows that the wall pier moment is strongly governed by higher modes, where the first mode can account for as little as 40% of the response. The first mode dominance in the total moment and the axial load, demonstrate the strong influence of the axial couple.



Figure 4.23 Comparison of first mode response to MRSA for (a) displacements, (b) moments, (c) shears, (d) coupling beam shear, (e) axial load, and (f) wall pier moment

4.6.7 Load distribution factor, λ

This section investigates the most effective static loading distribution for the SCCW system to accurately predict the coupling beams shear distribution. Goel et al. 2009 developed a static load distribution for link beams in eccentrically braced frames. Following the method by Goel and Chao (2008), the static loading distribution, at a given floor, x, is determined using Equation 4.29.

$$\lambda_{x} = (\beta_{x} - \beta_{x+1}) \frac{1}{\beta_{1}}$$
 Equation 4.29

In Equation 4.29, β_x is the coupling beam shear ($V_{CB,x}$), at a given story x normalized to the roof coupling beam shear ($V_{CB,r}$), the relation for β is show in Equation 4.30.

$$\beta_x = \frac{V_{CB,x}}{V_{CB,r}}$$
 Equation 4.30

In this section, the shear distribution β is determined by first developing relations to estimate the coupling beam shears ($V_{CB,x}$). To estimate the coupling beam shear, first shape functions, $S_{n,x}$, are developed to predict the response of coupling beams at mode n and height x of the building. These shape functions are shown in Equation 4.31, Equation 4.32, and Equation 4.33 for Mode 1, Mode 2, and Mode 3, respectively. Figure 4.24 shows that the simple estimate of shear shape functions gives a reasonable estimate of the MRSA analysis.

$$S_{1,x} = \sin\left((5.65 - 2.2\alpha_c)\frac{h_x}{H} + 2.2\alpha_c - 1.25\right) + 1.5/\alpha_c$$
 Equation 4.31

$$S_{2,x} = \sin\left(5\frac{h_x}{H} + 0.63\right) + 0.5\alpha_c - 0.25$$
 Equation 4.32

$$S_{3,x} = \sin\left(7.5\frac{h_x}{H} + 0.94\right) - 0.1\alpha_c^2$$
 Equation 4.33

The value of axial couple determined at the base of the wall is lower than the summation of the coupling beam shears. This difference is due to the higher mode effects, causing some of the coupling beams to reach the peak shear in the opposite direction as others. Figure 4.25 (a), Figure 4.25 (b), and Figure 4.25 (c) show the summation of absolute coupling beam shear forces

normalized by maximum axial force in the wall pier, $C_n = \frac{\sum |V_{CB,n}|}{P_p}$, for the first, second and third mode, respectively. The first mode is the highest contributor to the pier force, P_p . As shown in Figure 4.26 (a), Equation 4.34, Equation 4.35, and Equation 4.36 provide reasonable estimates of the contribution from the higher modes for the first, second mode and the third modes, respectively. As shown, higher values of α_c result in more contributions from the second and third modes. Hence, the higher relative stiffness of coupling beams, the more higher mode contribution.

$$C_{1} = \frac{\sum |V_{CB,1}|}{P_{p}} = 1.1 - 0.03\alpha_{c}^{3} \frac{S_{a}(T_{2})}{S_{a}(T_{1})} \le 1.05$$
 Equation 4.34

$$C_{2} = \frac{\sum |V_{CB,2}|}{P_{p}} = 0.22\alpha_{c} \frac{S_{a}(T_{2})}{S_{a}(T_{1})}$$
Equation 4.35

$$C_3 = \frac{\sum |V_{CB,3}|}{P_p} = 0.055\alpha_c \frac{S_a(T_3)}{S_a(T_1)}$$
 Equation 4.36



Figure 4.24 Coupling beam force shape factors (960 archetypes)



Figure 4.25 Contribution of each mode on coupling beam shears

Equation 4.37 was developed to estimate the coupling beam shear at any given story level, x. In Equation 4.37, each shape function has been normalized to the sum, $SN_{n,i} = \frac{S_{n,i}}{\sum |S_n|}$. Figure 4.26 shows the comparison between Equation 4.37 the MRSA results for an example 15-story, 30-story, and 45-story buildings. As shown, the recommended equation gives a reasonably good estimate of the behaviour for different building heights and locations. The equation has a less accurate prediction of the coupling beam shears at levels lower than the peak shear. However, for design purposes this equation is reasonable.

$$V_i = P_p \times \sqrt{\sum_{n=1}^{3} (C_n * SN_{n,i})^2}$$
 Equation 4.37

where C_n is the nth mode contribution; SN is the normalized shape function for the *n* mode; and $SN_{n,r}$ is the shape function at the roof coupling beam for the nth mode; and i is the story number.

The static shear distribution factor, β_x , is determined by normalizing the coupling beam shear to the roof coupling beam shear using Equation 4.38.



Figure 4.26 Estimated coupling beam shears for (a) 15-story, (b) 30-story, and (c) 45-story (Parameters = Vancouver site, ac = 0.7, Lcb = 1.5m, hf = 4m, mf = 600tonne, Lw, y = 10 m, Lw, x = 10m, tw = 0.6m)

4.7 Summary

In this chapter, the dynamic characteristics of the proposed earthquake-resilient RC core wall building system are examined. The dynamic analysis was conducted using modal response spectrum analysis. A total of 960 archetypes were developed for the CORW, and 960 archetypes were developed for the SCCW. Each archetype was analysed using two different design acceleration spectrums. As a result, a total of 3840 modal response spectrum analysis were conducted.

The dynamic analyses were used to estimate the effect of the building geometry and stiffnesses on forces and deformations. Additionally, this chapter also presented new empirical equations which were developed to estimate these dynamic characteristics.

Key observations from the CORW parametric study:

- 1. The natural period is governed entirely by the relative stiffness factor α_f . Where an infinitely rigid outrigger (i.e., $\alpha_f = 1$) would only result in about 38% reduction in fundamental period if compared to a system without an outrigger (i.e., $\alpha_f = 0$).
- 2. The modal mass participation shows that higher relative outrigger stiffness results in a more dominate first mode response.

- 3. The ratio between roof displacement to spectral displacement $(C_0 = \frac{\Delta r}{s_d})$ decreases with an increase in α_f .
- The factor shear amplification factor, M_v, depends on spectral shape, where M_v increases with an increase in Sa(T₂)/S_a(T₁).
- 5. Roof displacements are almost entirely governed by the first mode response regardless of spectral shape.
- 6. Overturning moments are dependent on spectral shape. A more constant spectral shape (i.e. S_a(T₂)/S_a(T₁) → 1 results in more first mode dominate response. On the other hand, S_a(T₂)/S_a(T₁) → ∞ results in significantly more higher mode response.
- 7. Base shear is dependant on spectral shape and have more significant higher mode contributions than overturning moments and displacements. A more constant spectral shape (i.e., S_a(T₂)/S_a(T₁) → 1) results in more first mode dominate response. On the other hand, S_a(T₂)/Sa(T₁) → ∞ results in significantly more higher mode response.
- 8. Inverted triangular static distribution gives a satisfactory estimate of the displacements and moments.
- 9. The ratio of the outrigger moment to the total overturning moment is similar for both the static and dynamic analysis.
- 10. The dynamic base moment is lower when compared to a static moment. An empirical equation is developed and presented to estimate these dynamic moments.

Key observations from the SCCW parametric study:

- 1. Natural period is most influenced by the relative stiffness of the coupling beams to the wall and the wall aspect ratios. Empirical equations used to estimate the response
- First mode modal mass participation ranges between 63- 72% depending on the relative stiffness of the coupling beams.
- 3. The C₀, the ratio between roof displacement and S_d, can be estimated as $C_0 = 1.5$.
- 4. The factor shear amplification factor, M_v , depends on spectral shape, and M_v value increases with an increase in $S_a(T_2)/S_a(T_1)$.
- 5. Degree-of-coupling can be estimated using simple empirical equations which depend on the geometry of the wall and the stiffness of the coupling beams.

- 6. Roof displacement, overturning moment, and wall pier axial loads are primary first mode governed.
- 7. Base shear, coupling beam shear, and wall pier moments have significant higher mode responses.
- 8. Static load distributions, determined by estimating the coupling beam shapes of the first three modes, are proposed to estimate the shear in the coupling beams.

The elastic dynamic response of structures is a critical component in performance-based design. Practitioners often use modal response spectrum analysis to design structures at the service level earthquake hazard like used within this chapter. Additionally, building codes, such as NBCC 2020, use elastic modal response spectrum analysis to approximate the higher mode effect in structures, despite expecting them to undergo extensive nonlinear action. Therefore, this chapter is crucial in understanding the behaviour of the CORW and SCCW, particularly in the attempt to design for multiple performance objectives and different shaking intensities, which is the main objective of the following chapter (Chapter 5).

Chapter 5: Proposed design procedure for CROCW

5.1 Overview

This chapter details the development of an equivalent energy-based design procedure (EEDP) used to design the CROCW system. The methods presented in this chapter may also be applied to design the CORW and SCCW independently.

The basic theory of EEDP is described in this chapter. Afterwards, the step-by-step process for designing the CROCW system for different hazard levels (e.g., service level earthquake (SLE), design-based earthquake (DBE), and the maximum considered earthquake (MCE)) is presented. This chapter explains how the empirical equations, developed in Chapter 4, are used to modify EEDP to account for higher mode effects. Additionally, the original derivation of EEDP assumed elastic-plastic hysteresis. In the CORW and SCCW systems, the behaviour incorporates flag-shaped hysteretic behaviour. This chapter presents new energy modification factors which account for these unique combinations of hysteretic shapes. This chapter concludes with an overview of four prototype building designs which use a CROCW system following the presented EEDP is presented. This chapter only summarises the design.

5.2 Introduction

This thesis proposes a procedure to design the CROCW system to meet different performance objectives at different shaking intensities. At a service level earthquake (SLE) shaking intensity, the design objective of the CROCW is to remain essentially elastic, where no yielding occurs. At a design-based earthquake (DBE) shaking intensity, the design objective is for the CROCW primary system dampers (i.e., outrigger dampers and SCFDs) to yield and dissipate energy. At a maximum considered earthquake (MCE) shaking intensity, the design objective of the CROCW primary and secondary system dampers (i.e., outrigger dampers, SCFDs, and wall-base dampers) is to yield and dissipate the earthquake energy.

The state-of-practice in designing structures to meet multiple performance objectives at different shaking intensities usually requires a nonlinear analysis performance-based design approach (e.g., LATBSDC, 2020). This design methodology is cumbersome and involves a substantial amount of design effort and a rigorous peer review process. To simplify this procedure, Yang et al. (2018)

developed the Equivalent Energy Design Procedure (EEDP), which utilizes a balanced energy concept to design structures to satisfy strength and displacement limitations without iteration. This procedure has been used for many different seismic design applications (Yang et al. (2018); Li et al. (2018); Etebarian et al. (2019); Yang et al. (2020)). However, the original derivation of EEDP was limited to structures which are not sensitive to higher modes and which have basic Elastic-plastic hysteretic behaviour.

In this chapter, modifications to the EEDP are recommended to design the newly proposed earthquake resilient RC core wall buildings. New dynamic factors are developed and are presented within this chapter. Additionally, new energy modification factors are developed to appropriately apply EEDP to the newly proposed CROCW, CORW and SCCW systems. This chapter concludes with example design of four prototype buildings, which will be used in subsequent chapters to investigate the seismic performance of the CROCW, CORW, and SCCW systems.

5.3 Designing steps using EEDP

EEDP uses the energy balanced concept to design dual-fused structures to meet both the target displacements and forces at multiple hazard intensities. EEDP approximates the behavior of the dual-fused system with an Equivalent Nonlinear Single Degree-of-Freedom (ENSDOF) system. The energy stored in ENSDOF (elastic strain energy, E_s , plus hysteretic energy, E_h) is related to the energy dissipated by its Equivalent Linear Single Degree-of-Freedom (ELSDOF) (elastic energy, E_a). A detailed derivation of EEDP can be found in Yang et al. (2018).

Figure 5.1 shows the system global force-deformation for the ELSDOF response and the ENSDOF response of the CROCW along the CORW and SCCW axis. As the original derivation of EEDP assumes that both primary fuse system and secondary fuse systems have an Elastic-Perfectly-Plastic (EPP) hysteretic behaviour, the design procedure herein is adjusted to account for the differences in the CROCW behaviour. Specifically, new energy modification factors are developed in a subsequent section (*Section 5.4 Development of energy modification factors* (γ_a and γ_b)). Additionally, the EEDP is modified to incorporate the dynamic response, using the relations developed in Chapter 4.



Figure 5.1 Desired nonlinear mechanisms of the CROCW

The following procedure allows engineers to design the complex behaviour of the CROCW to achieve all the desired performance objectives for the different hazards in a simple and noniterative format. In a typical design of RCCW, the architectural constraints, such as the number of elevators, elevator and stairway sizes, and utilization of floor space, which control the geometry and mass, are often decided prior to in-depth structural design. Similarly, in the following design of the CROCW, it is assumed that the core geometry and building masses can be reasonably estimated prior to the detailed design.

Step 1: Select SLE, DBE, and MCE shaking intensities

The first step in EEDP is for the designers to work with the owners to define the demand for the CROCW system. The designer shall select SLE, DBE, and MCE hazard levels, which will be applied to both the CORW and SCCW axes. At the SLE shaking intensity, CROCW is designed to remain elastic. At the DBE shaking intensity, the outrigger dampers within the CORW and the SCFD within the SCCW is designed to yield and start to dissipate the earthquake energy, while

the controlled rocking base in both systems is designed to remain elastic. At the MCE shaking intensity, the controlled rocking base in both systems is designed to yield.

Step 2: Create modified capacity design spectrum for the CORW axis

After the shaking intensities are determined, EEDP uses the capacity spectrum method concept (Freeman et al., 1975) to plot the demand and capacity curves on the same figure. In this step a modified capacity spectrum, shown in Figure 5.2, is developed for the CORW axis, where the roof displacements of the structure are estimated using the spectral displacement (S_d) multiplied by a $C_{0,CORW}$ factor (shown in the equation presented below) and the base shear is determined by using the higher mode force factor, $\beta_{o,CORW}$ (shown in the equation presented below) multiplied by the spectral acceleration (S_a) of the ELSDOF and the building mass (m). Details of the development of $C_{0,CORW}$ and $\beta_{o,CORW}$ are provided in Chapter 4.

$$C_{0,CORW} = 1.52 - \frac{0.05\alpha_f}{1.2 - \alpha_f}$$
 from Equation 4.9

$$\beta_{o,CORW} = 0.05 \frac{S_{a2}}{S_{a1}} + 0.5 \ge 0.75$$
 from Equation 4.15

where S_{a2} is the spectral acceleration at the second mode period and S_{a2} is the spectral acceleration at the first mode period, and α_f is a factor which relates the stiffness of the wall to the stiffness of the outrigger as shown in Equation 3.10 (from Chapter 3).

$$\alpha_f = \frac{1}{\frac{2E_c I_{w,CORW}}{k_o H} + 1}$$
 from Equation 3.10

where E_c is the modulus of elasticity; $I_{w,CORW}$ is the moment of inertia of the individual wall piers in the CORW direction; ko is the rotational stiffness of the outrigger system; and H is the height of the building.



COKW Sa SO,CORW

Figure 5.2 Develop modified capacity design spectrum for CORW axis

Step 3: Identify the CORW yield base shear ($F_{y,CORW}$) and yield roof displacement ($\Delta_{y,CORW}$)

In many designs, the period of the structure is usually governed by the architecture constraint. For instance, the size of the core wall and mass of the structure are usually decided based on the functionality of the structure. The CORW fundamental period governs the initial stiffness, and the selection of fundamental period will have various responses, as shown in Figure 5.3 (a). The period may be estimated using Equation 4.2 from Chapter 4, given the building geometry.

$$T_{1,CORW} \approx (1 - 0.37\alpha_f) 1.86 \sqrt{\frac{mH^3}{2E_c I_{w,CORW}}}$$
 from Equation 4.2

where E_c is the modulus of elasticity; $I_{w,CORW}$ is the moment of inertia of the individual wall piers in the CORW direction; ko is the rotational stiffness of the outrigger system; H is the height of the building, and α_f is a factor which relates the stiffness of the wall to the stiffness of the outrigger as shown in Equation 3.10 (from Chapter 3).

Once the period is defined, the yield base shear ($F_{y,CORW}$) and yield displacement ($\Delta_{y,CORW}$) of the CORW system can be identified from the interaction of the constant period line with the SLE demand curve Figure 5.3 (b) shows the intersection of the yield base shear ($F_{y,CORW}$) and yield displacement ($\Delta_{y,CORW}$) of the CROCW system.

Equation 5.1, Equation 5.2 and Equation 5.3 show the relationship of initial stiffness $F_{y,CORW}$, $\Delta_{y,CORW}$, and $K_{s,CORW}$ as a function of CROCW structural properties, respectively.

$$F_{y,CORW} = S_a(T_{1,CORW})\beta_{o,CORW}m$$
 Equation 5.1

$$\Delta_{y,CORW} = S_d(T_{1,CORW})C_{0,CORW}$$
 Equation 5.2

$$K_{s,CORW} = \frac{F_{y,CORW}}{\Delta_{y,CORW}} = \frac{4\pi^2 m \beta_{o,CORW}}{T_{1,CORW}^2 C_{0,CORW}}$$
Equation 5.3

where $C_{0,CORW}$ is the displacement amplification factor given in Equation 4.9; $\beta_{o,CORW}$ is the dynamic force factor given in Equation 4.15; m is the total building mass; $T_{1,CORW}$ is the fundamental period; and $S_a(T_{1,CORW})$ and $S_d(T_{1,CORW})$ is the spectral acceleration and displacement at the fundamental period ($T_{1,CORW}$), respectively.



Figure 5.3 SLE performance for CORW axis: (a) period design options and (b) SLE performance point

Step 4: Design CORW at the DBE shaking intensity

In this step the performance of the CORW at the DBE shaking intensity is determined based on the designer's selection of one of following three parameters: (1) the DBE base shear ($F_{p,CORW}$); (2) the DBE roof displacement ($\Delta_{p,CORW}$); or (3) the post yielding stiffness ratio (b_{CORW}). These three parameters are shown in Figure 5.4.

The post-yielding stiffness of CORW can be determined theoretically by assuming that the CORW system acts as a Euler-Bernoulli beam with a nonlinear spring (representing the controlled rocking base) at the base, and a nonlinear spring (represents the outrigger) at the roof. In this simple model, the earthquake loads are represented by an inverted triangular lateral distributed load pattern.

Equation 5.4 shows the post-yield stiffness of the, b_{CORW} , as a function of the normalized stiffness factor α_f derived in Chapter 3.

$$b_{CORW} = 1 - \frac{15}{22} \alpha_f$$
 Equation 5.4

Once b_{CORW} is known, EEDP uses an energy balanced concept to determine the DBE shaking intensity design base shear ($F_{p,CORW}$) and the design displacement ($\Delta_{p,CORW}$). In this process, the energy stored in an ELSDOF system is equated to that dissipated by the ENLSDOF. The incremental input energy, $\Delta E_{E1,CORW}$, shown in Figure 5.4, is determined from when the shaking intensity is increased from the SLE to the DBE shaking intensities and is given by Equation 5.5.

$$\Delta E_{E1,CORW} = \frac{1}{2} \left(F_{pe,CORW} + F_{y,CORW} \right) \left(\Delta_{pe,CORW} - \Delta_{y,CORW} \right)$$
Equation 5.5

where $F_{pe,CORW}$ and $F_{y,CORW}$ are the forces in the CORW ELSDOF at the DBE and SLE hazards, respectively. $\Delta_{pe,CORW}$ and $\Delta_{y,CORW}$ are the displacement of the CORW ELSDOF at the DBE and SLE hazards, respectively.

The corresponding incremental energy dissipated by the ENLSDOF which undergoes monotonic pushover ($\Delta E_{\text{NM1,CORW}}$) is determined using Equation 5.6.

$$\Delta E_{\text{NM1,CORW}} = \frac{1}{2} (F_{p,CORW} + F_{y,CORW}) (\Delta_{p,CORW} - \Delta_{y,CORW})$$
Equation 5.6

where $F_{p,CORW}$ is the CORW axis base shear at the DBE shaking intensity and $\Delta_{p,CORW}$ is the CORW axis roof displacement at the DBE shaking intensity.

The nonlinear dynamic energy ($\Delta E_{ND1,CORW}$), determined from the ENLSDOF, is converted to the nonlinear monotonic pushover energy ($\Delta E_{NM1,CORW}$) using the energy modification factor $\gamma_{a,CORW}$. This process is shown in Equation 5.7.

$$\Delta E_{E1,CORW} = \Delta E_{ND1,CORW} = \gamma_{a,CORW} \Delta E_{NM1,CORW}$$
Equation 5.7

where $\gamma_{a,CORW}$ is the energy modification, which can be calculated using Equation 5.8. Derivation of $\gamma_{a,CORW}$ is described in a subsequent section (Section 5.4 Development of energy modification factors (γ_a and γ_b)).

$$\gamma_{a,CORW} = (3.5b_{CORW} - 3)\Phi_{p,CORW} - 3.1b_{CORW} + 3.6$$
 Equation 5.8
where $\Phi_{p,CORW} = F_{y,CORW}/F_{pe,CORW}$

By substituting Equation 5.5 and Equation 5.6 in Equation 5.7, the corresponding design base shear at the DBE shaking intensity is determined by using Equation 5.9.

$$F_{p,CORW} = 2 \frac{\Delta E_{E1,CORW}}{\gamma_{a,CORW} (\Delta_{p,CORW} - \Delta_{y,CORW})} - F_{y,CORW}$$
Equation 5.9

where roof displacement at the DBE intensity ($\Delta_{p,CORW}$) is calculated with Equation 5.10.



Figure 5.4 DBE performance point on CORW axis

Step 5: Determine the performance of the CORW axis system at the MCE

In this step, the performance of the CORW at the MCE, shown in Figure 5.5, is determined. EEDP assumes that the energy dissipated by the energy stored in the ELSDOF is equal to the energy

dissipated by ENSDOF system. Equation 5.11 shows the incremental energy stored in the ELSDOF system when the shaking intensity changes from SLE to DBE.

$$\Delta E_{E2,CORW} = \frac{1}{2} \left(F_{ue,CORW} + F_{pe,CORW} \right) \left(\Delta_{ue,CORW} - \Delta_{pe,CORW} \right)$$
Equation 5.11

where $F_{ue,CORW}$ is the elastic base shear of the CORW ELSDOF system at the MCE hazard and $\Delta_{ue,CORW}$ is the CORW elastic roof displacement of the ELSDOF system at the MCE hazard. The corresponding incremental energy dissipated from the ENLSDOF, which undergoes monotonic pushover, is determined using Equation 5.12.

$$\Delta E_{\text{NM2,CORW}} = F_{p,CORW} \left(\Delta_{u,CORW} - \Delta_{p,CORW} \right)$$
Equation 5.12

Equation 5.13 shows the energy balanced equation.

$$\Delta E_{E2,CORW} = \Delta E_{ND2,CORW} = \gamma_{b,CORW} \Delta E_{NM2,CORW}$$
Equation 5.13

where $\gamma_{b,CORW}$ is the energy modification factor to relate the energy dissipated by the system under monotonic pushover to that under dynamic load when the hazard level increases from DBE to MCE. Equation 5.14 shows the empirical equation for $\gamma_{b,CORW}$, developed in a subsequent section (Section 5.4 Development of energy modification factors (γ_a and γ_b)).

$$\gamma_{b,CORW} = (5\lambda_{p,CORW}/\mu_{p,CORW} - 9.2)\Phi_{s,CORW} - 5\lambda_{p,CORW}/\mu_{p,CORW} + 8$$
Equation 5.14

where $\lambda_{p,CORW}$ is the ratio of the DBE base shear to the SLE base shear (i.e., $\lambda_{p,CORW} = F_{p,CORW}/F_{y,CORW}$); μ_p is the ratio of the DBE displacement to the SLE displacement (i.e., $\mu_{p,CORW} = \Delta_{p,CORW}/\Delta_{y,CORW}$); and $\Phi_{s,CORW}$ is the ratio of the MCE elastic base shear to the DBE elastic base shear (i.e., $\Phi_{s,CORW} = \Delta_{ue,CORW}/\Delta_{pe,CORW}$).



Figure 5.5 MCE performance point on CORW axis

The ultimate roof displacement ($\Delta_{u,CORW}$), can be determined by substituting Equation 5.11 and Equation 5.12 into Equation 5.13, and is shown in Equation 5.15.

$$\Delta_{u,CORW} = \frac{\Delta_{E2,CORW}}{\gamma_{b,CORW}F_{p,CORW}} + \Delta_{p,CORW}$$
Equation 5.15

where $\Delta_{E2,CORW}$ is the incremental energy stored in the ELSDOF shown in Equation 5.13; $F_{P,CORW}$ is the base shear at the DBE intensity determined in Equation 5.9.

Step 6: Determine the primary and secondary system base shear in the CORW axis

In the original derivation of EEDP, the primary fuse system and secondary fuse systems are assumed to act in parallel. This results in the tri-linear system backbone to be separated into two bilinear backbones (Figure 5.6 (a)). However, the CORW system is unique due to the high contribution of elastic deformations from the core wall. As a result, the portion of force distributed to the controlled rocking base (i.e., the secondary mechanism) varies depending on the yielding force of the outrigger (i.e., primary systems). As a result, the primary system (outrigger) is a bilinear backbone, while the secondary system (controlled rocking base) has a tri-linear backbone (Figure 5.6 (b)).

The portion the demand resisted by the primary (outrigger) system, $F_{PR,CORW}$ (Equation 5.16), is determined applying β_m factor, determined in Chapter 4, to the yield base shear ($F_{v,CORW}$). The

part of the load resisted by the secondary (controlled rocking base) system, $F_{PR,CORW}$, can be determined by subtracting the primary force from the DBE base shear, $F_{p,CORW}$, shown in Eq. (14).

$$F_{PR,CORW} = \beta_{\rm m} F_{y,CORW}$$
 Equation 5.16

$$F_{SC,CORW} = F_{p,CORW} - F_{PR,CORW}$$
Equation 5.17

where is the ratio between the outrigger demand and the total demand determined in Chapter 4, $\beta_m = \frac{3}{8} \alpha_f$.



Figure 5.6 EEDP force distribution to primary and secondary systems: (a) original EEDP force distribution and (b) CORW EEDP force distribution

Step 7: Design the primary and secondary system dampers in the CORW axis

In this step the CORW primary fuse system dampers (i.e., the outrigger dampers) and the secondary fuse system dampers (i.e., controlled rocking base) are determined. Figure 5.7 shows how the CORW mechanism (Figure 5.7 (a)) may be decomposed into the primary plastic mechanism (Figure 5.7 (b)) and secondary plastic mechanism (Figure 5.7 (c)). Equation 5.18 shows the external work done equation in the CORW primary fuse system.

$$W_{ExPr,CORW} = \theta F_{PR,CORW} \sum_{i=1}^{n} \lambda_{x,CORW} h_x$$
 Equation 5.18

where θ is the base rotation causing the mechanism; h_x is the elevation at a given floor, x; and $\lambda_{x,CORW}$ is the CORW force distribution, shown below. Details of the derivation of for the $\lambda_{x,CORW}$ can be identified in Chapter 4.

$$\lambda_{x,CORW} = \frac{w_x h_x}{\sum_{i=1}^{x} w_i h_i}$$
 from Equation 4.13

where w is the weight at each story at level x and h_x is the story elevation.



Figure 5.7 CORW axis (a) plastic mechanism, (b) primary fuse system mechanism and the (c) secondary fuse mechanism system

Equation 5.19 shows the internal work done by the primary system of the CORW.

$$W_{InPr,CORW} = \theta f_o L_o$$
 Equation 5.19

where f_o is the outrigger damper force L_o is the length of the outrigger

By equating Equation 5.18 to Equation 5.19 the forces in the outrigger are determined and shown in Equation 5.20.

$$f_o = F_{PR,CORW} \sum_{x=1}^{n} (\lambda_{x,CORW} h_x) / L_o / 2$$
 Equation 5.20

The plastic mechanism of the CORW's secondary system is shown in Figure 5.7 (c). The external work for the CORW secondary fuse system is shown in Equation 5.21, while the internal work is shown in Equation 5.22.

$$W_{ExSC,CORW} = \theta F_{SC,CORW} \sum_{x=1}^{n} (\lambda_{x,CORW} h_x) - P_g(\theta L_w)$$
Equation 5.21

where L_w is the length of the wall (see Figure 5.7) and P_g is the axial load in one wall pier.

$$W_{InSc,CORW} = \theta f_b (L_w + 2L_d)$$
Equation 5.22

where L_d is the distance between the wall and the damper (see Figure 5.7).

The minimum design base damper force, shown in Equation 5.23, is determined by equating the internal work (Equation 5.22) and the external work (Equation 5.21).

$$f_b = \left[F_{SC,CORW} \sum_{x=1}^n (\lambda_{x,CORW} h_x) - P_g(L_w) \right] / (L_w + 2L_d) / 2$$
 Equation 5.23

Step 8: Create modified capacity design spectrum for the SCCW system

Similar to Step 2, in this step the modified capacity design spectrum is developed for the SCCW axis and is shown in Figure 5.8. The SCCW roof displacements of the structure are estimated using the spectral displacement (S_d) multiplied by a $C_{0,SCCW}$ factor ($C_{0,SCCW} = 1.48$) and the base shear by the spectral acceleration (S_a) of the ELSDOF, the dynamic factor $\beta_{0,SCCW}$, and the building mass (m). The dynamic factor, $\beta_{0,SCCW}$, is determined using Equation 5.25, shown below.

$$\beta_{o,SCCW} = (c_1 + c_2 + c_3)^{0.5}$$
 Equation 5.24

where c_1 , c_2 , and c_3 are higher mode factors determined in Equation 4.34, Equation 4.35, and Equation 4.36, respectively.



Figure 5.8 Modified capacity design spectrum for all hazards along SCCW axis

Step 9: Identify SCCW yield base shear ($F_{y,SCCW}$) and yield roof displacement ($\Delta_{y,SCCW}$)

The performance at the SLE of the SCCW is determined by selection one of the following: (1) fundamental period in the SCCW axis ($T_{1,SCCW}$); (2) the yield base shear ($F_{y,SCCW}$); or (3) the yield displacement ($\Delta_{y,SCCW}$). In this example it is assumed that the geometry of the building can be estimated early in the design process. As a result, the period may be determined using the following equation, derived in Chapter 4.

$$T_{1,SCCW} \approx 1.85 \sqrt{\frac{mH^3}{E_c I_{Te}}}$$
 from Equation 4.16

where H is the total height of the wall; E_c is the elastic modulus of concrete; I_{Te} is the effective moment of inertia of the core, determined using parallel axis theorem which considers the moment of inertia of each wall pier and an effective area of each pier, shown in the following equation.

$$I_{Te} \approx 0.5 \alpha_2 A_w L_c^2 + 2I_w$$
 from Equation 4.17

where A_w is the cross-sectional area of a wall pier; I_w is the moment of inertia of a wall pier; d_c is the distance from the centroid of the wall pier to the centroid of the core wall; and α_2 , shown in the following equation, is a stiffness factor which relates the coupling beam stiffness to the stiffness of the wall.

$$\alpha_2 = \left(0.06 \frac{H}{L_c} + 0.3\right) \alpha_c \le \alpha_c^2$$
 from Equation 4.19

where α_c is relative stiffness parameter of the coupling beams to the walls developed in Chapter 4, shown in the following equation.

$$\alpha_c = \frac{1}{1 + \frac{E_c A_w}{n_f H k_{cb}}}$$
 from Equation 4.1

where n_f is the number of stories; E_c is the elastic modulus of the wall; k_{cb} is the initial stiffness of the coupling beams.

The period will determine the systems initial stiffness, as shown in Figure 5.9 (a). The intersection of the SLE and a line which passes the origin and has a stiffness equal to $K_{s,SCCW}$ represents the SLE performance point, shown in Figure 5.9 (b). Equation 5.25, Equation 5.26, and Equation 5.27 shows the relationship of yield base shear ($F_{y,SCCW}$), roof displacement at yield ($\Delta_{y,SCCW}$), and initial stiffness ($K_{s,SCCW}$), respectively.

$$F_{y,SCCW} = \beta_{o,SCCW} S_a(T_{1,SCCW}) m$$
 Equation 5.25

$$\Delta_{y,SCCW} = S_d(T_{1,SCCW})C_{0,SCCW}$$
 Equation 5.26

$$K_{s,SCCW} = \frac{F_{y,SCCW}}{\Delta_{y,SCCW}} = \frac{4\pi^2 \beta_{o,SCCW} m}{T_{1,SCCW}^2 C_{0,SCCW}}$$
Equation 5.27

where $S_a(T_{1,SCCW})$ and $S_d(T_{1,SCCW})$ is the spectral acceleration and displacement at the fundamental period($T_{1,SCCW}$), respectively; m is the building mass; $C_{0,SCCW}$ is the displacement amplification factor.



Figure 5.9 Select SLE performance for SCCW axis: (a) period options and (b) SLE performance point

Step 10: Determine relations for SCCW primary and secondary base shear

In this step the relations for the SCCW primary base shear ($F_{PR,SCCW}$) and the secondary base shear ($F_{SC,SCCW}$) are determined. In the original derivation of EEDP, the primary fuse system and secondary fuse system are assumed to act in parallel, resulting in a tri-linear system backbone to be separated into two bilinear backbones (Figure 5.10 (a)). However, in the SCCW system, a significant portion of the deformations are due to elastic deformation in the RC wall. The elastic stiffness of the core is sensitive to the boundary conditions on the wall, where, after the coupling beams yielded the elastic stiffness of the core decreases. As a result of this change in stiffness, the force-deformation relation of the secondary fuse system (i.e., the controlled rocking) shows a trilinear backbone. The primary fuse system and secondary fuse system relationship for the SCCW is shown in Figure 5.10 (b).



Figure 5.10 EEDP force distribution to primary fuse system and secondary fuse system: (a) original EEDP force distribution and (b) SCCW EEDP force distribution

The portion of the force resisted by the primary fuse system (i.e., $F_{PR,SCCW}$) is estimated based on the degree-of-coupling (DOC), which is defined as the ratio of the axial coupled from the coupling beams to the total overturning moment. This base shear is shown in Equation 5.28. Based on Figure 5.10 (b), the portion of the load resisted by the secondary fuse system (controlled rocking base), $F_{SC,SCCW}$, can be determined by subtracting the primary force from the DBE base shear, $F_{p,SCCW}$ as shown in Equation 5.29.

$$F_{PR,SCCW} = DOC * F_{y,SCCW}$$
 Equation 5.28
$$F_{SC,SCCW} = F_{p,SCCW} - F_{PR,SCCW}$$
 Equation 5.29

where *DOC* is the degree-of-coupling and can be conservatively estimated using the following equation. The development of Equation 4.28 is shown in Chapter 4.

$$DOC \approx 0.85 \frac{A_w L_c^2}{A_w L_c^2 + 4I_w}$$
 from Equation 4.28

where A_w is the cross-sectional area of a wall pier; I_w is the moment of inertia of a wall pier; L_c is the distance from the centroids of the wall piers.

Step 11: Use equal work to design the SCCW primary fuse and the secondary fuse

In this step the SCCW primary fuse system dampers (i.e., the SCFDs) and the secondary fuse system base shear ($F_{SC,SCCW}$) are determined. The damper design forces are determined by first separating the primary and secondary fuse system mechanisms, shown in Figure 5.11, and applying equal work. The SCCW primary fuse system mechanism, shown in Figure 5.11 (b), assumes free rotation at the base of the walls, and all the nonlinear deformation is within the coupling beams. The SCCW secondary fuse system mechanism, as shown in Figure 5.11 (c), assumes that there is no contribution from the coupling beams, and all the plastic deformation is concentrated in the controlled rocking base. Equation 5.30 shows the expression for external work done in the SCCW primary fuse system.

$$W_{ExPr,SCCW} = \theta F_{PR,SCCW} \sum_{i=1}^{n} \lambda_{x,SCCW} h_x$$
Equation 5.30

where θ is the base rotation causing the mechanism; h_x is the elevation at a given floor, x; and $\lambda_{x,SCCW}$ is the SCCW force distribution shown in the equation below. Details of the derivation of for the $\lambda_{x,SCCW}$ are shown in Chapter 4.

$$\lambda_{x,SCCW} = (\beta_x - \beta_{x+1}) \frac{1}{\beta_1}$$
 from Equation 4.29

where β_x is the coupling beam shear distribution at a given level, x, derived in Chapter 4. Equation 5.31 shows the internal work done by the primary system of the SCCW.

$$W_{InPr,SCCW} = \sum_{x=1}^{n} V_{P,r} \beta_x L_c \theta$$
 Equation 5.31

where $V_{p,r}$ is the coupling beam shear force at the top level and L_c is the distance between the centroids of the wall piers.

By equating Equation 5.31 to Equation 5.30 the forces in the coupling shear are determined and shown in Equation 5.32.

$$V_{CB,r} = \frac{F_{PR}}{2L_c} \frac{\sum_{i=1}^n \lambda_{x,SCCW} h_x}{\sum_{i=1}^n \beta_x}$$
Equation 5.32

Further, the coupling beam shear at any given level x is determined using Equation 5.33.

$$V_{CB,x} = \beta_x V_{CB,r}$$
 Equation 5.33



Figure 5.11 Plastic mechanisms for the SCCW system of a) the combined system; b) the primary fuse system and c) the secondary fuse system

The plastic mechanism of the SCCW's secondary system is shown in Figure 5.11 (c). The external work for the SCCW secondary fuse system is shown in Equation 5.34, while the internal work is shown in Equation 5.35.

$$W_{ExSC,SCCW} = \theta F_{SC} \sum_{x=1}^{n} (\lambda_x h_x) - P_g B_W \theta + P_p (-B_W + 2x_{cg})\theta$$
 Equation 5.34

$$W_{InSc,CORW} = 2f_b B_W \theta$$
 Equation 5.35

where B_W is the length of the wall pier; x_{cg} the distance from the rocking toe of the "tension" wall to the centerline of the wall; P_g is the gravity load in the wall pier; and Pp is the axial couple from the coupling beams, and is estimate as the axial coupled provided by the primary fuse system, shown in Equation 5.36.

$$P_p = \frac{F_{PR}}{\beta_{o,SCCW}L_c} \sum_{x=1}^{n} (\lambda_{x,SCCW} h_x)$$
Equation 5.36

By equating the SCCW secondary fuse system external work (Equation 5.34) and internal work (Equation 5.35) the secondary base shear along the SCCW axis can be determined and is shown in Equation 5.37.

$$F_{SC,SCWW} = \frac{P_g B_W + P_p (B_W - 2x_{cg}) + 2f_b B_W}{\sum_{x=1}^n (\lambda_{x,SCCW} h_x)}$$
Equation 5.37

The base shear at the DBE intensity for the SCCW ($F_{p,SCCW}$), shown in Equation 5.38, may now be determined by re-arranging Equation 5.38.

$$F_{p,SCCW} = F_{SC,SCWW} + F_{PR,SCCW}$$
Equation 5.38

Step 12: Determine the SCCW axis DBE roof displacement and post-yielding stiffness.

In this step, the DBE base shear ($F_{p,SCCW}$), determined in Step 11, and the equal energy concept is used to determine the DBE roof displacement ($\Delta_{p,SCCW}$) and the SCCW post-yielding stiffness (b_{SCCW}).

Using the EEDP, the incremental input energy, ΔE_{E1} in the SCCW axis, shown in Figure 5.12, is determined from when the shaking intensity is increased from the SLE to the DBE shaking intensities and is given by Equation 5.39.

$$E_{E1,SCCW} = \frac{1}{2} \left(F_{pe,SCCW} + F_{y,SCCW} \right) \left(\Delta_{pe,SCCW} - \Delta_{y,SCCW} \right)$$
Equation 5.39

where $F_{pe,SCCW}$ and $F_{y,SCCW}$ are the forces in the SCCW ELSDOF at the DBE and SLE hazards, respectively. $\Delta_{pe,SCCW}$ and $\Delta_{y,SCCW}$ are the displacement of the ELSDOF at the DBE and SLE hazards, respectively.

The corresponding incremental energy dissipated by the ENLSDOF which undergoes monotonic pushover ($\Delta E_{\text{NM1,SCCW}}$) is determined using Equation 5.40.

$$\Delta E_{\text{NM1,SCCW}} = \frac{1}{2} (F_{p,SCCW} + F_{y,SCCW}) (\Delta_{p,SCCW} - \Delta_{y,SCCW})$$
Equation 5.40

where $F_{p,SCCW}$ is the base shear at the DBE shaking intensity and $\Delta_{p,SCCW}$ is the roof displacement at the DBE shaking intensity.



Figure 5.12 Performance of SCCW under DBE intensity

The nonlinear dynamic energy ($\Delta E_{ND1,SCCW}$), determined from the ENLSDOF, is converted to the nonlinear monotonic pushover energy ($\Delta E_{NM1,SCCW}$) using the energy modification factor $\gamma_{a,SCCW}$. This process is shown in Equation 5.41.

$$\Delta E_{E1,SCCW} = \Delta E_{ND1,SCCW} = \gamma_{a,SCCW} \Delta E_{NM1,SCCW}$$
Equation 5.41

where $\gamma_{a,SCCW}$ is the energy modification factor shown in Equation 5.42. Details of the derivation are shown a subsequent section (*Section 5.4 Development of energy modification factors* $(\gamma_a \text{ and } \gamma_b)$).

$$\gamma_{a,SCCW} = (3.37b_{SCCW} - 2.5)\Phi_{p,SCCW} - 2.17b_{SCCW} + 2.8$$
 Equation 5.42

where $\Phi_{p,SCCW} = F_{pe,SCCW}/F_{y,SCCW}$

Rearranging Equation 5.42, gives the system post yielding stiffness shown in Equation 5.43.

$$b_{SCCW} = (\gamma_{a,SCCW} - 2.8 + 2.5\Phi_{p,SCCW})/(3.37\Phi_{p,SCCW} - 2.17)$$
 Equation 5.43

By substituting Equation 5.39 and Equation 5.40 in Equation 5.41, the corresponding design base shear at the DBE shaking intensity is determined by using Equation 5.44.

$$F_{p,CORW} = 2 \frac{\Delta E_{E1,CORW}}{\gamma_{a,CORW} (\Delta_{p,CORW} - \Delta_{y,CORW})} - F_{y,CORW}$$
Equation 5.44

Once $F_{p,SCCW}$ and b_{SCCW} are known, the SCCW roof displacement at the DBE intensity may be calculated using Equation 5.45.

$$\Delta_{p,SCCW} = \frac{F_{p,SCCW} - F_{y,SCCW}}{K_{s,SCCW} b_{SCCW}} + \Delta_{y,SCCW}$$
Equation 5.45

Step 13: Determine post yielding stiffness of coupling beam

In this step, the coupling beam post yield stiffness $(k_{cb,p})$ is determined. The relation between the SCCW system post yielding stiffness, b_{SCCW} , and the coupling beam post yielding stiffness $k_{p,post}$ is shown in Equation 5.46.

$$b_{SCCW} \approx \frac{\alpha_{2\text{kp}} A_w L_c^2 + 4I_w}{\alpha_2 A_w L_c^2 + 4I_w}$$
Equation 5.46

where A_w is the cross-sectional area of a wall pier; I_w is the moment of inertia of a wall pier; L_c is the distance from the centroids of the wall piers; α_2 , was calculated in Step 9, is a stiffness factor which relates the initial coupling beam stiffness to the stiffness of the wall; and α_{2kp} is a stiffness factor, shown in following equation, which relates the post-yielding coupling beam stiffness to the stiffness of the wall

$$\alpha_{2\rm kp} = \left(0.06 \frac{H}{L_c} + 0.3\right) \alpha_{2\rm kp} \le \alpha_{2\rm kp}^2 \qquad \text{from Equation 4.19}$$

where α_{2kp} is relative stiffness parameter of the coupling beams to the walls, shown in the following equation.

$$\alpha_c = \frac{1}{1 + \frac{E_c A_w}{n_f H k_{cb,p}}}$$
 from Equation 4.1

where $n_{\rm f}$ is the number of stories; $E_{\rm c}$ is the elastic modulus of the wall; $k_{cb,p}$ is the post yielding stiffness of the coupling beams.

Step 14: MCE performance point along SCCW axis

In this step, the performance of the SCCW at the MCE is determined and is shown in Figure 5.13. EEDP assumes that the energy dissipated by the energy stored in the ELSDOF is equal to the energy dissipated by ENSDOF system. Equation 5.47 shows the incremental energy stored in the ELSDOF system when the shaking intensity changes from SLE to DBE.

$$\Delta E_{E2,SCCW} = \frac{1}{2} \left(F_{ue,SCCW} + F_{pe,SCCW} \right) \left(\Delta_{ue,SCCW} - \Delta_{pe,SCCW} \right)$$
Equation 5.47

where $F_{ue,SCCW}$ is the elastic base shear of the SCCW ELSDOF system at the MCE hazard and $\Delta_{ue,SCCW}$ is the SCCW elastic roof displacement of the ELSDOF system at the MCE hazard. The corresponding incremental energy dissipated from the ENLSDOF, which undergoes monotonic pushover, is determined using Equation 5.48.

$$\Delta E_{\text{NM2,SCCW}} = F_{p,SCCW} \left(\Delta_{u,SCCW} - \Delta_{p,SCCW} \right)$$
Equation 5.48

Equation 5.49 shows the energy balanced equation.

$$\Delta E_{E2,SCCW} = \Delta E_{ND2,SCCW} = \gamma_{b,SCCW} \Delta E_{NM2,SCCW}$$
Equation 5.49

where $\gamma_{b,SCCW}$ is the energy modification factor to relate the energy dissipated by the system under monotonic pushover to that under dynamic load when the hazard level increases from DBE to MCE. Equation 5.50 shows the empirical equation for $\gamma_{b,SCCW}$, are shown a subsequent section (Section 5.4 Development of energy modification factors (γ_a and γ_b)).
$$\gamma_{b,SCCW} = (5\lambda_{p,SCCW}/\mu_{p,SCCW} - 9.5)\Phi_{s,SCCW} - 5.5\lambda_{p,SCCW}/\mu_{p,SCCW} + 9 \qquad \text{Equation 5.50}$$

where $\lambda_{p,SCCW}$ is the ratio of the DBE base shear to the SLE base shear (i.e., $\lambda_{p,SCCW} = F_{p,SCCW}/F_{y,SCCW}$); μ_p is the ratio of the DBE displacement to the SLE displacement (i.e., $\mu_{p,SCCW} = \Delta_{p,SCCW}/\Delta_{y,SCCW}$); and $\Phi_{s,SCCW}$ is the ratio of the MCE elastic base shear to the DBE elastic base shear (i.e., $\Phi_{s,SCCW} = \Delta_{ue,SCCW}/\Delta_{pe,SCCW}$).

By substituting Equation 5.47 and Equation 5.48 into Equation 5.49 the ultimate roof displacement $(\Delta_{u,CORW})$ can be determined, and is shown in Equation 5.51.

$$\Delta_{u,SCCW} = \frac{\Delta_{E2,SCCW}}{\gamma_{b,SCCW}F_{p,SCCW}} + \Delta_{p,SCCW}$$
Equation 5.51

where $\Delta_{E2,SCCW}$ is the incremental energy stored in the ELSDOF shown in Equation 5.49; $F_{P,SCCW}$ is the base shear at the DBE intensity determined in Equation 5.44.



Figure 5.13 MCE performance at SCCW axis

Step 15: Capacity design of the non-yielding elements

To ensure the CROCW will achieve the performance as specified in Step 1, the non-yielding elements such as the concrete core wall, outrigger system and foundation need to be capacity designed using the maximum probable forces from the damper. It should be noted that it maybe impractical to design RC structures to be fully elastic. Hence, in this research, the RC wall reinforcement is designed to remain elastic. To ensure the yielding stays within the dampers, the

connections, RC wall and foundation are designed for capacity using the maximum probable forces excreted from the dampers.

5.4 Development of energy modification factors (γ_a and γ_b)

As discussed in the previous section, EEDP assume the inelastic energy dissipated by the ENLSDOF system equals to the energy dissipated by the ELSDOF system. In 2018, Yang et al. proposed a set of γ_a and γ_b for factors to estimate the nonlinear dynamic energy dissipated by the ENLSDOF system using the nonlinear monotonic energy dissipated by the ENLSDOF system. In the original study, the system is assumed both the primary and secondary system have elastic-perfectly-plastic (EPP) force-deformation relationship. They proposed the relations for of γ_a and γ_b as a function of fundamental period and ductility factor ($\mu_p = \Delta_p / \Delta_y$). However, the CORW system utilizes the EPP hysteretic behavior for the primary system (outrigger) and flag-shaped hysteresis for the secondary system (SCFDs) and flag-shaped hysteresis for the secondary system (controlled rocking base). This unique combination requires an additional study to determine the relations for γ_a and γ_b .

To determine γ_a and γ_b factors, nonlinear time history analysis (NLTHA) was conducted on a series of SDOF systems with various parameters. The nonlinear hysteretic behavior is determined using two springs in parallel. The primary system is modelled using EPP behavior, while the secondary system utilizes a flag-shaped material following the approach proposed by Tremblay et al. (2008). In this study, a suite of 44 ground motions were adopted from the FEMA P695 study (FEMA 2009). Multiple DBE intensities ranging from 0.2, 0.4, 0.6 of the MCE level have been selected. Similarly, three additional shaking intensities ranging from 0.2, 0.4, 0.6 of the DBE hazard have been considered for SLE shaking intensity. For each combination of shaking intensities 16 nonlinear SDOF systems with 4 represented fundamental periods (T = 1, 2, 3, and 4) and 4 secondary fuse system stiffnesses of (b = 0.2, 0.4, 0.6 to 0.8) were analyzed. This results to a total of 6336 NLTHA for determining the γ_a and 6336 NLTHA for determining the γ_b .

From this analysis γ_a and γ_b factors for both the CORW and SCCW axis were developed. Equation 5.8, Equation 5.14, Equation 5.42, and Equation 5.50 shows the proposed relations for

 $\gamma_{a,CORW}$, $\gamma_{b,CORW}$, $\gamma_{a,SCCW}$, and $\gamma_{b,SCCW}$, respectively. These equations were presented in the prior sections. Figure 5.14 shows the data and the equations.



Figure 5.14 Numerical response of energy modification factors (a) $\gamma_{a,CORW}$, (b) $\gamma_{b,CORW}$, (c) $\gamma_{a,SCCW}$, and (d) $\gamma_{b,SCCW}$

5.5 Prototype building designs

This section presents four prototype CROCW system buildings designed using the proposed EEDP procedure presented in Section 5.3 of this chapter. The prototype buildings were designed for two hypothetical building sites. At each site, two different buildings, differentiated by number of stories were designed: 24-stories and 40- stories. In subsequent chapters, these prototype buildings will be used to investigate the seismic performance of the novel CROCW system. The following summarises the designs.

5.5.1 Prototype building site and hazard

Both prototype building sites are in South West coast of Canada. One site is located downtown Vancouver at longitude 48.419 and latitude -123.369 and the other site is downtown Victoria at longitude 48.419 and latitude -123.369. Vancouver was selected as a prototype site as it has one of the highest tall buildings per capita in the world (CTBUH, 2019) with virtually all tall buildings use RC core walls as the primary seismic force resisting systems (Adebar et al., 2017). Hence, Vancouver is an ideal prototype site to study the performance of the novel CROCW system. Victoria was selected as the second prototype site as it has the highest seismic hazard of any large Canadian city, comparable to cities such as Los Angeles in the US. This high seismic hazard makes Victoria another excellent site to study the performance of the novel systems introduced in this dissertation.

Both sites have an assumed shear wave velocity, Vs30, of 450 m/s, which is classified as site class C according to the NBCC, 2015. In this study, the Maximum considered earthquake (MCE) represents the 2% probability of exceedance in 50 years which follows the MCE design level in the ASCE (2016) and the NBCC (2015). In Canada, there is currently no recommendation for Service Level Earthquakes (SLE), therefore, the SLE uses the 50% probability of exceedance in 30 years recommended in the US for performance-based design (LATBSDC, 2020). The Design-based earthquake (DBE), at which the base dampers to remain elastic, was selected as the 20% probability of exceedance in 50 years.

The uniform hazard spectrums (UHSs) used for the designs are presented in Figure 5.15 (a) and (b) for the Vancouver site and the Victoria sites, respectively. These UHSs were developed using probabilistic seismic hazard analysis (PSHA). The PSHA is described in further detail in a

subsequent chapter (Chapter 7). As shown in Figure 5.15, spectral accelerations in the Victoria spectrum are approximately 60% greater than Vancouver values.



Figure 5.15 Uniform hazard spectrums for (a) Vancouver site and (b) Victoria site

5.5.2 Description of prototype building geometry and materials

Four prototype core wall buildings designed using the proposed EEDP and the UHS presented in Figure 5.15. For each location, two different number of stories were designed: 24 and 40. The story height is 4 m for all buildings, resulting in building heights, excluding the height of the outrigger, of 96m and 160m for the 24 and 40, buildings, respectively.

The Seismic Force Resisting System (SFRS) utilizes a central RC core-wall. The RC core is composed of two symmetric C-shaped walls with constant thicknesses up the height of the building. The core footprint is a square shape with dimension Lw x Lw. Along one principal axis (X-axis in (b)), the SFRS consists of a controlled rocking outriggered wall (CORW). Along the other principal axis (Y-axis (b)), the SFRS consists of the self-centring coupled wall (SCCW) system.

The gravity systems consist of flat slabs connected to RC columns. All prototype buildings utilize a square flat plate floor slab ($L_b x L_b$) with a slab thickness of 250mm. The gravity load and seismic mass were calculated assuming the structural component self-weights with an additional distributed load of 1 kPa for finishes. Previous studies have shown that minimal interaction between the rocking wall and the gravity system can be achieved through isolated wall-to-floor connection (Henry et al., 2012). However, the performance of the gravity system was not examined in this study.

The building geometric parameters were developed in consultation with practicing engineers familiar with the design practices in the United States of America and Canada. For simplicity, only two unique building geometry are considered for each location, however, the sizes of the dampers and vertical reinforcing steel differed between buildings. Figure 5.16 (a) shows the 24-story prototype elevations, Figure 5.16 (b) shows the 40-story prototype elevations, and Figure 5.16 (c) shows the plan view. Table 5.1 summarizes the geometry and dead load for the six prototype buildings.

Table 5.1 Characteristics of prototype building designs

Name	Location	Story #	H $[m]$	L_b^* [m]	L_w^* [m]	B_w^* [m]	t_w^* [m]	t_{f}^{*} [m]	W ^{**} [MN]	Pg ^{***} [MN]
Van24	Vancouver	24	96	26.85	9.25	3.39	0.58	0.99	179	41
V1c24	Victoria									
Van40	Vancouver	40	160	27.6	12.00	4 77	0.50	0.80	200	75
Vic40	Victoria	40	100	27.0	12.00	4.//	0.50	0.80	300	15

* Parameters L_w, B_w, t_w, t_f are shown in Figure 5.16; ** W is the total seismic weight of the building; *** P_g is gravity load on one C-shaped wall pier.



Figure 5.16 Prototype buildings (a) 24-story elevations; (b) 40-story elevations; and (c) plan view

The outrigger system consists of a steel truss system rigidly connected to the RC core and to exterior mega-columns. The detailed geometry of the outrigger system is shown in Figure 5.17.

The RC mega-columns are assumed to have a constant cross section of 1.2m x 1.2m over the entire height of the building. Symmetric friction dampers are fixed vertically between the mega-columns and the truss system. In these designs, the outrigger truss and columns are designed to be elastic. The outrigger system configuration used in this study are shown in Figure 5.17, however, other configurations could be equally suitable. For all prototype buildings, the outrigger depth was 8m, the top chords were W14x283 steel sections; and the Braces and Verticals were W14x426. The outrigger length is 25.75m and 26.5m for the 24-story and 40-story buildings, respectively.

The coupling beams utilize embedded steel beams connected to self-centering conical friction dampers (SCFDs). Figure 5.17 (b) shows the coupling beams embedded in the RC wall. The embedded detailing is assumed to follow the recommendations by AISC, 2016. In the prototype design, the SCFD are designed for the different force demands. However, it is assumed that all the Embedded beams are W24x335.



Figure 5.17 Prototype outrigger geometry

The prototype buildings were designed assuming that there were no subgrade floors. It is also assumed that the controlled rocking base is constrained from sliding either through friction or a mechanical shear key. The restoring forces for the controlled rocking base are assumed to be entirely from the gravity loads. Hence, there was no need for additional post-tensioning, typical of controlled rocking walls. The dampers within the controlled rocking base are sliding friction dampers (Rodgers et al., 2017).

All structural steel sections utilize grade ASTM A992/A992M steel, which has a yield stress of 345 MPa and a minimum tensile strength of 450MPa. All the reinforcing steel is assumed to follow CSA G30.18 Grade 400W grade reinforcing steel, which has a design yield strength of 400MPa. The elastic modulus for all steel is assumed to be Es = 200GPa. The design elastic modulus is

calculated following CSA A23.3 and is shown in Equation 5.52. For a specified compressive strength at 28 days of f'c = 50MPa and f'c = 60 MPa, for the 24-story and 40 story buildings respectively.

$$E_c = 3300\sqrt{f'c} + 6900 [MPa]$$
Equation 5.52

5.5.3 Summary of design parameters

Table 5.2 shows the design EEDP base shear, roof displacements, and damper forces for each prototype building. Table 5.3 shows the damper forces for the outrigger (f_o), base dampers (f_b), roof coupling beam shear (V_{pr}), and the post yielding stiffness of the coupling beam ($k_{c,b}$). Capacity design was conducted for each prototype building. For simplicity, a constant reinforcing ratio was used throughout the height of the building, Figure 5.18 shows the designed cross section for the RC walls.

	Name	T_1	γ _a	γ_b	b [-]	Δ_{y}	Δ_p	Δ_u	F_{y}	F_p
		[<i>s</i>]	[—]	[—]		[mm]	[mm]	[mm]	[MN]	[MN]
	Van24	- 2 10 -	1.40	2.70	0.61	40	115	435	3.38	10.75
RM	Vic24	2.19	1.40	2.69	0.61	60	175	680	5.42	17.20
Q	Van40	4 10	1.23	2.60	0.70	45	145	710	2.34	7.55
0 -	Vic40	- 4.19 -	1.22	2.61	0.70	75	225	1130	3.74	12.07
SCCW	Van24	2.00	1.77	4.83	0.24	45	130	545	2.95	9.62
	Vic24	- 3.00 -	1.80	4.88	0.22	65	205	885	4.72	15.38
	Van40	5.00	1.58	4.63	0.41	50	155	825	2.60	8.49
	Vic40	- 3.23 -	1.80	5.20	0.23	75	255	1475	4.16	13.58
Table 5.3 Damper design values										
Name			$f_o[kN]$		$f_b[kN]$		$V_{pr}[kN]$		k _{c,b} [kN/mm]	
Van24		920		3425		295		35		
Vic24		1470		15675		465		31		
Van40		800		2590		225		37		
Vic40		1280		3065		360		16		

Table 5.2 EEDP design values for prototype CROCW systems, along CORW and SCCW axis



Figure 5.18 Reinforcing detailing for each prototype building: (a) Vancouver 24-story, (b) Victoria 24story, (c) Vancouver 40-story, and (d) Victoria 40-story

5.6 Summary

This chapter presents a detailed EEDP for the CROCW system. While this chapter focused on designing the CROCW system, the same methods may be applied to both the SCCW and CORW systems independently. When designing independently, the governing base damper design should be used. This would result in the damper in one direction being overdesigned for the other. However, this is equally suitable, as it would be a conservative, albeit less efficient, design.

Modifications to the EEDP to account for higher mode effects are presented. Additionally, new energy modification factors are presented in this chapter, which accounts for the unique hysteretic shapes from the primary and secondary fuse systems.

The design procedure developed in this chapter is advantageous as, like other performance-based design methods, it allows the design of multiple performance objectives at different shaking intensities. However, unlike other performance-based design methods, EEDP allows for the design to be completed without iterations. This modified design process will allow for an efficient design process when implementing the novel CROCW, CORW, or SCCW systems.

This chapter concludes with the design of four prototype buildings. Two different story heights (24-story and 40-story) prototype buildings were designed for each of the two different locations

(Vancouver site and Victoria site). Buildings were designed to a Maximum considered earthquake (MCE) of 2% probability of exceedance in 50 years; a Design-based earthquake (DBE) of 20% probability of exceedance in 50 years; and a Service level earthquake (SLE) of 50% probability of exceedance in 30 years.

The next several chapters focus on investigating the nonlinear seismic performance of these systems using four prototype buildings. Chapter 6 describes the nonlinear modelling approach used for the remainder of the thesis. Chapter 7 describes the prototype site probabilistic hazard analysis, and ground motion selections for each prototype building site used throughout the remainder studies. Chapter 8, Chapter 9, and Chapter 10 investigate the seismic performance of the CORW and SCCW designed with the EEDP, which was presented in this chapter, for unidirectional loading in the CORW direction, unidirectional loading in the SCCW direction, and bidirectional loading, respectively.

Chapter 6: Nonlinear modelling approach

6.1 Overview

This chapter presents the nonlinear modelling approach, which is used in subsequent chapters to investigate the nonlinear dynamic response of the CORW and SCCW systems. Described in this chapter in the detailed 3-dimensional modelling approach for the overall structure and individual components. System-level assumptions, such as gravity loads, mass distribution, and damping are discussed. Each of the structural component numerical modelling methods, including RC wall, controlled rocking wall, SCFD, and axial friction dampers, were validated using experimental data.

6.2 System modelling

The numerical modelling utilises the commercially available software package PERFORM 3D (Computers and Structures, 2018). This software is commonly used for performance-based seismic design of tall buildings in Canada and the United States. The detailed modelling approach presented in this chapter, which uses an accessible and well-understood software, can allow for the newly proposed earthquake-resilient RC core wall system to be more easily adapted to industry.

To demonstrate the modelling approach, an example building is modelled which consists of a CORW along one principal axis (X-axis, Figure 6.1) and a SCCW along the other principal axis (Y-axis, Figure 6.1). The 3-dimensional model, shown in Figure 6.1, consists of components such as RC walls, controlled rocking base, SCFD, outrigger members and unidirectional friction dampers. The core wall utilizes 4-node *Shear Wall Elements*, situated at the centerline of the wall. The SCFDs are connected to the walls using elastic beam elements. The detailed modelling and validation of these components are discussed in Section *6.3 Component modeling*.

At typical story levels, the nodes were constrained using *Horizontal Rigid Floor* diaphragm constraint. The nodes at the base of the models were assigned restraints according to the desired fixity of degrees of freedom. Typically, vertical elements have all degrees of freedom restrained at the base. Seismic mass was calculated as 100% of the dead loads which includes self-weight of the structure and additional superimposed dead loads due to use and occupancy. The seismic mass was lumped onto a single node at the mass centroid and assigned the total lateral mass and torsional

mass moment of inertia. The rigid diaphragm constraints then ensure the inertial forces are distributed out to the components of the structure.



Figure 6.1 Numerical model of an example building shown in (a) 3D view, (b) Front Elevation, and (c) Side Elevation

The 3-dimensional model was developed assuming that the Seismic Force Resisting System (SFRS) resists 100% of the earthquake loads. The lateral resistance due to gravity members (i.e., slabs and columns) were excluded from analysis. However, under certain circumstances, the global stability of structures can be significantly affected by P-Delta effects (P- Δ), which refers to the second order forces induced when structures displace laterally while simultaneously being loaded with vertical (gravity) forces.

Since this research focusses on tall buildings, where both gravity loads and lateral drifts can be large, P- Δ forms an important part of the response. P- Δ effects were included in all time history analyses by using a "Leaning column" or "P-Delta column." These elements have no lateral stiffness of their own but are connected to the SFRS and will "lean" on it for stability as the structure displaces laterally. The P-Delta column is loaded with all the non-simulated gravity loads

from the secondary framing system, such that the total gravity load of the structure is included in the analysis: The SFRS includes its tributary gravity loads (which are needed to properly simulate the response of fiber elements) while the remainder of the gravity loads are placed on the P-Delta column. The columns were modelled using elastic frame elements with flexural releases ("pins") at the floor levels.

The nonlinear time-history analysis uses event-to-event solution technique which provides a stable and predictable output, even with highly nonlinear structures such as rocking systems. Damping for nonlinear time history analyses employed modal damping across all significant modes of the structure, plus a small amount of Rayleigh damping. The total viscous damping ratio was set to 2.5% in line with common practice in nonlinear time history analysis (LATBSDC, 2020). Modal damping accounted for 2.4% of that damping ratio, while Rayleigh mass and stiffness proportional damping accounted for the remaining 0.1%, anchored to the periods 2.0 T₁ and 0.1 T₁. This approach is recommended to avoid some of the issues encountered with solely using Rayleigh damping or solely modal damping (Hall, 2006; Chopra & McKenna, 2016; LATBSDC, 2020).

6.3 Component modeling

The complex nonlinear behaviour of RC elements can be simulated using three broadly categorized approaches: Lumped Plasticity Models, Continuum Models, and Distributed Plasticity Models. Lumped Plasticity models are the simplest and most computationally efficient, however their simplicity often comes at the expense of missing certain phenomenon that are of interest in RC structures. For instance, a concrete element might be simulated with an elastic frame and lumped plasticity moment-hinges at each end of the element, but this model will not be able to account for the interaction between moment and axial load, nor will it provide useful information about the strain distribution in the element. For the above reasons, other modelling approaches are often chosen for simulation of vertical elements (walls, columns) in concrete structures. Continuum Models offer the most detailed modelling approach, with the ability to capture most phenomenological effects. This approach is a good candidate for simulating the detailed response of a concrete element, however this comes at the expense of computational effort, and is generally difficult to implement for full building structures. The final modelling approach, distributed plasticity models, is the one utilized in this thesis for simulating RC elements. This approach

affords computational efficiency while still capturing many of the pertinent characteristics of the response of RC structures to static and dynamic loads.

PERFORM offers various component models to simulate the behaviour of structures, including both lumped plasticity and distributed plasticity components. In general, one or more structural behaviours can be defined, and then aggregated together into a compound element, which is assigned to the model geometry. For example, a shear wall fiber section can be aggregated with linear or nonlinear wall shear behaviour to form a complete shear wall compound, or nonlinear moment-rotation hinges can be aggregated with linear frame elements to form a nonlinear beam compound.

The adopted modelling approaches are discussed further in this section. The five main component types used in the numerical model are: the RC walls, the controlled rocking mechanism, the novel self-centering coupling beams spanning between wall piers, the hysteretic dampers used at the outrigger and wall-base, and the outrigger frame elements. Each component is described in the following sections, and nonlinear behaviour is validated with a previously conducted experiment.

6.3.1 Fixed base RC Walls

Nonlinear fiber elements are used to capture the hysteretic behaviour of the RC walls. In a fiber model, the material constitutive relations are assigned to discretized fibers over the cross section of the element. These elements are advantageous due to their computational efficiency and reasonable accuracy in predicting of the axial-flexural interaction. Typically, the shear behaviour is assigned as an uncoupled spring with linear elastic behaviour. The shear and flexural behaviour is decoupled; however, this simplification is common for modeling the SFRS in tall slender walls (LATBSDC, 2020).

6.3.1.1 Element formulation

Wall elements were modelled using *Shear Wall Element*. This is an engineering element, rather than a general-purpose finite element, and it is intended to capture the behaviour of RC walls without being excessively complex (Computers and Structures Inc., 2018). The shear wall element has 4 nodes and 24 degrees of freedom. Figure 6.2 shows a schematic representation of the eight in-plane deformations of the element. In this element, inelastic behaviour is modelled in the

axial/bending in the vertical direction and linear shear behaviour. Other degrees of freedom are modelled with linear elastic behaviour, with reduced stiffness to account for cracking.



Figure 6.2 PERFORM-3D Shear Wall Element (Computers and Structures Inc., 2018)

The complex hysteretic axial-flexural behaviour of RC walls is captured by multiple nonlinear uniaxial fiber elements arranged together into a wall cross section. Each fiber is assigned material properties, cross-sectional area, and a coordinate within the overall wall cross section. The shear properties are modeled using an uncoupled elastic spring, with a cracked stiffness of 0.5GA_v as recommended by Los Angeles Tall Building Council (LATBSDC 2020).

6.3.1.2 Material properties

The fiber element formulation for a typical RC wall relies on at least two uniaxial material formulations: Reinforcing Steel and Concrete. The Reinforcing steel was modelled using the *Inelastic Steel Material, Non-Buckling* material. The basic backbone and cyclic behaviour are shown in Figure 6.3 (a) and (b), respectively. The steel material is described with a trilinear backbone passing through the points (0, 0), (DY, FY), (DU, FU), (DX, FX), and is symmetric in tension and compression. The yield stress, FY, is chosen based on the measured (or expected) yield stress of the steel, and the yield strain, DY, corresponds to this stress divided by the modulus of elasticity for steel, assumed to be 200,000 MPa. The next point is defined by the measured (or expected) ultimate tensile stress of the steel, and an assumed strain of 0.075, which is

approximately half of the fracture strain. After this point, the stress plateaus, and the final strain is set to an assumed fracture strain of 0.15. Cyclic degradation was included in the steel material using the YX+3 option, with deformations set to (0.0025, 0.004, 0.006) and energy factors (0.7, 0.68, 0.64, 0.62, 0.60), which follows the approach in PEER Tall Building Initiative (PEER, 2012).



Figure 6.3 Reinforcing Steel Material in units of MPa for stress and strain is unitless (a) backbone, and (b) Cyclic behaviour

In this study a single unconfined concrete material was used. The cyclic behaviour of concrete was modelled using the *Inelastic 1D Concrete Material*. In compression, the concrete material is enveloped by a trilinear backbone, followed by linear strength loss, and finally a constant residual strength. In this model, the tension stress is set to 0. The initial stiffness was determined using the equation $4500\sqrt{f_c'}$ (MPa). The compressive stress and strains at the other key points on the curve were determined by approximating the high strength concrete model described by Razvi and Saatcioglu (1999).

6.3.1.3 Description of experimental test for validation

The numerical models developed in this section are validated to a RC wall specimen tested by Adebar et al. (2007), summarized in Figure 6.4. The specimen was intended to represent a typical Vancouver high-rise building wall, constructed at ¹/₄ scale. The specimen was 12.2 m tall and has a 1.625 m wide I-shaped cross section. The flanges and web were 203 mm and 127 mm, respectively. The reinforcing ratio in the flange was 0.65% provided by 5-10M vertical bars enclosed by #3@64mm ties. The reinforcing ratio in the web was 0.26%, provided by a single centered layer of 10M@305mm bars. Horizontal reinforcing was provided through the web and into the flanges with a single centered layer of 10M@305mm bars. The average compression strength (f^{*}c) from the concrete cylinder tests at the day of testing was 49MPa. Tensile testing of the reinforcing showed a yield and ultimate strength of 455 and 650 MPa, respectively. A constant

axial load was applied during the test with magnitude equal to $0.10f_c'A_g$ (1500 kN). Lateral load was applied 11.76m from the wall base using a hydraulic actuator using a constant displacement rate of 1mm/second. The loading protocol for the test included four cycles at each of the 13 displacement levels, which increased from ±15mm at Level 1 to ±300mm at Level 13.



Figure 6.4 Details of test wall specimen adapted from Adebar et al. (2006): (a) elevation; (b) cross section; (c) instrumentation

6.3.1.4 Validation of model

To validate the RC wall modeling approach, the experimental test by Adebar et al. (2007) was simulated using the methods describe above (shown in Figure 6.5). Shear wall elements were defined such that the outer element on either side of the wall encompassed the entire flange, while the web was discretized into 4 elements across its length. Both flange and web had 46 elements along their height. The reinforcing ratio in the flange and wall elements were 0.65% and 0.27%, respectively. The lowermost row of elements had double the reinforcing ratio, to simulate the extra rebar across the construction joint. Wall shear behaviour was modelled linearly with an effective shear modulus of G = 6250MPa, or approximately 0.5 times the uncracked value. One additional

row of elements was modeled above the point of lateral load application, like real wall geometry. Vertical gravity loads (PT loads on the real specimen) were applied to four nodes at the uppermost point on the wall, each with one quarter of the total applied PT force. The model was assumed to be fixed at the base, which coincides with the top of the foundation block in the real specimen.



Figure 6.5 Model of experimental test of RC wall

The model was loaded to each of the recorded displacements. As shown in Figure 6.6, the simple modeling approach presented in these sections provides a reasonable match to the measured experimental behaviour. At low drifts, both the specimen and the model exhibit a prominent pinching behaviour, due to the high axial stress closing cracks upon unloading. The largest discrepancy comes from an apparent dilation of this pinched region at the largest drift cycles, likely due to deterioration of the concrete in the flexural cracks which then prevents the cracks from fully closing upon load reversal. For clarity, only the response up to 1.5% drift is shown in Figure 6.6.



Figure 6.6 Comparison of (a) experimental from Adebar et al.(2007) and (b) numerical RC wall hysteresis

6.3.2 Controlled Rocking Base

The controlled rocking base is a critical contributor to the overall behaviour of the system. By allowing predictable rocking behaviour, the base of the structure can be engineered to be damage free, whereas conventional RC walls would see concentrated yielding and damage in this region. Nonlinear fiber elements are used to capture the behaviour of the controlled rocking base. The following summarizes the modeling approach and validation of the controlled rocking base.

6.3.2.1 Element formulation

The controlled rocking base uses the same 4-node Shear Wall elements as the RC walls. The key difference is that the rocking elements can only resist compression and shear. They do not contain any materials with tensile strength in the fiber section. The use of a 4-node element compatible with the RC walls above makes the modelling simple since the rocking based can be directly connected to the RC walls.

6.3.2.2 Material Properties

The compression resistance of the base was modelled with an Inelastic 1D Concrete Material, like the conventional RC wall elements. However, this base material can use a much simpler elasticplastic backbone curve in compression, and zero tension response, as shown in Figure 6.7 The initial compression stiffness was assumed to be the gross concrete stiffness, determined using the equation $4500\sqrt{f'_c}$ (MPa). The compression strength was assumed to be equal to f_c of the wall. In this study the strength loss and cyclic degradation of the base is not directly modelled, due to the presence of shielding and the base being a larger element with confining effects from the surrounding concrete. The shear response of the rocking elements was modelled elastically using the gross shear stiffness of the rocking base material.



Figure 6.7 Rocking base force-deformation response

6.3.2.3 Description of experimental test for validation

The numerical model described in this section was validated against a controlled rocking wall specimen tested by Perez et al. (2007) and summarized by Figure 6.8. Several different wall panels were included as part of their testing program, but TW2 was chosen for this validation study as it most closely resembled the conditions of the prototype buildings developed later in this thesis. The TW2 specimen had a rectangular cross section with dimensions 2540 mm by 152 mm (100 in by 6 in). The specimen height between the point of lateral loading and the base joint was 7232 mm (23.73 ft), and within this height was four wall panels connected by floor joints.



Figure 6.8 TW2 Specimen details from Perez et al. (2007): (a) Isometric View; (b) Elevation; (c) cross section

The wall was reinforced with a combination of conventional wall reinforcing and unbonded prestressing strands. In the lowermost two wall panels, each end of the wall had a 685 mm (27 in) long confined concrete region, using 8 overlapping spiral coils for the transverse reinforcing. The coils had yield and ultimate stresses of 414MPa and 614MPa (60ksi and 90ksi) respectively. The entire wall was enclosed by 4x4-W4.0xW4.0 welded wire mesh, with additional horizontal wires added in the top and bottom third of the wall panels.

Five unbonded prestressing tendons were symmetrically placed between the confined regions. Each tendon was stressed to $0.553 f_{pu}$ which results in an average concrete compressive stress of 8.2 MPa (1.19 ksi) after losses, but before application of additional axial load. Additional gravity load was applied using an external PT bar on each side of the specimen midpoint, with hydraulic actuators maintaining a constant applied axial load of 531.5 kN (119.5kip) throughout the experiment.

The loading protocol for the test included three loading and unloading cycles at each of the 9 drift levels, which increased from $\pm 0.05\%$ to $\pm 3.0\%$.

6.3.2.4 Validation of model

To validate the rocking wall modeling approach, the experimental test by Perez et al. (2007) using the component models described previously was used. Shear wall elements were defined such that the cross section had 8 elements across its length. The outermost two elements on each end of the wall were adjusted to encompass the confined regions of the wall. The concrete model described previously was adjusted to account for the level of confinement and measured material properties from the test. Both flange and web had 46 elements along their height.

The reinforcing ratio in the flange and wall elements were 0.65% and 0.27%, respectively. The lowermost row of elements had double the reinforcing ratio, to simulate the extra rebar across the construction joint. Wall shear behaviour was modelled linearly with an effective shear modulus of G = 6250MPa, or approximately 0.5 times the gross value.

One additional row of elements was modeled above the point of lateral load application, like real wall geometry. Vertical gravity loads (PT loads on the real specimen) were applied to four nodes at the uppermost point on the wall, each with one quarter of the total applied PT force. The base

of the wall was modelled with full fixity, while the PT strands were pinned. The model was loaded to each of the recorded displacements by a point load positioned like the actuator.

The response of the experimental and numerical responses is shown in Figure 6.9 (a) and Figure 6.9 (b), respectively. As shown, this modeling approach adequately follows the behaviour from experimental testing. Specifically, the backbone and initial stiffness are well matched. There is some further in-cycle degradation observed in the test which is not captured within in the model. However, the model is deemed sufficiently similar for system level modeling.



Figure 6.9 Comparison between (a) experiment from Perez et al. (2007) and (b) numerical response for controlled rocking wall

6.3.3 Self-centering coupling beams

Figure 6.10 shows the modeling approach for the coupling beam calibration model. The coupling beams are modeled assuming all the plasticity from the self-centering conical friction damper is lumped in the middle of the beam. The flexibility is modeled using elastic beams, which are connected to the wall. The wall nodes are free to rotate, therefore, a rigid beam is required to transfer the forces from the coupling beam into the walls.

There is currently no plastic hinge property within Perform3D that gives a self-centering hysteretic response. Therefore, to model the self-centering behaviour of a SCFD, an *Elastic nonlinear bar*, an *inelastic bar*, and a *tension only/compression only* bar are modeled in parallel (shown in Figure 6.11). The bars are connected to two rigid elements which connects to the elastic beams (shown in Figure 6.10).

In this model, the yield force (P_0) is taken as the sum of the yield of the inelastic bar ($P_{0,in}$) and the activation force of the nonlinear bar ($P_{0,nl}$). The hysteretic behaviour of the SCFD (shown Figure 6.11) is such that the loading stiffness (k_p) is different than the unloading stiffness ($k_{p,nl}$). In this model, the loading stiffness is taken as the sum of the Tension only/Compression only element stiffness (k_{tc}) and the stiffness of the nonlinear bar after the activation force is reached ($k_{p,nl}$). The Tension only/Compression only elements will not provide stiffness to the unloading, therefore the only stiffness from unloading is provided by the nonlinear bar stiffness ($k_{p,nl}$).



Figure 6.11 Perform3D modeling of self-centering coupling beam bars in parallel

6.3.3.1 Description of experimental test for validation

The modelling approach discussed above was validated using the experimental data from Yang et al. (2020). In their study, a 1/6.5 scale specimen was tested under quasi-static cyclic loads. Figure 6.12 shows the different components of the SCFD tested by Yang et al. (2020). The flat sliding surfaces are steel and Teflon (Figure 6.12 (a)).

The cones, shown in Figure 6.12 (b) and Figure 6.12 (d), were manufactured using stainless steel, where the friction coefficient is estimated to be $\mu = 0.3$ and the friction coefficient between steel and Teflon is estimated as $\mu' = 0.03$. Belleville washers (Figure 6.12c) were used as the PT tendons due to the scale of the specimen. Due to the cost of the manufacturing, only one cone with a slope angle of 22 degrees was used.

Figure 6.13 shows the test setup used for the SCFD. In this setup the SCFD is tested in shear loading. A pantograph system is used to prevent rotation of the loading beam. The actuator has a capacity of 1000kN and a stoke of +/- 150mm. The loading frame was designed to move in vertical (U1) direction. Displacements were recorded locally using LVDTs mounted across the device.



Figure 6.12 Prototype specimen from Yang et al. (2020) (a) Clamping plate B with stoppers (b) Clamping plate A with rectangle openings and female cone (c) Belleville washers (d) Inner plate with male cone



Figure 6.13 Experimental testing set-up for the SCFD from Yang et al. (2020)

6.3.3.2 Validation of model

Using the bars in parallel method described above, the experimental results of a specimen called *6P10W10*, from Yang et al., 2020, was modelled. The responses of the experimental and numerical responses are shown in Figure 6.14 (a) and Figure 6.14 (b), respectively. As shown, this modeling approach follows the behaviour from experimental testing. Specifically, the loading and unloading stiffnesses are well-matched.



Figure 6.14 SCFD modeling comparison between (a) experimental work by Yang et al. (2020) and (b) numerical model

6.3.4 Outrigger and base dampers

Unidirectional friction dampers are used in both the controlled rocking base and the outriggers, as shown in Figure 6.15. The dampers are modeled using truss elements. In the outrigger, these trusses are simply connected from the mega-columns to the outrigger truss. In the controlled rocking base, these dampers are connected from a pin support to a rigid beam offset, which is connected to a rigid offset beam. The wall shell element nodes are free to rotate, hence, the need for rigid beams.

The hysteresis is simply modeled using an inelastic bar with elastic-plastic behaviour. The following section describe the validation of this modelling approach with experimental tests conducted by Professor Geoffrey Rodgers from the University of Canterbury New Zealand in 2019.



Figure 6.15 unidirectional dampers in the outrigger and the wall base.

6.3.4.1 Description of experimental test for validation

In 2019, Professor Geoffrey Rodgers from the University of Canterbury New Zealand conducted a series of axial friction damper tests. Two unique specimens were experimentally tested by Prof. Geoffrey Rodgers. One damper was designed as a scaled outrigger damper, where a symmetric force displacement response is required, while the other was a scaled controlled rocking damper, where significant ductility is required in tension, and lower ductility is required in compression. Figure 6.16 shows the specimens tested. The specimens were reverse cyclically loaded and the global force and displacement were recorded.



Figure 6.16 Friction damper experimental tests from Prof. Geoffrey Rodgers, University of Canterbury: (a) front view, (b) side view, and (c) testing set-up

6.3.4.2 Validation of model

These dampers are modeled using inelastic truss elements which have elastic-perfectly-plastic behaviour. The experimental tests by Rodgers, 2019 are used to validate the modeling approach. Figure 6.17 shows the comparison between the numerical hysteresis and the experiment.



Figure 6.17 Comparison between experimental and numerical response of friction dampers for (a) outrigger damper and (b) base damper (experimental data courtesy of Rodgers, 2019)

6.3.5 Outrigger Members

The outrigger system was modelled elastically using frame elements. Nonlinearity is intended to be confined to the connected friction damper which act as fuses for the outrigger, ensuring that the outrigger members do not experience forces larger than intended. The outrigger system is checked in post-processing to ensure elastic behaviour.

6.4 Nonlinear modelling of prototype buildings

The nonlinear model was built using the methods described above and eigen-value analysis was conducted. The mode shapes for the 24 and 40 story prototype buildings are summarized Figure 6.18 show the prototype buildings first, second, and third modes are dominantly in the SCCW, CORW, and torsional, respectively.



6.5 Summary

This chapter presents the nonlinear modelling approach for conducting bidirectional nonlinear time-history analysis for the proposed CROCW system. The experimental validation of the numerical modelling approach for each structural component, including RC wall, controlled rocking wall, SCFD, and axial friction dampers is presented. Additionally, the assumptions about gravity loads, mass distribution, and damping are discussed.

The numerical modelling approach for the SCFD is a unique contribution within this thesis. The SCFD, only recently being developed, does not have a recommended modelling method. In this chapter, a method to model the unique characteristics of the SCFD are presented. The approach provides a reasonable estimate of the hysteretic response. Additionally, the SCFD can have different hysteretic shapes depending on the design geometry and friction coefficients. The modelling approach presented in this chapter can match well with all of these hysteretic shapes.

This chapter summarises the nonlinear modelling approach used to developed 3-dimensional numerical models of the prototype buildings designed in Chapter 5. The following chapter (Chapter 7) describes the prototype site PSHA and ground motion selection and scaling. The subsequent chapters utilise the nonlinear modelling approach and the selected ground motions to assess the seismic performance of the CORW, SCCW, and CROCW systems.

Chapter 7: Probabilistic seismic hazard analysis and ground motion selection and scaling

7.1 Overview

This chapter presents the Probabilistic Seismic Hazard Analysis (PSHA) used to develop the Uniform Hazard Spectrums (UHS) for both prototype sites (described in Chapter 5). After the presentation of the PSHA is the ground motion selection and scaling process. The selection of suitable ground motions is essential within the seismic performance assessment process. The ground motions selected within this chapter are used throughout the remainder of the thesis to study the nonlinear dynamic performance.

7.2 Introduction

This chapter describes the probabilistic seismic hazard analysis (PSHA) used to determine the uniform hazards spectrums (UHSs) and the ground motion selection and scaling. Selection of suitable ground motions is important within the performance assessment process. The selection of ground motions follows the NBCC Annexure J, where all the high contributing sources must have a unique suite of ground motions. These ground motions are used throughout the remainder of thesis to in the nonlinear modelling.

7.3 Site characteristics

Both cities are in the South West region of British Columbia which has an active seismic setting. Figure 7.1 shows a distribution of earthquake magnitudes in Canada between 1663 and 2006. As shown, the Southwestern region of Canada has some of the most powerful earthquakes in the country.

Despite the active seismic setting, there is a limited amount of data on damaging earthquakes. The first seismograph to be monitored continuously in Western Canada was situated in Victoria, British Columbia and began in 1898 (Cassidy, 2010) and the last recorded damaging earthquake was a magnitude 7.3 crustal event, which occurred in 1946 off the coast Vancouver Island. The regions near the epicenter of this historical event were sparsely populated and as such minimal damage

and casualties occurred. Despite the minimal damage caused by earthquakes in recent history, there is sufficient geological and historical evidence to conclude that large magnitude earthquakes will occur in Southwest Canada in the future.



Figure 7.1 Significant earthquakes between the years 1663 – 2006 (NRCAN, 2020)

The West Coast of Canada has highly complex tectonic plate movements due to the different plates (Explorer plate, Juan de Fuca Plate, South Gorda Plate, Pacific Plate, and the North American plate) interacting with one another, shown in Figure 7.2. The tectonic setting results in three main types of earthquakes: crustal, subduction intraslab (sometimes referred to as subcrustal), and subduction interface.

Crustal earthquakes, which occur in the oceanic crust and in the continental crust, have a strikeslip and thrust fault mechanisms. The epicenter could be between 0 to 30km deep. This earthquake can produce up to about a magnitude of 7.5.

Subcrustal (also referred to as subduction intraslab) earthquakes which occur within the Juan de Fuca plate. Subcrustal earthquakes in this region have a normal fault mechanism, can produce a magnitude 7 to 7.5 earthquake, and the epicenter of this earthquake could occur between 30km to 60km deep.

Subduction earthquake (also referred to as subduction interface) which can occur between the Juan de Fuca plate and the North American plate. Subduction earthquakes in this region could rupture a zone that could be greater than a 100km long and could produce up to a magnitude 9 earthquake.



Figure 7.2 Tectonic setting of southwest British Columbia (Adapted from Rogers et al., 2015)

Each type of earthquake (i.e., crustal, subduction intraslab, and subduction interface) must be considered when developing the hazard and selecting ground motions. The following sections describe how each type of earthquake hazard is considered.

7.4 Probabilistic seismic hazard analysis

PSHA is a powerful tool which helps identify the magnitude, distances, and earthquake types which can affect a building site. The PSHA utilizes Open File 7576, (Halchuk et al, 2014), for the NBCC 2015 model. In this PSHA, seismic sources are obtained from seismic zones and faults. If there is a range of uncertainty of the earthquake's characteristics over a region, a seismic zone is used. In the case where the fault geometry and location are well understood, faults are used as sources. The seismic sources used in this PSHA are shown in Table 7.1.

Ground motion prediction equations (GMPE) are models give the spectral accelerations or PGA of a site by a given source. They are a function of magnitude, distance to site, and source type. Different sources may require different GMPEs based on the characteristics of the source. The 2015 NBCC GMPEs used are based on the recommendations from (Atkinson and Adams, 2013).

Different GMPEs may require different distances to the site. Joyner-Boore(Rjb) distance, is the closest distance to the surface projection of rupture plane. The rupture distance (Rrup) is the closest distance to fault rupture surface. The hypocentral distance (Rhypo) is the distance to the point

where the fault begins to rupture. This is the location beneath the epicenter. For the Open File 7576 the Rjb is used for crustal sources, Rrup is used for subduction fault sources, and the Rhypo is used for the offshore area source and the subduction area sources. Table 7.1 shows the different distances used for each of the sources.

Name	Туре	Name	GMPE	Distance type
BRO	Area	Brooks Peninsula	Crustal	Rhypo D = 10km
CAS	Area	Cascade Mountains	Crustal	Rhypo D = 10km
CST	Area	Coastal Mountains	Crustal	Rhypo D = 10km
EXP	Area	Explorer plate bending	Crustal	Rhypo D = 10km
FHL	Area	Flathead Lake	Crustal	Rhypo D = 10km
HEC	Area	Hecate strait	Crustal	Rhypo D = 10km
JDFF	Area	Juan de Fuca plate bending, offshore	Crustal	Rhypo D = 10km
NBC	Area	Northern British Columbia	Crustal	Rhypo D = 10km
NOFR	Area	Nootka Fault	Crustal	Rhypo D = 10km
OLM	Area	Olympic Mountains	Crustal	Rhypo D = 10km
PGT	Area	Puget Sound shallow	Crustal	Rhypo D = 10km
ROCN	Area	Rocky Mountain fold/thrust belt North	Crustal	Rhypo D = 10km
SBC	Area	Southern British Columbia	Crustal	Rhypo D = 10km
VICM	Area	Vancouver Island Coast Mountains	Crustal	Rhypo D = 10km
QCFA	Area	Queen Charlotte Fault	Crustal	Rhypo D = 10km
FWFA	Area	Fairweather Fault	Crustal	Rhypo D = 10km
QCSS	Fault	Queen Charlotte Strike slip beta $= 0$	Crustal-Rjb	Joyner-Boore
QCSS	Fault	Queen Charlotte Strike slip beta = 1.84	Crustal-Rjb	Joyner-Boore
FWF	Fault	Fairweather Fault beta $= 0$	Crustal-Rjb	Joyner-Boore
FWF	Fault	Fairweather Fault beta $= 1.84$	Crustal-Rjb	Joyner-Boore
JDFN	Area	Juan de Fuca plate bending, onshore (deep)	Subduction inslab - D30	Rhypo D = 30km
GTP	Area	Georgia Strait/Puget Sound (deep)	Subduction inslab - D50	Rhypo D = 50km
CIS	Fault	Cascadia Interface source	Subduction Interface	Rrup
EISO	Fault	Explorer Interface outboard estimate of rupture	Subduction Interface	Rrup
EISB	Fault	Explorer Interface. Best estimate landward	Subduction Interface	Rrup
EISI	Fault	Explorer Interface. Inboard estimate of rupture	Subduction Interface	Rrup
HGT	Fault	Haida Gwaii thrust beta=0	Subduction Interface	Rrup
WIN	Fault	Winona thrust beta=0	Subduction Interface	Rrup
WIN	Fault	Winona thrust Activity Rate $= 0$	Subduction Interface	Rrup
OFS	Area	Offshore	Offshore crustal	Rhypo D = 10km

Table 7.1	Sources	used	in	PSHA
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Figure 7.3 shows each hazard of interested in each site, separated by source contribution. As shown in the figure, the subduction interface source is not representative of the SLE UHS. Both the SLE subduction intraslab hazards and the SLE crustal hazards have minimal contribution in the high periods.

The subduction intraslab DBE hazard follows the UHS shape in low period, but drops off rapidly in the higher period. At the high-period range, all source hazards tend to have similar spectral accelerations.

As shown in the figure, the crustal hazard is least representative of the MCE UHS compared to the other sources. The subduction interface hazard follows the MCE UHS closely at periods greater in the high period range (T>1s). The subduction intraslab follows the MCE UHS closely at low periods (T<1s).



Figure 7.3 Hazards by source contribution

7.5 Ground motion selection and scaling

To assess the seismic performance of the prototype buildings (which is completed in subsequent chapters) a suite of representative ground motions must be selected and scaled over an appropriate period range.

The tectonic activity near both sites consists of crustal, subduction intraslab (subcrustal), and subduction interface earthquakes. A UHS is a combination of various hazard sources, as a result it is difficult to match ground motion spectrums to the UHS over large period ranges. To address this, the NBCC 2015 permits scaling over a period range specific to the sources which have a higher contribution to the UHS (NBCC 2015, Option A in Annexure J).

The following sections discuss the source contributions to the hazard, the selected period ranges to scale the ground motions, and the highest contributing magnitudes and distances determined through deaggregation of the hazard. The chapter concludes with suites of ground motions which can be used to analyze the seismic performance of prototype buildings.

7.5.1 Period range for scaling and source contributions

NBCC 2015 suggest scaling to a lower-bound period of 0.15 times the fundamental period of the structure to capture aspects of the higher mode responses. It is also recommended that ground motions be scaled to an upper bound period of twice the fundamental period, to account for the softening of the structure from nonlinearity. To cover the range of periods, in this study the lower bound period is taken as 0.15 times the fundamental period of the 24-story building in the CORW direction, resulting in a lower bound $T_{min} = 0.3s$. The upper bound period at the MCE, where high degree of nonlinearity is expected, as $T_{max} = 10s$, as the UHS GMPEs are only reliable up to a natural period of 10s.

Ground motions are scaled following Method A in Annexure J of NBCC, 2015. This method uses one target spectrum, in this case the UHS, and scales a suite of ground motions to match a period range within the UHS. Ground motions of a particular source need only be scaled within a period range where that source is significantly contributing to the response. However, the NBCC does not have an explicit method to determine the range at which is considered "high contribution".

In this thesis it is assumed that the sources which contribute 15% or higher to the UHS are considered in the ground motion selection and scaling. Table 7.2 shows the percent contributing from each source at periods of 0.3, 0.5, 1, 2, and 3s and the corresponding selected period range used for ground motion scaling.

As shown in Table 7.2, the SLE (50% in 30-year hazard) is almost entirely governed by the crustal and subduction interface sources for both the Vancouver and Victoria sites. As a result, the period

range for crustal and subcrustal is selected as between the upper and lower bound (i.e., $T_{crus} = T_{intra} = 0.3s - 10s$). The subduction interface source has a low contribution to the SLE hazard (i.e., less than 15% over the entire range). Hence, no subduction interface ground motions were selected for this hazard.

The DBE (20% in 50-year hazard) at the Vancouver and Victoria sites, shown in Table 7.2, respectively, are primarily governed by subduction intraslab motions over most of the period range. As a result, DBE subduction intraslab motions are scaled over the entire range (i.e., T_{intra} =0.3s-10s). The DBE crustal source has less than a 15% contribution at 0.5s and 2s, and higher contributions at all other period ranges. Since the crustal source is not showing diminishing contribution at higher periods, the entire range is also considered (i.e., T_{crus} = 0.3s - 10s). The subduction interface source has a higher than 15% contribution under most periods, except for 0.3s. However, the entire range of periods is selected for subduction interface motions (i.e., T_{inter} = 0.3s-10s).

		SLE (50% in30 year)			DBE (20% in 50 years)			MCE (2% in 50 year)		
	Period [s]	Crustal	Subduction intraslab	Subduction interface	Crustal	Subduction intraslab	Subduction interface	Crustal	Subduction intraslab	Subduction interface
	0.3	23.2%	70.3%	6.8%	16.9%	71.4%	11.7%	20.0%	72.1%	8.0%
	0.5	22.2%	70.4%	8.0%	14.3%	70.1%	15.7%	15.6%	68.4%	16.0%
ver	1	29.9%	60.6%	10.4%	17.8%	57.5%	24.7%	15.2%	43.5%	41.4%
noou	2	26.4%	62.7%	12.4%	12.3%	56.6%	31.2%	8.4%	32.2%	59.4%
Van	3	33.6%	54.4%	14.5%	17.6%	44.7%	38.0%	9.0%	13.5%	77.5%
	Range	$\begin{array}{c} T_{crus} = \\ 0.3s - 10s \end{array}$	$\begin{array}{c} T_{intra} = 0.3s \\ -10s \end{array}$	-	$\begin{array}{c} T_{\rm crus} = \\ 0.3 {\rm s} -10 {\rm s} \end{array}$	$T_{intra} = 0.3s$ $-10s$	$\begin{array}{c} T_{inter} = 0.3s \\ -10s \end{array}$	$\begin{array}{l} T_{crus} = \\ 0.3s - 1s \end{array}$	$T_{intra} = 0.3s$ $-3s$	$T_{inter} = 0.3s$ $-10s$
	0.3	22.8%	71.0%	7.4%	19.7%	60.1%	20.5%	27.4%	44.1%	28.6%
	0.5	20.6%	73.2%	7.7%	16.2%	61.4%	22.8%	20.6%	41.9%	37.7%
a.	1	25.4%	67.4%	9.0%	18.1%	53.1%	29.2%	17.5%	21.9%	60.7%
ctor	2	21.4%	70.3%	10.5%	13.1%	54.2%	33.3%	9.9%	15.9%	74.3%
Vi	3	26.0%	64.4%	12.0%	16.9%	45.6%	38.0%	10.0%	5.3%	84.7%
	Range	$T_{crus} = 0.3s - 10s$	$T_{intra} = 0.3s$ -10s	-	$T_{\rm crus} = 0.3 {\rm s} -10 {\rm s}$	$T_{intra} = 0.3s$ -10s	$T_{inter} = 0.3s$ -10s	$T_{crus} = 0.3s - 1.5s$	$T_{intra} = 0.3s$ -2.5s	$T_{inter} = 0.3s$ $-10s$

Table 7.2 Hazard source contributions

The MCE (2% in 50-year hazard) at the Vancouver and Victoria sites, shown in Table 7.2, is dominated by the subduction interface source over most of the period range. As a result, the subduction motions are scaled over the entire range ($T_{inter} = 0.3s-10s$). In the Victoria site, the crustal and subduction intraslab sources diminish in contributions at higher periods. Less than 15%
contributions occur at a period of 2s for the crustal source and 3s for the subduction intraslab source. The Vancouver hazard has slightly higher contributions from the crustal source and slightly lower contribution from the subcrustal motions when compared o the Victoria source. As a result, the scaling period ranges are slightly different for the two sites. The Vancouver crustal motions are scaled over a range of $T_{crus} = 0.3s - 1s$, subduction intraslab motions are scaled over a range of $T_{intra} = 0.3s - 3s$. Victoria motions are scaled over a range of $T_{crus} = 0.3s - 3s$. Victoria motions are scaled over a range of $T_{crus} = 0.3s - 3s$. For crustal motions are scaled over a range of $T_{intra} = 0.3s - 3s$. Victoria motions are scaled over a range of $T_{crus} = 0.3s - 3s$.

7.5.2 De-aggregation of hazard

De-aggregation was conducted for each hazard at periods equal to 0.3s, 0.5s, 1s, 2s, and 3s. Figure 7.4 and Figure 7.5 show the result for a low period (T = 0.3s) and a high period (T = 3s), respectively. All other de-aggregation results are shown in Appendix B.

As shown in Figure 7.4, at a period of 0.3s, the SLE (50% in 30-year hazard) is dominated by magnitudes 5 -7.5 at distances between 50km to 150km for the Vancouver site and distances of 10km to 100km for Victoria for the site. These contributions are representative of the crustal and the subcrustal motions. At higher periods, the Vancouver SLE is dominated by sources which have magnitudes between 6 to 8 and between distances of 10km to 250km. The Victoria SLE is dominated by magnitudes between 5-7.5 with distances of about 50km to 200km. The subduction interface sources, identifiable from the magnitudes greater than 8.5, are shown to have a relatively low contribution at the SLE hazards.

The DBE (20% in 50 year) are dominated by magnitudes between 6 and 9 for both high and low periods and the Victoria and Vancouver sites. The Victoria hazard has the most contributions between distances of 50 km - 100 km, while the Vancouver hazard has the most significant contributions between 50 km and 200 km.

The MCE (2% in 50-year hazard) is almost entirely governed by the subduction motions at high periods. As a result, the Victoria site has high contributions at 50-100km away while the Vancouver site is of distance 100km – 150km away, this is approximately the distance of the Cascadia fault from each site. At low periods, there are contributions from the subcrustal and crustal motions, as shown by the lower magnitudes (6 – 8) at distances between 50km-140km and 50km-100km for the Vancouver and Victoria sites, respectively.



Figure 7.4 Deaggregation of the site hazards for period of T = 0.3s



Figure 7.5 Deaggregation of the site hazards for period of T = 3s

7.5.3 Record selection

Ground motions were selected such that they are representative of the site characteristics. Suites of 11 ground motions were selected for each source. The de-aggregation showed that the subduction motions have minimal contributions at the SLE, as a result, subduction interface motions were not selected for this hazard.

Ground motions were scaled such that the geomean of the two directions matched the UHS. Scaling factors were limited between 0.25-4. The individual ground motions were carefully selected such that the mean of all the unidirectional motions also matched the spectrums well.

Crustal ground motions were selected from the PEER West Strong motion database (PEER, 2018). Subduction motions were selected from the PEER Preliminary NGA Subduction Ground Motion Suite (UCLA, 2020). The full subduction ground motion database is not available for public use at the time of writing this dissertation. As a result, there are where a limited number of ground motions to select from. For this reason, more than two records from an individual event were selected in some cases. The individual ground motion name, year, distance, and scaling factors are shown in Table 7.3, Table 7.4, and Table 7.5 for the SLE, DBE, and MCE, respectively. Figure 7.6 show the individual, mean, and UHS response spectrums for the Vancouver and Victoria sites. Appendix C summarizes the X- and Y- component spectrums.

Vancouver								Victoria					
	NGA	Earthquake	Year	М	Rjb [km]	SF	NGA	Earthquake	Year	М	Rjb [km]	SF	
	17	Southern Calif	1952	6	73.34	1.13	13	Kern County	1952	7.36	122.6	0.70	
	21	Imperial Valley- 05	1955	5.4	13.78	1.24	17	Southern Calif	1952	6	73.3	1.63	
	295	Irpinia, Italy-02	1980	6.2	28.69	1.39	21	Imperial Valley5	1955	5.4	13.8	1.78	
	318	Westmorland	1981	5.9	19.26	0.82	295	Irpinia, Italy2	1980	6.2	28.7	2.00	
Crustal	323	Coalinga-01	1983	6.36	55.05	0.6	302	Irpinia, Italy2	1980	6.2	22.7	0.56	
	353	Coalinga-01	1983	6.36	40.13	0.57	318	Westmorland	1981	5.9	19.3	1.18	
	4314	Umbria-03, Italy	1984	5.6	40.71	1.12	353	Coalingal	1983	6.36	40.1	0.83	
	747	Loma Prieta	1989	6.93	68.22	1.19	4314	Italy	1984	5.6	40.7	1.61	
	782	Loma Prieta	1989	6.93	39.69	0.83	747	Loma Prieta	1989	6.93	68.2	1.72	
	7	Northwest Calif- 02	1941	6.6	91.15	1.11	782	Loma Prieta	1989	6.93	39.7	1.19	
	92	San Fernando	1971	6.61	68.38	2.01	7	Northwest Calif2	1941	6.6	91.2	1.61	
	2000007	Nisqually	2001	6.8	39	1.15	2000036	Nisqually	2001	6.8	70.2	1.06	
	2000071	Nisqually	2001	6.8	79	1	2000888	Ferndale	2010	6.55	83.1	3.99	
	2000888	Ferndale	2010	6.55	83	3.1	2000900	Ferndale	2010	6.55	45.3	1.38	
1	6001142	South America	2005	7.78	212	0.54	3000100	CA & Mexico	1992	6.51	86.5	1.08	
usta	5001487	New Zealand	2007	6.65	162	1.96	4032628	Kushiro-oki	1993	7.59	91.2	0.75	
Subcri	5001498	New Zealand	2007	6.65	149	2.41	5001497	New Zealand	2007	6.65	151.6	2.45	
	2000900	Ferndale	2010	6.55	45	0.96	6001148	Tarapaca	2005	7.78	149.7	0.67	
	7006355	Pingtung	2006	6.94	104	0.88	7006355	Pingtung	2006	6.94	104.1	1.27	
	5001499	New Zealand	2007	6.65	149	2.67	7006358	Pingtung	2006	6.94	158.4	1.66	
	4007388	Miyagi	2011	7.15	110	0.97	6001145	Tarapaca	2005	7.78	169.9	0.50	
	5001497	New Zealand	2007	6.65	152	1.7	5001487	New Zealand	2007	6.65	161.6	2.83	

Table 7.3 SLE hazard ground motion properties

	Vancouver						Victoria						
	NGA	Earthquake	Year	М	Rjb [km]	SF	NGA	Earthquake	Year	М	Rjb [km]	SF	
	131	Friuli, Italy2	1976	5.91	41	3.58	1787	Hector Mine	1999	7.13	10.4	0.59	
	164	Imperial Valley6	1979	6.53	15	0.69	285	Irpinia	1980	6.9	8.1	0.92	
	187	Imperial Valley6	1979	6.53	13	0.77	285	Irpinia	1980	6.9	8.1	0.67	
	285	Irpinia, Italy1	1980	6.9	8	0.54	30	Parkfield	1966	6.19	9.6	0.95	
stal	36	Borrego Mtn	1968	6.63	45	0.68	31	Parkfield	1966	6.19	12.9	1.58	
Jrus	4843	Chuetsu-oki	2007	6.8	18	0.7	353	Coalinga1	1983	6.36	40.1	2.65	
Ŭ	782	Loma Prieta	1989	6.93	40	2.6	4843	Chuetsu-oki	2007	6.8	18.2	1.03	
	796	Loma Prieta	1989	6.93	77	0.74	4859	Chuetsu-oki	2007	6.8	11.4	0.73	
	7	Northwest Calif2	1941	6.6	91	3.5	725	Superstition Hills2	1987	6.54	11.2	0.59	
	832	Landers	1992	7.28	69	0.61	782	Loma Prieta	1989	6.93	39.7	3.83	
	96	Managua	1972	5.2	4	0.76	796	Loma Prieta	1989	6.93	77.3	1.10	
	1002977	Iniskin	2016	7.15	226	3	2000036	Nisqually	2001	6.8	70.2	3.42	
	2000007	Nisqually	2001	6.8	39	3.62	2000049	Nisqually	2001	6.8	69.2	3.56	
	2000066	Nisqually	2001	6.8	21	1.02	3000185	CA & Mexico	1982	7.31	24.4	0.59	
	2000071	Nisqually	2001	6.8	79	3.14	4032462	Kushiro-oki	1993	7.59	180.1	0.98	
stal	4007388	Miyagi	2011	7.15	110	3.05	4032652	Hokkaido	1994	8.27	115.5	0.68	
cru	4032462	Kushiro-oki	1993	7.59	180	0.67	6001143	South America	2005	7.78	168.8	1.25	
Sub	4032463	Kushiro-oki	1993	7.59	105	0.5	6001149	South America	2005	7.78	66.6	1.37	
-	6001141	Tarapaca	2005	7.78	211	1.99	6001151	South America	2005	7.78	67.8	1.20	
	6001143	Tarapaca	2005	7.78	169	0.85	7006078	Pingtung	2006	7.02	11.7	0.60	
	6001151	Tarapaca	2005	7.78	68	0.81	7006502	Pingtung	2006	6.94	26.1	1.63	
	7006502	Pingtung	2006	6.94	26	1.1	7006538	Pingtung	2006	6.94	23.3	1.78	
	4000359	Tohoku	2011	9.11	126	0.73	3001962	Michoacan	1985	7.99	105.1	1.77	
	4000087	Tohoku	2011	9.11	111	0.76	4000244	Tohoku	2011	9.11	23.1	1.06	
	4001102	Tohoku	2011	9.11	162	0.85	4000369	Tohoku	2011	9.11	89.7	0.92	
	4000684	Tohoku	2011	9.11	144	0.98	4001239	Tohoku	2011	9.11	112.6	1.00	
Subduction	4028568	Tokachi-oki	2003	8.28	76	1.13	4028568	Tokachi-oki	2003	8.28	76.3	1.67	
	4028609	Tokachi-oki	2003	8.28	158	1.94	6001395	Iquique	2014	8.15	96.6	2.65	
	4032588	Tokachi-oki	1968	8.26	156	0.61	6001396	Iquique	2014	8.15	98.5	2.32	
	6001020	South Peru	2001	8.41	201	0.61	6001800	South America	2010	8.81	120.7	0.49	
	6001375	Iquique	2014	8.15	101	1.68	6001804	South America	2010	8.81	105.6	0.64	
	6001396	Iquique	2014	8.15	99	1.58	6004288	Iquique	2014	8.15	56.9	1.86	
	6002233	Chile	2015	8.31	111	1.78	6005357	Chile	2015	8.31	67.9	1.72	

Table 7.4 DBE hazard ground motion properties

	Vancouver						Victoria					
	NGA	Earthquake	Year	М	Rjb [km]	SF	NGA	Earthquake	Year	М	Rjb [km]	SF
	164	Imperial Valley6	1979	6.53	15	1.77	138	Tabas, Iran	1978	7.35	24.1	3.99
	1787	Hector Mine	1999	7.13	10	1.29	1633	Manjil, Iran	1990	7.37	12.6	1.49
	31	Parkfield	1966	6.19	13	2.71	164	Imperial Valley6	1979	6.53	15.2	2.96
	4843	Chuetsu-oki	2007	6.8	18	1.59	1787	Hector Mine	1999	7.13	10.4	1.78
	4855	Chuetsu-oki	2007	6.8	21	2.18	20	Northern Calif3	1954	6.5	26.7	2.27
stal	582	Taiwan	1986	7.3	55	1.61	281	Trinidad	1980	7.2	76.1	3.99
Crus	5837	El Mayor- Cucapah	2010	7.2	19	0.72	285	Irpinia, Italy1	1980	6.9	8.1	2.82
	68	San Fernando	1971	6.61	23	2.29	31	Parkfield	1966	6.19	12.9	3.99
	6959	Darfield	2010	7	19	0.94	3758	Landers	1992	7.28	36.9	3.97
	725	Superstition Hills2	1987	6.54	11	1.05	4843	Chuetsu-oki	2007	6.8	18.2	2.88
	832	Landers	1992	7.28	69	2.55	725	Superstition Hills2	1987	6.54	11.2	1.79
	2000053	Nisqually	2001	6.8	56	3.99	2001631	Ferndale	2010	6.55	34.3	3.99
	2000066	Nisqually	2001	6.8	21	3.07	3000185	CA & Mexico	1982	7.31	24.4	1.84
	2001631	Nisqually	2010	6.55	34	2.56	4032462	Kushiro-oki	1993	7.59	180.1	3.85
	4032462	Kushiro-oki	1993	7.59	180	2.33	4032479	Hokkaido	1994	8.27	218.8	1.54
stal	4032479	Hokkaido	1994	8.27	219	0.98	4032480	Hokkaido	1994	8.27	113.7	2.17
cru	4032480	Hokkaido	1994	8.27	114	1.39	6001143	Tarapaca	2005	7.78	168.8	3.91
Sub	6001145	Tarapaca	2005	7.78	170	2.46	6001145	Tarapaca	2005	7.78	169.9	3.81
-	6001149	Tarapaca	2005	7.78	67	2.74	7006504	Pingtung	2006	6.94	23.6	1.67
	7006502	Pingtung	2006	6.94	26	3.74	7006531	Pingtung	2006	6.94	23.4	2.28
	7006533	Pingtung	2006	6.94	19	1.37	7006532	Pingtung	2006	6.94	23.8	1.91
	7006538	Pingtung	2006	6.94	23	3.98	7006533	Pingtung	2006	6.94	19.0	2.22
	3001955	Michoacan	1985	7.99	8	2.42	3001955	Michoacan	1985	7.99	8.1	3.86
	4000026	Tohoku	2011	9.11	72	1.56	4000108	Tohoku	2011	9.11	5.9	1.57
	4000322	Tohoku	2011	9.11	135	3.08	4000842	Tohoku	2011	9.11	153.8	3.30
	4000836	Tohoku	2011	9.11	159	3.46	4001181	Tohoku	2011	9.11	88.2	3.98
u	4032552	Tokachi-oki	2003	8.28	18	0.96	4022853	Tokachi-oki	2003	8.28	125.3	3.77
Subductio	4028567	Tokachi-oki	2003	8.28	39	2.3	4022977	Tokachi-oki	2003	8.28	140.4	2.84
	4022977	Tokachi-oki	2003	8.28	140	1.78	4028563	Tokachi-oki	2003	8.28	49.3	2.32
	4022990	Tokachi-oki	2003	8.28	71	2.73	4028574	Tokachi-oki	2003	8.28	72.9	3.38
	6001803	South America	2010	8.81	113	1.3	6001801	South America	2010	8.81	132.8	2.92
	6001804	South America	2010	8.81	106	1.53	6001804	South America	2010	8.81	105.6	2.44
	6002259	Chile	2015	8.31	106	4	6001815	South America	2010	8.81	57.5	1.85

Table 7.5 MCE hazard ground motion properties



Figure 7.6 Response spectrum for ground motions for prototype sites, at all sites, at all sources

7.6 Summary

This chapter presents the prototype site probabilistic Seismic Hazard Analysis (PSHA), deaggregation, ground motion selection and scaling for each prototype site. The South West region of Canada, where these two sites are located, have a significant seismic hazard. With Victoria having about a 60% higher hazard when compared with Vancouver. De-aggregation showed that the subduction interface seismic source has minimal contribution to the SLE. As a result, the subduction interface source is not considered in the ground motion selection of the SLE. On the other hand, the subduction interface seismic source almost entirely governs the MCE for both sites.

The following chapters utilise the selected ground motions from this chapter, the nonlinear modelling approach from Chapter 6 to assess the seismic performance of the CORW, SCCW, and CROCW systems.

Chapter 8: Seismic performance assessment of CORW

8.1 Overview

This chapter presents the nonlinear time history results of the prototype buildings designed in Chapter 5 using EEDP, under unidirectional loading in the CORW direction. This chapter presents a discussion on the peak response of displacements, interstorey drift ratios, wall moments, shear forces, shear stresses, and wall strains at each story. Additionally, a comparison between the EEDP design objectives and the nonlinear response is provided.

8.2 Introduction

Four prototype buildings, 24-story and 40-story buildings located in Vancouver and 24-story and 40-story buildings situated in Victoria, are examined in this chapter. These prototype buildings were designed for the following design hazards: Service level earthquake (SLE) of 50% in 30 years; Design-based earthquake (DBE) of 20% in 50 years; and Maximum considered earthquake (MCE) of 2% in 50 years. The detailed steps to design the prototype building designs were presented in Chapter 5.

Based on the probabilistic seismic hazard analysis (PSHA) presented in Chapter 7, a suite of 11 ground motions was selected and scaled based on the three tectonic sources near the site: crustal, subduction intraslab, and subduction interface. The SLE hazard did not contain a suite of subduction interface ground motions due to the subduction source's limited contribution. As a result, two suites of 11 ground motions at the SLE and three suites of 11 ground motions at the DBE and MCE were used to conduct the nonlinear time-history analysis- a total of 660 analysis.

The prototype buildings were analysed using three-dimensional models with the modelling approach summarized in Chapter 6. The results presented in this chapter only considers the maximum ground motions applied along the CORW direction. The nonlinear analysis results showed that the CORW can behaved as the mechanism specified by EEDP in Chapter 5.

8.3 Global displacements and ISDRs

Roof drift ratio (RDR) and interstorey drift ratios (ISDRs) were determined for each ground motion, at each story level, at each of the eight nodes on the core wall, shown in Figure 8.1.

However, all responses are essentially the same for each node under this unidirectional loading, so only the WNW node is shown herein for brevity.



Figure 8.1 Displacement and interstorey drift post processing schematic for the CORW

8.3.1 Story displacements

Figure 8.2 shows the mean (μ) and standard deviation (σ) of the prototype building peak story displacements (Δ_s) normalised to the building height (H). The SLE peak μ roof drift ratio (RDR) is 0.046%, 0.054%, 0.035%, and 0.047% for the Van24, Vic24, Van40, and Vic40 buildings, respectively. The DBE peak RDR- μ is over double the SLE, with values of 0.14%, 0.21%, 0.10%, and 0.12% for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak RDR- μ is 0.53%, 0.85%, 0.46%, and 0.76% for the Van24, Vic24, Van40, and Vic40, respectively.

The peak RDR- σ at the SLE is 0.012%, 0.02%, 0.011%, and 0.011% for the Van24, Vic24, Van40, and Vic40, respectively. The peak RDR- σ at the DBE is 0.044%, 0.057%, 0.029%, and 0.044% for the Van24, Vic24, Van40, and Vic40, respectively. The peak for RDR- σ at the MCE is 0.28%, 0.30%, 0.18%, and 0.30% for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 8.2 CORW unidirectional normalised displacements (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Figure 8.3 compares the ratio of the mean roof displacement from the model (Δ_r) to the EEDP design roof displacement (Δ_{EEDP}). Generally, the response matches well - demonstrating the robustness of the EEDP design procedure.





8.3.2 Inter-story drift ratio (ISDR)

Figure 8.4 shows the mean (μ) and standard deviation (σ) prototype building peak global ISDR over the building height. As shown in the figure, the ISDR- μ at the SLE shaking intensity is 0.060%, 0.076%, 0.051%, and 0.076% for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak ISDR- μ is 0.19%, 0.27%, 0.17%, and 0.23% for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak ISDR- μ 0.60%, 1.05%, 0.67%, and 1.10% for the Van24, Vic24, Van40, and Vic40, respectively.

The peak σ for ISDR under SLE 0.015%, 0.02%, 0.012%, and 0.019% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under DBE is 0.055%, 0.083%, 0.041%, and 0.060% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under MCE is 0.26%, 0.31%, 0.20%, and 0.33% for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 8.4 CORW unidirectional ISDR (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

8.4 Outrigger dampers

Figure 8.5 compares the ratio of the mean outrigger damper force from the model (f_o) to the EEDP design yield force $(f_{o,y})$. As shown, the damper force is below yielding at the SLE. At the DBE and MCE all outrigger dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the outrigger system is designed to remain elastic at the SLE and yield at the DBE and MCE.



Figure 8.5 Outrigger damper force ratio for each prototype building at each hazard under unidirectional load in CORW direction.

8.5 Wall-base dampers

Figure 8.6 compares the ratio of the mean wall-base damper force from the model (f_b) to the EEDP design yield force $(f_{b,y})$. As shown, generally, the damper force is below yielding at the SLE and DBE. At the MCE all wall-base dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the wall-base dampers are designed to remain elastic at the SLE and DBE and yield at the MCE.



Figure 8.6 Wall-base damper force ratio under unidirectional load in CORW direction

8.6 Wall shear stress

Figure 8.7 shows the mean (μ) and standard deviation (σ) prototype building peak shear stress over the building height. The shear forces are determine using a section cut which includes only each wall web. The stress is determined by dividing this force by the wall length and the web thicknesses.

As shown in Figure 8.7, the shear stress μ at the SLE shaking intensity is 0.58 MPa, 0.81 MPa, 0.52 MPa, and 0.85 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak shear stress μ is 1.42 MPa, 1.95 MPa, 1.42 MPa, and 2.26 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak shear stress μ is 2.60 MPa, 3.69 MPa, 3.19 MPa, and 3.93 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) of the shear stress is highest for the Vic24, under all hazards. The peak σ of the shear stress under SLE is 0.16 MPa, 0.23 MPa, 0.15 MPa, and 0.31 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under DBE 0.35 MPa, 0.54 MPa, 0.52 MPa, and 0.60 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under MCE is 0.61 MPa, 0.83 MPa, 0.63 MPa, and 0.91 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

Overall, the shear stresses are low (less than 10%f'c), demonstrating this system resulting in reasonably low damage, as per the performance objective.



Figure 8.7 CORW unidirectional shear stress (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

8.7 Wall strain

Figure 8.8 shows the mean (μ) and standard deviation (σ) prototype building peak tension strain in the wall over the building height at all hazards. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The strains are normalised to the yield strain of rebar ($\varepsilon_y = 0.002$). Strains were evaluated at each node, and the peak responses are presented in Figure 8.8.

As shown in the figure, the mean strain ratio ($\varepsilon/\varepsilon_y - \mu$) at the SLE shaking intensity is 0.10, 0.10, 0.11, and 0.12 for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak $\varepsilon/\varepsilon_y - \mu$ 0.20, 0.24, 0.15, and 0.18 for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak $\varepsilon/\varepsilon_y - \mu$ is 0.39, 0.75, 0.45, and 0.91 for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) of the $\varepsilon/\varepsilon_y$ is highest for the Vic24, under all hazards. The peak σ for $\varepsilon/\varepsilon_y$ under SLE is 0.03, 0.03, 0.00, and 0.02 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under DBE is 0.04, 0.09, 0.04, and 0.11 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under MCE is 0.21, 0.31, 0.27, and 0.64 for the Van24, Vic24, Van40, and Vic24, Van40, and



Figure 8.8 CORW unidirectional strains (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

At the SLE and DBE hazard levels peak strains are observed at the top of building. This is due to the outrigger moment causing high flexural demands at the roof, where wall compression stresses are low. These strains are less prominent in the 40-story buildings, where the outrigger is less effective (i.e., lower α_f), and the higher mode moments are more prominent (i.e., more flexural strain demands are distributed over the height of the building.

At the MCE hazard, more prominent strains are observed near mid-height of the building. These high strains are more dominant in the Victoria 40-story building, where the mean of the mid-height strain is twice the mean of the base strain. The high mid-height strains are attributed to the base moment being capped by the yielding of the dampers, as a result, higher flexural demands occur over the height of the buildings.

Overall, the strains are below yielding (i.e., $\varepsilon/\varepsilon_y < 1$). The strains are highest near the mid-height of the building, demonstrating the governance of higher mode moments. However, the low strains demonstrate that the EEDP performance objectives have been met.

8.8 Summary

This chapter presented the nonlinear dynamic responses of four prototype buildings: 24-story located in Victoria and Vancouver, and 40-story located in Victoria and Vancouver. Using EEDP the systems were designed to meet different performance objectives at different shaking intensities. Specifically, at the SLE shaking intensity the system remains elastic, at the DBE the outrigger dampers yield and dissipate energy, while the system remains elastic, and at the MCE intensity the outrigger and base dampers yield and dissipate energy, while the remaining wall is elastic. The nonlinear analysis conducted in this chapter demonstrates the robustness of this design procedure in meeting these performance objectives. The limited damage is demonstrated by the low shear stresses (less than 10%f'c) and the low wall strains are low (less than $\varepsilon_y = 0.002$) for all prototype buildings.

This chapter presented the seismic performance assessment of the CORW system under unidirectional loading. The following chapter (Chapter 9) presents the seismic performance assessment of the SCCW under unidirectional loading.

Chapter 9: Seismic performance assessment of SCCW

9.1 Overview

This chapter presents the nonlinear time history results of the four prototype buildings, designed in Chapter 5, under unidirectional loading in the SCCW direction. The EEDP is validated within this chapter. Additionally, this chapter presents a discussion on the peak response at every story for displacements, inter-story drift ratio (ISDR), wall moments, shear forces, shear stresses, and wall strains.

9.2 Introduction

In Chapter 3 of this thesis the SCCW was proposed as an alternative to the conventional RC coupled wall system. In Chapter 5, a EEDP is developed, to design the SCCW. Using the modified EEDP, the SCCW system can be designed to meet different performance objectives at different shaking intensities. Specifically, at the SLE shaking intensity, the system remains elastic. At the DBE intensity, the coupling beams are designed to yield and dissipate energy, while the remaining system remains elastic. At the MCE intensity, the coupling beams are designed to yield and wall-base dampers are designed to yield and dissipate energy, while the remaining such as the system remains elastic.

In this chapter, the seismic performance of the SCCW system, designed with EEDP, under different levels of earthquake shaking is studied. Four prototype buildings are investigated: a 24-story and a 40-story building located in Vancouver and a 24-story and a 40-story building located in Victoria. These buildings were designed in Chapter 5 and considered the following shaking intensities in the design process: a Service level earthquake (SLE) of 50% in 30 year; a Designbased earthquake (DBE) of 20% in 50 year; and a Maximum considered earthquake (MCE) of 2% in 50 year.

In chapter 6, detailed numerical models were developed to consider the performance of the CROCW system, where along one principal axis is the CORW and along the other principal axis is the SCCW. In this chapter, only the response along the SCCW is studied. The numerical models were subjected to a set of source-specific ground motions. The ground motion suite selection and scaling are presented in Chapter 7.

A total of 660 nonlinear time-history analyses were conducted to encompass the four prototypes, under three hazards, and the difference earthquake sources. Key engineering design parameters for each analysis are studied, including: interstorey drifts (ISDR), wall forces, damper deformations, and wall strains. Only the governing ground motion source is shown within this chapter for brevity. The following sections present the numerical results and discuss the performance of the SCCW system. The nonlinear analysis results are used to validate the EEDP design procedure and understand the seismic response of the system.

9.3 Global displacements and Interstorey drifts

Lateral displacements and interstorey drift ratios (ISDRs) were analysed at each wall node shown in Figure 9.1. Displacements and ISDR was determined through analysing the peak response for each ground motion, at each story level, at each of the eight nodes on the core wall. However, all responses are essentially the same for each node under this unidirectional loading, so only the WNW node is shown herein for brevity.



Figure 9.1 Displacement and interstorey drift post processing schematic for SCCW

9.3.1 Global displacements

Figure 9.2 shows the mean (μ) and standard deviation (σ) prototype building peak global displacements of the SCCW over the building height. As shown in the figure, the SLE peak μ roof drift ratio (RDR) is 0.053%, 0.076%, 0.035%, and 0.045% for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak RDR- μ is over double the SLE, shown as 0.14%, 0.21%, 0.12%, and 0.15% for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak RDR- μ is 0.54%, 0.97%, 0.44%, and 0.69% for the Van24, Vic24, Van40, and Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) is highest for the Vic24, under all hazards. The peak RDR- σ under SLE is 0.015%, 0.03%, 0.008%, and 0.009% for the Van24, Vic24, Van40, and Vic40, respectively.

The peak RDR- σ under DBE is 0.051%, 0.071%, 0.046%, and 0.034% for the Van24, Vic24, Van40, and Vic40, respectively. The peak RDR- σ under MCE is 0.28%, 0.54%, 0.23%, and 0.29% for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 9.2 SCCW roof drift ratio (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Figure 9.3 compares the ratio of the mean roof displacement from the model (Δ_r) to the EEDP design roof displacement (Δ_{EEDP}). Generally, the response matches well - demonstrating the robustness of the EEDP design procedure.



Figure 9.3 Statistical comparison between nonlinear analysis and EEDP for the SCCW

9.3.2 Inter-story drift ratio (ISDR)

Figure 9.4 shows the mean (μ) and standard deviation (σ) prototype building peak global ISDR over the building height. As shown in the figure, the ISDR- μ at the SLE shaking intensity is 0.069%, 0.102%, 0.058%, and 0.084% for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak ISDR is 0.17%, 0.27%, 0.19%, and 0.27% for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak ISDR- μ is 0.63%, 1.14%, 0.66%, and 1.01% for the Van24, Vic24, Van40, and Vic40, Van40, and Vic40, respectively.

The standard deviation (σ) is highest for the Vic24, under all hazards. The peak σ for ISDR under SLE is 0.016%, 0.04%, 0.011%, and 0.015% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under DBE is 0.060%, 0.085%, 0.053%, and 0.056% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under MCE is 0.31%, 0.56%, 0.30%, and 0.44% for the Van24, Vic24, Van40, and Vic40, respectively. The ISDR shown in Figure 9.4 are relatively low, compared with the allowable 2.5% drifts allowed in the National building code of Canada (NBCC, 2015) permissible for normal importance buildings. However,

unlike conventional buildings, where yielding is expected over the wall height, these walls are expected to have minimal damages. Therefore, these low ISDR are to be expected.



Figure 9.4 SCCW unidirectional ISDR (governing source) : (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

9.4 Wall-base damper

Figure 9.5 compares the ratio of the mean wall-base damper force from the model (f_b) to the EEDP design yield force $(f_{b,y})$. As shown, the damper force is below yielding at the SLE and DBE. At the MCE all wall-base dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the wall-base dampers are designed to remain elastic at the SLE and DBE and yield at the MCE.



Figure 9.5 Base damper force ratio for each prototype building at each hazard under unidirectional load in SCCW direction

9.5 Coupling beam shear force

Figure 9.6 shows the mean (μ) and standard deviation (σ) of the ratio of coupling beam shear force (V_{CB}) to the yield coupling beam force ($V_{CB,y}$). The ratio $V_{CB}/V_{CB,y}$ was determined by taking the peak shear response of the North and South coupling beam at a given floor and dividing by the yield force.

As shown, generally, the coupling beam force is below yielding at the SLE. At the DBE and MCE all wall-base dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the wall-base dampers are designed to remain elastic at the SLE and yields at the DBE and MCE.





Figure 9.6 SCCW unidirectional coupling beam shear force ratio (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

9.6 Wall shear stress

Figure 9.7 shows the mean (μ) and standard deviation (σ) prototype building peak shear stress over the building height. The shear forces are determine using a section cut which includes only each wall flanges. The stress is determined by dividing this force by the flange length and the flange thicknesses. Figure 9.7 shows the maximum stress of the four flanges (i.e., North-West, North-East, South-West, South-East).

As shown in the figure, the μ shear stress at the SLE shaking intensity is 0.37 MPa, 0.58 MPa, 0.31 MPa, and 0.43 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak μ shear stress is 0.65 MPa, 1.03 MPa, 0.79 MPa, and 1.07 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak μ shear stress is 1.56 MPa, 2.15 MPa, 1.82 MPa, and 2.18 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

The peak σ of the shear stress under SLE is 0.09 MPa, 0.11 MPa, 0.06 MPa, and 0.11 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under DBE is 0.13 MPa, 0.14 MPa, 0.15 MPa, and 0.23 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under MCE is 0.34 MPa, 0.37 MPa, 0.38 MPa, and 0.45 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

Overall, the shear stresses are low (less than 10%f'c), demonstrating this system resulting in reasonably low damage, as per the performance objective.



Figure 9.7 SCCW shear stress under unidirectional loading(governing source) : (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

9.7 Wall strains

Figure 9.8 shows the mean (μ) and standard deviation (σ) prototype building peak tension strain in the wall over the building height at all hazards. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The strains are normalised to the yield strain of the reinforcing steel ($\varepsilon_y =$ 0.002). The strains are highest near the mid-height of the building, demonstrating the governance of higher mode moments. However, all strains are below yielding (i.e., $\varepsilon/\varepsilon_y < 1$), demonstrating the EEDP performance objectives have been met.

As shown in the figure, the mean strain ratio ($\varepsilon/\varepsilon_y$ - μ) at the SLE shaking intensity 0.072, 0.082, 0.10, and 0.11 for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak $\varepsilon/\varepsilon_y$ - μ is 0.10, 0.13, 0.14, and 0.16 for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak $\varepsilon/\varepsilon_y$ - μ is 0.29, 0.64, 0.31, and 0.80 for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) of the $\varepsilon/\varepsilon_y$ is highest for the Vic24, under all hazards. The peak σ for $\varepsilon/\varepsilon_y$ under SLE is 0.007, 0.012, 0.004, and 0.005 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under DBE is 0.017, 0.039, 0.018, and 0.04 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under MCE is 0.16, 0.28, 0.23, and 0.62 for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 9.8 Max strain profile response for all prototype buildings under unidirectional loading in the SCCW direction at the WNE corner: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

9.8 Summary

This chapter presented the nonlinear dynamic responses of four prototype buildings design with an SCCW: a 24-story located in Victoria and in Vancouver, and a 40-story located in Victoria and in Vancouver. The nonlinear analysis conducted in this chapter demonstrate the robustness of EEDP. Specifically, the performance objectives are met at the SLE shaking intensity, where the system remains elastic, at the DBE, where the SCFDs yield and dissipate energy, and at the MCE, where the SCFDs and base dampers yield and dissipate energy, while the remaining wall is elastic. The limited damage is demonstrated by the low shear stresses (less than 10%f^{*}c) and the low wall strains are low (less than $\varepsilon_v = 0.002$) for all prototype buildings.

This chapter presented the seismic performance assessment of the SCCW system under unidirectional loading. The following chapter (Chapter 10) shows the seismic performance assessment of the CROCW, which is designed with the CORW along one axis and the SCCW along the other axis, under bidirectional loading.

Chapter 10: Bidirectional performance CROCW system

10.1 Overview

This chapter presents the seismic assessment of the CROCW system under bidirectional loading. The CROCW, which has a CORW along one principal axis and an SCCW and the other, was designed in Chapter 5. Chapter 8 and Chapter 9 validated the EEDP procedure for the CORW and SCCW, respectively. This chapter validates the design of the bidirectional CROCW system.

10.2 Introduction

The CROCW system was designed in Chapter 5 using the modified EEDP. In this procedure, the CORW and the SCCW systems were designed independently but ensuring that assuming the wallbase damper has the same design force for each direction. Chapters 8 and 9 presented the results from applying unidirectional ground motions to the CROCW system along the CORW axis and the SCCW axis– validating the EEDP. This chapter presents the response of the CROCW under bidirectional loading and compares it to the unidirectional responses (i.e., responses from Chapter 8 and 9).

A total of 660 bidirectional nonlinear time-history analysis were conducted to encompass the four prototypes, under three hazards, and the difference earthquake sources. Key engineering design parameters for each bidirectional analysis are studied, including inter-story drifts (ISDR), wall forces, damper deformations, and wall strains. Additionally, a comparison of the CROCW system under unidirectional and bidirectional loading is provided. The governing ground motion source is shown herein for brevity.

10.3 Global displacements and Interstorey drifts

Lateral displacements and interstorey drifts are determined using the North-West node of the West wall, as shown in Figure 10.1. This node was selected as it showed the highest displacements and ISDR under bidirectional loading.



Figure 10.1 Displacement and interstorey drift post processing schematic

10.3.1 Global displacements

Figure 10.2 and Figure 10.3 shows the mean (μ) and standard deviation (σ) CROCW peak story displacements normalised to building height in the CORW direction and the SCCW direction, respectively.

As shown in Figure 10.2, the CORW direction SLE peak μ roof drift ratio (RDR) is 0.045%, 0.057%, 0.034%, and 0.049% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction DBE peak RDR- μ is over double the SLE, shown as 0.14%, 0.21%, 0.10%, and 0.13% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction MCE peak RDR- μ is 0.54%, 0.83%, 0.44%, and 0.75% for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) is highest for the Vic24, under all hazards. The CORW direction peak σ for RDR under SLE is 0.013%, 0.02%, 0.010%, and 0.012% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction peak σ for RDR under DBE is 0.041%, 0.063%, 0.028%, and 0.046% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction peak σ for RDR under MCE is 0.26%, 0.31%, 0.17%, and 0.28% for the Van24, Vic24, Van40, and Vic40, respectively.

As shown in Figure 10.3, the SCCW direction SLE peak roof drift ratio (RDR) is 0.053%, 0.076%, 0.035%, and 0.044% for the Van24, Vic24, Van40, and Vic40, respectively. The SCCW direction DBE peak RDR is over double the SLE, shown as 0.14%, 0.21%, 0.12%, and 0.15% for the Van24, Vic24, Van40, and Vic40, respectively. The SCCW direction MCE peak RDR is 0.61%, 1.02%, 0.50%, and 0.71% for the Van24, Vic24, Van40, and Vic40, respectively.

The SCCW direction peak σ for RDR under SLE is 0.015%, 0.03%, 0.007%, and 0.009% for the Van24, Vic24, Van40, and Vic40, respectively. The SCCW direction peak σ for RDR under DBE

is 0.052%, 0.073%, 0.045%, and 0.035% for the Van24, Vic24, Van40, and Vic40, respectively. The SCCW direction peak σ for RDR under MCE 0.29%, 0.58%, 0.25%, and 0.23% for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 10.2 CORW bidirectional displacements (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ



Figure 10.3 SCCW unidirectional displacements (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Figure 10.4 and Figure 10.5 compares the ratio of the mean roof displacement from the model (Δ_r) to the EEDP design roof displacement (Δ_{EEDP}) for the CORW direction and the SCCW direction

of the CROCW system, respectively. Generally, the response matches well - demonstrating the robustness of the EEDP design procedure.



Figure 10.4 Statistical comparison between nonlinear analysis and EEDP for the CROCW in the CORW direction



Figure 10.5 Statistical comparison between nonlinear analysis and EEDP for the CROCW in the SCCW direction
10.3.2 Inter-story drift ratio (ISDR)

Figure 10.6 and Figure 10.7 shows the mean (μ) and standard deviation (σ) prototype building peak global ISDR over the building height for the CORW and the SCCW direction, respectively.

As shown in Figure 10.6, the CORW direction ISDR- μ at the SLE shaking intensity is 0.060%, 0.079%, 0.050%, and 0.076% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction ISDR- μ at the DBE is 0.19%, 0.26%, 0.17%, and 0.23% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction ISDR- μ at the MCE 0.61%, 1.04%, 0.70%, and 1.21% for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) is highest for the Vic24, under all hazards. The CORW direction peak σ for ISDR under SLE is 0.019%, 0.02%, 0.012%, and 0.018% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction peak σ for ISDR under DBE 0.055%, 0.086%, 0.044%, and 0.066% for the Van24, Vic24, Van40, and Vic40, respectively. The CORW direction peak σ for ISDR under MCE is 0.24%, 0.32%, 0.20%, and 0.35% for the Van24, Vic24, Van40, and Vic40, respectively.

As shown in Figure 10.7, the SCCW direction ISDR at the SLE shaking intensity is 0.069%, 0.102%, 0.057%, and 0.084% for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak ISDR is 0.17%, 0.27%, 0.19%, and 0.28% for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak ISDR is 0.70%, 1.21%, 0.73%, and 1.07% for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) is highest for the Vic24, under all hazards. The peak σ for ISDR under SLE is 0.016%, 0.04%, 0.011%, and 0.015% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under DBE is 0.062%, 0.091%, 0.051%, and 0.064% for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for ISDR under MCE is 0.30%, 0.58%, 0.33%, and 0.44% for the Van24, Vic24, Van40, and Vic40, respectively.

The ISDR shown are relatively low, compared with the allowable 2.5% drifts allowed in the National building code of Canada (NBCC, 2015) permissible for normal importance buildings. However, unlike conventional buildings, where yielding is expected over the wall height, these walls are expected to have minimal damages. Therefore, these low ISDR are to be expected.



Figure 10.6 SCCW unidirectional ISDR (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ



Figure 10.7 SCCW unidirectional ISDR (governing source) : (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

10.4 Wall-base damper

Figure 10.8 compares the ratio of the mean wall-base damper force from the model (f_b) to the EEDP design yield force $(f_{b,y})$. As shown, the damper force is below yielding at the SLE and DBE. At the MCE all wall-base dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the wall-base dampers are designed to remain elastic at the SLE and DBE and yield at the MCE.





10.5 Outrigger dampers

Figure 10.9 compares the ratio of the mean outrigger damper force from the model (f_o) to the EEDP design yield force $(f_{o,y})$. As shown, the damper force is below yielding at the SLE. At the DBE and MCE all outrigger dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the outrigger system is designed to remain elastic at the SLE and yield at the DBE and MCE.



Figure 10.9 Outrigger damper force ratio for each prototype building at each hazard under bidirectional load in CORW direction.

10.6 Coupling beam shear force

Figure 10.10 shows the mean (μ) and standard deviation (σ) of the ratio of coupling beam shear force (V_{CB}) to the yield coupling beam force ($V_{CB,y}$). The ratio $V_{CB}/V_{CB,y}$ was determined by taking the peak shear response of each coupling beam at a given floor and dividing by the yield force. As shown, generally, the coupling beam force is below yielding at the SLE. At the DBE and MCE all wall-base dampers have yielded. These observed behaviors meet the performance objectives of the EEDP, where the wall-base dampers are designed to remain elastic at the SLE and yields at the DBE and MCE.



Figure 10.10 Bidirectional coupling beam shear force ratio (governing source): (a) SLE- μ; (b) DBE- μ;
(c) MCE- μ; (d) SLE- σ; (e) DBE- σ; and (f) MCE- σ

10.7 Wall shear stress

Figure 10.12 shows the mean (μ) and standard deviation (σ) prototype building peak shear stress in the wall flanges. The shear forces are determine using a section cut which includes only each wall flanges. The stress is determined by dividing this force by the wall length and the flange thicknesses. Figure 9.7 shows the maximum stress of the four flanges (i.e., North-West, North-East, South-West, South-East).

As shown in the figure, the μ shear stress at the SLE shaking intensity is 0.39 MPa, 0.601 MPa, 0.35 MPa, and 0.53 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak μ shear stress is 0.70 MPa, 1.17 MPa, 0.88 MPa, and 1.28 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak μ shear stress is 3 2.29 MPa, 3.29 MPa, 2.02 MPa, and 2.57 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

The peak σ of the shear stress under SLE is 0.13 MPa, 0.16 MPa, 0.066 MPa, and 0.15 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under DBE is 0.15 MPa, 0.22 MPa, 0.16 MPa, and 0.26 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under MCE is 0.70 MPa, 0.85 MPa, 0.39 MPa, and 0.53 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

Overall, the shear stresses are low (less than 10%f'c), demonstrating this system resulting in reasonably low damage, as per the performance objective.



Figure 10.11 CROCW flange shear stress under unidirectional loading(governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Figure 10.11 shows the mean (μ) and standard deviation (σ) prototype building peak shear stress in the web over the building height of the CROCW in the CORW direction. The shear forces are determine using a section cut which includes only each wall web. The stress is determined by dividing this force by the wall length and the web thicknesses.

As shown in the figure, the μ shear stress at the SLE shaking intensity is 0.58 MPa, 0.85 MPa, 0.52 MPa, and 0.80 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak μ shear stress is 1.34 MPa, 2.06 MPa, 1.45 MPa, and 2.04 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak μ shear stress is 3.03 MPa, 4.02 MPa, 3.26 MPa, and 4.31 MPa for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) of the shear stress is highest for the Vic24, under all hazards. The peak σ of the shear stress under SLE is 0.23 MPa, 0.26 MPa, 0.15 MPa, and 0.23 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under DBE is 0.39 MPa, 0.50 MPa, 0.47 MPa, and 0.54 MPa for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ of the shear stress under MCE is 0.54 MPa, 0.97 MPa, 0.70 MPa, and 0.96 MPa for the Van24, Vic24, Van40, and Vic24, vic24, Van40, and 0.96 MPa for the Van24, Vic24, Vic24, Van40, and 0.96 MPa for the Van24, Vic24, Vic24, Van40, and 0.96 MPa for the Van24, Vic24, Vic24, Van40, and 0.96 MPa for the Van24, Vic24, Van40, and Vic40, respectively. Overall, the shear stresses are low (less than 10%f'c), demonstrating this system resulting in reasonably low damage, as per the performance objective.



Figure 10.12 CROCW bidirectional web shear stress (governing source): (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

10.8 Wall strains

Figure 10.13 shows the mean (μ) and standard deviation (σ) prototype building peak tension strain in the wall over the building height at all hazards. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The tension strains are normalised to the yield strain of reinforcing steel ($\varepsilon_y =$ 0.002). The strains are highest near the mid-height of the building, demonstrating the governance of higher mode moments. However, generally the strains are low, demonstrating the EEDP performance objectives have been met.

As shown in the figure, the μ strain ratio ($\varepsilon/\varepsilon_y$) at the SLE shaking intensity is 0.092, 0.099, 0.11, and 0.12 for the Van24, Vic24, Van40, and Vic40, respectively. The DBE peak $\varepsilon/\varepsilon_y$ - μ is 0.18, 0.23, 0.17, and 0.19 for the Van24, Vic24, Van40, and Vic40, respectively. The MCE peak $\varepsilon/\varepsilon_y$ - μ is 0.44, 0.80, 0.54, and 1.09 for the Van24, Vic24, Van40, and Vic40, respectively.

The standard deviation (σ) of the $\varepsilon/\varepsilon_y$ is highest for the Vic24, under all hazards. The peak σ for $\varepsilon/\varepsilon_y$ under SLE is 0.044, 0.035, 0.005, and 0.026 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under DBE is 0.034, 0.067, 0.035, and 0.12 for the Van24, Vic24, Van40, and Vic40, respectively. The peak σ for $\varepsilon/\varepsilon_y$ under MCE is 0.18, 0.27, 0.26, and 0.84 for the Van24, Vic24, Van40, and Vic40, respectively.



Figure 10.13 Max strain profile response for all prototype buildings under the high hazard for bidirectional loading WNW corner: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

10.9 Summary

This chapter presented the bidirectional nonlinear dynamic responses of four prototype buildings: a 24-story located in Victoria and in Vancouver, and a 40-story located in Victoria and in Vancouver. Using EEDP described in Chapter 5, the CROCW were designed to meet different performance objectives at different shaking intensities.

The nonlinear analysis conducted in this chapter demonstrate the robustness of this design procedure in meeting these performance objectives. Specifically, at the SLE shaking intensity the system remains essentially elastic, at the DBE the outrigger dampers and the SCFD yield and dissipate energy, while the system remains, and at the MCE the outrigger, SCFD, and base dampers yield and dissipate energy, while the remaining wall has limited damage. The limited damage is demonstrated by the low shear stresses (less than 10%f'c) and the low wall strains are low for all prototype buildings.

Chapter 11: Comparison between CORW and SCCW with alternative RC core wall systems

11.1 Overview

The objective of this chapter is to demonstrate that the CORW and the SCCW can enhance seismic performance when compared with conventional RC wall systems. In this chapter the seismic response of the 40-story (tallest prototype) on the Victoria site (highest site hazard) is studied. The responses are discussed in detail and the end of this chapter provides some of the important conclusions from the comparisons.

11.2 Introduction

A main goal of this dissertation is to provide a novel RC core wall system which has higher seismic performance when compared to the conventional RC core wall system. To meet this goal, this thesis proposed two systems to achieve this goal: 1) the Controlled Outriggered Rocking Wall (CORW) as an alternative to the shear wall system and 2) the Self-Centering Coupled Wall (SCCW) system. Additionally, a design procedure for the proposed systems was recommended and validated using nonlinear time-history analysis. However, these studies have not yet demonstrated that the proposed systems could achieve higher performance than the conventional RC core walls. Further, the individual influence of the added technology (i.e., outrigger, controlled rocking mechanisms, and SCFDs) has yet to be examined. This chapter seeks to fill this knowledge gap through analysing and comparing different RC core wall system behavior for the shear wall and coupled wall systems.

11.3 Comparison of high-performance RC shear wall systems

This thesis has proposed the CORW system (Figure 11.1 (a)), which is an alternative system to the conventional fixed-base Shear Wall (SW), shown in Figure 11.1 (b). This chapter seeks to understand the CORW performance when compared with three alternative systems : (1) Shear Wall (SW) system (RC wall with a fixed base without an outrigger, shown in Figure 11.1 (b)); (2) Outriggered Wall (OW) system (RC wall with roof outrigger and a fixed base, shown in Figure

11.1 (c)); and (3) Controlled Rocking Wall (CRW) system (RC wall with controlled rocking base and without an outrigger, shown in Figure 11.1 (d)).

The SW system, shown in Figure 11.1 (b), consists of a fixed based shear wall which does not include an outrigger. The yielding mechanism of the SW is plastic hinging at the wall base (i.e., significant cracking of concrete and yielding of reinforcing steel). The OW, shown in Figure 11.1 (b), consists of a fixed based shear wall with a roof outrigger. Like the SW, the OW dissipates energy through plastic hinging near the wall base. However, the OW has additional energy dissipation and stiffness provided by the outrigger. The CRW, shown in Figure 11.1 (c), consists of a damped controlled rocking wall without an outrigger.



Figure 11.1 High-performance RC shear wall comparison: (a) Controlled Rocking Outriggered Wall (CORW); (b) Shear Wall (SW); (c) Outriggered Wall (OW); and (d) Controlled Rocking Wall (CRW)

To understand the differences in responses of the systems, it is important to first understand the different mechanics involved, particularly the differences between the controlled rocking and fixed base systems. The SW and the OW both form plastic hinging near the base of the wall. The plastic hinge occurs through the formation of cracks over a portion of the length of the wall. Within each open crack the rebar yields, resulting in the energy dissipation. On the other hand, the CRW and the CORW utilize controlled rocking systems at the base of the wall which essentially results in the formation of a single crack at the wall base. A large singular crack in a conventional wall would result in high localised strains and non-ductile behavior. Controlled rocking bases avoid such behaviour by not allowing rebar to cross the rocking interface, and in some cases, adding damping devices intended to withstand these high demands.

Despite the differences between the two systems' reaction mechanisms and curvatures, the global force deformation of a rocking wall system and a conventional wall system are similar when exposed to cyclic loads. For example, Figure 11.2 shows two experimental tests. Figure 11.2 (a) shows a wall test specimen design to represent a high-rise SW (Adebar et al., 2007), while Figure 11.2 (b) shows a rocking wall panel which uses post-tensioned (PT) to recenter the wall (Perez, 2013). In the SW, the energy dissipation (the enclosed hysteretic area) is primarily attributed to yielding of the wall reinforcing. Conversely, in/with the hysteretic energy dissipation/nonlinear mechanism in the rocking system, shown in Figure 11.2 (b), nonlinearity is primarily attributed to yielding of the PT tendons. For the rocking wall system, there is limited energy dissipation, and the wall behaves nonlinear elastically before the PT tendon yields.

As shown in Figure 11.2, both systems give a comparable force-deformation response. The highly pinched responses are due to the high contribution of axial load (*P*) to the overturning resistance in both the fixed base and rocking wall systems. As shown in Figure 11.2, hysteretic loops are essentially centered around the rocking moment ($M_{rocking} = PL_w/2$). The results exemplify the importance of axial load on the hysteretic performance of both systems.



Figure 11.2: Nonlinear behavior of conventional wall and rocking wall: (a) fix based wall (Adebar et al., 2007) and (b) Rocking wall (Perez, 2013)

In this chapter, a prototype building is analysed with the different SFRS systems and the systems are compared. The 40-story prototype building located in Victoria, designed in Chapter 5 and analysed in Chapter 8 and 10, is used for this comparison study. This prototype was selected due to it being the tallest building and the highest hazard.

There are numerous variations in design methods and equivalencies that could potentially occur in design practice. For simplicity, in this study, it is assumed that the reinforcing design, wall

geometry, and damper forces from the 40-story building design (from Chapter 5) are the same for all configurations.

The following sections describe the numerical modelling, seismic performance, and discussion on responses of the different systems.

11.3.1 Numerical modelling

The numerical modeling approach followed the same procedure as Chapter 6. The modelling of these systems results in the fundamental periods shown in Table 11.1. As shown, the fundamental periods of the SW and CRW are greater than the CORW by about 15.4% and 17.4% respectively. The CORW is stiffer than these systems due to the inclusion of an outrigger. The fixed based boundary condition of the SW gives this system a slightly higher stiffness than the CRW. For the same reason, the OW has a slightly lower period (about 0.74% less) than the CORW.

Table 11.1 Fundamental period for comparison study

	CORW	SW	OW	CRW
T1 [s]	4.08	4.71	4.05	4.79

The same ground motions used in previous studies are used for analysing these systems. For efficiency, and clarity in the comparison, only 2-dimensional analysis is presented.

11.3.2 Nonlinear time-history analysis on different high-performance shear walls

11.3.2.1 Story displacements and ISDR

Roof drift ratio (RDR) and interstorey drift ratios (ISDRs) were determined for each ground motion, at each story level, at each of the eight nodes on the core wall, shown in Figure 8.1. However, all responses are essentially the same for each node under this unidirectional loading, so only the WNW node is shown herein for brevity.



Figure 11.3 RDR and interstorey drift post processing schematic for the SW direction

11.3.2.1.1 Story displacements

Figure 11.4 (a), (b), and (c) shows the mean (μ) horizontal displacement normalised to the building height for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. Figure 11.4 (d), (e), and (f) shows the standard deviation (σ) horizontal displacement normalised to the building height for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively.

As shown in Figure 11.4, the μ roof drift ratio (RDR) at the SLE shaking intensity is 0.047%, 0.048%, 0.047%, and 0.049% for the CORW, SW, OW and CRW, respectively. The RDR- μ at the DBE shaking intensity is 0.13%, 0.16%, 0.13%, and 0.16% for the CORW, SW, OW and CRW, respectively. The RDR- μ at the MCE shaking intensity is 0.76%, 0.93%, 0.76%, and 0.89% for the CORW, SW, OW and CRW, respectively.

The standard deviation (σ) of the RDR at the SLE shaking intensity is 0.011%, 0.02%, 0.016%, and 0.019% for the CORW, SW, OW and CRW, respectively. The σ of the RDR at the DBE shaking intensity is 0.04%, 0.06%, 0.04%, and 0.06% for the CORW, SW, OW and CRW, respectively. The RDR- σ at the MCE shaking intensity is 0.30%, 0.33%, 0.30%, and 0.33% for the CORW, SW, OW and CRW, respectively.

The SLE shaking intensity difference between the RDR- μ of the CORW to the SW, OW and CRW were 1.9%, -0.02%, and 4.3%, respectively. The DBE shaking intensity RDR- μ SW, OW and CRW has a difference of 26.8%, 0.3%, and 27.9%, respectively, compared with the CORW. The MCE shaking intensity mean RDR- μ of the SW, OW and CRW were 22.5%, 1%, and 17.4% difference compared to the CORW systems.

The SLE shaking intensity RDR- σ had a difference of 69.4%, 51.1%, and 77.9% for the of the SW, OW and CRW, respectively, compared with the CORW system. The DBE shaking intensity RDR- σ of the SW, OW and CRW had a difference of 43.9%, -0.1%, and 35.4% compared to the CORW systems. The MCE shaking intensity RDR- σ of the SW, OW and CRW had a difference of 10.3%, -0.7%, and 8.7% compared to the CORW system.



Figure 11.4 Global displacements for each RC wall system: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

The higher displacements in the SW can be attributed to the higher level of cracking over the building height. The peak displacements near the roof in the OW are only slightly higher than the CORW system. This higher displacement may be attributed to the slight increase amount of cracking and wall yielding when compared to the CORW system. The CRW showed higher

displacements than the CORW, which is in part due to the higher period of the CRW and the slight increase in cracking.

11.3.2.1.2 Interstorey drift ratio

Figure 11.5 (a), (b), and (c) shows the mean (μ) interstorey drift ratio (ISDR) for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. Figure 11.5 (d), (e), and (f) shows the standard deviation (σ) interstorey drift ratio (ISDR) for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively.

As shown in Figure 11.5, the μ ISDR at the SLE shaking intensity is 0.076%, 0.090%, 0.076%, and 0.090% for the CORW, SW, OW and CRW, respectively. The peak ISDR- μ at the DBE shaking intensity is 0.23%, 0.29%, 0.22%, and 0.29% for the CORW, SW, OW and CRW, respectively. The peak ISDR- μ at the MCE shaking intensity is 1.10%, 1.67%, 1.18%, and 1.54% for the CORW, SW, OW and CRW, respectively.

The standard deviation (σ) of the ISDR at the SLE shaking intensity 0.019%, 0.03%, 0.020%, and 0.026% for the CORW, SW, OW and CRW, respectively. The σ of the ISDR at the DBE shaking intensity is 0.06%, 0.07%, 0.06%, and 0.08% for the CORW, SW, OW and CRW, respectively. The σ of the ISDR at the MCE shaking intensity is 0.33%, 0.45%, 0.37%, and 0.42% for the CORW, SW, OW and CRW, respectively.

The SLE shaking intensity ISDR- μ of the SW, OW and CRW were 18.0%, 0.001%, and 18.1% greater than the CORW systems. The DBE shaking intensity ISDR- μ of the SW, OW and CRW were 27.5%, -0.9%, and 26.0% greater than the CORW systems. The MCE shaking intensity ISDR- μ of the SW, OW and CRW were 52.1%, 8%, and 41.0% greater than the CORW systems.



Figure 11.5 ISDR for each of the shear wall systems: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

The SLE shaking intensity ISDR- σ of the SW, OW and CRW had a difference of 32.2%, 1.8%, and 36.6% difference than the CORW system, respectively. The DBE shaking intensity ISDR- σ of SW, OW and CRW had a difference of 23.1%, 2.1%, and 27.1% of the CORW system, respectively. The MCE shaking intensity ISDR- σ of SW, OW and CRW had a difference of 34.0%, 9.9%, and 25.2%, and 25.2% compared to the CORW system, respectively.

11.3.2.2 Wall shear stress

Figure 11.6 (a), (b), and (c) shows the mean (μ) shear stress for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. Figure 11.5 (d), (e), and (f) shows the standard deviation (σ) shear stress, each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. The shear stress was determined by dividing this force by the wall length and the web thicknesses. The shear force was determine using a section cut which includes only each wall web.

As shown in Figure 11.6, the peak μ shear stress at the SLE shaking intensity is 0.85 MPa, 0.83 MPa, 0.88 MPa, and 0.82 MPa for the CORW, SW, OW and CRW, respectively. The peak μ shear stress at the DBE shaking intensity is 2.26 MPa, 2.58 MPa, 2.29 MPa, and 2.57 MPa for the CORW, SW, OW and CRW, respectively. The peak shear stress μ at the MCE shaking intensity is 3.93 MPa, 4.48 MPa, 4.25 MPa, and 4.36 MPa for the CORW, SW, OW and CRW, respectively.

The standard deviation (σ) of the shear stress at the SLE shaking intensity is 0.31 MPa, 0.29 MPa, 0.33 MPa, and 0.27 MPa for the CORW, SW, OW and CRW, respectively. The σ shear stress at the DBE shaking intensity is 0.60 MPa, 0.60 MPa, 0.63 MPa, and 0.53 MPa for the CORW, SW, OW and CRW, respectively. The σ of the shear stress at the MCE shaking intensity and 0.91 MPa, 1.02 MPa, 1.04 MPa, and 0.96 MPa for the CORW, SW, OW and CRW, respectively.

The SLE shaking intensity μ shear stress of the SW, OW and CRW had a difference of -2.3%, 3.83%, and -3.9% compared to the CORW systems. The DBE shaking intensity μ shear stress of the SW, OW and CRW had a difference of 14.1%, 1.3%, and 14.0% compared to the CORW systems. The MCE shaking intensity μ shear stress of the SW, OW and CRW were 14.0%, 8%, and 11.0% greater than the CORW systems.

The SLE shaking intensity shear stress- σ of the SW, OW and CRW had a difference of -6.1%, 6.6%, and -15.0% difference than the CORW system, respectively. The DBE shaking intensity

shear stress $-\sigma$ of SW, OW and CRW had a difference of -0.1%, 3.7%, and -12.4% of the CORW system, respectively. The MCE shaking intensity shear stress- σ of SW, OW and CRW had a difference of 12.1%, 14.5%, and 6.1% compared to the CORW system, respectively.



Figure 11.6 Comparison of high-performance RC wall shear force: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

11.3.2.3 Vertical wall strains

Figure 11.7 (a), (b), and (c) shows the mean (μ) vertical strain for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. Figure 11.7 (d), (e), and (f) shows the standard deviation (σ) vertical strain of each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The strains are normalised to the yield strain of rebar ($\varepsilon_y = 0.002$). Strains were evaluated at each node, and the peak normalised responses are presented in Figure 11.7.

As shown in Figure 11.7, the μ strain ratio ($\varepsilon/\varepsilon_y$ - μ) at the SLE shaking intensity is 0.13, 0.01, 0.14, and 0.13 for the CORW, SW, OW and CRW, respectively. The peak $\varepsilon/\varepsilon_y$ - μ at the DBE shaking intensity is 0.19, 0.28, 0.21, and 0.21 for the CORW, SW, OW and CRW, respectively. The peak $\varepsilon/\varepsilon_y$ - μ at the MCE shaking intensity is 0.91, 5.41, 2.59, and 1.73 for the CORW, SW, OW and CRW, respectively.

The standard deviation (σ) of the $\varepsilon/\varepsilon_y$ at the SLE shaking intensity is 0.02, 0.02, 0.02, and 0.02 for the CORW, SW, OW and CRW, respectively. The σ of the $\varepsilon/\varepsilon_y$ at the DBE shaking intensity is 0.11, 0.18, 0.11, and 0.17 for the CORW, SW, OW and CRW, respectively. The σ of the $\varepsilon/\varepsilon_y$ at the MCE shaking intensity is 0.64, 2.60, 1.15, and 0.85 for the CORW, SW, OW and CRW, respectively.

The SLE shaking intensity $\varepsilon/\varepsilon_y - \mu$ of the SW, OW and CRW had a difference of 1.9%, 4.20%, and -0.1% compared to the CORW system. The DBE shaking intensity $\varepsilon/\varepsilon_y$ - μ of the SW, OW and CRW had a difference of 46.7%, 8.9%, and 10.4% compared to the CORW system. The MCE shaking intensity $\varepsilon/\varepsilon_y$ - μ SW, OW and CRW had a difference of 497%, 186%, and 90.3% compared to the CORW system.

The SLE shaking intensity $\varepsilon/\varepsilon_y$ - σ of the SW, OW and CRW had a difference of -13.2%, -1.3%, and -16.7% difference than of the CORW system, respectively. The DBE shaking intensity $\varepsilon/\varepsilon_y$ - σ of SW, OW and CRW had a difference of 58.9%, -2.6%, and 50.6% of the CORW system, respectively. The MCE shaking intensity $\varepsilon/\varepsilon_y$ - σ of SW, OW and CRW had a difference of 305%, 78.3%, and 32.5% compared to the CORW system, respectively.



Figure 11.7 Max strain profile response for all prototype buildings under the high hazard for unidirectional loading in the CORW direction: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Figure 11.7 shows the $\varepsilon/\varepsilon_y$ - μ for each system. The analysis showed the plastic hinging occurring at the base of the wall for both the SW and OW. Hence, the $\varepsilon/\varepsilon_y$ - μ are higher near the base of the wall compared to the other systems. The SW has the highest strains compared to all the other systems, while the CORW has the lowest.

Lower strains are observed in the outriggered fixed base wall (OW) when compared with the conventional fixed based wall (SW). Its important to note, that although the SW and OW have the same moment capacity (i.e., same reinforcing ratio), the SW had significantly more strain. This difference is due to the outrigger adding additional damping to the system, as well as controlling the deformations of the system due to reduced cracking, and increased stiffness due to the outrigger system.

11.4 Comparison of high-performance RC coupled wall systems

In this section, the SCCW system (Figure 11.8 (a)) is compared against an additional three alternative RC coupled wall systems. Two types of coupling beams are used in this study: 1) Diagonally Reinforced Coupling Beams (DRCB) and 2) the Self-centering Conical Friction Damper (SCFD).

The three systems studied are as follows: (1) Coupled Wall (CW) system (RC coupled wall with DRCB, and a fixed base shown in Figure 11.8 (b)); (2) Damped Coupled Wall (DCW) system (RC coupled wall with new SCFD coupling beams and a fixed base shown in Figure 11.8 (c)); and (3) Controlled Rocking Coupled Wall (CRCW) system (RC coupled wall with DRCB, and a controlled rocking base shown in Figure 11.8 (d))

In this chapter, the seismic performance of the different SFRS systems for one prototype building is assessed and compared. The 40-story prototype building located in Victoria, designed in Chapter 5 and analysed in Chapter 9 and 10, is used for this comparison study. This prototype was selected due to it being the tallest building and the highest hazard.



Figure 11.8 High-performance RC coupled wall comparison: (a) Self-Centering Coupled Wall (SCCW);(b) Coupled Wall (CW); (c) Damped Coupled Wall (DCW); and (d) Controlled Rocking Coupled Wall (CRCW)

There are numerous variations in design methods and equivalencies that could potentially occur in design practice. For simplicity, in this study, it is assumed that the reinforcing design, wall geometry, and damper forces from the Vic40 design are considered the same for all configurations. The yield coupling beam forces are assumed to be the same amongst the systems. However, the post-yielding stiffness of the SCFD and DRCB are expected to follow the unique characteristics of each coupling beam.

The following sections describe the numerical modelling, seismic performance, and discussion on responses of the different systems.

11.4.1 Numerical modelling

The numerical modeling approach followed the same procedure as Chapter 6. The DRCB model follow the same modelling approach as Ghodsi et al., 2010. The same ground motions used in previous analysis are used for analysing these systems. For efficiency, and clarity in the comparison, only unidirectional analysis is presented.

The modelling of these systems resulted in the fundamental periods shown in Table 11.1. As shown, the fundamental periods of the CW and CRCW are greater than the SCCW by about 14.7% and 16.4% respectively. The SCCW is stiffer than these systems due to the higher stiffness of the

coupling beams. The fixed based boundary condition of the CW gives this system a slightly higher stiffness than the CRCW. For the same reason, the DCW has a slightly lower period (about 1.6% less) than the SCCW.

Table 11.2 Fundamental period for comparison study

	SCCW	CW	DCW	CRCW
T1 [s]	5.17	5.93	5.09	6.02

11.4.2 Nonlinear time-history analysis on different high-performance shear walls

11.4.2.1 Global displacements and ISDRs

Lateral displacements and interstorey drift ratios (ISDRs) were analysed at each wall node shown in Figure 9.1. Displacements and ISDRs were determined through analysing the peak response for each ground motion, at each story level, at each of the eight nodes on the core wall. However, all responses are essentially the same for each node under this unidirectional loading, so only the WNW node is shown herein for brevity.



Figure 11.9 Displacement and interstorey drift post processing schematic for SCCW

11.4.2.1.1 Global displacements

Figure 11.10 (a), (b), and (c) shows the mean (μ) horizontal displacement normalised to the building height for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. Figure 11.10 (d), (e), and (f) shows the standard deviation (σ) horizontal displacement normalised to the building height for each system (CORW, SW, OW and CRW) for the SLE, DBE, and the MCE, respectively.

As shown in Figure 11.10, the mean (μ) roof drift ratio (RDR) at the SLE shaking intensity is 0.045%, 0.051%, 0.048%, and 0.051% for the SCCW, CW, DCW and CRCW, respectively. The

RDR- μ at the DBE shaking intensity is 0.16%, 0.17%, 0.15%, and 0.17% for the CORW, SW, OW and CRW, respectively. The RDR- μ at the MCE shaking intensity is 0.69%, 0.84%, 0.70%, and 0.86% for the SCCW, CW, DCW and CRCW, respectively.

The standard deviation (σ) of the RDR at the SLE shaking intensity is 0.009%, 0.02%, 0.014%, and 0.020% for the SCCW, CW, DCW and CRCW, respectively. The RDR- σ at the DBE shaking intensity is 0.03%, 0.07%, 0.04%, and 0.07% for the SCCW, CW, DCW and CRCW, respectively. The RDR- σ at the MCE shaking intensity is 0.29%, 0.22%, 0.31%, and 0.23% for the SCCW, CW, DCW and CRCW, respectively.

The SLE shaking intensity RDR- μ difference between the SCCW and the CW, DCW and CRCW were 15.3%, 8.34%, and 14.2%, respectively. The DBE shaking intensity RDR- μ between the SCCW of the CW, DCW and CRCW were 12.0%, -1.6%, and 12.5%, respectively. The MCE shaking intensity mean RDR of the CW, DCW and CRCW were 21.5%, 2%, and 23.7% greater than the CORW systems.

The SLE shaking intensity RDR $-\sigma$ of the CW, DCW and CRCW had a difference of 110.7%, 53.0%, and 113.4% difference than the SCCW system, respectively. The DBE shaking intensity RDR $-\sigma$ of CW, DCW and CRCW had a difference of 90.7%, 7.3%, and 111.5% of the SCCW system, respectively. The MCE shaking intensity RDR $-\sigma$ of CW, DCW and CRCW had a difference of -22.3%, 8.3%, and -20.0% compared to the SCCW system, respectively.

High displacements result in large ductility demands on all structural components. Often the displacements govern the design of secondary gravity elements. Therefore, limiting displacements is key in creating high-performance tall buildings. The SCCW demonstrated a lower peak roof displacement compared with all other coupled wall systems in this study. Hence, the SCCW has a superior displacement performance under these design conditions.

High displacements in the CW and CRCW compared with the SCCW and the DCW can be attributed to two main factors. First, the diagonally reinforced coupling beams have lower initial stiffness when compared with the SCFD, resulting in a longer period. Secondly, the high strains in the CW soften the structure in nonlinear analysis, resulting in higher displacements.



Figure 11.10 Global displacements for each RC wall system: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

11.4.2.1.2 Interstorey drift ratio

Figure 11.11 (a), (b), and (c) shows the mean (μ) interstorey drift ratio (ISDR) for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. Figure 11.11 (d), (e), and (f) shows the standard deviation (σ) interstorey drift ratio (ISDR) for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively.

As shown in Figure 11.11, the μ ISDR at the SLE shaking intensity is 0.084%, 0.105%, 0.094%, and 0.101% for the SCCW, CW, DCW and CRCW, respectively. The peak ISDR- μ at the DBE shaking intensity is 0.266%, 0.31%, 0.27%, and 0.30% for the SCCW, CW, DCW and CRCW respectively. The peak ISDR- μ at the MCE shaking intensity is 1.01%, 1.46%, 1.05%, and 1.45% for the SCCW, CW, DCW and CRCW, respectively.

The standard deviation (σ) of the ISDR at the SLE shaking intensity is 0.015%, 0.03%, 0.023%, and 0.033% for the SCCW, CW, DCW and CRCW, respectively. The ISDR- σ at the DBE shaking intensity is 0.06%, 0.11%, 0.05%, and 0.11% for the SCCW, CW, DCW and CRCW, respectively. The ISDR- σ at the MCE shaking intensity is 0.44%, 0.52%, 0.44%, and 0.52% for the SCCW, CW, DCW and CRCW, respectively.

The SLE shaking intensity ISDR- μ difference between the SCCW and the CW, DCW and CRCW were 24.7%, 11.62%, and 20.8%, respectively. The DBE shaking intensity ISDR- μ difference of the CW, DCW and CRCW were 15.7%, -0.3%, and 14.2% compared to the SCCW systems. The MCE shaking intensity ISDR- μ difference between the SCCW and the CW, DCW and CRCW were 44.5%, 5%, and 43.7%, respectively.

The SLE shaking intensity ISDR- σ SW, OW and CRW had a difference of 104%, 48.4%, and 117% compared to the CORW system, respectively. The DBE shaking intensity ISDR- σ of SW, OW and CRW had a difference of 93.2%, -4.1%, and 92.6% of the CORW system, respectively. The MCE shaking intensity ISDR- σ of SW, OW and CRW had a difference of 18.9%, 0.9%, and 19.2% compared to the CORW system, respectively.



Figure 11.11 ISDR for each of the shear wall systems: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

Like displacements, controlling ISDR is key in high performance structures. ISDR can control the design of secondary elements, such as the gravity system. Therefore, limiting ISDR is key in creating high-performance tall buildings. The SCCW demonstrated a lower peak ISDR when compared with all other coupled wall systems in this study.

The higher strains, natural period, and low post-yielding stiffness of the DRCB of the CW results in higher ISDR than any other system over most of the building height. Near the base of the wall, however, the SCCW and CRCW have high ISDR. This result is due to the controlled rocking wall opening at one concentrated location resulting in essentially rigid body rotation. It is important that this rotation be considered in the design of secondary elements. When compared to the DCW, the SCCW has similar ISDR over most of the building height, with the exception of the plastic hinge region. The slightly high peak ISDR in the DCW is attributed to the higher strains in the DCW system.

11.4.2.2 Shear stress

Figure 11.12 (a), (b), and (c) shows the mean (μ) shear stress for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. Figure 11.12 (d), (e), and (f) shows the standard deviation (σ) shear stress, each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. The shear forces are determined using a section cut which includes only each wall flanges. The stress is determined by dividing this force by the wall length and the flange thicknesses. Figure 11.12 shows the maximum stress of the four flanges (i.e., North-West, North-East, South-West, South-East).

As shown in Figure 11.12, peak μ shear stress at the SLE shaking intensity is 0.43 MPa, 0.36 MPa, 0.41 MPa, and 0.36 MPa for the SCCW, CW, DCW and CRCW, respectively. The peak μ shear stress at the DBE shaking intensity is 1.07 MPa, 0.95 MPa, 1.10 MPa, and 0.88 MPa for the SCCW, CW, DCW and CRCW, respectively. The peak μ shear stress at the MCE shaking intensity is 2.18 MPa, 2.00 MPa, 2.44 MPa, and 1.70 MPa for the SCCW, CW, DCW and CRCW, respectively.

The standard deviation (σ) of the shear stress at the SLE shaking intensity is 0.11 MPa, 0.11 MPa, 0.10 MPa, and 0.12 MPa for the SCCW, CW, DCW and CRCW, respectively. The σ shear stress at the DBE shaking intensity is 0.23 MPa, 0.33 MPa, 0.24 MPa, and 0.27 MPa for the SCCW,

CW, DCW and CRCW, respectively. The σ shear stress at the MCE shaking intensity is 0.45 MPa, 0.44 MPa, 0.52 MPa, and 0.32 MPa for the SCCW, CW, DCW and CRCW, respectively.

The SLE shaking intensity μ shear stress of the CW, DCW and CRCW were -15.6%, -5.61%, and -17.1% greater than the SCCW systems. The DBE shaking intensity μ shear stress of the CW, DCW and CRCW were -11.1%, 3.5%, and -17.2% greater than the CORW systems. The MCE shaking intensity μ shear stress of the CW, DCW and CRCW were -8.6%, 12%, and -21.9% greater than the SCCW systems.

The SLE shaking intensity shear stress- σ of the SW, OW and CRW had a difference of 3.1%, - 10.8%, and 4.8% difference than the CORW system, respectively. The DBE shaking intensity shear stress - σ of SW, OW and CRW had a difference of 46.5%, 4.8%, and 19.3% of the CORW system, respectively. The MCE shaking intensity shear stress- σ of SW, OW and CRW had a difference of -4.0%, 14.6%, and -30.5% compared to the CORW system, respectively.

Figure 11.12 shows the mean responses for all the systems. As shown, the shear force in the SCCW is greater than the CW and CRCW and less than the DRCW at all building heights. However, at the base, the shear force is similar in magnitude amongst these systems, despite the CW and CRCW having lower period, and lower core moment capacity. This response can be attributed to the higher core moments observed in the SCCW and DCW systems.



Figure 11.12 Shear stress in coupled wall flange: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

11.4.2.3 Coupling forces

Figure 11.13 (a), (b), and (c) shows the mean (μ) ratio of the coupling beam shear force to the yielding shear force ($V_{CB}/V_{CB,y}$) for each system (SCCW, CW, DCW and CRCW) for the SLE,

DBE, and the MCE, respectively. Figure 11.13 (d), (e), and (f) shows the standard deviation (σ) $V_{CB}/V_{CB,y}$ for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. The ratio $V_{CB}/V_{CB,y}$ was determined by taking the peak shear response of the North and South coupling beam at a given floor and dividing by the yield force.

As shown in Figure 11.13, the mean (μ) of the $V_{CB}/V_{CB,y}$ at the SLE shaking intensity is 0.93, 0.8, 0.99, and 0.74 for the SCCW, CW, DCW and CRCW, respectively. The peak mean (μ) of the $V_{CB}/V_{CB,y}$ at the DBE shaking intensity is 1.19, 1.04, 1.19, and 1.04 for the SCCW, CW, DCW and CRCW, respectively. The peak mean (μ) ISDR at the MCE shaking intensity is 1.80, 1.31, 1.83, and 1.30 for the SCCW, CW, DCW and CRCW, respectively.

The standard deviation (σ) of the $V_{CB}/V_{CB,y}$ at the SLE shaking intensity is 0.20, 0.21, 0.23, and 0.21 for the SCCW, CW, DCW and CRCW, respectively. The σ of the $V_{CB}/V_{CB,y}$ at the DBE shaking intensity is 0.05, 0.14, 0.12, and 0.15 for the SCCW, CW, DCW and CRCW, respectively. The σ of the $V_{CB}/V_{CB,y}$ at the MCE shaking intensity is 0.17, 0.09, 0.20, and 0.09 for the SCCW, CW, DCW and CRCW, respectively.

The SLE shaking intensity $V_{CB}/V_{CB,y}$ - μ of the CW, DCW and CRCW were -19.1%, 6.91%, and -20.5% greater than the SCCW systems. The DBE shaking intensity $V_{CB}/V_{CB,y}$ - μ of the SW, OW and CRW were -12.3%, 0.0%, and -12.4% greater than the SCCW systems. The MCE shaking intensity $V_{CB}/V_{CB,y}$ - μ of the CW, DCW and CRCW were -27.2%, 2%, and -27.4% greater than the SCCW systems.

The SLE shaking intensity $V_{CB}/V_{CB,y}$ - σ of the CW, DCW and CRCW had a difference of 3.4%, 13.1%, and 5.8% difference than the SCCW system, respectively. The DBE shaking intensity $V_{CB}/V_{CB,y}$ - σ of CW, DCW and CRCW had a difference of 166.3%, 114.3%, and 177.3% of the SCCW system, respectively. The MCE shaking intensity $V_{CB}/V_{CB,y}$ - σ of CW, DCW and CRCW had a difference of -45.7%, 18.7%, and -46.3% compared to the SCCW system, respectively.


Figure 11.13 Coupling shear ratio for each RC core wall system: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

11.4.2.4 Vertical wall strains

Figure 11.14 (a), (b), and (c) shows the mean (μ) vertical strain for each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. Figure 11.14 (d), (e), and (f) shows the standard deviation (σ) vertical strain, each system (SCCW, CW, DCW and CRCW) for the SLE, DBE, and the MCE, respectively. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The tension strains are normalised to the yield strain of rebar ($\varepsilon_y = 0.002$). The strains are highest near the mid-height of the building, demonstrating the governance of higher mode moments. However, all strains are below yielding (i.e., $\varepsilon/\varepsilon_y <1$), demonstrating the EEDP performance objectives have been met.

As shown in Figure 11.14, the mean (μ) strain ratio (ϵ/ϵ_y) at the SLE shaking intensity is 0.12, 0.11, 0.13, and 0.12 for the SCCW, CW, DCW and CRCW, respectively. The peak ϵ/ϵ_y - μ at the DBE shaking intensity is 0.19, 0.18, 0.20, and 0.17 for the SCCW, CW, DCW and CRCW, respectively. The peak ϵ/ϵ_y - μ at the MCE shaking intensity is 0.80, 0.81, 1.05, and 0.79 for the SCCW, CW, DCW and CRCW, respectively.

The standard deviation (σ) of the $\varepsilon/\varepsilon_{y}$ - σ at the SLE shaking intensity is 0 0.01, 0.01, 0.01, and 0.01 for the SCCW, CW, DCW and CRCW, respectively. The $\varepsilon/\varepsilon_{y}$ - σ at the DBE shaking intensity is 0.04, 0.11, 0.04, and 0.11 for the SCCW, CW, DCW and CRCW, respectively. The $\varepsilon/\varepsilon_{y}$ - σ at the MCE shaking intensity is 0.62, 0.63, 0.66, and 0.61 for the SCCW, CW, DCW and CRCW, respectively.

The SLE shaking intensity μ vertical strain of the CW, DCW and CRCW had a difference of 1.9%, 7.41%, and 0.001% when compared with the SCCW system. The DBE shaking intensity $\varepsilon/\varepsilon_y$ - μ of the CW, DCW and CRCW were -6.5%, 5.2%, and -10.8% greater than the SCCW systems. The MCE shaking intensity $\varepsilon/\varepsilon_y$ - μ difference of the CW, DCW and CRCW was 0.6%, 31%, and -1.2% compared to the SCCW system.

The SLE shaking intensity σ vertical strain of the CW, DCW and CRCW had a difference of 68.9%, 15.9%, and 62.9% when compared with the SCCW system. The DBE shaking intensity $\varepsilon/\varepsilon_{y}$ - σ of the CW, DCW and CRCW were 148.1%, -14.2%, and 146.3% greater than the SCCW

systems. The MCE shaking intensity $\varepsilon/\varepsilon_y$ - σ difference of the CW, DCW and CRCW was 1.6%, 6.2%, and -1.6% compared to the SCCW system.



Figure 11.14 Max strain profile for each RC wall system: (a) SLE- μ ; (b) DBE- μ ; (c) MCE- μ ; (d) SLE- σ ; (e) DBE- σ ; and (f) MCE- σ

11.5 Summary and conclusions

This chapter presents a comparison between alternative shear wall systems and the CORW systems and a comparison between alternative coupled wall systems and the SCCW systems. For efficiency, this comparison was completed on the 40-story (tallest prototype) on the Victoria site (highest site hazard).

The CORW was compared with three other RC wall systems: the CORW, the SW, the Outriggered Wall (OW), and the Controlled Rocking Wall (CRW).

The following are some of the important observations made in this study:

- 1. Displacements are 23% higher in the SW compared to the CORW
- 2. ISDR are 52% higher in the SW compared to the CORW
- 3. Shear stress in the SW is 14% higher than the CORW

The SCCW was compared with three other RC coupled wall systems: the Coupled Wall (CW), the Damped Coupled Wall (DCW), and the Controlled Rocking Coupled Wall (CRCW).

The following are some of the important observations made in this study:

- 1. Displacements are 22% higher in the CW compared to the SCCW
- 2. ISDR are 45% higher in the CW compared to the SCCW
- 3. Shear stress in the SW is 14% higher than the SCCW

It is important to note that this study was limited to one prototype designed using one design method. Further investigations should be completed to examine the influence of these systems on different building types. Further, this study did not investigate the influence of the design procedure on the system response. The prototype building used only the EEDP design of the SCCW. The results may vary if these systems were designed to conventional code-based methods.

These analyses demonstrate the high performance of the CORW and SCCW systems, using state of the art numerical models. However, there is limited experimental data on the dynamic behaviour of core wall buildings. The following Chapter 12 describes the work completed towards the shake-table testing of CORCW systems as well as the planned shake table testing programme.

Chapter 12: Design and construction of shake-table testing specimen.

12.1 Overview

This chapter gives the detailed design and construction of a shake-table testing program for the proposed high-performance RC core wall. This chapter begins with a description of the facility and the limitations of the shaking-table. Afterwards, a description of the specimen design simplifications and scaling is provided. Following this are the details of the specimen design and construction. This chapter also describes the planned testing phases, instrumentation plan, ground motion selection and scaling, and testing considerations. The end of this chapter describes a numerical model developed of the specimen, and the predicted results of the experimental test.

12.2 Introduction

The large scale of tall building structural systems makes them challenging to experimentally test. The only feasible way to obtain dynamic seismic response data for a full-scale tall building is through extensive instrumentation of as-built structures and analysis of the data after an earthquake occurs. This method is not practical when the objective is to immediately validate a new system; therefore, historically, researchers have tested large-scale portions of the structure, such as a wall or beam, under quasi-static cyclic loads. Although effective in examining the local behaviour, the interaction of elements and higher mode effects are not fully captured within this testing methodology.

An effective way to investigate the dynamic behavior of tall building systems is shake-table testing. However, due to the high expenses and limited availability of shake-table facilities, few low-damage RC wall systems have been experimentally tested using this method. Gavridou et al. (2017) studied a full-scale 4-story precast unbonded post-tensioned controlled rocking building. Their results demonstrated that the controlled rocking system could have minimal damage after strong dynamic shaking. Lu et al. 2018 tested a combined frame-controlled rocking wall system using 1/3 scale shaking table testing. The five-story scaled building showed negligible damage after shaking bi-directionally, demonstrating the effectiveness of this low damage system. In 2019,

Henry et al. conducted a full-scale shake table test on a two-story building under unidirectional and bidirectional loading. The system consisted of planar RC walls designed for controlled rocking. Dampers were added to the edges of the walls to help dissipate the earthquake energy. Overall, the results showed minimum damage to all levels of shaking. Although these systems showed promising results for low-damage buildings, there have been limited to low-rise buildings, and the experimental response of low-damage tall buildings remains untested.

In this research, a 1/6.5 scale RC core wall specimen was built for shake table testing at the Multifunction shake table array facility in Shanghai China. The specimen is a model of the 24-Story Vancouver prototype building (Van24) designed in Chapter 5 and analyzed in Chapter 8-10. The specimen was specially designed to use materials that would be used in actual building construction. The large geometry of this specimen makes it unique; it is the largest core-wall shake table specimen ever built. The following sections describe the shake table testing facility, specimen design and scaling, and specimen construction.

12.3 Testing facility

The specimen will be tested at the Multi-Array Shaking Table Facility located on the Jiading Campus of Tongji University in Shanghai, China (Figure 12.1). The shake tables within this facility have 3-degree-of-freedom, longitudinal, lateral, and yaw. Each shake table has a dimension of 6m x 4m. In this research, two shake tables will be used, each with a payload capacity of 70 tonne. Additionally, the height to the roof crane girder is 18m. Table 12.1 shows the shake table capacities. These capacities limit the size of the specimen and the amplitude of shaking.



Figure 12.1 Multi-function shake table facility, Jiading, Shanghai, China

Properties	Capacity (per table)
Allowable mass	70tonne
Stroke	+- 500 mm
Velocity	+- 1000 mm/s
Acceleration	+- 1.5g
Frequency of operation	0.1-50 Hz
Overturning moment	4000 kN-m

12.4 Similitude

The specimen was designed such that it represented the Vancouver 24-story prototype building presented in Chapter 5. The prototype building was scaled and simplified to facilitate experimental testing. The length scaling factor was determined based on the clearance of the crane within the laboratory (18m). The length scaling factor was selected as $s_l = 1/6.5$. Equation 12.1 shows the length scaling factor (s_l).

$$s_l = \frac{L_{sp}}{L_{pr}} = 6.5$$
 Equation 12.1

where L_{sp} is the length of any element on the specimen and L_{pr} is the corresponding element length on the prototype.

The concrete properties in the prototype building and in the specimen are assumed to be the same. Hence, the elastic modulus, stress, and strain are the same for both the prototype and specimen. Equation 12.2 shows the material scaling factor (S_e).

$$s_e = \frac{E_{sp}}{E_{pr}} = \frac{\sigma_{sp}}{\sigma_{pr}} = \frac{\varepsilon_{sp}}{\varepsilon_{pr}} = 1$$
 Equation 12.2

where E_{sp} , σ_{sp} , and ε_{sp} is the elastic modulus, stress, and strain, respectively, for the specimen and E_{pr} , σ_{pr} , and ε_{pr} is the elastic modulus, stress, and strain, respectively, for the prototype.

The ratio between force in the specimen to the force in the prototype is called the force scaling factor, s_f , and is shown in Equation 12.3. The force scaling factor is simplified by substituting the

material scaling factor, s_e (Equation 12.2), and the length scaling factor, s_l (Equation 12.1), in Equation 12.3.

$$s_f = \frac{F_{sp}}{F_{pr}} = \frac{\sigma_{sp}A_{sp}}{\sigma_{pr}A_{pr}} = s_e s_l^2 \cong 1/43$$
 Equation 12.3

where F_{sp} and A_{sp} are the specimen force and area, respectively, for the specimen and F_{pr} and A_{pr} are the specimen force and area, respectively, for the prototype.

The ratio between moment in the specimen to the moment in the prototype is called the moment scaling factor, s_m , and is shown in Equation 12.4. The moment scaling factor is simplified by substituting the material scaling factor, s_e (Equation 12.2), and the length scaling factor, s_l (Equation 12.1).

$$s_m = \frac{M_{sp}}{M_{pr}} = \frac{F_{sp}}{F_{pr}} \frac{L_{sp}}{L_{pr}} = s_e s_l^3 = 1/275$$
 Equation 12.4

where M_{sp} is the specimen moment and M_{pr} is the prototype moment.

The length, material, force, and moment scaling factors can be used to scale between the prototype building properties and the specimen properties. With these factors, the prototype building geometry and capacities were scaled, and the specimen was designed. The following sections describe the detailed design of the specimens.

12.5 Specimen design

This section describes the specimen design; Appendix D shows the detailed construction drawings. In addition to the scaling factors, there were several other simplifications made for constructability and testing. Specifically, the floor masses and coupling beams were lumped at 8 levels, instead of the 24 stories of the prototype. Additionally, the gravity loads were partially simulated using unbonded PT over the height of the building. Figure 12.2, Figure 12.3 (a), and Figure 12.3 (b) shows the specimen conceptual view, CORW elevation view, and the SCCW elevation view. As shown in the figures, the scaled specimen is 15.4m tall with a core wall footprint of 1.42m x 1.42m. The outrigger beam is 4.32m long and constructed using two W14X120 steel wide flange members. The outrigger columns are made from HSS6x6x3/8 steel sections and are pin-connected

at the base and outrigger. For stability, lateral supports are provided from the wall to the outrigger column at level 12 and 24. For the efficiency of construction and testing, story masses are lumped at every 3rd floor, where a total of 8 mass blocks are used. To assist with wall stability, a steel truss is bolted to the wall at every mass block level. The RC core wall was constructed in two pieces for transportation (10m wall and a 5m wall) and has a welded splice in between.

The following sections describe each component of the specimen in detail.



Figure 12.2 Conceptual view of specimen



Figure 12.3 Elevation views of the specimen: (a) CORW elevation and (SCCW) elevation

12.5.1 Outrigger system

The outrigger system was approximated to match the same scaled stiffness as the prototype building. The outrigger truss and columns were approximated as two equivalent steel W14x120 beams connected to HSS6x6x3/8 columns, which are laterally braced with steel angle $L2\frac{1}{2}x2\frac{1}{2}x^{14}$. The outrigger is connected to the wall using 16- cast in Dywidag rebar which are embedded into the wall with a 500mm embedment length. Figure 12.4 (a) presents the top view of the outrigger beams and Figure 12.4 (b) shows the side view.

The outrigger beams are pin connected to friction dampers. The HSS6x6x3/8 columns are pin connected at the top to the friction dampers and at the bottom to the foundation. The columns are

constructed into two pieces and spliced between L12 and L13 with a simple 4 bolt connection. The HSS6x6x3/8 are braced to the wall at L12 and L24. The wall is brace assembly, shown in Figure 12.5 (a) (concept) and shown in Figure 12.5 (b) (plan view), and consists of W6x16 beams which use $L2^{1}/_{2}x2^{1}/_{2}x^{1}/_{4}$ cross braces for stability. The braces are pin connected to the wall. The connection plate is connected to the wall using two $\frac{1}{2}$ " diameter through bolts with a back plate.



Figure 12.4 Specimen outrigger beam: (a) top view and (b) side view



Figure 12.5 outrigger system lateral bracing: (a) top view and (b) side view

12.5.2 Post-tensioning

Due to the payload limitation of the shaking tables, gravity loads are partly simulated using posttensioned (PT) strands anchored at level 15 and level 24. Four 0.5" diameter strands are anchored at level 15 and four 0.5" diameter strands are anchored at level 24. The strands are ASTM A 416 Grade 270 and have a specified yield strength of 1670 MPa, and ultimate strength of 1860MPa, a cross sectional area of 98.7mm², and a modulus of elasticity of 195,000MPa. Each strand is stressed until the design axial load taken by each PT strand is 125kN. Figure 12.6 (a) shows the elevation of the post-tensioned system, Figure 12.6 (b) shows the L24 top view PT connection, Figure 12.6 (c) shows the L15 top view PT connection, and Figure 12.6 (d) shows the top view PT connection to the foundation.

At L24 and L15 the PT strands are attached to double channel sections. The double channels have a shear key connection detailing to the wall connection plate. The connection plate and double channel assembly is shown in Figure 12.7. The connection plate has slotted holes to reduce any moment transfer into the connection. The connection plates are connected to the RC wall using through bolts and back plate. The bolts are designed to be post-tensioned, to provide a slip -critical joint between the wall connecting steel plates. Each PT strand is connected using reusable chucks (Figure 12.8), which use specially designed wedges to provide sufficient anchorage.



Figure 12.6 Post tensioned system: (a) elevation view; (b) L24 top view; (c) L 15 top view; and (d) foundation anchorage



Figure 12.7 Post-tensioning anchor beam: (a) concept and (b) connection plate



Figure 12.8 Reusable chuck for PT strand

12.5.3 RC core wall assembly

The RC core wall assembly mainly consists of two equally size C-shaped walls, shown in Figure 12.9. The C-shaped flanges are 150mm thick and 520mm long. The web is 90mm thick with a wall length of 1420mm. The reinforced core has constant cross-section reinforcing over the height of the wall. Each corner of the C-shaped walls has concentrated reinforcement, of 8-#3 bars, representative of zone reinforcing in typical RC shear walls. The flanges have distributed vertical reinforcing of #3 bars spaced at 3" at each face of the wall and horizontal reinforcing of #3 bars spaced at 6". The webs have distributed vertical reinforcing of #3 bars spaced at 5.5" at each face of the wall and horizontal reinforcing of #3 bars were hooked 90 degrees within the zone for constructability, while the flange horizontal bars had 180-degree hooks on either end of the bar. All reinforcing bars are specified ASTM 706 Grade 60. The concrete is specified as 50MPa with ¼" aggregate.



Figure 12.9 Reinforcing layout of the concrete wall. In this figure, #3 bars have a diameter of 3/8" and 10ga has a diameter of 0.1", the cover is 5/16".

The small scale limited the available reinforcing steel for ties. All ties were made from 1/8" mild steel wire. The ties around the zone are hooked 45-degrees around the vertical bar. The zone ties

at the coupled wall are spaced at 2 $\frac{1}{4}$ over the entire height of the wall. The zone at the corner of the C-shape has ties spaced 2 $\frac{1}{4}$ for the first 4 levels, spaced 6" from L5-L20, and again spaced 2 $\frac{1}{4}$ "from L21 to the roof.

The coupling beam dampers are connected to W8x15 steel beams which are embedded into the wall. Figure 12.10 (a) shows the elevation of the coupling beams anchored in the wall and Figure 12.10 (b) shows the conceptual view of the coupling beam. The detailing follows AISC 2016, adjusted for scale. Each beam has four 10M transfer rebars welded to the beams. Six shear studs are welded to the tops and bottom of the embedded beams. The wide flange beam passes through the zone, which obstructs the ties. In this area, ties are placed on either side of the beam with additional ties passing through the holes, shown in Figure 12.10 (c).



Figure 12.10 Anchored coupling beam details: (a) elevation view; b) concept view; and c) tie detailing.

The walls were designed to be constructed in two pieces (30' wall and an 18' wall), and later two wall-to-wall ½'' steel plates are welded together between L16 and L17. This wall-to-wall connection plate concept is shown in Figure 12.11 (a). Cross ties are provided to hook around all the distributed vertical bars, to provide confinement for the embedded 20M bars. Figure 12.11b and Figure 12.11 (c) show the wall-to-wall connection plate located at the bottom of the 18' wall

and the top of the 30' wall, respectively. The plate situation at the top of the 30' wall has an additional ¹/₂" perimeter around the wall edge (Figure 12.11 (c)), to allow for a fillet weld all around. Steel tabs are welded on the sides of the plate are for positioning the wall during the construction process.



Figure 12.11 Steel welded connection plate: (a) concept view; (b) 18' wall plate; and (c) 30' wall plate

To endure the multiple cycles of testing, the base of the wall was confined using external steel plates call base armouring. Figure 12.12 shows base armouring: concept view (Figure 12.12 (a)); web side elevation view (Figure 12.12 (b)); flange side elevation view (Figure 12.12 (c)); and plan view (Figure 12.12 (d)). As shown, the bottom of the wall has a 6" depth of ¼" steel plate, welded at all corners. Threaded rods connected the plates together, where the rods were welded to the inside plate, and bolted to the outside plate. All vertical zone bars are welded to the flange underside of the plate.



Figure 12.12 Armoring plate: (a) concept view; (b) web side elevation view; (c) flange side elevation view; and (d) plan view,

The small-scale of the test structure results in small deformations within the base dampers, particularly for low intensity earthquakes. As a result, the connection between the base damper and the core wall needed to be constructed with high tolerances. Figure 12.13 (a) shows the cross section of the wall-base damper wall connection plate. As shown, the wall-base dampers are connected to the wall with a bolted connection. The bolts are designed to be cast-in the concrete, and post-tensioned after curing. This grout allows for a tight, slip-critical connection.

Figure 12.13 (b) and Figure 12.13 (c) show the front and side view of the base damped connection. To facilitate tight tolerances, 50mm grout layer is poured between the damper connection plate and the foundation. This allows for any differences in construction tolerances to be accommodated.



Figure 12.13 Base connection anchorage: (a) plan view; (b) front view; (c) side view

12.5.4 Mass block frame and diaphragm

The mass is provided through the self-weight of the structure, and by additional 2 tonne steel blocks provided by the Tongji lab. A steel support frame, shown in Figure 12.14, is constructed using beams and angles to support the steel blocks. The blocks are supported in gravity by two steel wide flange sections H200x200x8x10 (Chinese sizes). The blocks are prevented from sliding and movement by surrounding each block with a perimeter of L160x10 steel angle.







Figure 12.15 Typical floor plan view

Steel angle (L8X6X3/4) seats are bolted to the wall flanges, and the mass block frame beams sit on these angles, shown in Figure 12.15. To provide additional support, the mass block frame is connected to the wall through bolted connection in the web.

In the real building application, slabs would be providing the wall with diaphragm connection. To simulate this and ensure the wall has sufficient stability, internal bracing is provided in the wall (shown in plan, Figure 12.15). This internal bracing also provides back plates to the mass block frame connection. To ensure the bracing does not interact with the coupling beams, the steel angle is reduced to a smaller cross section.

12.5.5 Foundation

The foundation is a 2.7m x 5.5m x 0.8m RC block, shown in Figure 12.16. The foundation is prevented from sliding through the clamping force on 36mm diameter rods which are connected to the tables. Additional steel angles are provided to ensure the foundation is prevented from slipping. The foundation construction was contracted to the Chinese construction company, Best Steel.



Figure 12.16 Foundation top view

12.5.6 Energy dissipation devices

The energy dissipation devices used in the outrigger and base dampers are the symmetric friction dampers designed and validated by Professor Geoffrey Rodgers. These devices are designed such that the scaled forces are the same from the Vancouver 24-story prototype building.

All of the SCFDs in the specimen are designed with the same geometry (i.e. cone slope, end plates, etc.) for manufacturing ease. The forces are adjusted by adjusting the post-tension force in the device. The SCFD had to be slightly modified from the original design due to equivalating the 24 beams to 8. Table 12.2 provides the design shear for each device the post yielding stiffness is designed to be 15kN/mm.

Table 12.2 Design SCFD specimen shear.

LVL	L4	L7	L10	L13	L16	L19	L21	L24
Design coupling beam shear, V [kN]	29	31	27	21	18	20	22	18

12.6 Specimen construction

The construction of the specimen was conducted on three continents. The RC core walls and the structural steel (except mass block) were construction in Vancouver, British Columbia, Canada. The foundation block and the mass block frames were constructed by Best Steel in Shanghai, China. The friction dampers were constructed at the University of Canterbury in New Zealand.

12.6.1 Construction at George Third and Son

All the structural steel, except for the SCFD and mass block frames, were constructed by George Third and Son, a high-quality structural steel manufacturer in Vancouver British Columbia. In addition to providing all the structural steel work, George Third and Son permitted the construction to occur in their yard, providing support for all aspects of construction.

12.6.2 RC core wall assembly

The reinforced core walls were constructed in four parts: two U-shaped walls that are 18' long and two C-shaped walls that are 30' walls. The U-shaped walls were constructed with the 3.5" web wall at the bottom with the two 6" flange sections pointed upward. To ensure the walls were constructed level, the formwork was built on top four large steel wide flange sections, that were

levelled prior to construction, shown in Figure 12.17. The embedded steel coupling beams and the steel plates for damper connection, PT connection, Wall-wall connection, and the confinement plates were all built into the formwork. The mass block connections and the diaphragm bolt holes were constructed using PVC pipe cast in. Due to delays in reinforcement placement, the formwork was exposed the weather for weeks prior to the concrete pour. As a result, the formwork warped in some areas, resulting in small gaps at form intersections.



Figure 12.17 Formwork of wall section

12.6.3 Reinforcing steel placement

The reinforcement steel was provided and placed by LMS reinforcing steel. The intricate nature of this scaled down specimen made the construction of the steel cages a complex task. The reinforcing cage had to be built within the formwork, and could not be prefabricated and placed, as is typical construction practice. Figure 12.18 (a) shows the overview of the reinforcing steel in the specimen.

Figure 12.18 (b) shows two pre-formed steel ties stacked on top of one another. The zone ties were formed from 10ga mild steel wire. The tight tolerances were achieved using a wire forming machine. The construction resulted in the tail of the bar being slightly longer than the other. Due to the built-up construction, the #3 zone bars were tied at least five of the corners of the bar. With over 1500 zone ties this resulted in over 7500 individual wire knots. The 10ga ties are shown in Figure 12.18 (c).



Figure 12.18 Reinforcing construction: (a) overview, (b) wire tie; (c) zone detailing

12.6.4 Concrete pour and curing

The 3.5" wall web and the 6" flanges were poured on two separate days. The 3.5" web wall was poured on 8am July 1, 2019 while the 6" wall flanges were poured on 8am on July 15, 2019. A self-consolidating concrete mix, called *Agilia* was donated by Lafarge. This high cement ratio mix is sensitive to vibrations and curing. The first pour, the weather was raining; during the pour, the mix began to clump, and the Lafarge quality control added proprietary admixtures. The second pour was a clear day, the concrete was poured until it leveled with the top of the formwork. To make sure it was level between the coupling beams, the sides of the formwork near the embedded steel beams were hammered during the pour.



Figure 12.19 concrete pour (a) pour 1 and (b) pour 2

Due to the high cement ratio of the mix, there were concerns that insufficient curing would result in large shrinkage cracks. Therefore, as soon as the concrete started to plasticize, soaker hoses were connected, and the walls were kept wet for 2 weeks for the web and 1 week for the flanges.

Concrete cylinders, with a 6in diameter, were sampled for both pours, moist cured, and tested both by UBC and by Lafarge. Table 12.3 shows the concrete strength at different testing dates. The specified design strength at 28 days was 50MPa. However, the stresses exceeded this design at both the 28 day and the 56-day strength.

	UE	BC	Lafarge		
Sample date	Days after pour	Concrete	Days after pour	Concrete strength	
		strength [MPa]		[MPa]	
Lula 25 2010	7	48	7	36.2	
July 25, 2019 (Elan and nouv)	28	53.7	28	57.7	
(Flanges pour)	56	62.5	56	68.6	
Lula 17 2010	11	63.6	7	42.4	
(Web pour)	28	64.5	28	66.2	
	56	74.6	56	74.1	

Table 12.3 Concrete cylinder testing strength

12.6.5 Transport PT

The walls had to undergo a tremendous amount of lifting and shipment from Vancouver to Shanghai. To avoid any unexpected damage, the walls were temporarily post tensioned. A double channel was bolt connected to the backside of the base connection, and a steel angle was placed at the top of the 10m wall. Two 0.5" strands, stressed to 80% fu were used to PT the wall. Figure 12.20 shows the temporary post-tensioning for the wall.



Figure 12.20 Temporary post-tensioning for wall

12.6.6 Assembly and shipment

The walls were assembled into a core configuration in Canada. This process involved the construction of temporary lifting lugs to tilt the wall up into a C-shaped configuration, shown in Figure 12.21 (a). The walls were then aligned, and the steel X-braces were installed inside the core. To accomplish this, the steel braces were fabricated slightly smaller, lifted into the core, then turned into place. The gap between the wall and the brace was grouted in. The grouted in cross-bracing is shown in Figure 12.21 (b).



Figure 12.21 Tilt up of the reinforced core wall

The core walls were then lifted into shipping containers using straps at two locations on the core wall, shown in Figure 12.22. Wood dunnage was laid out on the bottom of the container and the walls placed inside. Lateral bracing was applied to the walls to prevent them from sliding during the shipment.



Figure 12.22 Lifting into shipping container

12.6.7 Construction in China

The foundation and mass block frames were constructed by Best Steel in Shanghai China. The walls were tilted up using two cranes, as shown in Figure 12.23 (a). Both the 30' and 18' walls were erected and have been laterally secured, shown in Figure 12.23 (b). At the time of writing this dissertation, Covid-19 has resulted in global shutdowns and halted international collaborations, including this project. Therefore, final testing has not yet occurred and cannot be included within this thesis. The remaining planned steps in construction are shown in Appendix I.





Figure 12.23 Current status of construction at Tongji University: a) tilt-up of RC wall and b) tilted up walls

12.7 Testing plan overview

To study the performance of the CORW and SCCW behaviour, the specimen designed in Chapter 12 will undergo 5 phases of shaking table tests: Phase A, Phase B, Phase C, Phase D, and Phase E. Figure 12.24 shows each of these 5 phases.

In Phase A, the proposed CORW system will be tested under unidirectional shaking. Three shaking table intensities will be tested: SLE, DBE, and MCE. At the SLE shaking intensity, no yielding is expected within the specimen. At the DBE shaking intensity level, the dampers at the top are expected to dissipate the earthquake energy, while the base of the wall is designed to remain elastic.

At the MCE shaking intensity level, the dampers at the top of outrigger columns and the base of the wall are expected to dissipate the earthquake energy to prevent the structure from collapse.

In Phase B, the proposed SCCW will be tested under unidirectional shaking under four different shaking intensities. It is anticipated that these tests will show that there is no damage under SLE shaking intensity. The SCFDs will yield and dissipate energy at the DBE shaking intensity. Finally, at the MCE shaking intensity, the dampers at the base are expected to yield and start dissipating energy.

In Phase C, the CORW and the SCCW systems will be tested under bidirectional shaking, which will validate that the system can achieve the desired performance objectives.

In Phase D, the outrigger will be removed, and the rocking wall will be tested under unidirectional shaking. The result will be used to compare the rocking wall behaviour with and without an outrigger system. In Phase E, the base of the wall will be fixed, and the conventional shear wall will be tested under unidirectional shaking.



Figure 12.24 Shake table testing plan.

12.8 Ground motion selection and scaling

Only one ground motion is used for all phases of testing. Due to the limitations of the shaking table, only the horizontal motions are applied, and the vertical ground motion inputs are neglected. The dynamic testing of scaled specimens also requires the similitude scaling of the ground motions. The following sections discuss the methods to determine the scaling factors and the ground motion selection.

12.8.1 Similitude

The scaling factor for time and acceleration depends on the mass scaling factor. The total estimated mass of the specimen above the foundation is 97 tonnes, based on the mass of each component shown in Table 12.4. The total seismic mass in the prototype building is 17,812 tonnes. Hence, the scaling factor for mass is $s_m = \frac{1}{184}$, calculated using Equation 12.5.

$$s_m = \frac{M_{sp}}{M_{pr}} = \frac{97 \text{ tonne}}{17,812 \text{ tonne}} = \frac{1}{184}$$
 Equation 12.5

Mass block frame	1 tonne per floor x 8 floors = 8 tonnes
Core wall	20 tonne
Outrigger beam	1.7 tonne
Outrigger columns	2.5 tonnes
SCFDs	0.8tonne
Mass blocks	2tonne x 4 per floor x 8 floors = 64tonne
Total mass	97 tonne

Table 12.4 Estimate specimen masses

With the mass and length scaling factors, the acceleration, time, and velocity scaling factors can be calculated. Acceleration scaling factor, s_A , is determined by the ratio of the specimen acceleration to the prototype acceleration. The acceleration scaling factor is derived using Equation 12.6

$$s_A = \frac{acc_{sp}}{acc_{pr}} = \frac{F_{sp}}{M_{sp}} \frac{M_{pr}}{F_{pr}} = \frac{s_l^2}{s_m} = 4.36$$
 Equation 12.6

where acc_{sp} , F_{sp} , and M_{sp} is the acceleration, force, and mass, respectively, for the specimen and acc_{pr} , F_{pr} , and M_{pr} is the elastic modulus, stress, and strain, respectively, for the prototype.

The time scaling factor, s_t , is determined by the ratio of the specimen time to the prototype time. This scaling factor can be applied to any time unit used in the analysis of the specimen, such as natural period or ground motion time. The time scaling factor is derived in Equation 12.7

$$s_t = \frac{t_{sp}}{t_{pr}} = \sqrt{\frac{L_{sp}}{acc_{sp}}\frac{acc_{pr}}{L_{pr}}} = \sqrt{\frac{s_m}{s_l}} = 0.19$$
 Equation 12.7

where t_{sp} is the specimen time unit and t_{pr} is the prototype time unit.

The velocity scaling factor, s_v , is determined by the ratio of the specimen velocity to the prototype velocity. This scaling factor can be applied to any velocity unit used in the analysis of the specimen, such as ground motion velocity. The velocity scaling factor is derived in Equation 12.8.

$$s_v = \frac{vel_{sp}}{vel_{pr}} = \frac{t_{sp}}{t_{pr}} \frac{acc_{sp}}{acc_{pr}} = s_t s_a = \sqrt{\frac{s_l^3}{s_m^3}} = 1.22$$
 Equation 12.8

where vel_{sp} is the specimen velocity unit and vel_{pr} is the prototype velocity unit.

12.8.2 Ground motion selection

The ground motion was selected as the motion which is close to the uniform hazard spectrum (UHS) at the MCE, while having a low peak ground acceleration. The UHS of the prototype building located in Vancouver, Canada is used. To reduce the variabilities caused by using different ground motions for different hazards, the same ground motion was used to test the SLE, DBE, and MCE shaking levels. The SLE and DBE were estimated as 8.5% and 30% of the MCE, respectively.

The ground motion was selected to be from the 1992 Landers Earthquake, California (PEER ID: RSN 832). The selected ground motion was first scaled by factor of 0.19, 0.63, and 2.26 to match the prototype SLE, DBE, and MCE, respectively. The corresponding scaled acceleration spectrum for the North-South and West-East components, geomean, and design spectrums are shown in

Figure 12.25. In this figure $S_{a,s}$ = specimen pseudo acceleration, T_s = specimen period, S_A = acceleration scaling factor, and S_t = time scaling factor.



Figure 12.25 Scaled ground motion acceleration spectrums for the (a) SLE, (b) DBE, and (c) MCE hazards.

12.9 Instrumentation plan

In the proposed instrumentation there are 342 displacement gauges, 10 accelerometers, 16 load cells, and 202 strain gauges. Figure 12.26 shows the naming convention for all the planned instrumentation. Global displacements are measured using string pots which will be anchored to the walls and on to stationary frames situated off the table. Two lateral displacement measurements are recorded at three of the wall corners. In this test, it is assumed that each floor acts as a rigid diaphragm. Vertical deformations are measured using LVDTs at each corner of the core wall. Two additional vertical LVDTs are attached at the flange end. Figure 12.27 shows a typical plan view; Figure 12.28 shows the elevation view in the N-S direction; and Figure 12.29 shows the elevation view in the W-E direction.



Figure 12.26 Instrumentation naming convention



Figure 12.27 Instrumentation typical plan view



Figure 12.28 Displacement transducers (North-South) views (a) Elevation- Grid A and (b) Elevation – Grid E



Figure 12.29 Displacement transducers (East-West) views (a) Elevation- Grid A and (b) Elevation – Grid E

Strain gauges are attached to the top and bottom of two W8x15 beams on the North side of the wall. Due to limitations on instrumentation, strain gauges are attached to only one of the coupling beams on the south side. Figure 12.30 shows a typical LVDT layout each of the coupling beams. Two LVDTs are connected to estimate the shear deformation within the SCFD.



Figure 12.30 Coupling beam instrumentation plan

Strain gauges are attached to the outrigger beam, shown in Figure 12.31 (a) (side view) and in Figure 12.31 (b) (front view). These gauges will later be used to estimate the elastic stresses in the beam. LVDTs will be used to measure the deformations in all both the outrigger dampers and the wall-base dampers. Load pins will be used to measure the forces within the dampers. The PT strand forces are measured using force load cells for every PT strand.



Figure 12.31 Outrigger damper instrumentation

12.10 Predicted response

In this section, a detailed numerical model of the specimen is developed using Perform3D. The selected ground motion, shown in Figure 12.25 was simulated and the model, and the seismic performance evaluated.

12.10.1 Modelling approach

A detailed numerical model was developed to estimate the response of the shake table testing. The detailed model utilises the modelling approach described in Chapter 6. Like the modelling approach outlined in Chapter 6, a 3-dimensional model was developed using Perfom3D. Figure 12.32 (a) shows the model in the SCCW view, Figure 12.32 (b) shows the model in the CORW view, and Figure 12.32 (c) shows the plan view. Unlike the prototype building models, additional details unique to the specimen, such as the PT tendons, and the internal braces, were all added as elastic elements.



Figure 12.32 Numerical modelling of shake table specimen: (a) SCCW view, (b) CORW view, and (c) plan view

Eigen-value analysis was completed on the detailed model. Figure 12.33 shows the mode shapes and periods. The scaled specimen results in a scaled period of 3.10, 2.24, and 1.58 for the first, second, and third modes respectively. This is a 6.0%, 4.1%, and 3.8% difference in period with the prototype building, respectively.


Figure 12.33 Mode shapes for each specimen model, including: 24 story Mode 1 (a), Mode 2 (b),

12.10.2 Nonlinear responses

The nonlinear model was subject to the ground motion selected in Section 12.8.2. The model was subjected to both unidirectional and bidirectional loading. The peak displacements were recorded at each of the four corners of the RC core-wall, at the locations of the planned instrumentation.

Figure 12.34 shows the peak story displacements (Δ_s) for each hazard, at each wall corner, normalised to the building height (H). Generally, there is a negligible difference between the displacements at each corner for unidirectional loading (i.e., no torsion is observed).

At the SLE, there is minimal differences (less than 2%) between bidirectional and unidirectional displacements, indicating that there is minimal elastic torsion in the system. However, at the DBE and MCE, more significant differences are observed between the unidirectional and bidirectional motions.



Figure 12.34 Predicted normalised story displacements: (a) SLE-CORW; (b) SLE-SCCW; (c) DBE-CORW; (d) DBE-SCCW; (e) MCE-CORW; and (f) MCE-SCCW

Figure 12.35 shows the outrigger damper forces (f_o) normalised to the yield force (f_y) in each of the four outrigger dampers at each hazard level. As shown, at the SLE f_o/f_y dampers are below

yielding at the SLE, which is the performance objective at the EEDP design. At the DBE and MCE, the $f_o/f_y=1$ for all dampers, indicating yielding, which is the performance objective at the EEDP design.



Figure 12.35 Specimen outrigger damper predicted normalised force: (a) SLE shaking intensity; (b) DBE shaking intensity; (c) MCE shaking intensity

Figure 12.36 shows the wall-base damper forces (f_b) normalised to the yield force $(f_{b,y})$ in each of the four wall-base dampers at each hazard level. As shown, at the SLE $f_b/f_{b,y}$ dampers are below yielding at the SLE and DBE, which is the performance objective at the EEDP design. At the MCE, the $f_b/f_y=1$ for all dampers, indicating yielding, which is the performance objective at the EEDP design.

Figure 12.37 (a), (b), and (c) shows the coupling beam forces (V_{CB}) normalised to the yield force ($V_{CB,y}$) in each of the coupling beams at the SLE, DBE, and MCE hazards, respectively. As shown, at the SLE $V_{CB}/V_{CB,y}$ dampers are below yielding at the SLE, which is the performance objective at the EEDP design. At the DBE and MCE, the $V_{CB}/V_{CB,y}$ is greater than 1 for all dampers, indicating yielding, which is the performance objective at the EEDP design. Unlike the wall-base and outrigger dampers, the ratio of the $V_{CB}/V_{CB,y}$ is greater than 1 after yielding due to the post-yielding stiffness of the dampers.



Figure 12.36 Wall-base normalised wall-base damper force: (a) SLE shaking intensity; (b) DBE shaking intensity; (c) MCE shaking intensity



Figure 12.37 Specimen normalised coupling beam force: (a) SLE shaking intensity; (b) DBE shaking intensity; (c) MCE shaking intensity

Figure 12.38 shows the normalised strain for each hazard, for each direction of loading. At each node, strains are determined by taking the difference between the vertical displacements at each wall corner divided by the distance between the nodes. The tension strains are normalised to the

yield strain of rebar ($\varepsilon_y = 0.002$). All strains are below yielding (i.e., $\varepsilon/\varepsilon_y < 1$), demonstrating the EEDP performance objectives have been met.



Figure 12.38 Specimen normalised vertical wall strain: (a) SLE-CORW; (b) SLE-SCCW; (c) DBE-CORW; (d) DBE-SCCW; (e) MCE-CORW; and (f) MCE-SCCW

12.11 Summary

This chapter describes the work completed towards shake-table testing of the CROCW. An overview of the testing facility, specimen scaling, design, and construction is provided. The design

and construction of the testing specimen is a key contribution in this thesis. All components are currently in China, awaiting the opening of international borders at the time of writing this dissertation, such that experimental testing may continue.

This chapter also describes the testing plan which will be implemented after international borders open and research collaborations continue. A description of the testing phases and the instrumentation plan were provided. A detailed numerical model of the specimen was developed and was summarised in this chapter. The numerical model demonstrates expected high seismic performance of the proposed system.

Chapter 13: Summary of work and recommendations for future work

This thesis introduced three new seismic force resisting systems for earthquake-resilient high-rise buildings: the controlled rocking outrigger core wall (CROCW), the controlled outriggered rocking wall (CORW) and the self-centring coupled wall (SCCW). Additionally, a novel non-iterative design approach was developed, using the Equivalent energy-based design procedure (EEDP) with modifications to incorporate higher mode effects and hysteretic behaviour of the new systems. The presented EEDP is recommended to design the proposed systems for multiple performance objectives at different shaking intensities. The following describes some of the unique contributions of this work and ideas for future investigations.

13.1 Contributions

The overarching goal of this research was to provide alternative lateral force resisting systems and design methods for the RC core wall system which will result in enhanced seismic performance. The detailed contributions made towards these goals are discussed in the following sections.

13.1.1 Proposed a novel high-performance RC core wall system

This thesis provided an extensive literature review on energy dissipation devices and low-damage RC wall systems. From this review, a new RC core wall system was proposed. This new system is comprised of two unique systems along each principal axis, the CORW in the shear wall direction and the SCCW in the coupled wall direction.

Both systems are "dual-fused" and have a primary yielding mechanism and a secondary yielding mechanism. The systems are designed to remain elastic at a low hazard, yield the primary fuse system at the moderate hazard, and yield the primary fuse system and secondary fuse system at the high hazard.

The CORW primary fuse system is a damped outrigger, and its secondary fuse system is a damped controlled rocking base. This thesis details the mechanics of the primary fuse system and secondary fuse systems and provides discussion on the influence of important design parameters on the response.

The SCCW system's primary fuse system consists of novel self-centering coupling beams called Self-centering conical friction dampers (SCFD), and its secondary fuse system is a damped controlled rocking base. The author of this thesis heavily contributed to the development of the SCFD and is listed as a co-inventor on two patents and co-author on one journal paper. In this thesis, the behaviour of the SCFD and equations to estimate the hysteretic response are provided. Additionally, discussion on implementation of the SCFD in the SCCW is presented.

The use of a controlled rocking base as the secondary mechanisms in the CORW and SCCW allow this system to function efficiently under bidirectional loading. This thesis presented some of the challenges with the response of a controlled rocking base within a coupled wall system.

The outcome of this contribution is two unique systems which can be integrated into a centralised RC core wall, where the mechanical behaviour and plastic mechanisms are well understood. Hence, these systems may be used as a low-damage solution to RC core walls.

13.1.2 Quantified dynamic characteristics of the RC core wall systemS

This thesis studied the dynamic characteristics of the proposed earthquake-resilient RC core wall using linear dynamic analysis. From these studies empirical equations were developed to rapidly predict the higher mode responses of the systems.

From these analyses, it was observed that the CORW has a first mode dominant response in roof displacement, base moment, and outrigger moment. It was discovered that the CORW behaviour is highly dependent on the relative stiffness of the outrigger to the wall. This stiffness ratio was quantified using a factor α_f . Most of the empirical equations developed depend on this factor. Additionally, it was shown that the inverted triangular distribution gives a reasonable estimate of the force distribution to design the yielding mechanism for the CORW.

The dynamic analysis demonstrated that the SCCW has a significant higher mode response in the coupling beams and wall pier moments. As a result, a new static load distribution was developed to appropriately estimate the coupling beam shears.

13.1.3 Development of Equivalent Energy Based Design Procedure for earthquake resilient tall buildings

This thesis presented a design method for the proposed CROCW, CORW, and SCCW systems using EEDP with some modifications. The original derivation of EEDP assumes an elastic-plastic response for the primary fuse system and secondary fuse systems. The CORW has an elastic-plastic response for the outrigger system and a flag-shaped hysteretic response for the controlled rocking base. Therefore, new gamma factors were developed to apply EEPD to this system. Additionally, new gamma factors were developed for the SCCW, which has a self-centring hysteretic behaviour as the primary fuse system and secondary fuse system.

The original EEDP was developed for buildings which have a first mode response. The nature of tall core wall buildings is to have a dominant first mode response in displacements, but a higher mode response in terms of forces. In this study, the higher mode force response was estimated using linear dynamic models. The dynamic response was translated as factors to be incorporated with the EEDP.

The introduction of the higher mode factors and the gamma factors allows the SCCW and the CORW to be designed for multiple different performance objectives under different shaking intensities. A step-by-step guide to designing the SCCW and the CORW are provided, giving practitioners and researchers the tools to apply this procedure to design these new systems.

This thesis's design procedure has significant advantages compared with typical code-based design or other performance-based design methods. Specifically, it allows the design of multiple performance objectives at different shaking intensities. However, unlike other performance-based design methods, EEDP allows for the design to be completed without iterations. Hence, EEDP can be used as an efficient design method for the novel CORW and SCCW systems.

13.1.4 Assessed the performance of earthquake-resilient core wall buildings

A robust 3-dimensional modeling approach was developed for researchers and practitioners to use in design validation, performance-based design, and performance assessments of the CROCW system. The development of this modeling approach allows for researchers and practitioners alike to model the complex behaviour of the proposed earthquake resilient RC core wall system. Four prototype buildings, representing different building heights and hazards, were designed and analyzed using the 3-dimensional modeling approach. Nonlinear dynamic analysis, using both unidirectional and bidirectional motions, was conducted on the four prototype buildings that were designed using the new systems.

Within this dissertation is a comparison between alternative shear wall systems and the CORW systems and alternative coupled wall systems and the SCCW systems. The CORW was compared with three other RC wall systems: the shear wall (SW), the outriggered wall (OW), and the controlled rocking wall (CRW). The SCCW was compared with three other RC coupled wall systems: the coupled wall (CRW), the damped coupled wall (DCW), and the controlled rocking coupled wall (CRCW).

These comparisons demonstrated the significant higher performance when using the new technology and design methods in this thesis. For example, the CORW demonstrated 52% decrease in interstorey drifts and 22% decrease in roof displacements when compared with the conventional fixed based shear wall (SW). Similarly, the SCCW showed a 22% reduction in displacements when compared with conventional coupled wall (CW) building with the same yield force in the coupling beams. This outcome exemplifies the low damage performance of the proposed systems compared to existing technology.

13.1.5 Development of experimental program

As part of this work, a large-scale shake table specimen was designed and constructed. The objective was to design a large-scale specimen that could provide realistic local as well as global response of the system under earthquake loads. There was a significant effort to ensure the specimen was designed to meet the high-quality construction standards in typical Canadian practice. At the time of submitting this dissertation, Covid-19 travel restrictions have paused the experimental schedule.

13.2 Ongoing and Future work

This thesis presented the concept, design approach, numerical validation, and numerical comparison between the novel and conventional systems. There are several avenues for future work, particularly in experimental testing of this new system. The following describe some of the areas of future work.

The large-scale shake table testing will be completed once the Covid-19 related travel restrictions are lifted, and it is safe to continue testing. The remaining work involves specimen assembly, instrumentation, re-testing friction devices, and the shake table testing. The results from this experiment will help to validate the numerical models, developed as part of this research work. It is expected that the large-scale shake table testing will give key insight to the dynamic response of controlled rocking tall buildings.

After these shake table tests are completed, researchers can use this data to develop and calibrate numerical models of the CORW and SCCW systems. This data will allow for detailed study of the response of a range of prototype buildings, and further validate this new system. Additionally, researchers can use the data to further the understanding of coupling beam shear distribution in core wall buildings.

This thesis work focused on the response of the lateral force resisting system, while assuming the gravity system would be appropriately detailed to accommodate the deformation demands. Future research can be centered on the development of a gravity force resisting system which can work with the CORW and SCCW to ensure damage resistance.

The controlled rocking base is assumed to be on a fixed base; there was no investigation into the foundation design or the effects of podium on the response. Further investigations are required to understand the influence of the podium.

In this research, the controlled rocking base was assumed to be confined with steel plates and the sensitivity of this confinement was not examined. Future work could focus on detailed design procedures to determine appropriate rocking base design for wall systems.

The parametric analysis to quantify the higher mode effects of the system used idealized wall geometry and linear dynamic analysis. Future work could improve these design relations by conducting robust nonlinear parametric analysis using a variety of core wall geometry.

This work did not account for the wind demands within the design procedure. Wind can substantially increase the demands in high-rise building. Future work should address the consideration of wind demands.

This work did not consider the effect of vertical ground motions on the response of the controlled rocking system. Future work should include the effects of vertical ground motions on the controlled rocking tall building system.

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Appendix A: Parameter study results for all dynamic characteristic archetypes

These appendices provide the detailed numerical analysis for the LDA parametric studies provided in Chapter 4.

Name	af	nf I	hf mf	Ly	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment N	Wall Noment	Mode 1Disp Norf-1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN
V11111111	0.1	15	3 400	8	8	0.6	15	0.85	0.14	0.05	0.64	0 19	0.07	0.49	0.80	0.80	0.13	0.004	[MN-m] 618	[MR] 618	[MN-m] 25	(MN-m) 601	[m] [m]	[MN-m] 609	(MN) 18	m] 592
V11111111	0.1	15	3 400	8	8	0.6	2.5	0.93	0.15	0.05	0.64	0.19	0.07	0.45	0.89	0.89	0.14	0.004	573	573	23	558	0.14 0.0044	564	17	548
V21111111	0.3	15	3 400	8	8	0.6	1.5	0.79	0.13	0.05	0.65	0.18	0.07	0.52	0.89	0.89	0.12	0.003	661	661	80	590	0.12 0.0035	654	20	583
V21111112	0.3	15	3 400	8	8	0.6	2.5	0.86	0.15	0.05	0.65	0.18	0.07	0.48	0.89	0.89	0.13	0.004	613	613	74	548	0.13 0.0039	605	18	539
V31111111	0.5	15	3 400	8	8	0.6	1.5	0.73	0.13	0.05	0.66	0.17	0.06	0.56	0.89	0.89	0.11	0.003	/1/	/1/	145	584	0.11 0.0031	/11	22	577
V41111111	0.5	15	3 400	8	8	0.6	2.5	0.79	0.14	0.05	0.68	0.17	0.00	0.52	0.89	0.89	0.12	0.003	789	789	222	580	0.12 0.0034	785	20	574
V41111111	0.7	15	3 400	8	8	0.6	2.5	0.72	0.12	0.05	0.68	0.16	0.06	0.56	0.89	0.89	0.10	0.003	735	735	207	540	0.10 0.0030	731	22	534
V51111111	0.9	15	3 400	8	8	0.6	1.5	0.58	0.11	0.04	0.70	0.14	0.06	0.67	0.89	0.89	0.08	0.002	892	892	320	586	0.08 0.0024	889	28	582
V51111112	0.9	15	3 400	8	8	0.6	2.5	0.63	0.12	0.05	0.70	0.14	0.06	0.62	0.89	0.89	0.08	0.003	826	826	297	543	0.08 0.0027	823	26	539
V11111121	0.1	15	3 400	8	8	1	1.5	0.73	0.12	0.04	0.64	0.19	0.07	0.55	0.89	0.89	0.11	0.003	698	698	28	679	0.11 0.0033	690	21	672
V11111122	0.1	15	3 400	8	8	1	2.5	0.81	0.13	0.05	0.64	0.19	0.07	0.51	0.89	0.89	0.12	0.004	747	747	26	669	0.12 0.0037	741	22	619
V21111121 V21111122	0.3	15	3 400	8	8	1	2.5	0.08	0.12	0.04	0.65	0.18	0.07	0.55	0.89	0.89	0.10	0.003	688	688	84	615	0.11 0.0023	681	22	607
V31111121	0.5	15	3 400	8	8	1	1.5	0.63	0.11	0.04	0.66	0.17	0.06	0.63	0.89	0.89	0.09	0.003	806	806	163	656	0.09 0.0026	801	24	650
V31111122	0.5	15	3 400	8	8	1	2.5	0.69	0.12	0.05	0.66	0.17	0.06	0.58	0.89	0.89	0.10	0.003	748	748	151	609	0.10 0.0029	743	23	602
V41111121	0.7	15	3 400	8	8	1	1.5	0.57	0.11	0.04	0.68	0.16	0.06	0.68	0.89	0.89	0.08	0.002	890	890	251	653	0.08 0.0023	887	27	648
V41111122	0.7	15	3 400	8	8	1	2.5	0.63	0.12	0.04	0.68	0.16	0.06	0.63	0.89	0.89	0.09	0.003	821	821	231	603	0.09 0.0026	817	25	597
V51111121	0.9	15	3 400	8	8	1	2.5	0.50	0.09	0.04	0.70	0.14	0.06	0.74	0.89	0.89	0.06	0.002	988	988	334	609	0.06 0.0020	985	29	605
V11111211	0.1	15	3 400	8	10	0.6	1.5	2.10	0.34	0.12	0.64	0.19	0.07	0.24	0.85	0.89	0.39	0.012	315	315	12	306	0.39 0.0118	299	9	291
V11111212	0.1	15	3 400	8	10	0.6	2.5	2.32	0.38	0.14	0.64	0.19	0.07	0.21	0.83	0.89	0.42	0.013	280	280	11	272	0.42 0.0126	262	8	255
V21111211	0.3	15	3 400	8	10	0.6	1.5	1.95	0.33	0.12	0.65	0.18	0.07	0.26	0.85	0.89	0.36	0.011	340	340	41	306	0.36 0.0106	327	10	292
V21111212	0.3	15	3 400	8	10	0.6	2.5	2.15	0.37	0.13	0.65	0.18	0.07	0.23	0.84	0.89	0.39	0.012	304	304	36	274	0.39 0.0115	291	9	259
V31111211 V31111212	0.5	15	3 400	8	10	0.6	2.5	1.79	0.32	0.12	0.66	0.17	0.06	0.28	0.85	0.89	0.32	0.009	366	366	69	301	0.32 0.0093	356	11	288
V41111212	0.7	15	3 400	8	10	0.6	1.5	1.62	0.30	0.12	0.68	0.16	0.06	0.30	0.86	0.89	0.28	0.008	399	399	113	296	0.28 0.0082	391	12	286
V41111212	0.7	15	3 400	8	10	0.6	2.5	1.79	0.33	0.13	0.68	0.16	0.06	0.28	0.85	0.89	0.32	0.009	370	370	105	275	0.32 0.0093	362	11	265
V51111211	0.9	15	3 400	8	10	0.6	1.5	1.43	0.27	0.11	0.70	0.14	0.06	0.33	0.87	0.89	0.23	0.007	445	445	162	295	0.23 0.0072	439	14	288
V51111212	0.9	15	3 400	8	10	0.6	2.5	1.58	0.30	0.12	0.70	0.14	0.06	0.31	0.86	0.89	0.26	0.008	413	413	151	275	0.26 0.0082	408	13	267
V11111221	0.1	15	3 400	8	10	1	1.5	2.02	0.33	0.12	0.64	0.19	0.07	0.25	0.85	0.89	0.38	0.012	329	329	13	319	0.38 0.0115	313	9	304
V21111222	0.1	15	3 400	8	10	1	2.5	1.88	0.50	0.15	0.64	0.19	0.07	0.22	0.85	0.89	0.41	0.015	350	350	42	314	0.41 0.0125	337	10	301
V21111222	0.3	15	3 400	8	10	1	2.5	2.09	0.36	0.13	0.65	0.18	0.07	0.24	0.84	0.89	0.39	0.011	317	317	38	286	0.39 0.0113	304	9	271
V31111221	0.5	15	3 400	8	10	1	1.5	1.73	0.31	0.12	0.66	0.17	0.06	0.29	0.86	0.89	0.31	0.009	376	376	76	309	0.31 0.0090	366	11	296
V31111222	0.5	15	3 400	8	10	1	2.5	1.92	0.34	0.13	0.66	0.17	0.06	0.26	0.85	0.89	0.35	0.010	347	347	70	286	0.35 0.0101	336	10	273
V41111221	0.7	15	3 400	8	10	1	1.5	1.57	0.29	0.11	0.68	0.16	0.06	0.31	0.86	0.89	0.27	0.008	410	410	116	304	0.27 0.0079	402	12	294
V41111222	0.7	15	3 400	8	10	1	2.5	1.73	0.32	0.12	0.68	0.16	0.06	0.29	0.85	0.89	0.30	0.009	379	379	107	282	0.30 0.0089	3/1	14	2/1
V51111221	0.9	15	3 400	8	10	1	2.5	1.53	0.20	0.10	0.70	0.14	0.06	0.34	0.87	0.89	0.22	0.008	430	423	154	281	0.25 0.00079	418	13	273
V11112111	0.1	15	3 400	10	8	0.6	1.5	1.23	0.20	0.07	0.64	0.19	0.07	0.37	0.88	0.89	0.21	0.006	473	473	19	460	0.21 0.0063	461	14	449
V11112112	0.1	15	3 400	10	8	0.6	2.5	1.52	0.25	0.09	0.64	0.19	0.07	0.32	0.88	0.89	0.27	0.008	409	409	16	398	0.27 0.0082	396	12	385
V21112111	0.3	15	3 400	10	8	0.6	1.5	1.15	0.20	0.07	0.65	0.18	0.07	0.39	0.88	0.89	0.19	0.005	498	498	60	446	0.19 0.0055	488	15	435
V21112112	0.3	15	3 400	10	8	0.6	2.5	1.41	0.24	0.09	0.65	0.18	0.07	0.34	0.88	0.89	0.25	0.007	433	433	52	388	0.25 0.0072	422	13	377
V31112111 V31112112	0.5 0.5	15	3 400	10	8	0.0	2.5	1.05	0.19	0.07	0.00	0.17	0.06	0.41	0.88	0.89	0.10	0.005	329 463	463	107	432	0.10 0.0047	522 454	10	423
V41112111	0.7	15	3 400	10	8	0.6	1.5	0.95	0.18	0.07	0.68	0.16	0.06	0.44	0.89	0.89	0.14	0.004	579	579	164	427	0.14 0.0042	574	18	420
V41112112	0.7	15	3 400	10	8	0.6	2.5	1.17	0.22	0.08	0.68	0.16	0.06	0.38	0.88	0.89	0.19	0.005	501	501	142	371	0.19 0.0054	495	15	362
V51112111	0.9	15	3 400	10	8	0.6	1.5	0.84	0.16	0.06	0.70	0.14	0.06	0.49	0.89	0.89	0.12	0.004	656	656	237	432	0.12 0.0037	652	20	427
V51112112	0.9	15	3 400	10	8	0.6	2.5	1.04	0.19	0.08	0.70	0.14	0.06	0.41	0.88	0.89	0.15	0.005	551	551	199	364	0.15 0.0047	547	17	358
V11112121 V11112122	0.1	15	3 400	10	8	1	1.5 2.5	1.12	0.18	0.07	0.64	0.19	0.07	0.39	0.88	0.89	0.18	0.006	503	503 477	20	489	0.18 0.0055	492	15	4/9
V21112121	0.3	15	3 400	10	8	1	1.5	1.04	0.18	0.06	0.65	0.19	0.07	0.41	0.89	0.89	0.16	0.005	528	528	64	473	0.16 0.0048	519	16	463
V21112122	0.3	15	3 400	10	8	1	2.5	1.35	0.23	0.08	0.65	0.18	0.07	0.35	0.88	0.89	0.23	0.007	446	446	54	400	0.23 0.0068	436	13	388
V31112121	0.5	15	3 400	10	8	1	1.5	0.96	0.17	0.06	0.66	0.17	0.06	0.44	0.89	0.89	0.15	0.004	570	570	115	465	0.15 0.0042	563	17	456
V31112122	0.5	15	3 400	10	8	1	2.5	1.24	0.22	0.08	0.66	0.17	0.06	0.37	0.88	0.89	0.21	0.006	476	476	96	389	0.21 0.0059	468	14	379
V41112121	0.7	15	3 400	10	8	1	1.5	0.86	0.16	0.06	0.68	0.16	0.06	0.48	0.89	0.89	0.13	0.004	628	628	177	463	0.13 0.0037	623	19	456
V41112122 V51112121	0.7 0.9	15	3 400	10	8	1	2.5	0.76	0.21	0.08	0.68	0.16	0.06	0.53	0.88	0.89	0.18	0.005	515	515 710	256	380	0.18 0.0051	509 706	22	372
V51112122	0.9	15	3 400	10	8	1	2.5	0.99	0.19	0.07	0.70	0.14	0.06	0.43	0.88	0.89	0.14	0.004	569	569	205	376	0.14 0.0045	564	18	369
V11112211	0.1	15	3 400	10	10	0.6	1.5	3.24	0.53	0.19	0.64	0.19	0.07	0.14	0.72	0.88	0.53	0.017	191	191	7	185	0.53 0.0161	171	5	166
V11112212	0.1	15	3 400	10	10	0.6	2.5	4.29	0.70	0.25	0.64	0.19	0.07	0.10	0.58	0.88	0.66	0.021	140	140	5	136	0.66 0.0200	121	4	118
V21112211	0.3	15	3 400	10	10	0.6	1.5	3.01	0.51	0.19	0.65	0.18	0.07	0.15	0.74	0.88	0.50	0.015	205	205	24	186	0.50 0.0146	189	6	168
V21112212	0.3	15	3 400	10	10	0.6	2.5	3.99	0.68	0.25	0.65	0.18	0.07	0.11	0.59	0.88	0.62	0.019	149	149	17	136	0.62 0.0181	134	4	119
V31112211	0.5	15	3 400	10	10	0.6	1.5	3.66	0.49	0.18	0.66	0.17	0.06	0.17	0.75	0.88	0.46	0.014	162	162	45	136	0.40 0.0133	213	5 5	1/2
V41112211	0.7	15	3 400	10	10	0.6	1.5	2.50	0.46	0.18	0.68	0.16	0.06	0.19	0.77	0.88	0.42	0.012	256	256	73	193	0.42 0.0123	246	8	180
V41112212	0.7	15	3 400	10	10	0.6	2.5	3.31	0.61	0.24	0.68	0.16	0.06	0.13	0.64	0.88	0.52	0.015	182	182	52	139	0.51 0.0151	172	5	126
V51112211	0.9	15	3 400	10	10	0.6	1.5	2.21	0.41	0.17	0.70	0.14	0.06	0.22	0.81	0.89	0.37	0.012	303	303	112	203	0.37 0.0116	296	9	194

Name	af nf	hf	mf L	y La	c tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp [m]	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1 ISDR [-]	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
V51112212	0.9 15	53.	400 1	0 10) 0.6	2.5	2.93	0.55	0.22	0.70	0.14	0.06	0.16	0.70	0.88	0.45	0.014	213	213	80	145	0.45	0.0141	206	6	135
V11112221	0.1 15	53	400 1	.0 10) 1	1.5	3.17	0.52	0.19	0.64	0.19	0.07	0.14	0.73	0.88	0.53	0.016	196	196	7	190	0.52	0.0158	175	5	170
V11112222	0.1 15	53.	400 1	0 10) 1	2.5	4.29	0.70	0.25	0.64	0.19	0.07	0.10	0.58	0.88	0.66	0.021	140	140	5	136	0.66	0.0200	121	4	118
V21112221	0.3 15	3	400 1	$\frac{0}{0}$ 10	$\frac{1}{1}$	1.5	2.95	0.50	0.18	0.65	0.18	0.07	0.15	0.74	0.88	0.49	0.015	210	210	25	190	0.49	0.0144	194	6	173
V31112222	0.5 15	53.	400 1	0 10) 1	1.5	2.71	0.48	0.23	0.65	0.18	0.07	0.11	0.76	0.88	0.02	0.013	231	231	46	193	0.46	0.0131	218	7	115
V31112222	0.5 15	53.	400 1	.0 10) 1	2.5	3.67	0.65	0.24	0.66	0.17	0.06	0.12	0.61	0.88	0.57	0.017	162	162	32	136	0.57	0.0165	150	5	121
V41112221	0.7 15	53.	400 1	.0 10) 1	1.5	2.45	0.45	0.18	0.68	0.16	0.06	0.19	0.78	0.89	0.41	0.012	262	262	75	198	0.41	0.0121	253	8	185
V41112222	0.7 15	53.	400 1	0 10	$\frac{1}{1}$	2.5	3.32	0.61	0.24	0.68	0.16	0.06	0.13	0.64	0.88	0.52	0.015	182	182	52	139	0.51	0.0151	172	5	126
V51112221	0.9 1	53.	400 1	0 10	$\frac{1}{1}$	2.5	2.17	0.41	0.10	0.70	0.14	0.00	0.25	0.82	0.89	0.30	0.011	213	213	80	145	0.30	0.0114	206	5	135
V11121111	0.1 15	5 3	600	8 8	3 0.6	1.5	1.04	0.17	0.06	0.64	0.19	0.07	0.41	0.89	0.89	0.17	0.005	787	787	32	765	0.17	0.0050	771	23	750
V11121112	0.1 15	53	600	88	3 0.6	2.5	1.14	0.19	0.07	0.64	0.19	0.07	0.39	0.88	0.89	0.19	0.006	745	745	30	725	0.19	0.0057	729	22	709
V21121111	0.3 15	5 3 1	600 600	88	3 0.6	2.5	0.97	0.17	0.06	0.65	0.18	0.07	0.44	0.89	0.89	0.15	0.004	836	836	101	748	0.15	0.0044	824	25	735
V31121112	0.5 1	5 3 1	600	8 8	3 0.6	1.5	0.89	0.18	0.07	0.65	0.18	0.07	0.41	0.89	0.89	0.17	0.003	904	904	183	702	0.17	0.0049	894	25	725
V31121112	0.5 15	5 3 1	600	8 8	3 0.6	2.5	0.97	0.17	0.06	0.66	0.17	0.06	0.43	0.89	0.89	0.15	0.004	841	841	170	686	0.15	0.0043	831	25	674
V41121111	0.7 15	53	600	88	3 0.6	1.5	0.81	0.15	0.06	0.68	0.16	0.06	0.51	0.89	0.89	0.12	0.003	1002	1002	283	737	0.12	0.0034	994	31	727
V41121112	0.7 15	5 3 1	600	8 8	3 0.6	2.5	0.88	0.16	0.06	0.68	0.16	0.06	0.47	0.89	0.89	0.13	0.004	926	926	261	682	0.13	0.0038	918	28	671
V51121111 V51121112	0.9 15	3	600 600	8 8	3 0.6	2.5	0.71	0.13	0.05	0.70	0.14	0.06	0.57	0.89	0.89	0.10	0.003	1050	1050	378	691	0.10	0.0031	1045	33	739 684
V11121121	0.1 15	5 3	600	8 8	3 1	1.5	0.90	0.15	0.05	0.64	0.19	0.07	0.46	0.89	0.89	0.14	0.004	883	883	36	858	0.14	0.0042	868	26	845
V11121122	0.1 19	5 3 1	600	8 8	3 1	2.5	0.99	0.16	0.06	0.64	0.19	0.07	0.43	0.89	0.89	0.16	0.005	815	815	33	793	0.16	0.0047	800	24	778
V21121121	0.3 15	5 3	600	8 8	3 1	1.5	0.84	0.14	0.05	0.65	0.18	0.07	0.50	0.89	0.89	0.13	0.004	947	947	115	847	0.13	0.0037	936	28	834
V21121122	0.3 15	3	600 600	88	s 1 z 1	2.5	0.92	0.16	0.06	0.65	0.18	0.07	0.46	0.89	0.89	0.14	0.004	871 1024	871	207	779 834	0.14	0.0041	1015	26	766
V31121121 V31121122	0.5 15	5 3 1	600 600	8 8	3 1	2.5	0.85	0.14	0.05	0.66	0.17	0.00	0.49	0.89	0.89	0.11	0.003	946	946	191	771	0.11	0.0033	936	29	759
V41121121	0.7 15	53	600	88	31	1.5	0.70	0.13	0.05	0.68	0.16	0.06	0.58	0.89	0.89	0.10	0.003	1134	1134	320	833	0.10	0.0029	1128	35	825
V41121122	0.7 15	5 3	600	8 8	3 1	2.5	0.77	0.14	0.05	0.68	0.16	0.06	0.53	0.89	0.89	0.11	0.003	1041	1041	294	766	0.11	0.0032	1034	32	756
V51121121	0.9 15	2 3	600 600	8 8	<u>5</u> 1 21	2.5	0.61	0.11	0.05	0.70	0.14	0.06	0.64	0.89	0.89	0.08	0.003	1278	1278	459	840	0.08	0.0026	1273	27	833
V11121211	0.1 15	5 3 1	600	8 10) 0.6	1.5	2.57	0.42	0.15	0.64	0.14	0.07	0.18	0.81	0.89	0.45	0.003	373	373	42.5	361	0.45	0.0136	344	10	334
V11121212	0.1 15	53	600	8 10	0.6	2.5	2.84	0.46	0.17	0.64	0.19	0.07	0.16	0.77	0.89	0.48	0.015	333	333	13	322	0.48	0.0146	302	9	294
V21121211	0.3 15	53	600	8 10) 0.6	1.5	2.39	0.41	0.15	0.65	0.18	0.07	0.20	0.82	0.89	0.42	0.013	403	403	48	364	0.42	0.0123	381	12	339
V21121212	0.3 15	5 3 1	600 600	8 10	0.6	2.5	2.64	0.45	0.16	0.65	0.18	0.07	0.18	0.78	0.89	0.45	0.014	358	358	42	324	0.45	0.0132	335	10	298
V31121211 V31121212	0.5 15	5 3 1	600 600	8 10) 0.6	2.5	2.20	0.39	0.15	0.66	0.17	0.06	0.25	0.85	0.89	0.39	0.011	397	397	90 79	329	0.39	0.0115	378	12	307
V41121211	0.7 15	5 3 1	600	8 10) 0.6	1.5	1.99	0.37	0.14	0.68	0.16	0.06	0.26	0.84	0.89	0.36	0.010	510	510	145	381	0.36	0.0104	497	15	364
V41121212	0.7 15	5 3 1	600	8 10) 0.6	2.5	2.19	0.41	0.16	0.68	0.16	0.06	0.23	0.82	0.89	0.38	0.011	452	452	128	339	0.38	0.0112	439	13	321
V51121211	0.9 15	3	600 600	8 10) 0.6	1.5	1.75	0.33	0.13	0.70	0.14	0.06	0.28	0.85	0.89	0.29	0.009	572	572	209	382	0.29	0.0093	563	18	369
V11121212	0.9 1	5 3 1	600 600	8 10) 0.8	1.5	2.48	0.30	0.13	0.70	0.14	0.08	0.20	0.84	0.89	0.55	0.010	388	388	194	376	0.55	0.0104	359	10	350
V11121222	0.1 1	5 3 1	600	8 10) 1	2.5	2.75	0.45	0.16	0.64	0.19	0.07	0.17	0.79	0.89	0.47	0.015	345	345	13	334	0.47	0.0142	315	9	307
V21121221	0.3 19	53	600	8 10) 1	1.5	2.31	0.39	0.14	0.65	0.18	0.07	0.21	0.83	0.89	0.41	0.012	420	420	50	380	0.41	0.0121	399	12	356
V21121222	0.3 15	5 3	600	8 10	$\frac{1}{1}$	2.5	2.55	0.44	0.16	0.65	0.18	0.07	0.18	0.79	0.89	0.44	0.013	372	372	44	337	0.44	0.0129	349	11	311
V31121221	0.5 1	5 3 1	600 600	8 10	$\frac{1}{1}$	2.5	2.35	0.38	0.14	0.66	0.17	0.00	0.24	0.85	0.89	0.38	0.011	408	408	83	342	0.38	0.0111	395	14	320
V41121221	0.7 15	53	600	8 10) 1	1.5	1.92	0.36	0.14	0.68	0.16	0.06	0.26	0.84	0.89	0.34	0.010	525	525	149	392	0.34	0.0100	513	16	375
V41121222	0.7 15	53	600	8 10) 1	2.5	2.12	0.39	0.15	0.68	0.16	0.06	0.24	0.83	0.89	0.37	0.011	471	471	134	353	0.37	0.0110	458	14	335
V51121221	0.9 15	5 3 1	600 600	8 10	$\frac{1}{1}$	1.5	1.69	0.32	0.13	0.70	0.14	0.06	0.29	0.86	0.89	0.28	0.009	588	588	215	392	0.28	0.0089	580	18	379
V111222111	0.9 15	5 3	600 1	0 8	, 1 3 0.6	1.5	1.51	0.55	0.14	0.64	0.14	0.00	0.27	0.88	0.89	0.52	0.010	617	617	25	503	0.52	0.0081	597	18	549
V11122112	0.1 15	5 3	600 1	0 8	3 0.6	2.5	1.86	0.30	0.11	0.64	0.19	0.07	0.27	0.86	0.89	0.35	0.011	529	529	21	514	0.35	0.0105	507	15	493
V21122111	0.3 15	5 3 1	600 1	.0 E	3 0.6	1.5	1.40	0.24	0.09	0.65	0.18	0.07	0.34	0.88	0.89	0.24	0.007	652	652	79	584	0.24	0.0071	636	19	567
V21122112	0.3 15	3	600 1	0 8	3 0.6	2.5	1.73	0.29	0.11	0.65	0.18	0.07	0.29	0.86	0.89	0.32	0.009	560	560	67	503	0.32	0.0092	542	16	483
V31122111 V31122112	0.5 15	5 3 1	600 1 600 1	08	3 0.6	2.5	1.59	0.25	0.09	0.66	0.17	0.06	0.31	0.87	0.89	0.22	0.008	601	601	141	493	0.22	0.0081	587	18	476
V41122111	0.7 15	5 3	600 1	.0 8	3 0.6	1.5	1.17	0.22	0.08	0.68	0.16	0.06	0.38	0.88	0.89	0.18	0.005	755	755	213	558	0.18	0.0054	745	23	545
V41122112	0.7 15	5 3 1	600 1	0 8	3 0.6	2.5	1.44	0.27	0.10	0.68	0.16	0.06	0.33	0.87	0.89	0.24	0.007	654	654	185	485	0.24	0.0071	643	20	471
V51122111	0.9 15	5 3 1	500 1	3 0	3 0.6	1.5	1.03	0.19	0.08	0.70	0.14	0.06	0.41	0.88	0.89	0.15	0.005	830	830	300	548	0.15	0.0047	823	26	539
V11122112	0.9 19	5 3 1	600 1 600 1	υ 8 0 8	s 0.6 3 1	2.5	1.37	0.24	0.10	0.70	0.14	0.06	0.36	0.88	0.89	0.20	0.006	661	661	263	481 642	0.20	0.0062	642	19	470
V11122122	0.1 15	5 3	600 1	0 8	3 1	2.5	1.78	0.29	0.10	0.64	0.19	0.07	0.28	0.86	0.89	0.33	0.010	546	546	22	531	0.33	0.0100	524	16	510
V21122121	0.3 15	5 3 1	600 1	0 8	3 1	1.5	1.27	0.22	0.08	0.65	0.18	0.07	0.36	0.88	0.89	0.22	0.006	697	697	84	624	0.21	0.0063	682	21	608
V21122122	0.3 15	3	600 1	3 0	3 1	2.5	1.66	0.28	0.10	0.65	0.18	0.07	0.30	0.87	0.89	0.30	0.009	578	578	70	519	0.30	0.0087	560	17	500
V31122121 V31122122	0.5 15	3	600 1	0 8	s 1 3 1	2.5	1.1/	0.21	0.08	0.66	0.17	0.06	0.38	0.88	0.89	0.19	0.005	/44 610	610	150	508	0.19	0.0055	732	19	593
V41122121	0.7 15	5 3	600 1	0 8	3 1	1.5	1.06	0.20	0.08	0.68	0.16	0.06	0.41	0.88	0.89	0.16	0.005	802	802	227	592	0.16	0.0047	793	24	580
V41122122	0.7 15	5 3	600 1	0.8	31	2.5	1.38	0.26	0.10	0.68	0.16	0.06	0.34	0.88	0.89	0.23	0.007	674	674	191	499	0.23	0.0067	663	20	485
V51122121	0.9 15	5 3	600 1	3 0	3 1	1.5	0.93	0.18	0.07	0.70	0.14	0.06	0.45	0.89	0.89	0.13	0.004	899	899	325	593	0.13	0.0042	893	28	585
V11122122 V11122211	0.9 15	5 3 1	600 1	0 10	s 1) 06	2.5	3.96	0.65	0.09	0.70	0.14	0.06	0.37	0.88	0.89	0.19	0.006	229	229	2/1	222	0.19	0.0059	200	23	484

Name	af	nf	hf mf	Ly L	x tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
1001100010	0.3	15	3 600	10 1	0 0		1.99	0.83	0.30	0.65	0.18	0.07	0.09	0.50	0.86	[m]	0.022	[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	NDR [-]	[MN-m]	[MN]	m]
V31122212	0.5	15	3 600		0 0.	5 1	5 3 39	0.83	0.30	0.05	0.10	0.07	0.08	0.50	0.80	0.73	0.022	267	267	53	224	0.73	0.0213	247	8	201
V31122211	0.5	15	3 600	10 1	0 0.1	5 2.	5 4.49	0.80	0.30	0.66	0.17	0.00	0.13	0.51	0.86	0.67	0.020	194	194	38	164	0.67	0.0193	176	5	143
V41122211	0.7	15	3 600	10 1	0 0.	6 1.5	5 3.06	0.57	0.22	0.68	0.16	0.06	0.15	0.68	0.88	0.49	0.014	300	300	86	229	0.48	0.0142	285	9	208
V41122212	0.7	15	3 600	10 1	0 0.	5 2.5	5 4.06	0.75	0.29	0.68	0.16	0.06	0.10	0.54	0.86	0.60	0.018	216	216	62	166	0.60	0.0177	202	6	148
V51122211	0.9	15	3 600	10 1	.0 0.	6 1.5	5 2.71	0.51	0.20	0.70	0.14	0.06	0.17	0.74	0.88	0.42	0.013	352	352	131	239	0.42	0.0134	341	11	. 223
V51122212	0.9	15	3 600	10 1	.0 0.	5 2.5	5 3.58	0.67	0.27	0.70	0.14	0.06	0.12	0.60	0.87	0.52	0.017	251	251	95	172	0.52	0.0165	240	7	157
V11122221	0.1	15	3 600	10 1	.0	1 1.5	5 3.89	0.63	0.23	0.64	0.19	0.07	0.11	0.62	0.88	0.61	0.019	234	234	9	226	0.61	0.0185	205	6	199
V11122222	0.1	15	3 600	10 1	0	1 2.5	5 5.25	0.85	0.31	0.64	0.19	0.07	0.08	0.49	0.86	0.77	0.024	167	167	6	161	0.77	0.0232	140	4	137
V21122221	0.3	15	3 600	10 1	.0	1 1.5	5 3.61	0.62	0.22	0.65	0.18	0.07	0.12	0.64	0.88	0.57	0.017	249	249	29	227	0.57	0.0167	226	7	201
V21122222	0.3	15	3 600	10 1	0	1 2.5	5 4.88 - 1.11	0.83	0.30	0.65	0.18	0.07	0.08	0.50	0.86	0.73	0.022	1/8	1/8	20	163	0.73	0.0213	157	5	140
V31122221	0.5	15	3 600		0	1 1.3	5 1.10	0.59	0.22	0.00	0.17	0.06	0.13	0.00	0.88	0.53	0.016	2/3	273	38	164	0.55	0.0155	234	5	1/13
V41122222	0.5	15	3 600		0	1 2	5 3.00	0.80	0.30	0.00	0.17	0.00	0.09	0.51	0.80	0.07	0.020	307	307	88	234	0.07	0.0193	292	9	2143
V41122222	0.7	15	3 600	10 1	0	1 2.5	5 4.06	0.75	0.29	0.68	0.16	0.06	0.10	0.54	0.86	0.60	0.018	216	216	62	166	0.60	0.0177	202	6	147
V51122221	0.9	15	3 600	10 1	.0	1 1.5	5 2.65	0.50	0.20	0.70	0.14	0.06	0.18	0.75	0.88	0.42	0.013	361	361	135	245	0.42	0.0132	349	11	. 229
V51122222	0.9	15	3 600	10 1	0	1 2.5	5 3.59	0.67	0.27	0.70	0.14	0.06	0.12	0.60	0.87	0.52	0.017	251	251	95	172	0.52	0.0165	240	7	157
V11211111	0.1	15	4 400	8	8 0.	5 1.5	5 0.79	0.13	0.05	0.64	0.19	0.07	0.52	0.89	0.89	0.12	0.003	873	873	35	849	0.12	0.0027	862	19	838
V11211112	0.1	15	4 400	8	8 0.	5 2.5	5 0.84	0.14	0.05	0.64	0.19	0.07	0.49	0.89	0.89	0.13	0.003	830	830	34	808	0.13	0.0029	818	18	796
V21211111	0.3	15	4 400	8	8 0.	5 1.5	5 0.74	0.13	0.05	0.65	0.18	0.07	0.55	0.89	0.89	0.11	0.002	934	934	114	835	0.11	0.0024	925	21	. 825
V21211112	0.3	15	4 400	8	8 0.1	5 2.	0.78	0.13	0.05	0.65	0.18	0.07	0.52	0.89	0.89	0.12	0.003	887	1017	108	793	0.12	0.0026	8/8	20	783
V31211111	0.5	15	4 400	8	8 0.	5 1.3 5 D I	0.68	0.12	0.05	0.66	0.17	0.06	0.59	0.89	0.89	0.10	0.002	1013	1013	205	824	0.10	0.0021	1006	23	816
V31211112	0.5	15	4 400	9	8 0.	5 1 9	5 0.72	0.15	0.05	0.00	0.17	0.00	0.50	0.89	0.89	0.11	0.002	1114	1114	314	219	0.11	0.0025	1100	22	
V41211111 V41211112	0.7	15	4 400	8	8 0.1	5 2.	5 0.65	0.12	0.04	0.68	0.16	0.00	0.61	0.89	0.89	0.09	0.002	1059	1059	299	778	0.08	0.0020	1054	20	771
V51211111	0.9	15	4 400	8	8 0.1	5 1.5	5 0.54	0.10	0.04	0.70	0.14	0.06	0.71	0.89	0.89	0.07	0.002	1254	1254	450	823	0.07	0.0017	1250	29	818
V51211112	0.9	15	4 400	8	8 0.	5 2.5	5 0.58	0.11	0.04	0.70	0.14	0.06	0.68	0.89	0.89	0.08	0.002	1198	1198	430	787	0.08	0.0018	1195	28	782
V11211121	0.1	15	4 400	8	8	1 1.5	5 0.65	0.11	0.04	0.64	0.19	0.07	0.61	0.89	0.89	0.10	0.002	1024	1024	42	996	0.10	0.0022	1014	23	987
V11211122	0.1	15	4 400	8	8	1 2.5	5 0.70	0.11	0.04	0.64	0.19	0.07	0.58	0.89	0.89	0.10	0.002	974	974	40	948	0.10	0.0024	964	22	938
V21211121	0.3	15	4 400	8	8	1 1.5	5 0.61	0.10	0.04	0.65	0.18	0.07	0.65	0.89	0.89	0.09	0.002	1096	1096	134	979	0.09	0.0019	1089	25	971
V21211122	0.3	15	4 400	8	8	1 2.5	5 0.65	0.11	0.04	0.65	0.18	0.07	0.61	0.89	0.89	0.09	0.002	1037	1037	126	927	0.09	0.0021	1029	23	918
V31211121	0.5	15	4 400	8	8	1 1.	5 0.56	0.10	0.04	0.66	0.17	0.06	0.69	0.89	0.89	0.08	0.002	1186	1186	240	964	0.08	0.0017	1180	27	956
V31211122	0.5	15	4 400	8	8	1 2.3	5 0.59	0.11	0.04	0.66	0.17	0.06	0.66	0.89	0.89	0.08	0.002	1127	127	228	916	0.08	0.0018	1120	20	908
V41211121	0.7	15	4 400	8	8	1 2 1	5 0.50	0.09	0.04	0.08	0.10	0.00	0.74	0.89	0.89	0.07	0.001	1237	1235	3/8	906	0.07	0.0015	1202	29	938
V51211122	0.9	15	4 400	8	8	1 1.	5 0.44	0.08	0.04	0.70	0.14	0.06	0.79	0.89	0.89	0.05	0.001	1392	1392	499	914	0.05	0.0012	1389	33	909
V51211122	0.9	15	4 400	1 8	8	1 2.5	5 0.48	0.09	0.04	0.70	0.14	0.06	0.76	0.89	0.89	0.06	0.001	1352	1352	485	887	0.06	0.0014	1348	32	883
V11211211	0.1	15	4 400	81	0 0.	6 1.5	5 1.88	0.31	0.11	0.64	0.19	0.07	0.27	0.86	0.89	0.35	0.008	467	467	19	454	0.35	0.0079	447	10	435
V11211212	0.1	15	4 400	81	0 0.	5 2.5	5 2.01	0.33	0.12	0.64	0.19	0.07	0.25	0.85	0.89	0.38	0.009	442	442	17	429	0.38	0.0086	421	10	410
V21211211	0.3	15	4 400	81	.0 0.1	6 1.5	5 1.75	0.30	0.11	0.65	0.18	0.07	0.28	0.86	0.89	0.32	0.007	494	494	59	444	0.32	0.0070	478	11	. 426
V21211212	0.3	15	4 400	81	.0 0.	5 2.5	5 1.87	0.32	0.12	0.65	0.18	0.07	0.27	0.85	0.89	0.35	0.008	469	469	56	422	0.35	0.0076	453	10	404
V31211211	0.5	15	4 400	8 1	0 0.1	5 1.5	5 1.60	0.29	0.11	0.66	0.17	0.06	0.30	0.87	0.89	0.28	0.006	530	530	107	435	0.28	0.0061	518	12	420
V31211212	0.5	15	4 400		0 0.	5 2.5	5 1.72	0.31	0.11	0.66	0.17	0.06	0.29	0.86	0.89	0.31	0.007	504	504	101	414	0.31	0.0067	491	11	. 398
V41211211	0.7	15	4 400	81	0 0.	6 J.	5 1.45	0.27	0.10	0.68	0.16	0.06	0.33	0.87	0.89	0.24	0.005	5/8	578	103	428	0.24	0.0054	508	13	415
V51211212	0.9	15	4 400	8 1	0 0.	5 1.	5 1.28	0.24	0.10	0.70	0.14	0.06	0.36	0.88	0.89	0.20	0.005	641	641	233	400	0.20	0.0047	635	15	415
V51211212	0.9	15	4 400	8 1	0 0.	5 2.5	5 1.37	0.26	0.10	0.70	0.14	0.06	0.34	0.88	0.89	0.22	0.005	611	611	222	406	0.22	0.0051	605	14	396
V11211221	0.1	15	4 400	8 1	.0	1 1.5	5 1.72	0.28	0.10	0.64	0.19	0.07	0.29	0.87	0.89	0.32	0.007	499	499	20	485	0.32	0.0072	480	11	. 467
V11211222	0.1	15	4 400	81	.0	1 2.5	5 1.84	0.30	0.11	0.64	0.19	0.07	0.27	0.86	0.89	0.34	0.008	474	474	19	460	0.34	0.0078	453	10	441
V21211221	0.3	15	4 400	81	.0	1 1.	5 1.60	0.27	0.10	0.65	0.18	0.07	0.31	0.87	0.89	0.29	0.006	528	528	64	474	0.29	0.0063	513	12	457
V21211222	0.3	15	4 400	81	.0	1 2.5	5 1.72	0.29	0.11	0.65	0.18	0.07	0.29	0.86	0.89	0.31	0.007	501	501	60	450	0.31	0.0068	485	11	. 433
V31211221	0.5	15	4 400	81	0	1 1.5	1.47	0.26	0.10	0.66	0.17	0.06	0.33	0.87	0.89	0.25	0.006	566	566	114	464	0.25	0.0055	554	13	449
V31211222	0.5	15	4 400	81	0	1 1	5 1.58	0.28	0.10	0.66	0.1/	0.06	0.31	0.8/	0.89	0.28	0.006	53/ 615	53/ 615	108	441	0.28	0.0060	525	12	425
V41211221	0.7	15	4 400	81	0	1 2 9	5 1 43	0.25	0.09	0.08	0.10	0.00	0.55	0.87	0.89	0.22	0.003	585	585	1/4	433	0.22	0.0048	575	14	445
V51211222	0.9	15	4 400	81	0	1 1.	5 1.17	0.22	0.09	0.70	0.14	0.06	0.38	0.88	0.89	0.18	0.004	681	681	247	450	0.18	0.0042	674	16	441
V51211222	0.9	15	4 400	81	0	1 2.5	5 1.26	0.24	0.09	0.70	0.14	0.06	0.36	0.88	0.89	0.19	0.005	649	649	235	430	0.19	0.0046	642	15	420
V11212111	0.1	15	4 400	10	8 0.	5 1.5	5 1.11	0.18	0.06	0.64	0.19	0.07	0.40	0.88	0.89	0.18	0.004	673	673	27	654	0.18	0.0041	658	15	641
V11212112	0.1	15	4 400	10	8 0.	5 2.5	5 1.26	0.21	0.07	0.64	0.19	0.07	0.36	0.88	0.89	0.21	0.005	620	620	25	603	0.21	0.0049	604	14	588
V21212111	0.3	15	4 400	10	8 0.	5 1.5	5 1.03	0.18	0.06	0.65	0.18	0.07	0.41	0.89	0.89	0.16	0.004	707	707	86	632	0.16	0.0036	695	16	620
V21212112	0.3	15	4 400	10	8 0.	5 2.5	5 1.17	0.20	0.07	0.65	0.18	0.07	0.38	0.88	0.89	0.19	0.004	654	654	79	586	0.19	0.0042	641	15	572
V31212111	0.5	15	4 400	10	8 0.1	5 1.5	5 0.95	0.17	0.06	0.66	0.17	0.06	0.44	0.89	0.89	0.14	0.003	764	764	154	623	0.14	0.0031	754	17	612
V31212112	0.5	15	4 400	10	8 0.	b 2.5	1.08	0.19	0.07	0.66	0.17	0.06	0.40	0.88	0.89	0.17	0.004	696	696	141	568	0.17	0.0037	686	16	556
V41212111	0.7	15	4 400	10	0 U.I 8 0 I	0 1.5 6 0 1	5 0.86	0.10	0.06	0.68	0.16	0.06	0.48	0.89	0.89	0.13	0.003	842	842	238	550	0.13	0.0028	835	19	511 ξλο
V51212112	0.7	15	4 400	10	8 0	5 1	5 0.76	0.14	0.06	0.08	0.14	0.00	0.54	0.89	0.89	0.10	0.003	952	952	343	627	0.10	0.0032	947	27	620
V51212112	0.9	15	4 400	10	8 0.	5 2.5	5 0.86	0.16	0.06	0.70	0.14	0.06	0.48	0.89	0.89	0.12	0.003	856	856	309	564	0.12	0.0029	851	20	557
V11212121	0.1	15	4 400	10	8	1 1.5	5 0.95	0.15	0.06	0.64	0.19	0.07	0.45	0.89	0.89	0.15	0.003	754	754	30	733	0.15	0.0033	741	17	721
V11212122	0.1	15	4 400	10	8	1 2.5	5 1.10	0.18	0.06	0.64	0.19	0.07	0.40	0.88	0.89	0.18	0.004	678	678	27	659	0.18	0.0040	663	15	645
V21212121	0.3	15	4 400	10	8	1 1.5	5 0.88	0.15	0.05	0.65	0.18	0.07	0.47	0.89	0.89	0.13	0.003	804	804	98	719	0.13	0.0029	793	18	707

Name	af n	f hf	mf	Ly	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	M <i>o</i> de 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN
V21212122	0.3 1	5 4	400	10	8	1	2.5	1.02	0.17	0.06	0.65	0.18	0.07	0.42	0.89	0.89	0.16	0.004	(MN-m) 711	(AR) 711	[MN•m] 86	[MN-m] 636	0.16	0.0035	(MN-m) 700	(MA) 16	m] 624
V31212121	0.5 1	5 4	400	10	8	1	1.5	0.81	0.14	0.05	0.66	0.17	0.06	0.51	0.89	0.89	0.12	0.003	875	875	177	712	0.12	0.0026	867	20	703
V31212122	0.5 1	54	400	10	8	1	2.5	0.94	0.17	0.06	0.66	0.17	0.06	0.45	0.89	0.89	0.14	0.003	771	771	156	629	0.14	0.0031	762	17	617
V41212121	0.7 1	54	400	10	8	1	1.5	0.73	0.14	0.05	0.68	0.16	0.06	0.56	0.89	0.89	0.10	0.002	965	965	272	710	0.10	0.0023	959	22	702
V41212122	0.7 1	54	400	10	8	1	2.5	0.85	0.16	0.06	0.68	0.16	0.06	0.49	0.89	0.89	0.12	0.003	1097	1097	240	626	0.12	0.0027	1083	19	617
V51212121	0.9 1	5 4 5 4	400	10	8	1	2.5	0.05	0.12	0.05	0.70	0.14	0.00	0.01	0.89	0.89	0.09	0.002	967	967	346	633	0.09	0.0020	957	23	626
V11212211	0.1 1	5 4	400	10	10	0.6	1.5	2.78	0.45	0.16	0.64	0.19	0.07	0.17	0.78	0.89	0.48	0.002	302	302	12	293	0.48	0.0108	275	6	268
V11212212	0.1 1	54	400	10	10	0.6	2.5	3.28	0.53	0.19	0.64	0.19	0.07	0.13	0.72	0.88	0.54	0.013	252	252	10	244	0.54	0.0122	224	5	218
V21212211	0.3 1	54	400	10	10	0.6	1.5	2.59	0.44	0.16	0.65	0.18	0.07	0.18	0.79	0.89	0.45	0.010	326	326	39	295	0.45	0.0098	305	7	272
V21212212	0.3 1	54	400	10	10	0.6	2.5	3.05	0.52	0.19	0.65	0.18	0.07	0.15	0.73	0.88	0.50	0.011	269	269	32	244	0.50	0.0110	248	6	221
V31212211	0.5 1	54	400	10	10	0.6	2.5	2.38	0.42	0.16	0.66	0.17	0.06	0.20	0.80	0.89	0.42	0.009	361	361	72	299	0.41	0.0090	270	8	279
V41212212	0.7 1	5 4	400	10	10	0.6	1.5	2.15	0.40	0.15	0.68	0.16	0.06	0.23	0.82	0.89	0.38	0.008	412	412	117	309	0.38	0.0083	400	9	292
V41212212	0.7 1	5 4	400	10	10	0.6	2.5	2.53	0.47	0.18	0.68	0.16	0.06	0.19	0.77	0.88	0.42	0.009	336	336	95	253	0.42	0.0093	323	7	236
V51212211	0.9 1	54	400	10	10	0.6	1.5	1.90	0.36	0.14	0.70	0.14	0.06	0.27	0.84	0.89	0.32	0.008	478	478	175	319	0.32	0.0077	469	11	307
V51212212	0.9 1	5 4	400	10	10	0.6	2.5	2.24	0.42	0.17	0.70	0.14	0.06	0.22	0.81	0.89	0.37	0.009	397	397	147	267	0.37	0.0088	388	9	254
V11212221	0.1 1	54	400	10	10	1	1.5	2.61	0.42	0.15	0.64	0.19	0.07	0.18	0.80	0.89	0.45	0.011	326	326	13	316	0.45	0.0103	300	/	292
V11212222	0.1 1	5 4	400	10	10	1	2.5	3.12	0.51	0.18	0.64	0.19	0.07	0.14	0.74	0.88	0.52	0.012	200	352	42	318	0.52	0.0117	238	2	232
V21212222	0.3 1	5 4	400	10	10	1	2.5	2.90	0.49	0.18	0.65	0.18	0.07	0.16	0.75	0.88	0.49	0.011	285	285	33	258	0.49	0.0106	263	6	235
V31212221	0.5 1	54	400	10	10	1	1.5	2.23	0.40	0.15	0.66	0.17	0.06	0.22	0.82	0.89	0.40	0.009	392	392	79	324	0.40	0.0086	376	9	305
V31212222	0.5 1	54	400	10	10	1	2.5	2.67	0.48	0.18	0.66	0.17	0.06	0.17	0.76	0.88	0.45	0.010	314	314	63	261	0.45	0.0097	297	7	241
V41212221	0.7 1	5 4	400	10	10	1	1.5	2.01	0.37	0.14	0.68	0.16	0.06	0.25	0.84	0.89	0.36	0.008	448	448	127	334	0.36	0.0079	436	10	319
V41212222	0.7 1	5 4	400	10	10	1	2.5	2.41	0.45	0.17	0.68	0.16	0.06	0.20	0.79	0.89	0.41	0.009	357	503	101	269	0.41	0.0090	344	12	251
V51212222	0.9 1	5 4	400	10	10	1	2.5	2.13	0.40	0.15	0.70	0.14	0.06	0.23	0.82	0.89	0.36	0.007	422	422	155	283	0.36	0.0085	413	10	270
V11221111	0.1 1	54	600	8	8	0.6	1.5	0.97	0.16	0.06	0.64	0.19	0.07	0.44	0.89	0.89	0.15	0.003	1106	1106	45	1076	0.15	0.0034	1086	25	1057
V11221112	$0.1\ 1$	54	600	8	8	0.6	2.5	1.03	0.17	0.06	0.64	0.19	0.07	0.41	0.89	0.89	0.16	0.004	1055	1055	43	1025	0.16	0.0037	1033	23	1006
V21221111	0.3 1	5 4	600	8	8	0.6	1.5	0.90	0.15	0.06	0.65	0.18	0.07	0.46	0.89	0.89	0.14	0.003	1180	1180	143	1055	0.14	0.0030	1164	26	1038
V21221112	0.3 1	5 4	600	8	8	0.6	2.5	0.96	0.16	0.06	0.65	0.18	0.07	0.44	0.89	0.89	0.15	0.003	1124	1124	136	1006	0.15	0.0033	1108	25	98/
V31221111 V31221112	0.5 1	5 4	600	8	8	0.6	2.5	0.83	0.15	0.00	0.66	0.17	0.00	0.30	0.89	0.89	0.12	0.003	1203	1215	233	990	0.12	0.0027	1202	27	974
V41221111	0.7 1	5 4	600	8	8	0.6	1.5	0.75	0.14	0.05	0.68	0.16	0.06	0.54	0.89	0.89	0.11	0.002	1413	1413	399	1039	0.11	0.0024	1404	32	1027
V41221112	0.7 1	54	600	8	8	0.6	2.5	0.80	0.15	0.06	0.68	0.16	0.06	0.52	0.89	0.89	0.12	0.003	1345	1345	380	990	0.12	0.0025	1336	31	977
V51221111	0.9 1	5 4	600	8	8	0.6	1.5	0.66	0.12	0.05	0.70	0.14	0.06	0.60	0.89	0.89	0.09	0.002	1600	1600	575	1052	0.09	0.0021	1594	37	1043
V51221112	0.9 1	5 4	600	8	8	0.6	2.5	0.70	0.13	0.05	0.70	0.14	0.06	0.57	0.89	0.89	0.10	0.002	1525	1525	548	1003	0.10	0.0023	1518	36	1351
V11221121	0.1 1	5 4	600	8	8	1	2.5	0.85	0.15	0.05	0.64	0.19	0.07	0.32	0.89	0.89	0.12	0.003	1234	1234	50	1207	0.12	0.0028	1200	29	1183
V21221121	0.3 1	5 4	600	8	8	1	1.5	0.74	0.13	0.05	0.65	0.18	0.07	0.55	0.89	0.89	0.11	0.002	1393	1393	169	1245	0.11	0.0024	1380	31	1230
V21221122	0.3 1	54	600	8	8	1	2.5	0.79	0.13	0.05	0.65	0.18	0.07	0.52	0.89	0.89	0.12	0.003	1319	1319	160	1179	0.12	0.0026	1305	30	1164
V31221121	0.5 1	54	600	8	8	1	1.5	0.68	0.12	0.05	0.66	0.17	0.06	0.59	0.89	0.89	0.10	0.002	1512	1512	306	1230	0.10	0.0021	1502	34	1217
V31221122	0.5 1	54	600	8	8	1	2.5	0.73	0.13	0.05	0.66	0.17	0.06	0.56	0.89	0.89	0.11	0.002	1432	1432	290	1166	0.11	0.0023	1421	32	1152
V41221121	0.7 1	5 4 5 4	600	8	8	1	2.5	0.62	0.11	0.04	0.68	0.16	0.06	0.64	0.89	0.89	0.09	0.002	1576	1576	469	1158	0.09	0.0019	1568	38	1210
V51221121	0.9 1	5 4	600	8	8	1	1.5	0.54	0.10	0.04	0.70	0.14	0.06	0.71	0.89	0.89	0.07	0.002	1873	1873	672	1230	0.07	0.0017	1867	44	1222
V51221122	0.9 1	54	600	8	8	1	2.5	0.58	0.11	0.04	0.70	0.14	0.06	0.67	0.89	0.89	0.08	0.002	1782	1782	640	1171	0.08	0.0018	1777	42	1163
V11221211	0.1 1	54	600	8	10	0.6	1.5	2.30	0.37	0.13	0.64	0.19	0.07	0.21	0.84	0.89	0.42	0.010	565	565	22	548	0.42	0.0094	529	12	515
V11221212	0.1 1	54	600	8	10	0.6	2.5	2.46	0.40	0.14	0.64	0.19	0.07	0.19	0.82	0.89	0.44	0.010	522	522	20	506	0.44	0.0099	484	11	471
V21221211 V21221212	0.3 1	5 4	600	8	10	0.6	2.5	2.14	0.30	0.13	0.65	0.18	0.07	0.23	0.84	0.89	0.39	0.009	566	566	67	511	0.39	0.0086	588	13	5Z4 479
V31221211	0.5 1	5 4	600	8	10	0.6	1.5	1.97	0.35	0.13	0.66	0.17	0.06	0.26	0.84	0.89	0.36	0.008	680	680	137	560	0.36	0.0078	659	15	534
V31221212	0.5 1	5 4	600	8	10	0.6	2.5	2.10	0.37	0.14	0.66	0.17	0.06	0.24	0.84	0.89	0.38	0.008	631	631	127	521	0.38	0.0083	608	14	493
V41221211	0.7 1	5 4	600	8	10	0.6	1.5	1.78	0.33	0.13	0.68	0.16	0.06	0.28	0.85	0.89	0.31	0.007	744	744	211	554	0.31	0.0069	728	17	532
V41221212	0.7 1	5 4	600	8	10	0.6	2.5	1.90	0.35	0.14	0.68	0.16	0.06	0.27	0.84	0.89	0.34	0.007	705	705	200	526	0.34	0.0075	688	16	504
V51221211 V51221212	0.9 1	54	600	8	10	0.0	2.5	1.57	0.29	0.12	0.70	0.14	0.06	0.31	0.86	0.89	0.20	0.000	831 789	789	288	526	0.20	0.0066	820 778	19	509
V11221221	0.1 1	5 4	600	8	10	1	1.5	2.10	0.34	0.12	0.64	0.19	0.07	0.24	0.85	0.89	0.39	0.009	628	628	25	610	0.39	0.0089	595	13	579
V11221222	0.1 1	5 4	600	8	10	1	2.5	2.26	0.37	0.13	0.64	0.19	0.07	0.22	0.84	0.89	0.41	0.009	577	577	23	560	0.41	0.0093	541	12	527
V21221221	0.3 1	5 4	600	8	10	1	1.5	1.96	0.33	0.12	0.65	0.18	0.07	0.26	0.85	0.89	0.36	0.008	678	678	81	610	0.36	0.0080	653	15	582
V21221222	0.3 1	54	600	8	10	1	2.5	2.10	0.36	0.13	0.65	0.18	0.07	0.24	0.84	0.89	0.39	0.009	629	629	75	566	0.39	0.0085	602	14	537
V31221221 V31221222	0.5 1	5 A	600 H	8	10	1	2.5	1.80	0.32	0.12	0.66	0.17	0.06	0.28	0.85	0.89	0.33	0.007	690	690	14/	568	0.33	0.0070	709	10	575
V41221221	0.7 1	5 4	600	8	10	1	1.5	1.63	0.30	0.12	0.68	0.16	0.06	0.30	0.86	0.89	0.28	0.006	796	796	225	591	0.28	0.0062	780	18	571
V41221222	0.7 1	54	600	8	10	1	2.5	1.75	0.32	0.12	0.68	0.16	0.06	0.28	0.85	0.89	0.31	0.007	754	754	213	561	0.31	0.0067	738	17	540
V51221221	0.9 1	54	600	8	10	1	1.5	1.44	0.27	0.11	0.70	0.14	0.06	0.33	0.87	0.89	0.23	0.005	887	887	323	589	0.23	0.0055	877	21	574
V51221222	0.9 1	5 4	600	8	10	1	2.5	1.54	0.29	0.12	0.70	0.14	0.06	0.31	0.87	0.89	0.25	0.006	842	842	307	560	0.25	0.0060	831	19	544
V11222111 V11222112	0.1 1	5 4 5 4	E 600	10	8	0.6	1.5	1.36	0.22	0.08	0.64	0.19	0.07	0.34	0.88	0.89	0.24	0.005	885	885	35	796	0.24	0.0054	860	19	837
V21222111	0.3 1	5 4	600	10	8	0.6	1.5	1.27	0.23	0.09	0.65	0.19	0.07	0.31	0.88	0.89	0.20	0.005	933	933	113	836	0.28	0.0047	913	21	814
V21222112	0.3 1	5 4	600	10	8	0.6	2.5	1.44	0.24	0.09	0.65	0.18	0.07	0.33	0.88	0.89	0.25	0.006	855	855	103	767	0.25	0.0055	834	19	743
V31222111	0.5 1	54	600	10	8	0.6	1.5	1.16	0.21	0.08	0.66	0.17	0.06	0.38	0.88	0.89	0.19	0.004	995	995	201	813	0.19	0.0041	980	22	794

Name	af	nf b	ıf mf	Lv	Lx	tŵ	Lch	T1 [s]	T2 [s]	T3 [s]	ML FT	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
				_,						[-]				100.000	(Cartoria)	00.000	[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V31222112	0.5	15	4 600) 10	8	0.6	2.5	1.32	0.24	0.09	0.66	0.17	0.06	0.35	0.88	0.89	0.22	0.005	915	915	185	749	0.22	0.0048	898	21	728
V41222111	0.7	15	4 600) 10	8	0.6	2.5	1.05	0.19	0.08	0.68	0.16	0.06	0.41	0.88	0.89	0.10	0.004	1074	1074	303	792	0.10	0.0035	1062	24	716
V51222112	0.9	15	4 600) 10	8	0.6	1.5	0.93	0.17	0.07	0.70	0.14	0.06	0.45	0.89	0.89	0.13	0.003	1205	1205	435	795	0.13	0.0031	1197	23	783
V51222112	0.9	15	4 600) 10	8	0.6	2.5	1.06	0.20	0.08	0.70	0.14	0.06	0.41	0.88	0.89	0.15	0.004	1091	1091	395	721	0.15	0.0036	1083	25	709
V11222121	0.1	15	4 600) 10	8	1	1.5	1.16	0.19	0.07	0.64	0.19	0.07	0.39	0.88	0.89	0.19	0.004	983	983	40	956	0.19	0.0043	961	22	935
V11222122	0.1	15	4 600) 10	8	1	2.5	1.35	0.22	0.08	0.64	0.19	0.07	0.35	0.88	0.89	0.23	0.005	892	892	36	866	0.23	0.0053	867	20	843
V21222121	0.3	15	4 600) 10	8	1	1.5	1.08	0.18	0.07	0.65	0.18	0.07	0.40	0.88	0.89	0.17	0.004	1034	1034	125	926	0.17	0.0038	1017	23	906
V21222122	0.3	15	4 600	10	8	1	2.5	1.25	0.21	0.08	0.65	0.18	0.07	0.37	0.88	0.89	0.21	0.005	1105	1105	223	84Z 001	0.21	0.0046	921	21	821
V31222121	0.5	15	4 600) 10	8	1	2.5	1.15	0.21	0.08	0.66	0.17	0.06	0.39	0.88	0.89	0.19	0.003	1002	1002	202	819	0.19	0.0033	986	23	800
V41222121	0.7	15	4 600) 10	8	1	1.5	0.90	0.17	0.06	0.68	0.16	0.06	0.47	0.89	0.89	0.13	0.003	1217	1217	343	896	0.13	0.0029	1206	28	882
V41222122	0.7	15	4 600) 10	8	1	2.5	1.04	0.19	0.07	0.68	0.16	0.06	0.41	0.88	0.89	0.16	0.003	1080	1080	305	797	0.16	0.0035	1069	25	782
V51222121	0.9	15	4 600) 10	8	1	1.5	0.79	0.15	0.06	0.70	0.14	0.06	0.52	0.89	0.89	0.11	0.003	1381	1381	497	910	0.11	0.0026	1374	32	899
V51222122	0.9	15	4 600) 10	10	1	2.5	0.92	0.17	0.07	0.70	0.14	0.06	0.46	0.89	0.89	0.13	0.003	1216	1216	439	802	0.13	0.0031	1208	28	790
V11222211 V11222212	0.1	15	4 600) 10	10	0.6	2.5	3.41	0.55	0.20	0.64	0.19	0.07	0.13	0.70	0.88	0.55	0.013	361	361	14	201	0.55	0.0125	320	/	312
V21222211	0.3	15	4 600) 10	10	0.6	1.5	3.17	0.54	0.20	0.65	0.19	0.07	0.14	0.71	0.88	0.52	0.013	386	386	45	351	0.52	0.0114	354	8	316
V21222212	0.3	15	4 600) 10	10	0.6	2.5	3.73	0.64	0.23	0.65	0.18	0.07	0.11	0.62	0.88	0.59	0.013	320	320	37	291	0.59	0.0129	289	7	258
V31222211	0.5	15	4 600) 10	10	0.6	1.5	2.91	0.52	0.19	0.66	0.17	0.06	0.16	0.73	0.88	0.48	0.011	425	425	85	355	0.48	0.0104	398	9	323
V31222212	0.5	15	4 600) 10	10	0.6	2.5	3.43	0.61	0.23	0.66	0.17	0.06	0.13	0.64	0.88	0.54	0.012	350	350	70	294	0.54	0.0117	325	7	263
V41222211	0.7	15	4 600) 10	10	0.6	1.5	2.63	0.49	0.19	0.68	0.16	0.06	0.18	0.75	0.88	0.44	0.010	480	480	137	363	0.43	0.0096	460	11	337
V41222212	0.7	15	4 600) 10	10	0.6	2.5	3.10	0.57	0.22	0.68	0.16	0.06	0.14	0.68	0.88	0.49	0.011	394	567	210	300	0.49	0.0108	553	13	2/3
V51222211	0.9	15	4 600) 10	10	0.6	2.5	2.33	0.51	0.21	0.70	0.14	0.06	0.17	0.73	0.88	0.43	0.000	462	462	173	314	0.43	0.0101	447	10	293
V11222221	0.1	15	4 600) 10	10	1	1.5	3.19	0.52	0.19	0.64	0.19	0.07	0.14	0.73	0.88	0.53	0.012	389	389	15	377	0.53	0.0119	348	8	338
V11222222	0.1	15	4 600) 10	10	1	2.5	3.82	0.62	0.22	0.64	0.19	0.07	0.11	0.63	0.88	0.61	0.014	318	318	12	307	0.60	0.0137	278	6	271
V21222221	0.3	15	4 600) 10	10	1	1.5	2.97	0.51	0.18	0.65	0.18	0.07	0.15	0.74	0.88	0.49	0.011	416	416	49	378	0.49	0.0108	384	9	343
V21222222	0.3	15	4 600) 10	10	1	2.5	3.56	0.61	0.22	0.65	0.18	0.07	0.12	0.65	0.88	0.57	0.013	338	338	39	308	0.57	0.0124	307	7	274
V31222221	0.5	15	4 600) 10	10	1	2.5	2.73	0.49	0.18	0.66	0.17	0.06	0.17	0.70	0.88	0.40	0.010	459	459	92	382	0.46	0.0099	2455	10	351
V41222221	0.7	15	4 600) 10	10	1	1.5	2.47	0.38	0.18	0.68	0.16	0.00	0.14	0.78	0.88	0.42	0.0012	521	521	148	393	0.32	0.0091	501	12	367
V41222222	0.7	15	4 600) 10	10	1	2.5	2.96	0.55	0.21	0.68	0.16	0.06	0.15	0.70	0.88	0.47	0.010	418	418	119	318	0.47	0.0104	398	9	291
V51222221	0.9	15	4 600) 10	10	1	1.5	2.18	0.41	0.16	0.70	0.14	0.06	0.23	0.81	0.89	0.36	0.009	616	616	227	414	0.36	0.0086	603	14	394
V51222222	0.9	15	4 600) 10	10	1	2.5	2.61	0.49	0.20	0.70	0.14	0.06	0.18	0.75	0.88	0.41	0.010	491	491	183	333	0.41	0.0098	476	11	311
V12111111	0.1	30	3 400) 8	8	0.6	1.5	1.01	0.17	0.06	0.63	0.19	0.07	0.42	0.89	0.89	0.16	0.002	2077	2077	82	2005	0.16	0.0025	2041	31	1967
V12111112	0.1	30 20	3 400	1 8	8	0.6	2.5	1.05	0.17	0.06	0.63	0.19	0.07	0.41	0.89	0.89	0.17	0.003	2034	2034	262	1963	0.17	0.0026	2104	30	1925
V22111111 V221111112	0.3	30	3 400) 8	8	0.6	2.5	0.93	0.10	0.00	0.64	0.18	0.00	0.43	0.89	0.89	0.15	0.002	2156	2156	203	1973	0.15	0.0022	2134	32	1941
V32111111	0.5	30	3 400) 8	8	0.6	1.5	0.87	0.16	0.06	0.65	0.17	0.06	0.48	0.89	0.89	0.13	0.002	2405	2405	474	1948	0.13	0.0019	2380	36	1918
V32111112	0.5	30	3 400) 8	8	0.6	2.5	0.90	0.16	0.06	0.65	0.17	0.06	0.46	0.89	0.89	0.14	0.002	2330	2330	459	1888	0.14	0.0020	2304	35	1857
V42111111	0.7	30	3 400) 8	8	0.6	1.5	0.79	0.15	0.06	0.66	0.16	0.06	0.52	0.89	0.89	0.12	0.002	2651	2651	730	1943	0.12	0.0017	2632	41	1918
V42111112	0.7	30	3 400) 8	8	0.6	2.5	0.82	0.15	0.06	0.66	0.16	0.06	0.50	0.89	0.89	0.12	0.002	2573	2573	708	1886	0.12	0.0018	2553	39	1860
V52111111 V521111112	0.9	30 30	3 400	18	8	0.6	2.5	0.70	0.13	0.05	0.69	0.14	0.06	0.58	0.89	0.89	0.10	0.002	2993	2993	1052	1967	0.10	0.0015	2980	4/	1949
V12111112	0.1	30	3 400	8	8	1	1.5	0.79	0.14	0.05	0.63	0.14	0.07	0.52	0.89	0.89	0.12	0.002	2558	2558	1013	2468	0.12	0.0010	2528	38	2437
V12111122	0.1	30	3 400) 8	8	1	2.5	0.82	0.13	0.05	0.63	0.19	0.07	0.50	0.89	0.89	0.13	0.002	2479	2479	98	2391	0.13	0.0019	2448	37	2359
V22111121	0.3	30	3 400) 8	8	1	1.5	0.74	0.13	0.05	0.64	0.18	0.06	0.55	0.89	0.89	0.11	0.002	2736	2736	325	2426	0.11	0.0016	2711	41	2398
V22111122	0.3	30	3 400) 8	8	1	2.5	0.77	0.13	0.05	0.64	0.18	0.06	0.53	0.89	0.89	0.12	0.002	2643	2643	313	2343	0.12	0.0017	2616	40	2314
V32111121	0.5	30	3 400	8	8	1	1.5	0.68	0.12	0.05	0.65	0.17	0.06	0.59	0.89	0.89	0.10	0.001	2964	2964	585	2397	0.10	0.0014	2943	45	2372
V32111122	0.5	50 30	3 400	1 8	8	1	2.5	0.71	0.13	0.05	0.65	0.17	0.06	0.57	0.89	0.89	0.11	0.002	28/2	28/2	567	2323	0.11	0.0015	2851	44	2297
V42111121	0.7	30	3 400) 8	8	1	2.5	0.62	0.11	0.04	0.66	0.16	0.06	0.64	0.89	0.89	0.09	0.001	3140	3140	864	2298	0.09	0.0013	3124	48	2557
V52111121	0.9	30	3 400) 8	8	1	1.5	0.55	0.10	0.04	0.69	0.14	0.06	0.70	0.89	0.89	0.07	0.001	3647	3647	1279	2393	0.07	0.0011	3636	57	2377
V52111122	0.9	30	3 400) 8	8	1	2.5	0.57	0.11	0.04	0.69	0.14	0.06	0.68	0.89	0.89	0.07	0.001	3543	3543	1242	2325	0.07	0.0012	3531	55	2309
V12111211	0.1	30	3 400) 8	10	0.6	1.5	2.30	0.38	0.13	0.63	0.19	0.07	0.21	0.84	0.89	0.43	0.007	1094	1094	42	1059	0.43	0.0064	1032	16	994
V12111212	0.1	30	3 400	8	10	0.6	2.5	2.40	0.39	0.14	0.63	0.19	0.07	0.20	0.83	0.89	0.44	0.007	1041	1041	40	1008	0.44	0.0066	976	15	941
V22111211	0.3	30	3 400	8	10	0.6	1.5	2.15	0.37	0.13	0.64	0.18	0.06	0.23	0.84	0.89	0.40	0.006	1195	1195	139	1069	0.40	0.0058	1143	1/	1011
V32111212	0.5	30	3 400	, 0) 8	10	0.6	1.5	1.98	0.35	0.14	0.65	0.18	0.06	0.22	0.85	0.89	0.41	0.000	1322	1322	259	1018	0.41	0.0053	1280	20	1032
V32111212	0.5	30	3 400) 8	10	0.6	2.5	2.06	0.37	0.14	0.65	0.17	0.06	0.25	0.84	0.89	0.38	0.006	1266	1266	248	1037	0.38	0.0055	1223	19	985
V42111211	0.7	30	3 400) 8	10	0.6	1.5	1.79	0.33	0.13	0.66	0.16	0.06	0.28	0.85	0.89	0.32	0.005	1444	1444	399	1071	0.32	0.0047	1412	22	1029
V42111212	0.7	30	3 400) 8	10	0.6	2.5	1.87	0.35	0.13	0.66	0.16	0.06	0.27	0.85	0.89	0.34	0.005	1398	1398	386	1038	0.34	0.0050	1365	21	995
V52111211	0.9	30	3 400	8	10	0.6	1.5	1.59	0.30	0.12	0.69	0.14	0.06	0.31	0.86	0.89	0.26	0.004	1608	1608	572	1068	0.26	0.0042	1585	25	1036
V52111212	0.9	30 20	3 400	18	10	0.6	2.5	1.66	0.31	0.12	0.69	0.14	0.06	0.30	0.86	0.89	0.28	0.004	1557	1557	555	1036	0.28	0.0054	1534	24	1003
V12111221	0.1	50 30	3 400) 8) 9	10	1	2.5	2.03	0.32	0.11	0.03	0.19	0.07	0.20	0.80	0.89	0.37	0.006	1325	1325	10	1281	0.37	0.0056	1271	19	1225
V22111221	0.3	30	3 400) 8	10	1	1.5	1.81	0.31	0.12	0.64	0.19	0.06	0.23	0.85	0.89	0.34	0.005	1408	1408	165	1256	0.34	0.0050	1362	21	1204
V22111222	0.3	30	3 400) 8	10	1	2.5	1.89	0.32	0.12	0.64	0.18	0.06	0.27	0.85	0.89	0.36	0.005	1361	1361	159	1215	0.36	0.0052	1314	20	1162
V32111221	0.5	30	3 400) 8	10	1	1.5	1.67	0.30	0.11	0.65	0.17	0.06	0.30	0.86	0.89	0.30	0.004	1509	1509	296	1232	0.30	0.0044	1471	23	1186
V32111222	0.5	30	3 400) 8	10	1	2.5	1.74	0.31	0.12	0.65	0.17	0.06	0.29	0.86	0.89	0.32	0.005	1460	1460	287	1193	0.32	0.0046	1422	22	1146
V42111221	0.7	30	3 400) 8	10	1	1.5	1.51	0.28	0.11	0.66	0.16	0.06	0.32	0.87	0.89	0.26	0.004	1640	1640	453	1213	0.26	0.0038	1611	25	1174

Name	af n	f h	f mf	Ly	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
1357 C 1946 B	229 - 223		4 873764		00.0040	22463	00000							(41.9) S	10210134	3639720	[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V42111222	0.7 3	0 3	3 400	8	10	1	2.5	1.58	0.29	0.11	0.66	0.16	0.06	0.31	0.86	0.89	0.28	0.004	1590	1590	439	1176	0.27	0.0040	1560	24	1137
V52111221	0.9 3	0 3	3 400	8	10	1	1.5	1.34	0.25	0.10	0.69	0.14	0.06	0.35	0.88	0.89	0.21	0.003	1819	1819	645	1205	0.21	0.0034	1799	28	1176
V52111222	0.9 3	0 3	3 400	8	10	1	2.5	1.40	0.26	0.11	0.69	0.14	0.06	0.34	0.87	0.89	0.22	0.004	1763	1763	626	1169	0.22	0.0036	1742	27	1139
V12112111	0.1 3	0 :	3 400	10	8	0.6	1.5	1.38	0.23	0.08	0.63	0.19	0.07	0.34	0.88	0.89	0.25	0.004	1/05	1/05	6/	164/	0.25	0.0037	1662	25	1601
V72112112	0.1 5	0:0	3 400	10	8	0.6	1.5	1.48	0.24	0.09	0.05	0.19	0.07	0.52	0.88	0.89	0.27	0.004	1803	1803	213	1604	0.27	0.0041	1765	24	1520
V22112111	0.3 3	0 3	3 400	10	8	0.6	2.5	1.38	0.22	0.09	0.64	0.18	0.06	0.34	0.88	0.89	0.22	0.003	1716	1716	202	1527	0.22	0.0032	1677	26	1483
V32112111	0.5 3	0 3	3 400	10	8	0.6	1.5	1.19	0.21	0.08	0.65	0.17	0.06	0.38	0.88	0.89	0.20	0.003	1922	1922	378	1562	0.20	0.0028	1891	29	1524
V32112112	0.5 3	0 3	3 400	10	8	0.6	2.5	1.27	0.23	0.08	0.65	0.17	0.06	0.36	0.88	0.89	0.22	0.003	1832	1832	361	1490	0.22	0.0031	1800	28	1451
V42112111	0.7 3	0 3	3 400	10	8	0.6	1.5	1.07	0.20	0.08	0.66	0.16	0.06	0.40	0.88	0.89	0.17	0.002	2070	2070	570	1523	0.17	0.0025	2047	32	1491
V42112112	0.7 3	0 3	3 400	10	8	0.6	2.5	1.15	0.21	0.08	0.66	0.16	0.06	0.39	0.88	0.89	0.18	0.003	1979	1979	545	1457	0.18	0.0027	1954	30	1424
V52112111	0.9 3	0 3	3 400	10	8	0.6	1.5	0.95	0.18	0.07	0.69	0.14	0.06	0.44	0.88	0.89	0.14	0.002	2303	2303	812	1519	0.14	0.0022	2287	36	1495
V52112112	0.9 3	0 :	3 400	10	8	0.6	2.5	1.02	0.19	0.08	0.69	0.14	0.06	0.42	0.88	0.89	0.15	0.002	2168	2168	/65	1431	0.15	0.0023	2151	34	1406
V12112121	0.1.3	0:	3 400	10	8	1	2.5	1.10	0.18	0.06	0.63	0.19	0.07	0.40	0.88	0.89	0.18	0.003	1978	1978	78	1909	0.18	0.0028	1940	29	1870
V22112122	0.1 3	0. 0.:	3 400	10	8	1	1.5	1.13	0.19	0.07	0.03	0.19	0.07	0.38	0.88	0.89	0.20	0.003	2078	2078	246	1846	0.20	0.0031	2045	31	1809
V22112121	0.3 3	0 3	3 400	10	8	1	2.5	1.11	0.19	0.07	0.64	0.18	0.06	0.40	0.88	0.89	0.18	0.002	1983	1983	234	1762	0.18	0.0027	1948	30	1723
V32112121	0.5 3	0 3	3 400	10	8	1	1.5	0.95	0.17	0.06	0.65	0.17	0.06	0.45	0.89	0.89	0.15	0.002	2246	2246	443	1821	0.15	0.0021	2219	34	1788
V32112122	0.5 3	0 3	3 400	10	8	1	2.5	1.02	0.18	0.07	0.65	0.17	0.06	0.42	0.88	0.89	0.16	0.002	2103	2103	414	1706	0.16	0.0023	2074	32	1672
V42112121	0.7 3	0 3	3 400	10	8	1	1.5	0.86	0.16	0.06	0.66	0.16	0.06	0.48	0.89	0.89	0.13	0.002	2471	2471	680	1812	0.13	0.0019	2450	38	1785
V42112122	0.7 3	0 3	3 400	10	8	1	2.5	0.93	0.17	0.07	0.66	0.16	0.06	0.45	0.89	0.89	0.14	0.002	2312	2312	637	1697	0.14	0.0021	2290	35	1669
V52112121	0.9 3	0 3	3 400	10	8	1	1.5	0.76	0.14	0.06	0.69	0.14	0.06	0.54	0.89	0.89	0.11	0.002	2779	2779	977	1828	0.11	0.0017	2765	43	1808
V52112122	0.9 3	0 3	3 400	10	10	1	2.5	0.82	0.15	0.06	0.69	0.14	0.06	0.50	0.89	0.89	0.12	0.002	2610	2610	918	1718	0.12	0.0018	2595	41	1697
V12112211	0.1.3	0 :	3 400	10	10	0.6	1.5	3.30	0.54	0.19	0.63	0.19	0.07	0.13	0.71	0.88	0.55	0.009	725	725	2/	637	0.55	0.0083	600	01	629
V22112212	0.1 5	0 : n :	3 400	10	10	0.6	1.5	3.07	0.59	0.21	0.05	0.19	0.07	0.12	0.07	0.88	0.59	0.009	787	787	24	706	0.59	0.0089	720	11	637
V22112212	0.3 3	0 3	3 400	10	10	0.6	2.5	3.36	0.57	0.21	0.64	0.18	0.06	0.13	0.68	0.88	0.55	0.008	706	706	80	638	0.55	0.0080	645	10	570
V32112211	0.5 3	0 3	3 400	10	10	0.6	1.5	2.83	0.50	0.19	0.65	0.17	0.06	0.16	0.74	0.88	0.48	0.007	860	860	167	713	0.48	0.0069	809	12	652
V32112212	0.5 3	0 3	3 400	10	10	0.6	2.5	3.09	0.55	0.21	0.65	0.17	0.06	0.15	0.70	0.88	0.51	0.007	774	774	150	644	0.51	0.0074	723	11	583
V42112211	0.7 3	0 3	3 400	10	10	0.6	1.5	2.56	0.47	0.18	0.66	0.16	0.06	0.18	0.76	0.88	0.43	0.006	971	971	269	730	0.43	0.0064	932	14	679
V42112212	0.7 3	0 3	3 400	10	10	0.6	2.5	2.80	0.52	0.20	0.66	0.16	0.06	0.16	0.73	0.88	0.46	0.007	872	872	242	658	0.46	0.0068	831	13	606
V52112211	0.9 3	0 3	3 400	10	10	0.6	1.5	2.27	0.43	0.17	0.69	0.14	0.06	0.22	0.80	0.89	0.38	0.006	1140	1140	411	766	0.38	0.0060	1112	17	727
V52112212	0.9 3	0:0	3 400	10	10	0.6	2.5	2.48	0.47	0.19	0.69	0.14	0.05	0.19	0.77	0.88	0.40	0.006	1020	1020	369	689	0.40	0.0054	990	10	548
V12112221	0.1 3	0 3	2 400	10	10	1	2.5	2.65	0.40	0.17	0.05	0.19	0.07	0.10	0.77	0.89	0.50	0.008	767	767	20	744	0.49	0.0074	695	11	670
V22112221	0.3 3	0 3	3 400	10	10	1	1.5	2.65	0.45	0.17	0.64	0.18	0.06	0.18	0.78	0.89	0.46	0.007	927	927	107	834	0.46	0.0067	868	13	768
V22112222	0.3 3	0 3	3 400	10	10	1	2.5	2.92	0.50	0.18	0.64	0.18	0.06	0.16	0.75	0.88	0.50	0.007	828	828	95	747	0.50	0.0072	767	12	678
V32112221	0.5 3	0 3	3 400	10	10	1	1.5	2.44	0.43	0.16	0.65	0.17	0.06	0.20	0.79	0.89	0.43	0.006	1026	1026	200	846	0.43	0.0062	978	15	788
V32112222	0.5 3	0 3	3 400	10	10	1	2.5	2.69	0.48	0.18	0.65	0.17	0.06	0.17	0.76	0.88	0.46	0.007	912	912	178	756	0.46	0.0066	862	13	695
V42112221	0.7 3	0 3	3 400	10	10	1	1.5	2.21	0.41	0.16	0.66	0.16	0.06	0.22	0.81	0.89	0.39	0.006	1166	1166	323	871	0.39	0.0057	1130	17	823
V42112222	0.7 3	0 3	3 400	10	10	1	2.5	2.44	0.45	0.17	0.66	0.16	0.06	0.20	0.78	0.89	0.42	0.006	1033	1033	286	775	0.42	0.0061	994	15	724
V52112221	0.9 3	0 :	3 400	10	10	1	1.5	1.96	0.37	0.15	0.69	0.14	0.06	0.26	0.84	0.89	0.34	0.005	1362	1362	48/	910	0.34	0.0054	1337	21	8/4
V52112222	0.9 3	0 :	3 400	10	10	0.6	2.5	1.24	0.41	0.10	0.69	0.14	0.00	0.23	0.82	0.89	0.37	0.000	2750	2750	437	2654	0.30	0.0058	2699	19	2501
V12121111	0.1 3	0 3	3 600	8	8	0.6	2.5	1.24	0.20	0.08	0.63	0.19	0.07	0.36	0.88	0.89	0.22	0.003	2683	2683	105	2590	0.21	0.0034	2620	40	2525
V22121111	0.3 3	0 3	3 600	8	8	0.6	1.5	1.16	0.20	0.07	0.64	0.18	0.06	0.39	0.88	0.89	0.19	0.003	2899	2899	342	2577	0.19	0.0028	2846	43	2517
V22121112	0.3 3	0 3	3 600	8	8	0.6	2.5	1.20	0.20	0.07	0.64	0.18	0.06	0.38	0.88	0.89	0.20	0.003	2832	2832	334	2517	0.20	0.0030	2778	42	2457
V32121111	0.5 3	0 3	3 600	8	8	0.6	1.5	1.07	0.19	0.07	0.65	0.17	0.06	0.41	0.88	0.89	0.17	0.002	3081	3081	607	2501	0.17	0.0024	3037	47	2448
V32121112	0.5 3	0 3	3 600	8	8	0.6	2.5	1.11	0.20	0.07	0.65	0.17	0.06	0.40	0.88	0.89	0.18	0.003	3011	3011	593	2445	0.18	0.0026	2967	45	2391
V42121111	0.7 3	0 3	3 600	8	8	0.6	1.5	0.97	0.18	0.07	0.66	0.16	0.06	0.44	0.88	0.89	0.15	0.002	3352	3352	923	2463	0.15	0.0021	3319	51	2418
V42121112	0.7 3	0	s 600	8	8	0.6	2.5	1.00	0.19	0.07	0.66	0.16	0.06	0.42	0.88	0.89	0.15	0.002	3243	3243	1220	2383	0.15	0.0022	3208	50	2338
V52121111	0.9 3	0 3	3 600	8	8	0.0	2.5	0.80	0.10	0.00	0.69	0.14	0.06	0.48	0.89	0.89	0.12	0.002	3649	3649	1330	2488	0.12	0.0019	3626	57	2455
V12121112	0.1 3	0 3	3 600	8	8	1	1.5	0.97	0.16	0.06	0.63	0.19	0.07	0.44	0.89	0.89	0.16	0.002	3240	3240	128	3126	0.16	0.0023	3187	48	3072
V12121122	0.1 3	0 3	3 600	8	8	1	2.5	1.01	0.16	0.06	0.63	0.19	0.07	0.42	0.89	0.89	0.16	0.002	3129	3129	123	3019	0.16	0.0024	3074	47	2963
V22121121	0.3 3	0 3	3 600	8	8	1	1.5	0.90	0.15	0.06	0.64	0.18	0.06	0.46	0.89	0.89	0.14	0.002	3458	3458	409	3069	0.14	0.0021	3413	52	3018
V22121122	0.3 3	0 3	3 600	8	8	1	2.5	0.94	0.16	0.06	0.64	0.18	0.06	0.45	0.89	0.89	0.15	0.002	3353	3353	397	2976	0.15	0.0022	3307	50	2924
V32121121	0.5 3	0 3	3 600	8	8	1	1.5	0.83	0.15	0.06	0.65	0.17	0.06	0.50	0.89	0.89	0.13	0.002	3754	3754	740	3040	0.13	0.0018	3718	57	2996
V32121122	0.5 3	0 3	3 600	8	8	1	2.5	0.87	0.15	0.06	0.65	0.17	0.06	0.48	0.89	0.89	0.13	0.002	3625	3625	715	2937	0.13	0.0019	3588	55	2891
V42121121	0.7 3	0 3	5 600	8	8	1	1.5	0.75	0.14	0.05	0.66	0.16	0.06	0.54	0.89	0.89	0.11	0.002	4121	4121	1134	3019	0.11	0.0016	4094	63	2983
V42121122	0.7 3	0 3	3 600	8	8 Q	1	2.5	0.78	0.15	0.06	0.00	0.10	0.06	0.52	0.89	0.89	0.12	0.002	4651	2994	1632	2927	0.11	0.001/	4622	01 72	2889
V52121122	0.9 3	0	3 600	8	8	1	2.5	0.70	0.13	0.05	0.69	0.14	0.06	0.58	0.89	0.89	0.10	0.002	4510	4510	1584	2964	0.10	0.0015	4491	71	2936
V12121211	0.1 3	0 3	3 600	8	10	0.6	1.5	2.82	0.46	0.17	0.63	0.19	0.07	0.16	0.78	0.89	0.49	0.008	1295	1295	49	1256	0.49	0.0074	1191	18	1148
V12121212	0.1 3	0 3	3 600	8	10	0.6	2.5	2.94	0.48	0.17	0.63	0.19	0.07	0.15	0.76	0.89	0.51	0.008	1236	1236	47	1198	0.51	0.0076	1130	17	1089
V22121211	0.3 3	0 3	3 600	8	10	0.6	1.5	2.63	0.45	0.16	0.64	0.18	0.06	0.18	0.78	0.89	0.46	0.007	1404	1404	162	1262	0.46	0.0067	1315	20	1163
V22121212	0.3 3	0 3	3 600	8	10	0.6	2.5	2.74	0.47	0.17	0.64	0.18	0.06	0.17	0.77	0.89	0.48	0.007	1338	1338	154	1204	0.47	0.0069	1248	19	1104
V32121211	0.5 3	0 3	3 600	8	10	0.6	1.5	2.42	0.43	0.16	0.65	0.17	0.06	0.20	0.80	0.89	0.43	0.006	1554	1554	303	1281	0.43	0.0062	1482	23	1194
V32121212	0.5 3	0 3	3 600	8	10	0.6	2.5	2.52	0.45	0.17	0.65	0.17	0.06	0.19	0.78	0.89	0.44	0.006	1477	1477	288	1220	0.44	0.0063	1403	21	1131
V42121211	0.7 3	0 3	3 600	8	10	0.6	1.5	2.19	0.41	0.16	0.66	0.16	0.06	0.23	0.82	0.89	0.39	0.006	1/6/	1676	489	1320	0.39	0.0057	1/13	26	1248
V52121212	0.7 3	0 3	3 600	8	10	0.6	1.5	1.95	0.42	0.15	0.60	0.10	0.06	0.21	0.80	0.89	0.40	0.005	2057	2057	736	1373	0.40	0.0053	2019	32	1320
and the second second second second	0.0 0	<u> </u>		-						0.20	0.00												0.00				2020

Name	af nf	hf	mf	Ly I	Lx I	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
						_							1012010.00	-			[IIII]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	PD4 [-]	[MN-m]	[MN]	m)
V52121212	0.9 30	3	600	8	10	0.6	2.5	2.03	0.38	0.15	0.69	0.14	0.06	0.25	0.83	0.89	0.35	0.006	1979	1979	709	1323	0.35	0.0055	1940	30	1269
V121212221	0.1 30	13	600	8	10	1	2.5	2.58	0.59	0.14	0.63	0.19	0.07	0.20	0.82	0.89	0.44	0.007	1499	1499	57	1451	0.44	0.0003	1399	22	1349
V22121221	0.3 30	3	600	8	10	1	1.5	2.22	0.38	0.14	0.64	0.18	0.06	0.22	0.83	0.89	0.41	0.006	1722	1722	200	1542	0.41	0.0060	1641	25	1451
V22121222	0.3 30) 3	600	8	10	1	2.5	2.32	0.40	0.14	0.64	0.18	0.06	0.21	0.82	0.89	0.42	0.006	1633	1633	190	1464	0.42	0.0061	1550	24	1371
V32121221	0.5 30	3	600	8 :	10	1	1.5	2.04	0.36	0.14	0.65	0.17	0.06	0.25	0.84	0.89	0.38	0.006	1917	1917	376	1571	0.38	0.0055	1853	28	1493
V32121222	0.5 30	3	600	8	10	1	2.5	2.13	0.38	0.14	0.65	0.17	0.06	0.23	0.83	0.89	0.39	0.006	1814	1814	355	1489	0.39	0.0056	1748	27	1408
V421212221	0.7 30	13	600	8.	10	1	2.5	1.85	0.34	0.13	0.66	0.16	0.06	0.27	0.85	0.89	0.33	0.005	2110	2110	563	1567	0.33	0.0049	1088	32	1502
V52121222	0.9 30	3	600	8 :	10	1	1.5	1.64	0.31	0.12	0.69	0.10	0.06	0.30	0.86	0.89	0.27	0.003	2349	2349	837	1562	0.27	0.0032	2315	36	1514
V52121222	0.9 30	3	600	8	10	1	2.5	1.72	0.32	0.13	0.69	0.14	0.06	0.29	0.85	0.89	0.29	0.005	2274	2274	811	1514	0.29	0.0046	2239	35	1464
V12122111	0.1 30) 3	600	10	8	0.6	1.5	1.69	0.28	0.10	0.63	0.19	0.07	0.29	0.87	0.89	0.32	0.005	2212	2212	86	2137	0.32	0.0048	2137	32	2060
V12122112	0.1 30	3	600	10	8	0.6	2.5	1.82	0.30	0.11	0.63	0.19	0.07	0.28	0.86	0.89	0.35	0.005	2097	2097	82	2026	0.35	0.0052	2019	31	1946
V22122111	0.3 30	13	600	10	8	0.6	1.5	1.58	0.27	0.10	0.64	0.18	0.06	0.31	0.87	0.89	0.29	0.004	2344	2344	2/6	2089	0.29	0.0042	2280	35	2017
V32122112	0.5 30	13	600	10	8	0.0	1.5	1.09	0.29	0.11	0.64	0.18	0.00	0.25	0.87	0.89	0.31	0.005	2223	2509	493	2044	0.31	0.0040	2157	38	1908
V32122112	0.5 30	3	600	10	8	0.6	2.5	1.56	0.28	0.10	0.65	0.17	0.06	0.31	0.87	0.89	0.28	0.004	2382	2382	468	1942	0.28	0.0040	2327	36	1876
V42122111	0.7 30	3	600	10	8	0.6	1.5	1.32	0.24	0.09	0.66	0.16	0.06	0.35	0.88	0.89	0.22	0.003	2721	2721	750	2007	0.22	0.0032	2680	41	1953
V42122112	0.7 30	3	600	10	8	0.6	2.5	1.41	0.26	0.10	0.66	0.16	0.06	0.34	0.87	0.89	0.24	0.004	2587	2587	713	1910	0.24	0.0035	2544	39	1854
V52122111	0.9 30	3	600	10	8	0.6	1.5	1.17	0.22	0.09	0.69	0.14	0.06	0.38	0.88	0.89	0.18	0.003	2999	2999	1061	1983	0.18	0.0028	2971	47	1942
V52122112	0.9 30	13	600	10	8	1.0	2.5	1.25	0.23	0.09	0.69	0.14	0.05	0.37	0.88	0.89	0.19	0.003	2859	2859	1012	2510	0.19	0.0031	2829	29	2442
V12122121	0.1 30	3	600	10	8	1	2.5	1.35	0.22	0.08	0.63	0.19	0.07	0.33	0.88	0.89	0.24	0.004	2000	2461	96	2376	0.24	0.0030	2393	36	2306
V22122121	0.3 30	3	600	10	8	1	1.5	1.26	0.21	0.08	0.64	0.18	0.06	0.36	0.88	0.89	0.22	0.003	2745	2745	324	2441	0.22	0.0031	2689	41	2378
V22122122	0.3 30	3	600	10	8	1	2.5	1.36	0.23	0.08	0.64	0.18	0.06	0.34	0.88	0.89	0.24	0.004	2602	2602	307	2315	0.24	0.0035	2544	39	2250
V32122121	0.5 30	3	600	10	8	1	1.5	1.16	0.21	0.08	0.65	0.17	0.06	0.39	0.88	0.89	0.19	0.003	2924	2924	576	2376	0.19	0.0027	2879	44	2320
V32122122	0.5 30	3	600	10	8	1	2.5	1.25	0.22	0.08	0.65	0.17	0.06	0.37	0.88	0.89	0.21	0.003	2776	2776	546	2257	0.21	0.0030	2729	42	2199
V42122121	0.7 30	13	600	10	8	1	2.5	1.05	0.19	0.08	0.66	0.16	0.06	0.41	0.88	0.89	0.10	0.002	3148	3148	807	2315	0.10	0.0024	2050	48	2208
V52122122	0.9 30	1 3	600	10	8	1	1.5	0.93	0.17	0.07	0.69	0.10	0.06	0.45	0.89	0.89	0.13	0.002	3520	3520	1241	2321	0.13	0.0021	3496	55	2286
V52122122	0.9 30	3	600	10	8	1	2.5	1.01	0.19	0.08	0.69	0.14	0.06	0.42	0.88	0.89	0.15	0.002	3286	3286	1160	2169	0.15	0.0023	3260	51	2132
V12122211	0.1 30	3	600	10	10	0.6	1.5	4.04	0.66	0.24	0.63	0.19	0.07	0.10	0.61	0.88	0.65	0.010	865	865	32	841	0.64	0.0097	764	12	737
V12122212	0.1 30	3	600	10 :	10	0.6	2.5	4.41	0.72	0.26	0.63	0.19	0.07	0.09	0.56	0.88	0.70	0.011	786	786	29	764	0.69	0.0104	687	10	662
V22122211	0.3 30	13	600	10 :	10	0.6	2.5	3.76	0.64	0.23	0.64	0.18	0.06	0.11	0.62	0.88	0.60	0.009	928	928	105	762	0.60	0.0088	841	13	/43
V32122212	0.5 30	3	600	10 :	10	0.6	1.5	3.46	0.62	0.23	0.65	0.10	0.06	0.13	0.64	0.88	0.56	0.008	1016	1016	197	848	0.56	0.0080	942	14	759
V32122212	0.5 30	3	600	10 :	10	0.6	2.5	3.78	0.67	0.25	0.65	0.17	0.06	0.11	0.59	0.88	0.60	0.009	917	917	177	767	0.59	0.0086	844	13	680
V42122211	0.7 30	3	600	10 :	10	0.6	1.5	3.14	0.58	0.22	0.66	0.16	0.06	0.14	0.67	0.88	0.50	0.007	1140	1140	317	865	0.50	0.0074	1080	17	787
V42122212	0.7 30	3	600	10 :	10	0.6	2.5	3.43	0.64	0.24	0.66	0.16	0.06	0.13	0.62	0.88	0.54	0.008	1026	1026	285	780	0.54	0.0079	967	15	705
V52122211	0.9 30	13	600	10 :	10	0.6	1.5	2.78	0.52	0.21	0.69	0.14	0.06	0.17	0.73	0.88	0.44	0.007	1329	1329	484	902	0.44	0.0069	1283	20	839
V121222212	0.1 30	3	600	10 1	10	1	1.5	3.48	0.57	0.20	0.63	0.14	0.00	0.13	0.68	0.88	0.58	0.007	1021	1021	430	991	0.40	0.0086	915	14	882
V12122222	0.1 30	3	600	10 :	10	1	2.5	3.84	0.63	0.22	0.63	0.19	0.07	0.11	0.63	0.88	0.62	0.010	915	915	34	889	0.62	0.0093	812	12	783
V22122221	0.3 30	3	600	10 :	10	1	1.5	3.25	0.55	0.20	0.64	0.18	0.06	0.14	0.70	0.88	0.54	0.008	1100	1100	126	994	0.54	0.0078	1008	15	892
V22122222	0.3 30	3	600	10 :	10	1	2.5	3.58	0.61	0.22	0.64	0.18	0.06	0.12	0.65	0.88	0.58	0.009	983	983	112	890	0.58	0.0084	894	14	791
V32122221	0.5 30	3	600	10	10	1	1.5	2.99	0.53	0.20	0.65	0.17	0.06	0.15	0.72	0.88	0.50	0.007	1208	1208	235	1004	0.50	0.0072	1131	17	912
V42122222	0.5 50	13	600	10 .	10	1	1.5	2.71	0.59	0.22	0.65	0.17	0.00	0.15	0.74	0.88	0.34	0.008	1362	1362	378	1027	0.35	0.0077	1302	20	948
V42122222	0.7 30	3	600	10 :	10	1	2.5	2.99	0.55	0.21	0.66	0.16	0.06	0.15	0.70	0.88	0.48	0.007	1211	1211	336	917	0.48	0.0071	1150	18	838
V52122221	0.9 30	3	600	10 :	10	1	1.5	2.40	0.45	0.18	0.69	0.14	0.06	0.20	0.78	0.88	0.39	0.006	1595	1595	576	1075	0.39	0.0062	1551	24	1014
V52122222	0.9 30	3	600	10 :	10	1	2.5	2.65	0.50	0.20	0.69	0.14	0.06	0.18	0.75	0.88	0.42	0.007	1413	1413	513	957	0.42	0.0067	1367	21	894
V12211111	0.1 30	4	400	8	8	0.6	1.5	0.98	0.16	0.06	0.63	0.19	0.07	0.43	0.89	0.89	0.16	0.002	2852	2852	113	2752	0.16	0.0018	2805	32	2703
V22211112	0.1 30	, 4) 4	400	8	8	0.0	2.5	0.91	0.16	0.06	0.63	0.19	0.07	0.42	0.89	0.89	0.16	0.002	2/84	2/84	361	2687	0.16	0.0018	3007	31	2637
V22211112	0.3 30	4	400	8	8	0.6	2.5	0.94	0.16	0.06	0.64	0.18	0.06	0.45	0.89	0.89	0.15	0.002	2983	2983	353	2648	0.15	0.0016	2942	34	2602
V32211111	0.5 30) 4	400	8	8	0.6	1.5	0.84	0.15	0.06	0.65	0.17	0.06	0.49	0.89	0.89	0.13	0.001	3305	3305	652	2677	0.13	0.0014	3272	38	2637
V32211112	0.5 30) 4	400	8	8	0.6	2.5	0.86	0.15	0.06	0.65	0.17	0.06	0.48	0.89	0.89	0.13	0.001	3226	3226	636	2614	0.13	0.0014	3193	37	2573
V42211111	0.7 30	4	400	8	8	0.6	1.5	0.76	0.14	0.05	0.66	0.16	0.06	0.53	0.89	0.89	0.11	0.001	3626	3626	998	2657	0.11	0.0012	3601	42	2624
V42211112	0.7 30	4	400	8	8	0.6	2.5	0.78	0.15	0.06	0.66	0.16	0.06	0.52	0.89	0.89	0.11	0.001	3553	3553	9/8	2604	0.11	0.0013	3528	41	2571
V52211112	0.9 30	4	400	8	8	0.6	2.5	0.69	0.13	0.05	0.69	0.14	0.06	0.59	0.89	0.89	0.10	0.001	4013	4013	1410	2637	0.10	0.0011	3996	48	2613
V12211121	0.1 30) 4	400	8	8	1	1.5	0.75	0.12	0.04	0.63	0.19	0.07	0.54	0.89	0.89	0.12	0.001	3579	3579	142	3452	0.12	0.0013	3541	40	3412
V12211122	0.1 30) 4	400	8	8	1	2.5	0.77	0.13	0.05	0.63	0.19	0.07	0.53	0.89	0.89	0.12	0.001	3493	3493	138	3369	0.12	0.0013	3454	39	3329
V22211121	0.3 30) 4	400	8	8	1	1.5	0.70	0.12	0.04	0.64	0.18	0.06	0.58	0.89	0.89	0.11	0.001	3837	3837	455	3401	0.11	0.0011	3805	43	3365
V22211122	0.3 30	4	400	8	8	1	2.5	0.72	0.12	0.04	0.64	0.18	0.06	0.57	0.89	0.89	0.11	0.001	3749	3749	445	3324	0.11	0.0012	3716	42	3287
V32211121	0.5 30	4	400	8	8	1	2.5	0.04	0.11	0.04	0.05	0.17	0.06	0.62	0.89	0.89	0.09	0.001	4127	4127	815 708	3271	0.09	0.0010	4101	4/	3305
V42211121	0.7 30) 4	400	8	8	1	1.5	0.58	0.12	0.04	0.66	0.16	0.06	0.67	0.89	0.89	0.08	0.001	4553	4553	1252	3329	0.08	0.0009	4533	53	3303
V42211122	0.7 30) 4	400	8	8	1	2.5	0.60	0.11	0.04	0.66	0.16	0.06	0.66	0.89	0.89	0.08	0.001	4451	4451	1224	3255	0.08	0.0009	4430	51	3228
V52211121	0.9 30	4	400	8	8	1	1.5	0.52	0.10	0.04	0.69	0.14	0.06	0.73	0.89	0.89	0.07	0.001	5061	5061	1774	3320	0.07	0.0008	5047	59	3300
V52211122	0.9 30) 4	400	8	8	1	2.5	0.53	0.10	0.04	0.69	0.14	0.06	0.72	0.89	0.89	0.07	0.001	4969	4969	1742	3260	0.07	0.0008	4954	58	3240
v12211211	0.1 30	4	400	8	10	U.6	1.5	2.18	0.36	0.13	0.63	0.19	0.07	0.23	0.84	0.89	0.41	0.005	1556	1556	60	1505	0.41	0.0046	1477	17	1423

Name	af nf	hf n	մե	v Lx	tw	Lcb	T1 [s]	[2 [s]	T3 [s]	M1 (-) -)	M2 F1 1	M3 (-)	Sa	Sa2	Sa3	Disp	ISDR [-]	Moment	Shear	Moment	Moment ::	1 Disp	Mode 1	Moment	Shear N	Mode 1 Wall Aoment [MN+
																[m]		[MN-m]	[MIN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V12211212	0.1 30	4 4	100	8 10	0.6	2.5	2.25	0.37	0.13	0.63	0.19	0.07	0.22	0.84	0.89	0.42	0.005	1501	1501	58	1452	0.42	0.0047	1419	16	1368
V22211211	0.3 30	4 4	100	8 10	0.6	2.5	2.03	0.35	0.13	0.64	0.18	0.06	0.25	0.85	0.89	0.39	0.004	1643	1643	199	1/69	0.39	0.0042	1038	19	1449
V32211212	0.5 30	4 4	100	8 10	0.6	1.5	1.87	0.33	0.12	0.65	0.17	0.06	0.24	0.85	0.89	0.35	0.004	1842	1842	361	1506	0.35	0.0038	1788	21	1441
V32211212	0.5 30	4 4	100	8 10	0.6	2.5	1.93	0.34	0.13	0.65	0.17	0.06	0.26	0.85	0.89	0.36	0.004	1798	1798	353	1472	0.36	0.0039	1744	20	1405
V42211211	0.7 30	4 4	100	8 10	0.6	1.5	1.70	0.31	0.12	0.66	0.16	0.06	0.29	0.86	0.89	0.30	0.003	2007	2007	554	1487	0.30	0.0033	1966	23	1432
V42211212	0.7 30	4 4	100	8 10	0.6	2.5	1.75	0.32	0.12	0.66	0.16	0.06	0.28	0.85	0.89	0.31	0.003	1961	1961	542	1454	0.31	0.0034	1919	22	1398
V52211211	0.9 30	4 4	100	8 10	0.6	1.5	1.51	0.28	0.11	0.69	0.14	0.06	0.32	0.87	0.89	0.25	0.003	2231	2231	793	1481	0.25	0.0029	2201	26	1439
V52211212	0.9 30	4 4	100	8 10	0.6	2.5	1.55	0.29	0.12	0.69	0.14	0.06	0.31	0.86	0.89	0.26	0.003	2183	2183	776	1450	0.26	0.0030	2153	25	1408
V12211221	0.1.30	4 4	100	8 10	1	2.5	1.75	0.29	0.10	0.63	0.19	0.07	0.28	0.87	0.89	0.33	0.004	1906	1906	74	1841	0.33	0.0037	1838	21	1770
V12211222	0.1 30	4 4	100	8 10	1	1.5	1.62	0.30	0.11	0.63	0.19	0.07	0.28	0.80	0.89	0.30	0.004	2020	2020	237	1801	0.30	0.0033	1962	20	1725
V22211222	0.3 30	4 4	100	8 10	1	2.5	1.69	0.29	0.11	0.64	0.18	0.06	0.29	0.87	0.89	0.31	0.003	1976	1976	232	1761	0.31	0.0034	1917	22	1695
V32211221	0.5 30	4 4	100	8 10	1	1.5	1.51	0.27	0.10	0.65	0.17	0.06	0.32	0.87	0.89	0.27	0.003	2164	2164	425	1763	0.27	0.0029	2116	24	1705
V32211222	0.5 30	4 4	100	8 10	1	2.5	1.56	0.28	0.10	0.65	0.17	0.06	0.31	0.87	0.89	0.28	0.003	2117	2117	416	1726	0.28	0.0030	2068	24	1667
V42211221	0.7 30	4 4	100	8 10	1	1.5	1.37	0.25	0.10	0.66	0.16	0.06	0.34	0.88	0.89	0.23	0.003	2349	2349	648	1734	0.23	0.0025	2312	27	1685
V42211222	0.7 30	4 4	100	8 10	1	2.5	1.41	0.26	0.10	0.66	0.16	0.06	0.34	0.87	0.89	0.24	0.003	2299	2299	634	1697	0.24	0.0026	2261	26	1647
V52211221	0.9 30	4 4	100	8 10	1	1.5	1.22	0.23	0.09	0.69	0.14	0.06	0.37	0.88	0.89	0.19	0.002	2596	2596	919	1/1/	0.19	0.0022	2570	30	1680
V52211222	0.9 30	4 4	100	8 10	0.6	2.5	1.25	0.24	0.09	0.69	0.14	0.06	0.37	0.88	0.89	0.19	0.002	2341	2341	900	2268	0.19	0.0023	2514	30	2209
V12212111	0.1 30	4 4	100	10 8	0.6	2.5	1.32	0.22	0.08	0.63	0.19	0.07	0.34	0.88	0.89	0.25	0.003	2268	2268	89	2190	0.25	0.0028	2210	25	2130
V22212111	0.3 30	4 4	100	10 8	0.6	1.5	1.23	0.21	0.08	0.64	0.18	0.06	0.37	0.88	0.89	0.21	0.002	2480	2480	293	2205	0.21	0.0023	2431	28	2150
V22212112	0.3 30	4 4	100	10 8	0.6	2.5	1.29	0.22	0.08	0.64	0.18	0.06	0.36	0.88	0.89	0.22	0.002	2399	2399	283	2133	0.22	0.0024	2348	27	2077
V32212111	0.5 30	4 4	100	10 8	0.6	1.5	1.13	0.20	0.08	0.65	0.17	0.06	0.39	0.88	0.89	0.18	0.002	2638	2638	519	2142	0.18	0.0020	2598	30	2093
V32212112	0.5 30	4 4	100	10 8	0.6	2.5	1.19	0.21	0.08	0.65	0.17	0.06	0.38	0.88	0.89	0.20	0.002	2557	2557	503	2078	0.20	0.0021	2516	29	2028
V42212111	0.7 30	4 4	100	10 8	0.6	1.5	1.03	0.19	0.07	0.66	0.16	0.06	0.42	0.88	0.89	0.16	0.002	2836	2836	781	2085	0.16	0.0017	2805	33	2044
V42212112	0.7 30	4 4	100	10 8	0.6	2.5	1.08	0.20	0.08	0.60	0.16	0.06	0.40	0.88	0.89	0.17	0.002	2755	2/55	1122	2026	0.17	0.0019	2123	32	2071
V52212111	0.9 30	4 4	100	10 8	0.0	2.5	0.91	0.17	0.07	0.69	0.14	0.06	0.40	0.89	0.89	0.13	0.002	3062	3062	1080	2019	0.13	0.0015	3040	36	1988
V12212121	0.1 30	4 4	100	10 8	1	1.5	1.02	0.17	0.06	0.63	0.19	0.07	0.42	0.89	0.89	0.16	0.002	2761	2761	109	2665	0.16	0.0019	2712	31	2614
V12212122	0.1 30	4 4	100	10 8	1	2.5	1.08	0.18	0.06	0.63	0.19	0.07	0.40	0.89	0.89	0.18	0.002	2678	2678	105	2584	0.18	0.0020	2628	30	2532
V22212121	0.3 30	4 4	100	10 8	1	1.5	0.95	0.16	0.06	0.64	0.18	0.06	0.44	0.89	0.89	0.15	0.002	2953	2953	350	2622	0.15	0.0016	2912	33	2575
V22212122	0.3 30	4 4	100	10 8	1	2.5	1.00	0.17	0.06	0.64	0.18	0.06	0.42	0.89	0.89	0.16	0.002	2819	2819	333	2504	0.16	0.0017	2776	32	2455
V32212121	0.5 30	4 4	100	10 8	1	1.5	0.88	0.16	0.06	0.65	0.17	0.06	0.48	0.89	0.89	0.13	0.001	3189	3189	629	2584	0.13	0.0014	3156	36	2543
V32212122	0.5 30	4 4	100	10 8	1	2.5	0.92	0.15	0.06	0.65	0.17	0.06	0.45	0.89	0.89	0.14	0.002	3054	2510	602	24/6	0.14	0.0015	3019	35	2433
V42212121 V42212122	0.7 30	4 4	100	10 8	1	2.5	0.75	0.15	0.00	0.00	0.10	0.00	0.52	0.89	0.89	0.12	0.001	3367	3367	903	2380	0.12	0.0013	3494	30	2340
V52212121	0.9 30	4 4	100	10 8	1	1.5	0.70	0.13	0.05	0.69	0.14	0.06	0.57	0.89	0.89	0.10	0.001	3973	3973	1396	2611	0.10	0.0011	3955	47	2586
V52212122	0.9 30	4 4	100	10 8	1	2.5	0.74	0.14	0.06	0.69	0.14	0.06	0.55	0.89	0.89	0.10	0.001	3792	3792	1333	2493	0.10	0.0012	3774	44	2468
V12212211	0.1 30	4 4	100	10 10	0.6	1.5	3.07	0.50	0.18	0.63	0.19	0.07	0.15	0.74	0.88	0.52	0.006	1046	1046	39	1015	0.52	0.0059	950	11	916
V12212212	0.1 30	4 4	100	10 10	0.6	2.5	3.27	0.53	0.19	0.63	0.19	0.07	0.14	0.72	0.88	0.55	0.006	976	976	36	947	0.55	0.0062	880	10	849
V22212211	0.3 30	4 4	100	10 10	0.6	1.5	2.86	0.49	0.18	0.64	0.18	0.06	0.16	0.75	0.88	0.49	0.005	1130	1130	130	1018	0.49	0.0054	1049	12	928
V22212212	0.5 20	4 4	100	10 10	0.6	2.5	3.04	0.52	0.19	0.65	0.18	0.06	0.15	0.73	0.88	0.51	0.005	1054	1054	242	1020	0.51	0.0056	972	11	859
V32212211 V32212212	0.5 30	4 4	100	10 10	0.0	2.5	2.80	0.47	0.10	0.65	0.17	0.00	0.16	0.75	0.89	0.45	0.005	1159	1159	243	961	0.43	0.0049	1091	13	880
V42212211	0.7 30	4 4	100	10 10	0.6	1.5	2.39	0.44	0.17	0.66	0.16	0.06	0.20	0.79	0.89	0.41	0.005	1410	1410	391	1057	0.41	0.0045	1359	16	991
V42212212	0.7 30	4 4	100	10 10	0.6	2.5	2.54	0.47	0.18	0.66	0.16	0.06	0.19	0.77	0.88	0.43	0.005	1309	1309	363	984	0.43	0.0047	1257	15	916
V52212211	0.9 30	4 4	100	10 10	0.6	1.5	2.12	0.40	0.16	0.69	0.14	0.06	0.24	0.82	0.89	0.36	0.004	1663	1663	597	1114	0.36	0.0043	1627	19	1064
V52212212	0.9 30	4 4	100	10 10	0.6	2.5	2.25	0.42	0.17	0.69	0.14	0.06	0.22	0.80	0.89	0.38	0.004	1538	1538	554	1034	0.37	0.0045	1501	18	981
V12212221	0.1 30	4 4	100	10 10	1	1.5	2.53	0.41	0.15	0.63	0.19	0.07	0.19	0.81	0.89	0.45	0.005	1309	1309	50	1268	0.45	0.0051	1220	14	1176
V22212222	0.1 30	4 4	100	10 10	1	2.5	2.70	0.44	0.10	0.03	0.19	0.07	0.1/	0.79	0.89	0.48	0.005	1/14	1/14	40	1270	0.48	0.0054	1252	13	1082
V22212222	0.3 30	4 4	100	10 10	1	2.5	2.55	0.43	0.15	0.64	0.18	0.06	0.19	0.80	0.89	0.45	0.005	1317	1317	157	1183	0.45	0.0049	1240	14	1097
V32212221	0.5 30	4 4	100	10 10	1	1.5	2.17	0.39	0.14	0.65	0.17	0.06	0.23	0.83	0.89	0.40	0.004	1585	1585	310	1302	0.40	0.0043	1525	18	1229
V32212222	0.5 30	4 4	100	10 10	1	2.5	2.31	0.41	0.15	0.65	0.17	0.06	0.21	0.81	0.89	0.41	0.005	1461	1461	286	1203	0.41	0.0045	1399	16	1127
V42212221	0.7 30	4 4	100	10 10	1	1.5	1.96	0.36	0.14	0.66	0.16	0.06	0.26	0.84	0.89	0.36	0.004	1789	1789	494	1330	0.36	0.0039	1744	20	1271
V42212222	0.7 30	4 4	100	10 10	1	2.5	2.10	0.39	0.15	0.66	0.16	0.06	0.24	0.83	0.89	0.38	0.004	1665	1665	461	1241	0.38	0.0042	1618	19	1179
V52212221	0.9 30	4 4	100	10 10	1	1.5	1.74	0.33	0.13	0.69	0.14	0.06	0.29	0.85	0.89	0.29	0.003	1998	1998	712	1331	0.29	0.0035	1966	23	1286
V12221111	0.9 30	4 4	500	8 8	0.6	1.5	1.20	0.55	0.14	0.69	0.14	0.00	0.27	0.88	0.89	0.52	0.004	3749	3749	147	3619	0.52	0.0058	3668	42	3536
V12221112	0.1 30	4 6	500	8 8	0.6	2.5	1.23	0.20	0.07	0.63	0.19	0.07	0.37	0.88	0.89	0.21	0.002	3683	3683	145	3555	0.21	0.0024	3601	41	3471
V22221111	0.3 30	4 (500	8 8	0.6	1.5	1.12	0.19	0.07	0.64	0.18	0.06	0.39	0.88	0.89	0.18	0.002	3948	3948	466	3508	0.18	0.0020	3879	44	3430
V22221112	0.3 30	4 6	500	8 8	0.6	2.5	1.15	0.20	0.07	0.64	0.18	0.06	0.39	0.88	0.89	0.19	0.002	3881	3881	458	3450	0.19	0.0021	3811	43	3371
V32221111	0.5 30	4 (500	8 8	0.6	1.5	1.03	0.18	0.07	0.65	0.17	0.06	0.41	0.88	0.89	0.16	0.002	4189	4189	825	3399	0.16	0.0018	4132	47	3329
V32221112	0.5 30	4 6	500	8 8	0.6	2.5	1.06	0.19	0.07	0.65	0.17	0.06	0.41	0.88	0.89	0.17	0.002	4124	4124	812	3348	0.17	0.0018	4066	47	3277
V42221111	0.7 30	4 (500	8 8	0.6	1.5	0.93	0.1/	0.07	0.66	0.16	0.06	0.45	0.89	0.89	0.14	0.002	4597	4597	1266	33/6	0.14	0.0016	4554	53	3318
V52221112	0.7 30	4 0	500	8 8	0.6	2.5	0.90	0.18	0.06	0.69	0.10	0.06	0.44	0.88	0.89	0.14	0.002	4490 5180	5180	1238	3416	0.14	0.0016	4451 5150	52	3243
V52221112	0.9 30	4 6	500	8 8	0.6	2.5	0.85	0.16	0.06	0.69	0.14	0.06	0.49	0.89	0.89	0.12	0.001	5068	5068	1784	3337	0.12	0.0014	5038	59	3294
V12221121	0.1 30	4 (500	8 8	1	1.5	0.92	0.15	0.05	0.63	0.19	0.07	0.46	0.89	0.89	0.15	0.002	4532	4532	179	4373	0.15	0.0016	4465	51	4303
V12221122	0.1 30	4 6	500	8 8	1	2.5	0.94	0.15	0.06	0.63	0.19	0.07	0.45	0.89	0.89	0.15	0.002	4433	4433	175	4278	0.15	0.0017	4365	50	4207
V22221121	0.3 30	4 6	500	8 8	1	1.5	0.85	0.15	0.05	0.64	0.18	0.06	0.49	0.89	0.89	0.13	0.001	4852	4852	575	4305	0.13	0.0014	4796	55	4241

Name	af	nf	hf mf	Ly	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
V22221122	0.2	20	4 600	9	0	1	2.5	0.99	0.15	0.05	0.64	0.19	0.06	0.47	0.90	0.90	0.14	0.001	[MN-m]	MN 4720	[MN-m]	[MN-m]	0.14	SUR [-]	[MN-m]	(MN) 52	m] 4122
V32221122	0.5	30 30	4 600	8	8	1	1.5	0.88	0.13	0.05	0.65	0.18	0.06	0.47	0.89	0.89	0.14	0.001	5251	5251	1036	4197	0.14	0.0013	5205	60	4152
V32221122	0.5	30	4 600	8	8	1	2.5	0.81	0.14	0.05	0.65	0.17	0.06	0.51	0.89	0.89	0.12	0.001	5137	5137	1013	4159	0.12	0.0013	5090	58	4101
V42221121	0.7	30	4 600	8	8	1	1.5	0.71	0.13	0.05	0.66	0.16	0.06	0.57	0.89	0.89	0.10	0.001	5792	5792	1594	4241	0.10	0.0011	5757	67	4195
V42221122	0.7	30	4 600	8	8	1	2.5	0.73	0.14	0.05	0.66	0.16	0.06	0.56	0.89	0.89	0.11	0.001	5654	5654	1556	4141	0.11	0.0012	5618	65	4093
V52221121	0.9	30	4 600	8	8	1	1.5	0.63	0.12	0.05	0.69	0.14	0.06	0.63	0.89	0.89	0.08	0.001	6486	6486	2276	4259	0.08	0.0010	6461	76	4225
V12221122	0.9	30	4 600	8	10	0.6	1.5	2.67	0.12	0.05	0.63	0.14	0.00	0.01	0.89	0.89	0.09	0.001	1837	1837	70	1780	0.09	0.0010	1700	19	1639
V12221212	0.1	30	4 600	8	10	0.6	2.5	2.76	0.45	0.16	0.63	0.19	0.07	0.17	0.78	0.89	0.48	0.006	1775	1775	67	1720	0.48	0.0054	1636	19	1577
V22221211	0.3	30	4 600	8	10	0.6	1.5	2.49	0.42	0.16	0.64	0.18	0.06	0.19	0.80	0.89	0.44	0.005	1996	1996	231	1792	0.44	0.0048	1881	21	1663
V22221212	0.3	30	4 600	8	10	0.6	2.5	2.57	0.44	0.16	0.64	0.18	0.06	0.18	0.79	0.89	0.45	0.005	1927	1927	223	1732	0.45	0.0050	1810	21	1601
V32221211	0.5	30	4 600	8	10	0.6	1.5	2.29	0.41	0.15	0.65	0.17	0.06	0.21	0.81	0.89	0.41	0.004	2213	2213	433	1821	0.41	0.0045	2120	24	1708
V32221212	0.5	30	4 600	8	10	0.6	2.5	2.36	0.42	0.15	0.65	0.17	0.06	0.20	0.81	0.89	0.42	0.005	2133	2133	698	1/58	0.42	0.0045	2039	23	1643
V42221211	0.7	30	4 600	8	10	0.6	2.5	2.00	0.40	0.15	0.66	0.16	0.06	0.23	0.82	0.89	0.38	0.004	2426	2426	671	1810	0.38	0.0041	2355	27	1716
V52221211	0.9	30	4 600	8	10	0.6	1.5	1.84	0.35	0.14	0.69	0.14	0.06	0.27	0.85	0.89	0.31	0.004	2867	2867	1024	1911	0.31	0.0037	2817	33	1842
V52221212	0.9	30	4 600	8	10	0.6	2.5	1.90	0.36	0.14	0.69	0.14	0.06	0.27	0.84	0.89	0.33	0.004	2798	2798	1000	1867	0.33	0.0039	2748	32	1797
V12221221	0.1	30	4 600	8	10	1	1.5	2.16	0.35	0.13	0.63	0.19	0.07	0.23	0.84	0.89	0.41	0.005	2365	2365	91	2288	0.41	0.0046	2247	26	2166
V12221222	0.1	30	4 600	8	10	1	2.5	2.23	0.36	0.13	0.63	0.19	0.07	0.22	0.84	0.89	0.42	0.005	2280	2280	202	2206	0.42	0.0047	2158	24	2080
V22221221	0.3	30	4 600	8	10	1	2.5	2.01	0.34	0.13	0.64	0.18	0.00	0.23	0.85	0.89	0.38	0.004	2495	2495	291	22312	0.38	0.0042	2395	27	2203
V32221221	0.5	30	4 600	8	10	1	1.5	1.85	0.33	0.12	0.65	0.17	0.06	0.27	0.85	0.89	0.34	0.004	2788	2788	547	2279	0.34	0.0037	2708	31	2182
V32221222	0.5	30	4 600	8	10	1	2.5	1.91	0.34	0.13	0.65	0.17	0.06	0.26	0.85	0.89	0.36	0.004	2720	2720	534	2225	0.36	0.0038	2639	30	2126
V42221221	0.7	30	4 600	8	10	1	1.5	1.68	0.31	0.12	0.66	0.16	0.06	0.29	0.86	0.89	0.30	0.003	3036	3036	838	2249	0.30	0.0033	2975	35	2168
V42221222	0.7	30	4 600	8	10	1	2.5	1.73	0.32	0.12	0.66	0.16	0.06	0.29	0.85	0.89	0.31	0.003	2966	2966	819	2198	0.31	0.0034	2903	34	2115
V52221221	0.9	30 30	4 600	8	10	1	2.5	1.49	0.28	0.11	0.69	0.14	0.00	0.32	0.87	0.89	0.24	0.003	3298	3298	1172	2240	0.24	0.0029	3254	38	21/8
V12222111	0.1	30	4 600	10	8	0.6	1.5	1.62	0.26	0.09	0.63	0.19	0.07	0.30	0.87	0.89	0.30	0.003	3051	3051	119	2947	0.30	0.0034	2954	34	2847
V12222112	0.1	30	4 600	10	8	0.6	2.5	1.70	0.28	0.10	0.63	0.19	0.07	0.29	0.87	0.89	0.32	0.004	2942	2942	115	2842	0.32	0.0036	2842	32	2739
V22222111	0.3	30	4 600	10	8	0.6	1.5	1.51	0.26	0.09	0.64	0.18	0.06	0.32	0.88	0.89	0.27	0.003	3231	3231	380	2877	0.27	0.0030	3148	36	2784
V22222112	0.3	30	4 600	10	8	0.6	2.5	1.58	0.27	0.10	0.64	0.18	0.06	0.31	0.87	0.89	0.29	0.003	3118	3118	367	2778	0.29	0.0032	3033	35	2682
V32222111	0.5	30 30	4 600	10	8	0.6	2.5	1.39	0.25	0.09	0.65	0.17	0.06	0.34	0.88	0.89	0.24	0.003	3454	3454	656	2812	0.24	0.0026	3380	39	2729
V42222112	0.7	30	4 600	10	8	0.6	1.5	1.40	0.23	0.09	0.66	0.16	0.06	0.36	0.88	0.89	0.20	0.003	3741	3741	1031	2757	0.20	0.0023	3689	43	2688
V42222112	0.7	30	4 600	10	8	0.6	2.5	1.32	0.24	0.09	0.66	0.16	0.06	0.35	0.88	0.89	0.22	0.002	3619	3619	998	2669	0.22	0.0024	3565	41	2598
V52222111	0.9	30	4 600	10	8	0.6	1.5	1.11	0.21	0.08	0.69	0.14	0.06	0.40	0.88	0.89	0.17	0.002	4118	4118	1455	2721	0.17	0.0020	4081	48	2669
V52222112	0.9	30	4 600	10	8	0.6	2.5	1.17	0.22	0.09	0.69	0.14	0.06	0.38	0.88	0.89	0.18	0.002	3990	3990	1411	2638	0.18	0.0021	3952	47	2584
V12222121	0.1	30 30	4 600	10	8	1	2.5	1.25	0.20	0.07	0.63	0.19	0.07	0.37	0.88	0.89	0.22	0.002	3652	3652	143	3526	0.22	0.0024	3570	41	3440
V22222122	0.3	30	4 600	10	8	1	1.5	1.16	0.21	0.07	0.64	0.15	0.06	0.38	0.88	0.89	0.19	0.003	3852	3852	455	3424	0.19	0.0020	3782	43	3344
V22222122	0.3	30	4 600	10	8	1	2.5	1.23	0.21	0.08	0.64	0.18	0.06	0.37	0.88	0.89	0.21	0.002	3722	3722	439	3309	0.21	0.0023	3649	42	3227
V32222121	0.5	30	4 600	10	8	1	1.5	1.07	0.19	0.07	0.65	0.17	0.06	0.40	0.88	0.89	0.17	0.002	4094	4094	806	3323	0.17	0.0019	4036	46	3252
V32222122	0.5	30	4 600	10	8	1	2.5	1.13	0.20	0.08	0.65	0.17	0.06	0.39	0.88	0.89	0.18	0.002	3959	3959	780	3216	0.18	0.0020	3899	45	3142
V42222121	0.7	30	4 600	10	8	1	2.5	0.97	0.18	0.07	0.66	0.16	0.06	0.44	0.88	0.89	0.15	0.002	4448	4448	1225	3268	0.15	0.0016	4403	51	3208
V52222122	0.9	30	4 600	10	8	1	1.5	0.86	0.16	0.06	0.69	0.10	0.06	0.42	0.89	0.89	0.12	0.002	5011	5011	1765	3300	0.12	0.0014	4981	59	3257
V52222122	0.9	30	4 600	10	8	1	2.5	0.91	0.17	0.07	0.69	0.14	0.06	0.46	0.89	0.89	0.13	0.002	4786	4786	1686	3154	0.13	0.0015	4754	56	3108
V12222211	0.1	30	4 600	10	10	0.6	1.5	3.76	0.61	0.22	0.63	0.19	0.07	0.11	0.64	0.88	0.61	0.007	1247	1247	46	1212	0.61	0.0069	1109	13	1069
V12222212	0.1	30	4 600	10	10	0.6	2.5	4.00	0.65	0.23	0.63	0.19	0.07	0.11	0.61	0.88	0.64	0.008	1166	1166	43	1133	0.64	0.0072	1031	12	993
V222222211	0.3	30	4 600	10	10	0.6	2.5	3.51	0.60	0.22	0.64	0.18	0.06	0.12	0.65	0.88	0.57	0.005	1341	1341	142	1214	0.57	0.0062	1221	14	1080
V32222211	0.5	30	4 600	10	10	0.6	1.5	3.23	0.58	0.22	0.65	0.17	0.06	0.12	0.68	0.88	0.53	0.006	1470	1470	285	1225	0.53	0.0057	1369	16	1103
V32222212	0.5	30	4 600	10	10	0.6	2.5	3.43	0.61	0.23	0.65	0.17	0.06	0.13	0.64	0.88	0.55	0.006	1369	1369	265	1143	0.55	0.0060	1270	15	1023
V42222211	0.7	30	4 600	10	10	0.6	1.5	2.93	0.54	0.21	0.66	0.16	0.06	0.16	0.71	0.88	0.48	0.005	1654	1654	459	1251	0.48	0.0053	1573	18	1146
V42222212	0.7	30	4 600	10	10	0.6	2.5	3.11	0.58	0.22	0.66	0.16	0.06	0.14	0.68	0.88	0.50	0.006	1537	1537	427	1166	0.50	0.0055	1457	17	1062
V52222211	0.9	30 30	4 600	10	10	0.6	1.5	2.59	0.49	0.20	0.69	0.14	0.06	0.18	0.76	0.88	0.42	0.005	1932	1932	701	1216	0.41	0.0049	18/2	22	1224
V12222221	0.1	30	4 600	10	10	1	1.5	3.09	0.50	0.18	0.63	0.19	0.07	0.14	0.74	0.88	0.53	0.006	1558	1558	58	1511	0.53	0.0059	1414	16	1363
V12222222	0.1	30	4 600	10	10	1	2.5	3.30	0.54	0.19	0.63	0.19	0.07	0.13	0.71	0.88	0.55	0.006	1447	1447	54	1404	0.55	0.0062	1303	15	1256
V22222221	0.3	30	4 600	10	10	1	1.5	2.88	0.49	0.18	0.64	0.18	0.06	0.16	0.75	0.88	0.49	0.006	1683	1683	193	1517	0.49	0.0054	1561	18	1380
V222222222	0.3	30	4 600	10	10	1	2.5	3.08	0.52	0.19	0.64	0.18	0.06	0.15	0.72	0.88	0.52	0.006	1561	1561	179	1409	0.52	0.0056	1438	16	1272
V32222221	0.5	30 30	4 600	10	10	1	1.5	2.65	0.47	0.18	0.65	0.17	0.06	0.18	0.77	0.89	0.46	0.005	1854	1854	362	1535	0.46	0.0049	1755	20	1414
V42222221	0.7	30	4 600	10	10	1	1.5	2.40	0.45	0.19	0.66	0.16	0.06	0.10	0.74	0.89	0.41	0.005	2100	2100	582	1575	0.48	0.0046	2023	23	1474
V42222222	0.7	30	4 600	10	10	1	2.5	2.57	0.48	0.18	0.66	0.16	0.06	0.18	0.76	0.88	0.43	0.005	1938	1938	537	1458	0.43	0.0048	1860	22	1355
V52222221	0.9	30	4 600	10	10	1	1.5	2.13	0.40	0.16	0.69	0.14	0.06	0.23	0.82	0.89	0.36	0.004	2474	2474	888	1658	0.36	0.0043	2419	28	1582
V52222222	0.9	30	4 600	10	10	1	2.5	2.28	0.43	0.17	0.69	0.14	0.06	0.21	0.80	0.89	0.38	0.005	2275	2275	820	1530	0.38	0.0045	2219	26	1451
V13111111 V13111111	0.1	45 45	3 400	8	8	0.6	2.5	1.18	0.19	0.07	0.62	0.19	0.07	0.38	0.88	0.89	0.20	0.002	4238	4238	165	4084	0.20	0.0020	4150	42	3992
V23111111	0.3	45	3 400	8	8	0.6	1.5	1.10	0.20	0.07	0.63	0.19	0.07	0.40	0.88	0.89	0.18	0.002	4460	4460	523	3958	0.18	0.0018	4385	41	3930
V23111112	0.3	45	3 400	8	8	0.6	2.5	1.12	0.19	0.07	0.63	0.18	0.06	0.39	0.88	0.89	0.19	0.002	4395	4395	515	3900	0.19	0.0018	4318	44	3814
V33111111	0.5	45	3 400	8	8	0.6	1.5	1.01	0.18	0.07	0.64	0.17	0.06	0.42	0.88	0.89	0.16	0.002	4728	4728	925	3834	0.16	0.0015	4665	48	3757

Name	af	nf	nf mf	Lv	Ia	tw	Lch	T1 [s]	T7 [s]	T3 [s]	MLEI	M2 EI	M3 [-]	Sa	Sa?	Sa3	Disp	SDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disn	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
Hame	a	iu .	u nu	ц,	LA		1100	11 [3]	1 - [9]	19 [9]	mr []	M=[]	mo [1	Ju	002	542	[m]	0011 []	[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V33111112	0.5	45	3 400	8	8	0.6	2.5	1.03	0.18	0.07	0.64	0.17	0.06	0.41	0.88	0.89	0.16	0.002	4662	4662	912	3781	0.16	0.0016	4598	47	3703
V43111111	0.7	45	3 400	8	8	0.6	1.5	0.92	0.17	0.07	0.66	0.16	0.06	0.46	0.89	0.89	0.14	0.001	5206	5206	1423	3822	0.14	0.0014	5158	53	3758
V43111112	0.7	45	3 400	8	8	0.6	2.5	0.94	0.17	0.07	0.66	0.16	0.06	0.45	0.89	0.89	0.14	0.001	5109	5109	1397	3752	0.14	0.0014	5060	52	3687
V53111111	0.9	45	3 400	8	8	0.6	2.5	0.81	0.15	0.06	0.68	0.14	0.06	0.51	0.89	0.89	0.11	0.001	58/3	5757	2052	3869	0.11	0.0012	5840	60	3822
V1311112	0.9	45	3 400	8	8	1	1.5	0.83	0.10	0.00	0.68	0.14	0.00	0.30	0.89	0.89	0.12	0.001	5169	5169	2012	4979	0.12	0.0012	5096	52	4903
V13111122	0.1	45	3 400	8	8	1	2.5	0.91	0.15	0.05	0.62	0.19	0.07	0.46	0.89	0.89	0.15	0.001	5076	5076	199	4889	0.15	0.0015	5001	51	4812
V23111121	0.3	45	3 400	8	8	1	1.5	0.83	0.14	0.05	0.63	0.18	0.06	0.50	0.89	0.89	0.13	0.001	5551	5551	653	4919	0.13	0.0013	5490	56	4849
V23111122	0.3	45	3 400	8	8	1	2.5	0.85	0.14	0.05	0.63	0.18	0.06	0.49	0.89	0.89	0.13	0.001	5436	5436	639	4817	0.13	0.0013	5373	55	4746
V33111121	0.5	45	3 400	8	8	1	1.5	0.76	0.14	0.05	0.64	0.17	0.06	0.53	0.89	0.89	0.11	0.001	5983	5983	1172	4840	0.11	0.0011	5933	61	4778
V33111122	0.5	45	3 400	8	8	1	2.5	0.78	0.14	0.05	0.64	0.17	0.06	0.52	0.89	0.89	0.12	0.001	5875	5875	1151	4753	0.12	0.0011	5824	60	4690
V43111121	0.7	45	3 400	8	8	1	1.5	0.69	0.13	0.05	0.66	0.16	0.06	0.58	0.89	0.89	0.10	0.001	6608	6608	1805	4838	0.10	0.0010	6570	68	4788
V43111122	0.7	45	3 400	8	8	1	2.5	0.71	0.13	0.05	0.66	0.16	0.06	0.57	0.89	0.89	0.10	0.001	54//	54//	1//0	4/43	0.10	0.0010	6438	6/	4692
V53111121	0.9	45	3 400	8	8	1	2.5	0.01	0.12	0.05	0.08	0.14	0.00	0.64	0.89	0.89	0.08	0.001	7245	7245	2525	4807	0.08	0.0009	7217	76	402.5
V13111211	0.1	45	3 400	8	10	0.6	1.5	2.60	0.42	0.15	0.62	0.19	0.07	0.18	0.80	0.89	0.47	0.005	2117	2117	80	2047	0.47	0.0047	1967	20	1892
V13111212	0.1	45	3 400	8	10	0.6	2.5	2.67	0.44	0.16	0.62	0.19	0.07	0.17	0.79	0.89	0.48	0.005	2053	2053	77	1986	0.47	0.0047	1901	19	1829
V23111211	0.3	45	3 400	8	10	0.6	1.5	2.42	0.41	0.15	0.63	0.18	0.06	0.20	0.81	0.89	0.44	0.004	2301	2301	265	2063	0.44	0.0042	2175	22	1921
V23111212	0.3	45	3 400	8	10	0.6	2.5	2.49	0.42	0.15	0.63	0.18	0.06	0.19	0.80	0.89	0.45	0.004	2231	2231	256	2000	0.45	0.0043	2102	21	1857
V33111211	0.5	45	3 400	8	10	0.6	1.5	2.23	0.40	0.15	0.64	0.17	0.06	0.22	0.82	0.89	0.41	0.004	2553	2553	496	2098	0.41	0.0039	2450	25	1973
V33111212	0.5	45	3 400	8	10	0.6	2.5	2.29	0.41	0.15	0.64	0.17	0.06	0.21	0.81	0.89	0.41	0.004	2471	2471	480	2032	0.41	0.0040	2367	24	1906
V43111211	0.7	45	3 400	8	10	0.6	1.5	2.02	0.37	0.14	0.66	0.16	0.06	0.25	0.84	0.89	0.37	0.004	2907	2907	798	2163	0.37	0.0036	2830	29	2062
V43111212	0.7	45	3 400	8	10	0.6	2.5	2.08	0.38	0.15	0.60	0.16	0.06	0.24	0.83	0.89	0.38	0.004	2815	2815	1154	2097	0.38	0.0037	2736	28	2007
V53111211	0.9	45	3 400	8	10	0.0	2.5	1.80	0.34	0.14	0.08	0.14	0.00	0.28	0.85	0.89	0.31	0.003	3197	3197	1132	21/3	0.31	0.0032	3137	34	2057
V13111221	0.1	45	3 400	8	10	1	1.5	2.07	0.34	0.12	0.62	0.19	0.07	0.24	0.85	0.89	0.40	0.003	2782	2782	1102	2686	0.40	0.0040	2657	27	2556
V13111222	0.1	45	3 400	8	10	1	2.5	2.12	0.35	0.12	0.62	0.19	0.07	0.24	0.85	0.89	0.41	0.004	2690	2690	103	2598	0.41	0.0041	2561	26	2464
V23111221	0.3	45	3 400	8	10	1	1.5	1.93	0.33	0.12	0.63	0.18	0.06	0.26	0.85	0.89	0.37	0.004	2992	2992	348	2668	0.37	0.0036	2886	29	2549
V23111222	0.3	45	3 400	8	10	1	2.5	1.98	0.34	0.12	0.63	0.18	0.06	0.26	0.85	0.89	0.38	0.004	2926	2926	340	2610	0.38	0.0037	2818	29	2489
V33111221	0.5	45	3 400	8	10	1	1.5	1.77	0.32	0.12	0.64	0.17	0.06	0.28	0.86	0.89	0.33	0.003	3210	3210	626	2621	0.33	0.0031	3123	32	2515
V33111222	0.5	45	3 400	8	10	1	2.5	1.82	0.32	0.12	0.64	0.17	0.06	0.28	0.85	0.89	0.34	0.003	3146	3146	613	2570	0.34	0.0033	3057	31	2462
V43111221	0.7	45	3 400	8	10	1	1.5	1.61	0.30	0.11	0.66	0.16	0.06	0.30	0.86	0.89	0.28	0.003	2493	2493	957	2585	0.28	0.0028	3425	25	2496
V43111222	0.7	45	3 400	8	10	1	1.5	1.05	0.31	0.12	0.68	0.10	0.06	0.30	0.80	0.89	0.25	0.003	3421	3971	1365	2572	0.23	0.0025	3333	40	2443
V53111221	0.9	45	3 400	8	10	1	2.5	1.47	0.28	0.11	0.68	0.14	0.06	0.33	0.87	0.89	0.24	0.002	3799	3799	1340	2523	0.23	0.0025	3750	39	2454
V13112111	0.1	45	3 400	10	8	0.6	1.5	1.58	0.26	0.09	0.62	0.19	0.07	0.31	0.88	0.89	0.29	0.003	3467	3467	135	3343	0.29	0.0029	3361	34	3233
V13112112	0.1	45	3 400	10	8	0.6	2.5	1.64	0.27	0.10	0.62	0.19	0.07	0.30	0.87	0.89	0.31	0.003	3357	3357	130	3238	0.31	0.0031	3248	33	3125
V23112111	0.3	45	3 400	10	8	0.6	1.5	1.47	0.25	0.09	0.63	0.18	0.06	0.33	0.88	0.89	0.26	0.003	3670	3670	429	3263	0.26	0.0026	3579	36	3161
V23112112	0.3	45	3 400	10	8	0.6	2.5	1.53	0.26	0.10	0.63	0.18	0.06	0.32	0.87	0.89	0.28	0.003	3557	3557	415	3164	0.28	0.0027	3463	35	3059
V33112111	0.5	45	3 400	10	8	0.6	1.5	1.35	0.24	0.09	0.64	0.17	0.06	0.35	0.88	0.89	0.23	0.002	3921	3921	766	3188	0.23	0.0023	3846	39	3097
V33112112	0.5	45	3 400	10	8	0.6	1.5	1.41	0.25	0.09	0.64	0.17	0.06	0.34	0.88	0.89	0.25	0.002	3803	3803	1160	3095	0.25	0.0024	3726	38	3001
V43112111	0.7	45	3 400	10	8	0.6	2.5	1.25	0.23	0.09	0.66	0.16	0.06	0.36	0.88	0.89	0.20	0.002	4117	4117	1127	3035	0.20	0.0020	4058	42	2957
V53112111	0.9	45	3 400	10	8	0.6	1.5	1.09	0.20	0.08	0.68	0.14	0.06	0.40	0.88	0.89	0.16	0.002	4655	4655	1633	3078	0.16	0.0017	4614	48	3020
V53112112	0.9	45	3 400	10	8	0.6	2.5	1.14	0.21	0.09	0.68	0.14	0.06	0.39	0.88	0.89	0.17	0.002	4528	4528	1590	2995	0.17	0.0018	4486	47	2936
V13112121	0.1	45	3 400	10	8	1	1.5	1.21	0.20	0.07	0.62	0.19	0.07	0.38	0.88	0.89	0.21	0.002	4171	4171	163	4020	0.21	0.0021	4082	41	3927
V13112122	0.1	45	3 400	10	8	1	2.5	1.26	0.21	0.07	0.62	0.19	0.07	0.36	0.88	0.89	0.22	0.002	4044	4044	158	3898	0.22	0.0022	3952	40	3802
V23112121	0.3	45	3 400	10	8	1	1.5	1.12	0.19	0.07	0.63	0.18	0.06	0.39	0.88	0.89	0.19	0.002	4391	4391	515	3897	0.19	0.0018	4314	44	3811
V23112122	0.3	45	3 400	10	8	1	2.5	1.18	0.20	0.07	0.63	0.18	0.06	0.41	0.88	0.89	0.20	0.002	4268	4268	500	3789	0.20	0.0019	4189	43	3700
V33112121	0.5 0.5	45	3 400	10	8	1	2.5	1.04	0.18	0.07	0.64	0.17	0.06	0.41	0.88	0.89	0.10	0.002	4059	4059	911 887	3670	0.10	0.0010	4393	4/	3700
V43112121	0.7	45	3 400	10	8	1	1.5	0.94	0.17	0.07	0.66	0.16	0.06	0.45	0.89	0.89	0.14	0.001	5104	5104	1395	3748	0.14	0.0014	5055	52	3683
V43112122	0.7	45	3 400	10	8	1	2.5	0.98	0.18	0.07	0.66	0.16	0.06	0.43	0.88	0.89	0.15	0.001	4908	4908	1342	3606	0.15	0.0015	4857	50	3539
V53112121	0.9	45	3 400	10	8	1	1.5	0.83	0.16	0.06	0.68	0.14	0.06	0.50	0.89	0.89	0.12	0.001	5751	5751	2010	3790	0.12	0.0012	5718	60	3742
V53112122	0.9	45	3 400	10	8	1	2.5	0.87	0.16	0.07	0.68	0.14	0.06	0.48	0.89	0.89	0.12	0.001	5518	5518	1930	3638	0.12	0.0013	5483	58	3588
V13112211	0.1	45	3 400	10	10	0.6	1.5	3.64	0.59	0.21	0.62	0.19	0.07	0.12	0.66	0.88	0.60	0.006	1444	1444	53	1400	0.60	0.0060	1289	13	1240
V13112212	0.1	45	3 400	10	10	0.6	2.5	3.84	0.63	0.22	0.62	0.19	0.07	0.11	0.63	0.88	0.63	0.006	1361	1361	170	1320	0.62	0.0062	1209	12	1163
V23112211	0.3	45	3 400	10	10	0.6	2.5	3.39	0.58	0.21	0.63	0.18	0.06	0.13	0.67	0.88	0.56	0.006	11554	1054	1/6	1202	0.56	0.0054	1220	14	1253
V33112212	0.5	45	3 400	10	10	0.0	1.5	3.12	0.01	0.22	0.64	0.18	0.00	0.12	0.69	0.88	0.55	0.000	1703	1703	328	1417	0.58	0.0050	1589	16	1280
V33112212	0.5	45	3 400	10	10	0.6	2.5	3.29	0.59	0.22	0.64	0.17	0.06	0.13	0.67	0.88	0.54	0.005	1601	1601	308	1334	0.54	0.0052	1489	15	1199
V43112211	0.7	45	3 400	10	10	0.6	1.5	2.83	0.52	0.20	0.66	0.16	0.06	0.16	0.72	0.88	0.47	0.005	1916	1916	528	1448	0.47	0.0046	1825	19	1330
V43112212	0.7	45	3 400	10	10	0.6	2.5	2.99	0.55	0.21	0.66	0.16	0.06	0.15	0.70	0.88	0.49	0.005	1798	1798	496	1362	0.49	0.0048	1708	18	1244
V53112211	0.9	45	3 400	10	10	0.6	1.5	2.52	0.47	0.19	0.68	0.14	0.06	0.19	0.77	0.88	0.41	0.004	2235	2235	804	1511	0.41	0.0043	2168	23	1419
V53112212	0.9	45	3 400	10	10	0.6	2.5	2.65	0.50	0.20	0.68	0.14	0.06	0.18	0.75	0.88	0.42	0.004	2095	2095	756	1420	0.42	0.0045	2027	21	1327
V13112221	0.1	45	3 400	10	10	1	1.5	2.94	0.48	0.17	0.62	0.19	0.07	0.15	0.76	0.89	0.51	0.005	1838	1838	69	1779	0.51	0.0051	1680	17	1617
V13112222	0.1	45	3 400	10	10	1	2.5	3.11	0.51	0.18	0.62	0.19	0.07	0.14	0.74	0.88	0.53	0.005	1020	1020	54	1799	0.53	0.0053	1955	10	1629
V23112221	0.3	45	3 400	10	10	1	2.5	2.90	0.49	0.18	0.63	0.18	0.06	0.16	0.75	0.88	0.50	0.005	1969	1969	212	1677	0.48	0.0040	1777	19	1525
V33112221	0.5	45	3 400	10	10	1	1.5	2.52	0.45	0.17	0.64	0.17	0.06	0.19	0.78	0.89	0.44	0.004	2194	2194	425	1811	0.44	0.0043	2085	21	1679
V33112222	0.5	45	3 400	10	10	1	2.5	2.67	0.48	0.18	0.64	0.17	0.06	0.17	0.76	0.88	0.46	0.004	2052	2052	397	1697	0.46	0.0044	1940	20	1562
V43112221	0.7	45	3 400	10	10	1	1.5	2.29	0.42	0.16	0.66	0.16	0.06	0.21	0.80	0.89	0.40	0.004	2488	2488	684	1861	0.40	0.0039	2404	25	1752

Name	af nf	hf mi	Lv	Lx	tw	Lch	T1 [s]	T2 [s]	T3 [s]	ML FT	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Diso	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
									[-]				00.000	(Constraint)	CHERCER D	[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V43112222	0.7 45	3 40	0 10	10	1	2.5	2.42	0.45	0.17	0.66	0.16	0.06	0.20	0.78	0.89	0.42	0.004	2320	2320	638	1741	0.42	0.0041	2234	23	1628
V53112221	0.9 45	3 40	0 10	10	1	1.5	2.03	0.38	0.15	0.68	0.14	0.06	0.25	0.83	0.89	0.35	0.004	2932	2932	1043	1962	0.35	0.0037	2873	30	1880
V33112222	0.9 45	3 40	0 10	10	0.6	2.5	2.15	0.40	0.16	0.68	0.14	0.06	0.23	0.82	0.89	0.37	0.004	5544	5544	972	5345	0.36	0.0039	5304	28	5189
V13121112	0.1 45	3 60	08	8	0.6	2.5	1.44	0.23	0.00	0.62	0.19	0.07	0.32	0.88	0.89	0.27	0.003	5452	5452	210	5256	0.27	0.0027	5300	54	5099
V23121111	0.3 45	3 60	0 8	8	0.6	1.5	1.34	0.23	0.08	0.63	0.18	0.06	0.35	0.88	0.89	0.24	0.002	5861	5861	686	5208	0.24	0.0023	5732	58	5063
V23121112	0.3 45	3 60	08	8	0.6	2.5	1.37	0.23	0.09	0.63	0.18	0.06	0.34	0.88	0.89	0.24	0.002	5764	5764	674	5122	0.24	0.0024	5633	57	4976
V33121111	0.5 45	3 60	08	8	0.6	1.5	1.24	0.22	0.08	0.64	0.17	0.06	0.37	0.88	0.89	0.21	0.002	6249	6249	1221	5076	0.21	0.0020	6143	63	4947
V33121112	0.5 45	3 60	08	8	0.6	2.5	1.27	0.23	0.08	0.64	0.17	0.06	0.36	0.88	0.89	0.22	0.002	6148	6148	1201	4996	0.21	0.0021	6041	62	4865
V43121111	0.7 45	3 60	0 8	8	0.6	1.5	1.12	0.21	0.08	0.66	0.16	0.06	0.39	0.88	0.89	0.18	0.002	6736	6736	1843	4957	0.18	0.0017	6654	69	4849
V43121112	0.7 45	3 60	n s	8	0.6	2.5	1.15	0.21	0.08	0.68	0.16	0.06	0.39	0.88	0.89	0.18	0.002	7402	7402	2504	4885	0.18	0.0018	7344	58	4775
V53121111	0.9 45	3 60	0 8	8	0.0	2.5	1.00	0.19	0.07	0.08	0.14	0.00	0.43	0.88	0.89	0.14	0.002	7261	7261	2545	4003	0.14	0.0015	7201	76	4800
V13121121	0.1 45	3 60	0 8	8	1	1.5	1.09	0.18	0.06	0.62	0.19	0.07	0.40	0.88	0.89	0.18	0.002	6675	6675	261	6431	0.18	0.0018	6548	66	6300
V13121122	0.1 45	3 60	08	8	1	2.5	1.11	0.18	0.07	0.62	0.19	0.07	0.40	0.88	0.89	0.19	0.002	6574	6574	257	6335	0.19	0.0019	6446	65	6202
V23121121	0.3 45	3 60	08	8	1	1.5	1.01	0.17	0.06	0.63	0.18	0.06	0.42	0.89	0.89	0.16	0.002	7004	7004	822	6213	0.16	0.0016	6895	70	6090
V23121122	0.3 45	3 60	0 8	8	1	2.5	1.04	0.18	0.06	0.63	0.18	0.06	0.41	0.89	0.89	0.17	0.002	6909	6909	811	6130	0.17	0.0016	6799	69	6005
V33121121	0.5 45	3 60	08	8	1	1.5	0.93	0.17	0.06	0.64	0.17	0.06	0.45	0.89	0.89	0.15	0.001	7591	7591	1485	6150	0.15	0.0014	7502	77	6042
V33121122	0.5 45	3 60	0 8	8	1	2.5	0.96	0.17	0.06	0.64	0.17	0.06	0.44	0.89	0.89	0.15	0.001	/440	/440	1456	6029	0.15	0.0014	7350	75	5919
V43121121	0.7 45	3 60	0 0	8	1	2.5	0.85	0.10	0.00	0.00	0.10	0.00	0.49	0.89	0.89	0.13	0.001	8170	8170	22.65	5995	0.15	0.0012	8101	84	5904
V53121122	0.9 45	3 60	0 8	8	1	1.5	0.75	0.14	0.06	0.68	0.14	0.06	0.54	0.89	0.89	0.10	0.001	9388	9388	3278	6180	0.10	0.0011	9341	98	6113
V53121122	0.9 45	3 60	0 8	8	1	2.5	0.77	0.14	0.06	0.68	0.14	0.06	0.53	0.89	0.89	0.11	0.001	9194	9194	3211	6053	0.11	0.0011	9146	96	5986
V13121211	0.1 45	3 60	08	10	0.6	1.5	3.18	0.52	0.19	0.62	0.19	0.07	0.14	0.73	0.88	0.54	0.006	2521	2521	94	2442	0.54	0.0054	2281	23	2195
V13121212	0.1 45	3 60	08	10	0.6	2.5	3.27	0.53	0.19	0.62	0.19	0.07	0.14	0.72	0.88	0.55	0.006	2446	2446	91	2371	0.55	0.0055	2207	22	2124
V23121211	0.3 45	3 60	0 8	10	0.6	1.5	2.97	0.51	0.18	0.63	0.18	0.06	0.15	0.74	0.88	0.51	0.005	2721	2721	310	2451	0.51	0.0049	2516	26	2222
V23121212	0.3 45	3 60	0 8	10	0.6	2.5	3.05	0.52	0.19	0.63	0.18	0.06	0.15	0.73	0.88	0.52	0.005	2640	2640	300	2380	0.52	0.0050	2434	25	2150
V33121211 V33121212	0.5 45	3 60	08	10	0.6	2.5	2.73	0.49	0.18	0.64	0.17	0.06	0.17	0.76	0.88	0.47	0.005	2993	2993	561	2478	0.47	0.0045	2824	29	2275
V43121211	0.7 45	3 60	0 8	10	0.6	1.5	2.48	0.46	0.18	0.66	0.16	0.06	0.19	0.78	0.89	0.43	0.003	3382	3382	930	2540	0.40	0.0042	3251	34	2369
V43121212	0.7 45	3 60	0 8	10	0.6	2.5	2.55	0.47	0.18	0.66	0.16	0.06	0.19	0.77	0.88	0.43	0.004	3275	3275	901	2462	0.43	0.0042	3143	33	2290
V53121211	0.9 45	3 60	08	10	0.6	1.5	2.20	0.41	0.17	0.68	0.14	0.06	0.22	0.81	0.89	0.37	0.004	3972	3972	1418	2668	0.37	0.0039	3879	41	2539
V53121212	0.9 45	3 60	08	10	0.6	2.5	2.26	0.42	0.17	0.68	0.14	0.06	0.22	0.80	0.89	0.38	0.004	3839	3839	1373	2582	0.38	0.0040	3744	39	2450
V13121221	0.1 45	3 60	08	10	1	1.5	2.53	0.41	0.15	0.62	0.19	0.07	0.19	0.81	0.89	0.46	0.005	3273	3273	124	3165	0.46	0.0046	3050	31	2935
V13121222	0.1 45	3 60	08	10	1	2.5	2.60	0.42	0.15	0.62	0.19	0.07	0.18	0.80	0.89	0.47	0.005	3172	3172	120	3069	0.47	0.0047	2947	30	2836
V23121221	0.3 45	3 60	0 0	10	1	2.5	2.50	0.40	0.15	0.63	0.18	0.00	0.21	0.82	0.89	0.45	0.004	3/50	3/50	307	3092	0.45	0.0042	3260	22	2962
V33121222	0.5 45	3 60	0 8	10	1	1.5	2.17	0.39	0.13	0.64	0.17	0.06	0.23	0.83	0.89	0.40	0.004	3957	3957	769	3248	0.40	0.0038	3806	39	3065
V33121222	0.5 45	3 60	0 8	10	1	2.5	2.23	0.40	0.15	0.64	0.17	0.06	0.22	0.82	0.89	0.41	0.004	3826	3826	743	3144	0.41	0.0039	3672	38	2958
V43121221	0.7 45	3 60	08	10	1	1.5	1.97	0.36	0.14	0.66	0.16	0.06	0.26	0.84	0.89	0.36	0.004	4469	4469	1226	3323	0.36	0.0035	4356	45	3174
V43121222	0.7 45	3 60	08	10	1	2.5	2.03	0.37	0.14	0.66	0.16	0.06	0.25	0.84	0.89	0.37	0.004	4357	4357	1196	3243	0.37	0.0036	4242	44	3091
V53121221	0.9 45	3 60	08	10	1	1.5	1.75	0.33	0.13	0.68	0.14	0.06	0.28	0.85	0.89	0.30	0.003	4987	4987	1766	3324	0.30	0.0031	4906	52	3211
V53121222	0.9 45	3 60	0 10	10	1	2.5	1.80	0.34	0.14	0.68	0.14	0.06	0.28	0.85	0.89	0.31	0.003	4885	4885	1/31	3258	0.31	0.0032	4803	50	3143
V13122111 V13122112	0.1 45	3 60	0 10	8	0.6	2.5	2.01	0.31	0.11	0.62	0.19	0.07	0.26	0.85	0.89	0.37	0.004	4462	4462	1/2	4306	0.37	0.0037	4283	43	3965
V23122112	0.3 45	3 60	0 10	8	0.6	1.5	1.80	0.31	0.11	0.63	0.18	0.06	0.23	0.86	0.89	0.34	0.003	4736	4736	551	4220	0.34	0.0033	4584	47	4049
V23122112	0.3 45	3 60	0 10	8	0.6	2.5	1.88	0.32	0.12	0.63	0.18	0.06	0.27	0.85	0.89	0.36	0.003	4582	4582	533	4085	0.36	0.0035	4426	45	3909
V33122111	0.5 45	3 60	0 10	8	0.6	1.5	1.66	0.30	0.11	0.64	0.17	0.06	0.30	0.86	0.89	0.30	0.003	5075	5075	990	4139	0.30	0.0029	4949	51	3985
V33122112	0.5 45	3 60	0 10	8	0.6	2.5	1.73	0.31	0.12	0.64	0.17	0.06	0.29	0.86	0.89	0.32	0.003	4914	4914	958	4010	0.32	0.0031	4785	49	3853
V43122111	0.7 45	3 60	0 10	8	0.6	1.5	1.50	0.28	0.11	0.66	0.16	0.06	0.32	0.87	0.89	0.26	0.003	5513	5513	1510	4076	0.26	0.0025	5416	56	3946
V43122112	0.7 45	3 60	0 10	8	0.6	2.5	1.57	0.29	0.11	0.66	0.16	0.06	0.31	0.86	0.89	0.27	0.003	5344	5344	1464	3953	0.27	0.0027	5244	54	3821
V53122111	0.9 45	3 60	0 10	8	0.6	2.5	1.35	0.25	0.10	0.68	0.14	0.06	0.34	0.88	0.89	0.21	0.002	5918	5918	2085	3927	0.21	0.0022	5847	61	3827
V13122121	0.1 45	3 60	0 10	8	1	1.5	1.48	0.24	0.09	0.62	0.19	0.07	0.32	0.88	0.89	0.27	0.003	5447	5447	212	5252	0.27	0.0027	5295	54	5094
V13122122	0.1 45	3 60	0 10	8	1	2.5	1.55	0.25	0.09	0.62	0.19	0.07	0.31	0.88	0.89	0.29	0.003	5271	5271	205	5083	0.29	0.0029	5114	52	4920
V23122121	0.3 45	3 60	0 10	8	1	1.5	1.38	0.23	0.09	0.63	0.18	0.06	0.34	0.88	0.89	0.24	0.002	5759	5759	674	5118	0.24	0.0024	5628	57	4971
V23122122	0.3 45	3 60	0 10	8	1	2.5	1.44	0.25	0.09	0.63	0.18	0.06	0.33	0.88	0.89	0.26	0.003	5579	5579	652	4960	0.26	0.0025	5445	55	4809
V33122121	0.5 45	3 60	0 10	8	1	1.5	1.27	0.23	0.08	0.64	0.17	0.06	0.36	0.88	0.89	0.22	0.002	6143	6143	1200	4992	0.22	0.0021	6036	62	4861
V33122122	0.5 45	3 60	0 10	8	1	2.5	1.33	0.24	0.09	0.64	0.17	0.06	0.35	0.88	0.89	0.23	0.002	5959	5959	1914	4845	0.23	0.0022	5849	60	4710
V43122121	0.7 45	3 60	0 10	8	1	2.5	1.20	0.21	0.09	0.66	0.10	0.06	0.39	0.88	0.89	0.20	0.002	6441	6441	1763	4744	0.18	0.0019	6355	66	4631
V53122121	0.9 45	3 60	0 10	8	1	1.5	1.02	0.19	0.08	0.68	0.14	0.06	0.42	0.88	0.89	0.15	0.002	7256	7256	2544	4794	0.15	0.0016	7196	76	4710
V53122122	0.9 45	3 60	0 10	8	1	2.5	1.07	0.20	0.08	0.68	0.14	0.06	0.41	0.88	0.89	0.16	0.002	7065	7065	2478	4670	0.16	0.0017	7004	74	4584
V13122211	0.1 45	3 60	0 10	10	0.6	1.5	4.46	0.73	0.26	0.62	0.19	0.07	0.09	0.56	0.87	0.71	0.007	1731	1731	63	1681	0.70	0.0070	1514	15	1456
V13122212	0.1 45	3 60	0 10	10	0.6	2.5	4.70	0.77	0.28	0.62	0.19	0.07	0.09	0.53	0.87	0.74	0.008	1635	1635	59	1587	0.73	0.0073	1423	14	1369
V23122211	0.3 45	3 60	0 10	10	0.6	1.5	4.15	0.71	0.26	0.63	0.18	0.06	0.10	0.57	0.88	0.66	0.007	1853	1853	208	1680	0.66	0.0064	1662	17	1468
V23122212	0.5 45	3 60	0 10	10	0.6	2.5	4.38	0.75	0.27	0.63	0.18	0.06	0.09	0.55	0.87	0.69	0.007	2020	2020	195	1586	0.68	0.0066	1957	16	1379
V33122211	0.5 45	3 60	0 10	10	0.6	2.5	4.03	0.08	0.25	0.64	0.17	0.06	0.11	0.59	0.87	0.63	0.006	1903	1903	364	1594	0.63	0.0058	1743	19	1496
V43122211	0.7 45	3 60	0 10	10	0.6	1.5	3.47	0.64	0.25	0.66	0.16	0.06	0.13	0.62	0.88	0.55	0.005	2256	2256	622	1717	0.54	0.0053	2124	22	1548
V43122212	0.7 45	3 60	0 10	10	0.6	2.5	3.66	0.68	0.26	0.66	0.16	0.06	0.12	0.59	0.87	0.57	0.006	2123	2123	585	1619	0.57	0.0056	1991	21	1451
V53122211	0.9 45	3 60	0 10	10	0.6	1.5	3.08	0.58	0.23	0.68	0.14	0.06	0.15	0.67	0.88	0.47	0.005	2618	2618	950	1786	0.47	0.0050	2514	26	1645

Name	af n	f hf	mf	Ly Ly	tw	Lch	T1 [s]	T7 [s]	T3 [s]	MI FI	M2 [-]	M3 (-)	Sa	Sa?	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disn	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
Hante	a i	i iu	mu	Ly Ls		ЦСО	11 [9]	17 [9]	12 [9]		M2 []	m5 [-]	Ju	502	3022	[m]	0011 [1	[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
V53122212	0.9 4	53	600	10 10	0.6	2.5	3.25	0.61	0.24	0.68	0.14	0.06	0.14	0.65	0.88	0.49	0.005	2458	2458	893	1679	0.49	0.0052	2354	25	1540
V13122221	0.1 4	53	600	10 10		1.5	3.60	0.59	0.21	0.62	0.19	0.07	0.12	0.67	0.88	0.60	0.006	2193	2193	81	2126	0.59	0.0059	1959	20	1884
V13122222	0.1 4	5 3	600	10 10	1 1	2.5	3.81	0.62	0.22	0.62	0.19	0.07	0.11	0.63	0.88	0.62	0.006	2058	2058	267	2121	0.62	0.0062	2156	22	1005
V23122221	0.3 4	53	600	10 10) 1	2.5	3.55	0.57	0.21	0.63	0.18	0.00	0.13	0.65	0.88	0.58	0.000	2300	2212	250	2131	0.50	0.0054	2130	20	1777
V33122221	0.5 4	5 3	600	10 10) 1	1.5	3.09	0.55	0.21	0.64	0.17	0.06	0.15	0.70	0.88	0.51	0.005	2587	2587	499	2151	0.51	0.0049	2417	25	1946
V33122222	0.5 4	53	600	10 10	1	2.5	3.27	0.58	0.22	0.64	0.17	0.06	0.14	0.67	0.88	0.54	0.005	2421	2421	466	2017	0.53	0.0051	2252	23	1814
V43122221	0.7 4	53	600	10 10) 1	1.5	2.81	0.52	0.20	0.66	0.16	0.06	0.16	0.73	0.88	0.46	0.005	2911	2911	802	2199	0.46	0.0046	2775	29	2022
V43122222	0.7 4	53	600	10 10) 1	2.5	2.97	0.55	0.21	0.66	0.16	0.06	0.15	0.70	0.88	0.48	0.005	2721	2721	750	2060	0.48	0.0047	2585	27	1883
V53122221	0.9 4	53	600	10 10) 1	1.5	2.49	0.47	0.19	0.68	0.14	0.06	0.19	0.77	0.88	0.40	0.004	3399	3399	1222	2297	0.40	0.0043	3299	35	2159
V53122222	0.9 4	53	600	10 10	$\frac{1}{2}$	2.5	2.63	0.49	0.20	0.68	0.14	0.06	0.18	0.75	0.88	0.42	0.004	3170	3170	1143	2148	0.42	0.0044	3067	32	2008
V13211111	0.1 4	54	400	5 6	2 0.6	1.5	1.15	0.19	0.07	0.62	0.19	0.07	0.39	0.88	0.89	0.20	0.001	5/2/	5/2/	224	5519	0.20	0.0015	2011	43	5398
V23211112	0.1 4	5 4	400	8 8	0.0	1.5	1.17	0.19	0.07	0.62	0.19	0.07	0.58	0.88	0.89	0.20	0.002	6078	6028	707	5348	0.20	0.0013	5928	42	5236
V23211112	0.3 4	54	400	8 8	3 0.6	2.5	1.09	0.19	0.07	0.63	0.18	0.06	0.40	0.88	0.89	0.18	0.001	5965	5965	700	5293	0.18	0.0013	5864	45	5180
V33211111	0.5 4	54	400	8 8	3 0.6	1.5	0.99	0.18	0.07	0.64	0.17	0.06	0.43	0.89	0.89	0.15	0.001	6432	6432	1258	5214	0.15	0.0011	6349	49	5113
V33211112	0.5 4	54	400	8 8	3 0.6	2.5	1.01	0.18	0.07	0.64	0.17	0.06	0.42	0.88	0.89	0.16	0.001	6332	6332	1239	5134	0.16	0.0011	6248	48	5032
V43211111	0.7 4	54	400	88	3 0.6	1.5	0.90	0.17	0.06	0.66	0.16	0.06	0.47	0.89	0.89	0.13	0.001	7059	7059	1930	5181	0.13	0.0010	6996	54	5098
V43211112	0.7 4	54	400	8 8	3 0.6	2.5	0.91	0.17	0.07	0.66	0.16	0.06	0.46	0.89	0.89	0.14	0.001	6967	6967	1905	5115	0.14	0.0010	6903	54	5030
V53211111	0.9 4	5 4	400	8 8	3 0.6	1.5	0.80	0.15	0.06	0.68	0.14	0.06	0.52	0.89	0.89	0.11	0.001	7972	7972	2785	5250	0.11	0.0009	7928	62	5189
V53211112	0.9 4	54	400	8 8	3 0.6	2.5	0.81	0.15	0.06	0.68	0.14	0.06	0.51	0.89	0.89	0.11	0.001	/862	7862	2/4/	51/9	0.11	0.0009	/818	62	5117
V13211121	0.1 4	5 4 5 4	400	8 8	$\frac{1}{2}$ 1	2.5	0.85	0.14	0.05	0.62	0.19	0.07	0.49	0.89	0.89	0.14	0.001	7142	7142	280	6760	0.14	0.0010	6032	54	6670
V23211122	0.1 4	5 4	400	8 8	, 1 3 1	1.5	0.8/	0.14	0.05	0.62	0.19	0.07	0.48	0.89	0.89	0.14	0.001	7640	7640	899	6768	0.14	0.0010	7560	58	6678
V23211122	0.3 4	5 4	400	8 8	3 1	2.5	0.81	0.14	0.05	0.63	0.18	0.06	0.51	0.89	0.89	0.13	0.001	7533	7533	887	6673	0.13	0.0009	7452	57	6582
V33211121	0.5 4	5 4	400	8 8	3 1	1.5	0.73	0.13	0.05	0.64	0.17	0.06	0.55	0.89	0.89	0.11	0.001	8267	8267	1619	6685	0.11	0.0008	8202	63	6606
V33211122	0.5 4	54	400	8 8	3 1	2.5	0.75	0.13	0.05	0.64	0.17	0.06	0.54	0.89	0.89	0.11	0.001	8136	8136	1593	6580	0.11	0.0008	8070	62	6499
V43211121	0.7 4	54	400	8 8	31	1.5	0.67	0.12	0.05	0.66	0.16	0.06	0.60	0.89	0.89	0.10	0.001	9080	9080	2480	6646	0.10	0.0007	9030	70	6580
V43211122	0.7 4	54	400	8 8	3 1	2.5	0.68	0.13	0.05	0.66	0.16	0.06	0.59	0.89	0.89	0.10	0.001	8959	8959	2447	6558	0.10	0.0007	8908	69	6492
V53211121	0.9 4	54	400	8 8	3 1	1.5	0.59	0.11	0.04	0.68	0.14	0.06	0.66	0.89	0.89	0.08	0.001	10204	10204	3555	6704	0.08	0.0006	10169	80	6656
V53211122	0.9 4	54	400	8 8	5 1	2.5	0.60	0.11	0.05	0.68	0.14	0.06	0.65	0.89	0.89	0.08	0.001	10055	10055	3504	6607	0.08	0.0006	10020	79	6558
V13211211	0.1 4	5 4	400	8 10	0.0	2.5	2.51	0.41	0.15	0.62	0.19	0.07	0.19	0.81	0.89	0.40	0.005	2959	2959	109	2642	0.45	0.0034	2745	21	2059
V23211212	0.3 4	5 4	400	8 10	0.6	1.5	2.34	0.40	0.15	0.63	0.18	0.06	0.21	0.82	0.89	0.43	0.003	3203	3203	369	2868	0.43	0.0031	3037	23	2683
V23211212	0.3 4	5 4	400	8 10	0.6	2.5	2.38	0.41	0.15	0.63	0.18	0.06	0.20	0.82	0.89	0.43	0.003	3128	3128	360	2802	0.43	0.0031	2960	23	2615
V33211211	0.5 4	54	400	8 10	0.6	1.5	2.15	0.38	0.14	0.64	0.17	0.06	0.23	0.83	0.89	0.40	0.003	3555	3555	691	2917	0.40	0.0029	3422	26	2756
V33211212	0.5 4	54	400	8 10	0.6	2.5	2.20	0.39	0.15	0.64	0.17	0.06	0.23	0.83	0.89	0.40	0.003	3474	3474	675	2853	0.40	0.0029	3339	26	2689
V43211211	0.7 4	54	400	8 10	0.6	1.5	1.95	0.36	0.14	0.66	0.16	0.06	0.26	0.84	0.89	0.36	0.003	4003	4003	1098	2976	0.36	0.0026	3903	30	2844
V43211212	0.7 4	54	400	8 10	0.6	2.5	1.99	0.37	0.14	0.66	0.16	0.06	0.26	0.84	0.89	0.36	0.003	3938	3938	1081	2929	0.36	0.0027	3837	30	2796
V53211211	0.9 4	54	400	8 10	0.6	1.5	1.73	0.33	0.13	0.68	0.14	0.06	0.29	0.85	0.89	0.29	0.002	4465	4465	1581	2976	0.29	0.0023	4393	35	28/6
V13211212	0.9 4	5 4	400	8 10	1 1	1.5	1.94	0.55	0.15	0.08	0.14	0.00	0.28	0.85	0.89	0.50	0.002	3954	3954	1537	3816	0.30	0.0024	3795	24	3651
V13211222	0.1 4	5 4	400	8 10	1	2.5	1.97	0.32	0.12	0.62	0.19	0.07	0.26	0.85	0.89	0.38	0.003	3891	3891	150	3755	0.38	0.0029	3730	28	3589
V23211221	0.3 4	5 4	400	8 10) 1	1.5	1.80	0.31	0.11	0.63	0.18	0.06	0.28	0.86	0.89	0.34	0.003	4198	4198	489	3741	0.34	0.0025	4062	31	3588
V23211222	0.3 4	54	400	8 10	1	2.5	1.84	0.31	0.11	0.63	0.18	0.06	0.27	0.86	0.89	0.35	0.003	4137	4137	481	3687	0.35	0.0025	3999	31	3532
V33211221	0.5 4	54	400	8 10) 1	1.5	1.66	0.30	0.11	0.64	0.17	0.06	0.30	0.86	0.89	0.30	0.002	4498	4498	877	3669	0.30	0.0022	4386	34	3532
V33211222	0.5 4	5 4	400	8 10	1	2.5	1.70	0.30	0.11	0.64	0.17	0.06	0.29	0.86	0.89	0.31	0.002	4434	4434	865	3618	0.31	0.0022	4321	33	3480
V43211221	0.7 4	54	400	8 10	1	1.5	1.51	0.28	0.11	0.66	0.16	0.06	0.32	0.87	0.89	0.26	0.002	4886	4886	1339	3612	0.26	0.0019	4799	37	3497
V43211222	0.7 4	54 54	400	8 10	1	2.5	1.54	0.28	0.11	0.66	0.16	0.06	0.31	0.87	0.89	0.27	0.002	4819	4819	1320	3563	0.27	0.0020	5251	37	3447
V53211221	0.9 4	5 4	400	8 10	, <u>1</u>	2.5	1.34	0.25	0.10	0.68	0.14	0.06	0.33	0.88	0.89	0.21	0.002	5337	5337	1903	3540	0.21	0.0017	5274	42	3452
V13212111	0.1 4	5 4	400	10 8	3 0.6	1.5	1.53	0.25	0.09	0.62	0.19	0.07	0.32	0.88	0.89	0.28	0.002	4721	4721	183	4552	0.28	0.0021	4582	35	4408
V13212112	0.1 4	5 4	400	10 8	3 0.6	2.5	1.58	0.26	0.09	0.62	0.19	0.07	0.31	0.88	0.89	0.29	0.002	4619	4619	179	4454	0.29	0.0022	4477	34	4308
V23212111	0.3 4	54	400	10 8	3 0.6	1.5	1.42	0.24	0.09	0.63	0.18	0.06	0.33	0.88	0.89	0.26	0.002	4997	4997	584	4442	0.25	0.0019	4878	37	4309
V23212112	0.3 4	54	400	10 8	3 0.6	2.5	1.47	0.25	0.09	0.63	0.18	0.06	0.33	0.88	0.89	0.27	0.002	4889	4889	571	4348	0.26	0.0019	4768	36	4212
V33212111	0.5 4	54	400	10 8	8 0.6	1.5	1.31	0.23	0.09	0.64	0.17	0.06	0.35	0.88	0.89	0.23	0.002	5337	5337	1042	4338	0.23	0.0016	5239	40	4219
V33212112	0.5 4	54	400	10 8	0.6	2.5	1.35	0.24	0.09	0.64	0.17	0.06	0.35	0.88	0.89	0.23	0.002	5224	5224	1020	4248	0.23	0.0017	5124	39	4127
V43212111	0.7 4	5 4 5 4	400	10 8	0.6	2.5	1.19	0.22	0.08	0.66	0.16	0.06	0.38	0.88	0.89	0.19	0.001	5640	5640	1578	4240	0.19	0.0014	5570	44	4146
V53212112	0.9 4	5 4	400	10 8	, 0.0 3 ().6	1.5	1.25	0.20	0.08	0.68	0.10	0.06	0.41	0.88	0.89	0.16	0.001	6321	6321	2217	4177	0.16	0.0013	6267	49	4000
V53212112	0.9 4	5 4	400	10 8	3 0.6	2.5	1.09	0.20	0.08	0.68	0.14	0.06	0.40	0.88	0.89	0.16	0.001	6203	6203	2176	4101	0.16	0.0013	6148	48	4024
V13212121	0.1 4	54	400	10 8	3 1	1.5	1.15	0.19	0.07	0.62	0.19	0.07	0.39	0.88	0.89	0.19	0.001	5739	5739	224	5530	0.19	0.0015	5624	43	5410
V13212122	0.1 4	54	400	10 8	3 1	2.5	1.18	0.19	0.07	0.62	0.19	0.07	0.38	0.88	0.89	0.20	0.002	5627	5627	220	5422	0.20	0.0015	5509	42	5300
V23212121	0.3 4	54	400	10 8	3 1	1.5	1.07	0.18	0.07	0.63	0.18	0.06	0.41	0.88	0.89	0.17	0.001	6040	6040	709	5359	0.17	0.0013	5941	45	5247
V23212122	0.3 4	54	400	10 8	3 1	2.5	1.10	0.19	0.07	0.63	0.18	0.06	0.40	0.88	0.89	0.18	0.001	5923	5923	695	5256	0.18	0.0013	5821	44	5142
V33212121	0.5 4	5 4	400	10 8	s 1	1.5	0.98	0.18	0.07	0.64	0.17	0.06	0.43	0.89	0.89	0.15	0.001	6452	6452	1262	5230	0.15	0.0011	6370	49	5130
V 33212122	0.5 4	5 A	400	10 8	2 1	2.5	0.80	0.18	0.07	0.64	0.17	0.06	0.42	0.88	0.89	0.10	0.001	7079	7079	1025	5088	0.10	0.0012	7015	48	4985
V43212121	0.7 4	5 4	400	10 8	3 1	2.5	0.92	0.17	0.07	0.66	0.16	0.06	0.45	0.89	0.89	0.13	0.001	6904	6904	1888	5069	0.13	0.0010	6840	53	4985
V53212121	0.9 4	5 4	400	10 8	3 1	1.5	0.79	0.15	0.06	0.68	0.14	0.06	0.52	0.89	0.89	0.11	0.001	7994	7994	2793	5265	0.11	0.0009	7951	63	5204
V53212122	0.9 4	54	400	10 8	3 1	2.5	0.82	0.15	0.06	0.68	0.14	0.06	0.50	0.89	0.89	0.12	0.001	7787	7787	2721	5130	0.12	0.0009	7743	61	5068
V13212211	0.1 4	54	400	10 10	0.6	1.5	3.48	0.57	0.20	0.62	0.19	0.07	0.13	0.68	0.88	0.58	0.005	2027	2027	75	1965	0.58	0.0043	1817	14	1748
Name																										
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| V13212212
 | 0.1 45
 | 4 400

 | 10 10
 | 0.6 | 2.5
 | 3.61 | 0.59 | 0.21 | 0.62
 | 0.19 | 0.07 | 0.12 | 0.66
 | 0.88 | 0.60 | 0.005 | 1944 | 1944
 | 72 | 1885 | 0.60 | 0.0045
 | 1736 | 13 | 1670 |
| V23212211
 | 0.3 45
 | 4 400

 | 10 10
 | 0.6 | 1.5
 | 3.24 | 0.55 | 0.20 | 0.63
 | 0.18 | 0.06 | 0.14 | 0.70
 | 0.88 | 0.54 | 0.004 | 2183 | 2183
 | 247 | 1970 | 0.54 | 0.0039
 | 2001 | 15 | 1768 |
| V23212212
 | 0.3 45
 | 4 400

 | 10 10
 | 0.6 | 2.5
 | 3.36 | 0.57 | 0.21 | 0.63
 | 0.18 | 0.06 | 0.13 | 0.68
 | 0.88 | 0.56 | 0.004 | 2092 | 2092
 | 237 | 1889 | 0.56 | 0.0040
 | 1911 | 15 | 1688 |
| V33212211
 | 0.5 45
 | 4 400

 | 10 10
 | 0.6 | 2.5
 | 2.99 | 0.55 | 0.20 | 0.64
 | 0.17 | 0.06 | 0.15 | 0.72
 | 0.88 | 0.50 | 0.004 | 2390 | 2390
 | 462 | 1990 | 0.50 | 0.0030
 | 2244 | 1/ | 1725 |
| V43212212
 | 0.7 45
 | 4 400

 | 10 10
 | 0.0 | 1.5
 | 2 71 | 0.55 | 0.21 | 0.64
 | 0.17 | 0.00 | 0.14 | 0.70
 | 0.88 | 0.52 | 0.004 | 2699 | 2699
 | 743 | 2035 | 0.31 | 0.0037
 | 2142 | 20 | 1879 |
| V43212212
 | 0.7 45
 | 4 400

 | 10 10
 | 0.6 | 2.5
 | 2.81 | 0.52 | 0.20 | 0.66
 | 0.16 | 0.06 | 0.16 | 0.73
 | 0.88 | 0.47 | 0.003 | 2580 | 2580
 | 711 | 1949 | 0.46 | 0.0034
 | 2459 | 19 | 1792 |
| V53212211
 | 0.9 45
 | 4 400

 | 10 10
 | 0.6 | 1.5
 | 2.40 | 0.45 | 0.18 | 0.68
 | 0.14 | 0.06 | 0.20 | 0.78
 | 0.88 | 0.39 | 0.003 | 3154 | 3154
 | 1132 | 2128 | 0.39 | 0.0031
 | 3067 | 24 | 2007 |
| V53212212
 | 0.9 45
 | 4 400

 | 10 10
 | 0.6 | 2.5
 | 2.50 | 0.47 | 0.19 | 0.68
 | 0.14 | 0.06 | 0.19 | 0.77
 | 0.88 | 0.41 | 0.003 | 3012 | 3012
 | 1083 | 2035 | 0.40 | 0.0032
 | 2923 | 23 | 1913 |
| V13212221
 | 0.1 45
 | 4 400

 | 10 10
 | 1 | 1.5
 | 2.72 | 0.44 | 0.16 | 0.62
 | 0.19 | 0.07 | 0.17 | 0.79
 | 0.89 | 0.48 | 0.004 | 2682 | 2682
 | 101 | 2595 | 0.48 | 0.0036
 | 2478 | 19 | 2384 |
| V13212222
 | 0.1 45
 | 4 400

 | 10 10
 | 1 | 2.5
 | 2.83 | 0.46 | 0.17 | 0.62
 | 0.19 | 0.07 | 0.16 | 0.78
 | 0.89 | 0.50 | 0.004 | 2565 | 2565
 | 96 | 2482 | 0.49 | 0.0037
 | 2358 | 18 | 2268 |
| V23212221
 | 0.3 45
 | 4 400

 | 10 10
 | 1 | 1.5
 | 2.53 | 0.43 | 0.16 | 0.63
 | 0.18 | 0.06 | 0.19 | 0.80
 | 0.89 | 0.45 | 0.003 | 2910 | 2910
 | 334 | 2611 | 0.45 | 0.0033
 | 2738 | 21 | 2418 |
| V23212222
 | 0.3 45
 | 4 400

 | 10 10
 | 1 | 2.5
 | 2.63 | 0.45 | 0.16 | 0.63
 | 0.18 | 0.06 | 0.18 | 0.78
 | 0.89 | 0.47 | 0.003 | 2778 | 2778
 | 318 | 2495 | 0.46 | 0.0034
 | 2603 | 20 | 2299 |
| V33212221
 | 0.5 45
 | 4 400

 | 10 10
 | 1 | 1.5
 | 2.33 | 0.42 | 0.16 | 0.64
 | 0.17 | 0.06 | 0.21 | 0.81
 | 0.89 | 0.42 | 0.003 | 3224 | 3224
 | 626 | 2653 | 0.42 | 0.0030
 | 3084 | 24 | 2484 |
| V33212222
 | 0.5 45
 | 4 400

 | 10 10
 | 1 | 2.5
 | 2.43 | 0.43 | 0.15 | 0.64
 | 0.17 | 0.06 | 0.20 | 0.80
 | 0.89 | 0.43 | 0.003 | 3072 | 3072
 | 1007 | 2532 | 0.43 | 0.0031
 | 2930 | 23 | 2359 |
| V43212221
 | 0.7 45
 | 4 400

 | 10 10
 | 1 | 2.5
 | 2.12 | 0.39 | 0.15 | 0.66
 | 0.16 | 0.06 | 0.24 | 0.83
 | 0.89 | 0.38 | 0.003 | 2400 | 2400
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 | 1442 | 2715 | 0.34 | 0.0027
 | 3985 | 31 | 2608 |
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 | 8 8
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 | 292 | 7234 | 0.25 | 0.0019
 | 7307 | 55 | 7030 |
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 | 288 | 7150 | 0.26 | 0.0020
 | 7217 | 55 | 6943 |
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 | 7764 | 59 | 6858 |
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Name	af 1	մ հք	mf	Ly I	x tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MM
0.000000000	0.2.4		0.0	40		2.5	4.35	0.00	0.00	0.60	2.40	0.00	0.25	0.00	0.00		0.000	[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	SDR [-]	[MN-m]	[MN]	m]
V23222122 V33222121	0.5 4	546	500 500	10 10	8 1	. 2.5	1.35	0.23	0.08	0.63	0.18	0.06	0.35	0.88	0.89	0.24	0.002	8476	8476	1657	6884	0.24	0.0017	8337	58	6718
V33222122	0.5 4	5 4 6	500	10	8 1	2.5	1.25	0.22	0.08	0.64	0.17	0.06	0.37	0.88	0.89	0.21	0.002	8294	8294	1621	6738	0.21	0.0015	8152	63	6565
V43222121	0.7 4	546	500	10	8 1	1.5	1.09	0.20	0.08	0.66	0.16	0.06	0.40	0.88	0.89	0.17	0.001	9129	9129	2497	6717	0.17	0.0013	9021	70	6573
V43222122	0.7 4	546	000	10	8 1	2.5	1.13	0.21	0.08	0.66	0.16	0.06	0.39	0.88	0.89	0.18	0.001	8942	8942	2447	6582	0.18	0.0013	8832	69	6436
V53222121	0.9 4	546	500	10 10	8 1	2.5	1.00	0.18	0.07	0.68	0.14	0.06	0.44	0.88	0.89	0.14	0.001	9809	9809	3438	6479	0.14	0.0011	9731	79	6369
V13222211	0.1 4	5 4 6	600	10 1	0 0.6	1.5	4.26	0.69	0.25	0.62	0.19	0.07	0.10	0.58	0.88	0.68	0.005	2426	2426	88	2355	0.68	0.0051	2130	16	2050
V13222212	0.1 4	546	500	10 1	0 0.6	2.5	4.42	0.72	0.26	0.62	0.19	0.07	0.09	0.56	0.88	0.70	0.005	2329	2329	85	2261	0.70	0.0052	2038	15	1961
V23222211	0.3 4	5 4 6	00	10 1	0 0.6	1.5	3.97	0.68	0.25	0.63	0.18	0.06	0.11	0.59	0.88	0.64	0.005	2599	2599	292	2354	0.63	0.0046	2341	18	2068
V33222212	0.5 4	5 4 6	500 500	10 10	0 0.6	1.5	3.66	0.65	0.20	0.65	0.18	0.06	0.10	0.58	0.88	0.58	0.003	2834	2834	544	2368	0.58	0.0047	2616	20	2107
V33222212	0.5 4	546	500	10 1	0 0.6	2.5	3.80	0.68	0.25	0.64	0.17	0.06	0.11	0.59	0.88	0.60	0.004	2718	2718	521	2273	0.60	0.0043	2501	19	2014
V43222211	0.7 4	546	500	10 1	0 0.6	1.5	3.32	0.61	0.24	0.66	0.16	0.06	0.13	0.64	0.88	0.53	0.004	3174	3174	875	2412	0.53	0.0039	2996	23	2183
V43222212	0.7 4	546	000	10 1	0.6	2.5	3.44	0.64	0.25	0.66	0.16	0.06	0.13	0.62	0.88	0.54	0.004	3037	3037	837	2310	0.54	0.0040	2861	22	2085
V53222211	0.9 4	5 4 6	500	10 1	0.0	2.5	3.06	0.55	0.22	0.68	0.14	0.00	0.15	0.68	0.88	0.40	0.004	3525	3525	1279	2404	0.40	0.0037	3386	20	2322
V13222221	0.1 4	546	500	10 1	0 1	1.5	3.33	0.54	0.19	0.62	0.19	0.07	0.13	0.71	0.88	0.56	0.004	3198	3198	118	3099	0.56	0.0042	2880	22	2771
V13222222	0.1 4	546	500	10 1	D 1	2.5	3.46	0.56	0.20	0.62	0.19	0.07	0.13	0.69	0.88	0.58	0.004	3059	3059	113	2966	0.58	0.0043	2744	21	2640
V23222221	0.3 4	5 4 6	000	10 1	D 1	2.5	3.10	0.53	0.19	0.63	0.18	0.06	0.14	0.72	0.88	0.52	0.004	3448	3448	392	3109	0.52	0.0038	3174	24	2803
V33222221	0.5 4	5 4 6	500	10 1	0 1	. 1.5	2.86	0.55	0.20	0.64	0.18	0.00	0.14	0.74	0.88	0.49	0.004	3789	3789	732	3142	0.48	0.0035	3561	27	2868
V33222222	0.5 4	546	600	10 1	0 1	2.5	2.97	0.53	0.20	0.64	0.17	0.06	0.15	0.72	0.88	0.50	0.004	3617	3617	698	3004	0.50	0.0036	3389	26	2729
V43222221	0.7 4	546	600	10 1	D 1	. 1.5	2.59	0.48	0.18	0.66	0.16	0.06	0.18	0.76	0.88	0.44	0.003	4273	4273	1176	3215	0.44	0.0032	4095	32	2984
V43222222	0.7 4	5 4 6	00	10 1	0 1	2.5	2.69	0.50	0.19	0.66	0.16	0.06	0.17	0.75	0.88	0.45	0.003	4074	4074	1122	3072	0.45	0.0033	3895	30	2838
V53222221	0.9 4	546	500 500	10 1	0 1	2.5	2.30	0.43	0.17	0.68	0.14	0.06	0.21	0.80	0.89	0.38	0.003	4762	4762	1792	3370	0.38	0.0030	4879	38	3193
M11111111	0.1 1	5 3 4	100	8	8 0.6	1.5	0.85	0.14	0.05	0.64	0.19	0.07	0.18	0.71	0.71	0.05	0.001	235	235	9	228	0.05	0.0014	220	7	214
M11111112	0.1 1	534	100	8	8 0.6	2.5	0.93	0.15	0.05	0.64	0.19	0.07	0.16	0.69	0.71	0.05	0.002	216	216	8	209	0.05	0.0016	200	6	195
M21111111	0.3 1	5 3 4	00	8	8 0.6	1.5	0.79	0.13	0.05	0.65	0.18	0.07	0.19	0.71	0.71	0.04	0.001	251	251	30	227	0.04	0.0013	240	7	214
M31111111	0.5 1	5 3 4	100	8	8 0.6	1.5	0.86	0.15	0.05	0.65	0.18	0.07	0.17	0.69	0.71	0.05	0.001	230	230	55	208	0.05	0.0014	218	8	216
M31111112	0.5 1	5 3 4	100	8	8 0.6	2.5	0.79	0.14	0.05	0.66	0.17	0.06	0.19	0.70	0.71	0.04	0.001	251	251	50	208	0.04	0.0012	241	7	196
M41111111	0.7 1	534	100	8	8 0.6	1.5	0.66	0.12	0.05	0.68	0.16	0.06	0.23	0.71	0.71	0.04	0.001	307	307	87	229	0.04	0.0010	300	9	220
M41111112	0.7 1	5 3 4	100	8	8 0.6	2.5	0.72	0.13	0.05	0.68	0.16	0.06	0.21	0.71	0.71	0.04	0.001	281	281	120	210	0.04	0.0011	274	11	200
M51111112	0.9 1	5 3 4	100	8	8 0.6	2.5	0.58	0.11	0.04	0.70	0.14	0.06	0.27	0.71	0.71	0.03	0.001	322	322	118	230	0.03	0.0010	317	10	208
M11111121	0.1 1	5 3 4	100	8	8 1	1.5	0.73	0.12	0.04	0.64	0.19	0.07	0.21	0.71	0.71	0.04	0.001	271	271	11	263	0.04	0.0012	257	8	250
M11111122	0.1 1	534	100	8	8 1	2.5	0.81	0.13	0.05	0.64	0.19	0.07	0.19	0.71	0.71	0.05	0.001	247	247	10	240	0.05	0.0014	232	7	226
M21111121	0.3 1	5 3 4	100	8	8 1	1.5	0.68	0.12	0.04	0.65	0.18	0.07	0.22	0.71	0.71	0.04	0.001	292	292	35	262	0.04	0.0011	281	9	251
M31111122	0.5 1	5 3 4	100	8	8 1	1.5	0.73	0.15	0.03	0.65	0.18	0.07	0.20	0.71	0.71	0.04	0.001	318	318	64	261	0.04	0.0012	310	9	223
M31111122	0.5 1	534	100	8	8 1	2.5	0.69	0.12	0.05	0.66	0.17	0.06	0.22	0.71	0.71	0.04	0.001	290	290	58	239	0.04	0.0011	281	9	228
M41111121	0.7 1	534	100	8	8 1	. 1.5	0.57	0.11	0.04	0.68	0.16	0.06	0.27	0.71	0.71	0.03	0.001	361	361	102	267	0.03	0.0009	355	11	260
M41111122	0.7 1	5 3 4	100	8	8 1	2.5	0.63	0.12	0.04	0.68	0.16	0.06	0.24	0.71	0.71	0.03	0.001	323	323	91	240	0.03	0.0010	317	10	232
M51111121 M51111122	0.9 1	5 3 4	100	8	8 1	2.5	0.55	0.09	0.04	0.70	0.14	0.00	0.51	0.71	0.71	0.03	0.001	377	377	149	275	0.03	0.0008	373	12	207
M11111211	0.1 1	5 3 4	100	8 1	0 0.6	1.5	2.10	0.34	0.12	0.64	0.19	0.07	0.06	0.42	0.71	0.10	0.003	94	94	3	91	0.10	0.0031	79	2	77
M11111212	0.1 1	534	100	8 1	0.6	2.5	2.32	0.38	0.14	0.64	0.19	0.07	0.05	0.39	0.71	0.11	0.003	83	83	3	80	0.11	0.0033	68	2	66
M21111211	0.3 1	5 3 4	00	8 1	0.6	1.5	1.95	0.33	0.12	0.65	0.18	0.07	0.07	0.43	0.71	0.10	0.003	100	100	11	92	0.10	0.0028	88	3	78
M31111212	0.5 1	534	100	8 1	0.0	1.5	1.79	0.37	0.13	0.65	0.18	0.07	0.08	0.40	0.71	0.10	0.003	108	108	21	91	0.10	0.0030	98	2	79
M31111212	0.5 1	5 3 4	100	8 1	0 0.6	2.5	1.98	0.35	0.13	0.66	0.17	0.06	0.07	0.41	0.71	0.10	0.003	98	98	19	83	0.10	0.0028	87	3	71
M41111211	0.7 1	534	100	8 1	0 0.6	1.5	1.62	0.30	0.12	0.68	0.16	0.06	0.09	0.46	0.71	0.08	0.002	119	119	34	92	0.08	0.0023	111	3	81
M41111212	0.7 1	534	00	8 1	0.6	2.5	1.79	0.33	0.13	0.68	0.16	0.06	0.08	0.43	0.71	0.09	0.003	108	108	31	83	0.09	0.0025	99	3	73
M51111211 M51111212	0.9 1	534	100	8 1	0 0.6 0 0.6	2.5	1.43	0.27	0.11	0.70	0.14	0.06	0.10	0.49	0.71	0.07	0.002	130	123		94 85	0.07	0.0021	130	4	85 76
M11111221	0.1 1	5 3 4	100	8 1	0_1	. 1.5	2.02	0.33	0.12	0.64	0.19	0.07	0.07	0.43	0.71	0.10	0.003	99	99	4	95	0.10	0.0031	83	3	81
M11111222	0.1 1	534	100	8 1	0 1	2.5	2.24	0.36	0.13	0.64	0.19	0.07	0.06	0.40	0.71	0.11	0.003	87	87	3	84	0.11	0.0032	71	2	69
M21111221	0.3 1	5 3 4	00	8 1	0 1	1.5	1.88	0.32	0.12	0.65	0.18	0.07	0.07	0.44	0.71	0.09	0.003	104	104	12	95	0.09	0.0028	91	3	82
M31111222	0.5 1	534	100	8 1	0 1 D 1	2.5	1.73	0.36	0.13	0.65	0.18	0.07	0.06	0.41	0.71	0.10	0.003	93 112	93	22	85 95	0.10	0.0030	102	2	82
M31111222	0.5 1	5 3 4	100	8 1	0 1	. 2.5	1.92	0.34	0.13	0.66	0.17	0.06	0.07	0.42	0.71	0.09	0.003	101	101	20	86	0.09	0.0027	91	3	74
M41111221	0.7 1	534	100	8 1	0 1	1.5	1.57	0.29	0.11	0.68	0.16	0.06	0.09	0.47	0.71	0.08	0.002	124	124	35	95	0.08	0.0023	115	4	84
M41111222	0.7 1	534	100	8 1	0 1	2.5	1.73	0.32	0.12	0.68	0.16	0.06	0.08	0.44	0.71	0.08	0.003	111	111	32	86	0.08	0.0025	103	3	75
M51111221	0.9 1	531	100 100	8 1	0 1 D 1	2.5	1.58	0.26	0.10	0.70	0.14	0.06	0.10	0.50	0.71	0.07	0.002	141	141	53	97	0.07	0.0021	135	4	89 79
M11112111	0.1 1	5 3 4	100	10	8 0.6	1.5	1.23	0.20	0.07	0.64	0.19	0.07	0.12	0.60	0.71	0.07	0.002	162	162		157	0.07	0.0020	146	4	142
M11112112	0.1 1	534	100	10	8 0.6	2.5	1.52	0.25	0.09	0.64	0.19	0.07	0.09	0.52	0.71	0.08	0.002	131	131	5	127	0.08	0.0024	115	3	112
M21112111	0.3 1	5 3 4	00	10	8 0.6	1.5	1.15	0.20	0.07	0.65	0.18	0.07	0.13	0.61	0.71	0.06	0.002	172	172	20	157	0.06	0.0018	159	5	142
M21112112 M31112111	0.3 1	5 3 4	100	10	8 0.6 8 0.4	2.5	1.41	0.24	0.09	0.65	0.18	0.07	0.10	0.53	0.71	0.07	0.002	139	139	16	126	0.07	0.0021	126	4	112
TTTTTTT	0.0 1.	, , , ,	00	20	. 0.0		1.00	0.17	0.07	0.00	0.11	0.00	0.14	0.02	0.71	0.00	0.002	109	103	50	1.51	0.00	0.0010	110	5	747

Name	af nf	hf	mf L	v Lx	tw	Lch	T1 [s]	T2 [s]	T3 [s]	M1 F1	M2 FI	M3 [-]	Sa	Sa2 Sa	3	Disp	ISDR [-]	Total Moment	Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
													00.20			[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
M31112112	0.5 1	53	400	10 8	3 0.6	2.5	1.30	0.23	0.09	0.66	0.17	0.06	0.11	0.55 0.	71	0.07	0.002	151	151	30	126	0.07	0.0019	140	4	114
M41112111	0.7 1	53	400	10 8	3 0.6	1.5	0.95	0.18	0.07	0.68	0.16	0.06	0.16	0.64 0.	/1	0.05	0.001	211	211	60	159	0.05	0.0015	203	6	148
M51112112	0.7 1	5 3	400	10 6	3 0.0	1.5	0.84	0.22	0.06	0.08	0.10	0.06	0.12	0.57 0.	71	0.00	0.002	242	242	90	163	0.00	0.0018	236	7	155
M51112112	0.9 1	5 3	400	10 8	3 0.6	2.5	1.04	0.19	0.08	0.70	0.14	0.06	0.14	0.61 0.	71	0.05	0.002	195	195	73	132	0.05	0.0016	188	6	133
M11112121	0.1 1	53	400	10 8	31	1.5	1.12	0.18	0.07	0.64	0.19	0.07	0.13	0.63 0.	71	0.06	0.002	179	179	7	174	0.06	0.0018	163	5	158
M11112122	0.1 1	53	400	10 8	31	2.5	1.46	0.24	0.08	0.64	0.19	0.07	0.10	0.54 0.	71	0.08	0.002	137	137	5	133	0.08	0.0023	121	4	117
M21112121	0.3 1	53	400	10 8	3 1	1.5	1.04	0.18	0.06	0.65	0.18	0.07	0.14	0.64 0.	71	0.06	0.002	192	192	23	174	0.06	0.0016	179	5	159
M21112122	0.3 1	53	400	10 8	<u>31</u> 21	2.5	1.35	0.23	0.08	0.65	0.18	0.07	0.10	0.55 0.	/1	0.07	0.002	145	145	1/	132	0.07	0.0021	132	4	118
M31112121	0.5 1	53	400	10 8	<u>s 1</u> 3 1	2.5	1.24	0.17	0.08	0.66	0.17	0.06	0.10	0.05 0.	71	0.05	0.002	158	158	31	132	0.05	0.0013	199	4	101
M41112121	0.7 1	53	400	10 8	3 1	1.5	0.86	0.16	0.06	0.68	0.16	0.06	0.17	0.67 0.	71	0.05	0.001	232	232	66	175	0.05	0.0013	224	7	164
M41112122	0.7 1	53	400	10 8	31	2.5	1.13	0.21	0.08	0.68	0.16	0.06	0.13	0.59 0.	71	0.06	0.002	176	176	50	134	0.06	0.0017	168	5	123
M51112121	0.9 1	53	400	10 8	31	1.5	0.76	0.14	0.06	0.70	0.14	0.06	0.20	0.70 0.	71	0.04	0.001	266	266	98	179	0.04	0.0012	260	8	170
M51112122	0.9 1	53	400	10 8	3 1	2.5	0.99	0.19	0.07	0.70	0.14	0.06	0.15	0.63 0.	71	0.05	0.002	204	204	76	138	0.05	0.0016	197	6	129
M11112211 M11112212	0.1 1	53	400	$\frac{10}{10}$ 10) 0.6 1 0.6	2.5	3.24 4.79	0.53	0.19	0.64	0.19	0.07	0.03	0.29 0.	51	0.13	0.004	20	50 40	2	38	0.13	0.0039	41	1	40
M21112211	0.3 1	5 3	400	10 10	0.6	1.5	3.01	0.51	0.19	0.65	0.18	0.07	0.04	0.30 0.	62	0.12	0.004	58	58	6	54	0.12	0.0036	46	1	41
M21112212	0.3 1	53	400	10 10	0.6	2.5	3.99	0.68	0.25	0.65	0.18	0.07	0.02	0.22 0.	52	0.14	0.005	41	41	4	38	0.14	0.0042	31	1	28
M31112211	0.5 1	53	400	10 10	0.6	1.5	2.77	0.49	0.18	0.66	0.17	0.06	0.04	0.31 0.	63	0.12	0.003	63	63	12	55	0.11	0.0033	53	2	43
M31112212	0.5 1	53	400	10 10	0.6	2.5	3.66	0.65	0.24	0.66	0.17	0.06	0.03	0.23 0.	53	0.14	0.004	44	44	8	39	0.13	0.0039	35	1	29
M41112211	0.7 1	5 3	400	$\frac{10}{10}$ 10	J U.6	2.5	2.50	0.46	0.18	0.68	0.16	0.06	0.05	0.33 0.	64 54	0.11	0.003	/0	/0	20	20	0.11	0.0031	62	2	46
M51112212	0.9 1	53	400	10 10) 0.6	1.5	2.21	0.41	0.24	0.08	0.10	0.06	0.03	0.36 0	66	0.12	0.004	83	83	32	59	0.12	0.0030	41	2	50
M51112212	0.9 1	5 3	400	10 10	0.6	2.5	2.93	0.55	0.22	0.70	0.14	0.06	0.04	0.28 0.	57	0.11	0.004	57	57	23	41	0.11	0.0035	51	2	33
M11112221	0.1 1	53	400	10 10) 1	1.5	3.17	0.52	0.19	0.64	0.19	0.07	0.03	0.30 0.	63	0.13	0.004	57	57	2	55	0.13	0.0038	42	1	41
M11112222	0.1 1	53	400	10 10) 1	2.5	4.29	0.70	0.25	0.64	0.19	0.07	0.02	0.22 0.	51	0.15	0.005	40	40	1	38	0.15	0.0046	28	1	27
M21112221	0.3 1	53	400	10 10	$\frac{1}{1}$	1.5	2.95	0.50	0.18	0.65	0.18	0.07	0.04	0.31 0.	63	0.12	0.004	60	60	6	55	0.12	0.0035	48	1	42
M21112222 M31112221	0.3 1	53	400	10 10) <u>1</u>) 1	2.5	2.99	0.68	0.25	0.65	0.18	0.07	0.02	0.22 0.	5Z 64	0.14	0.005	41	41	12	38	0.14	0.0042	55	2	28
M31112222	0.5 1	53	400	10 10	$\frac{1}{1}$	2.5	3.67	0.65	0.24	0.66	0.17	0.06	0.03	0.23 0.	52	0.14	0.004	44	44	8	39	0.13	0.0039	35	1	29
M41112221	0.7 1	53	400	10 10) 1	1.5	2.45	0.45	0.18	0.68	0.16	0.06	0.05	0.34 0.	64	0.11	0.003	72	72	21	57	0.11	0.0031	64	2	47
M41112222	0.7 1	53	400	10 10) 1	2.5	3.32	0.61	0.24	0.68	0.16	0.06	0.03	0.25 0.	54	0.12	0.004	49	49	14	39	0.12	0.0036	41	1	30
M51112221	0.9 1	53	400	10 10) 1	1.5	2.17	0.41	0.16	0.70	0.14	0.06	0.06	0.37 0.	67	0.10	0.003	86	86	33	60	0.09	0.0030	80	2	52
M51112222	0.9 1	53	400	8 9	3 0 6	2.5	2.93	0.55	0.22	0.70	0.14	0.06	0.04	0.28 0.	57 71	0.11	0.004	280	280	23	281	0.11	0.0035	265	2	258
M11121111 M11121112	0.1 1	53	600	8 8	3 0.6	2.5	1.14	0.19	0.07	0.64	0.19	0.07	0.13	0.63 0.	71	0.06	0.002	263	263	10	255	0.00	0.0018	238	7	230
M21121111	0.3 1	53	600	8 8	3 0.6	1.5	0.97	0.17	0.06	0.65	0.18	0.07	0.15	0.66 0.	71	0.05	0.002	309	309	37	279	0.05	0.0015	290	9	258
M21121112	0.3 1	53	600	8 8	3 0.6	2.5	1.06	0.18	0.07	0.65	0.18	0.07	0.14	0.64 0.	71	0.06	0.002	282	282	33	256	0.06	0.0017	263	8	234
M31121111	0.5 1	53	600	8 8	3 0.6	1.5	0.89	0.16	0.06	0.66	0.17	0.06	0.17	0.67 0.	71	0.05	0.001	335	335	67	278	0.05	0.0014	320	10	259
M31121112	0.5 1	5 3	600	8 8	3 0.6	2.5	0.97	0.17	0.06	0.66	0.17	0.06	0.15	0.65 0.	/1	0.05	0.002	308	308	62	256	0.05	0.0015	292	11	237
M41121111 M41121112	0.7 1	53	600	8 8	3 0.6	2.5	0.81	0.15	0.00	0.68	0.10	0.00	0.15	0.67 0	71	0.04	0.001	341	341	97	257	0.04	0.0013	303	10	200
M51121111	0.9 1	5 3	600	8 8	3 0.6	1.5	0.71	0.13	0.05	0.70	0.14	0.06	0.21	0.71 0.	71	0.04	0.001	433	433	159	290	0.04	0.0012	425	13	278
M51121112	0.9 1	53	600	8 8	3 0.6	2.5	0.78	0.15	0.06	0.70	0.14	0.06	0.19	0.70 0.	71	0.04	0.001	393	393	145	264	0.04	0.0012	384	12	251
M11121121	0.1 1	53	600	8 8	31	1.5	0.90	0.15	0.05	0.64	0.19	0.07	0.17	0.69 0.	71	0.05	0.002	334	334	13	324	0.05	0.0015	310	9	302
M11121122	0.1 1	53	600	8 8	3 1	2.5	0.99	0.16	0.06	0.64	0.19	0.07	0.15	0.67 0.	71	0.05	0.002	304	304	12	295	0.05	0.0016	280	10	272
M21121121 M21121122	0.3 1	53	600	8 5	<u>s 1</u>	2.5	0.64	0.14	0.05	0.65	0.18	0.07	0.16	0.70 0.	71	0.05	0.001	324	324	38	293	0.05	0.0015	306	01	272
M31121121	0.5 1	5 3	600	8 8	3 1	1.5	0.77	0.14	0.05	0.66	0.17	0.06	0.20	0.71 0.	71	0.04	0.001	389	389	78	321	0.04	0.0012	374	11	303
M31121122	0.5 1	53	600	8 8	3 1	2.5	0.85	0.15	0.06	0.66	0.17	0.06	0.18	0.69 0.	71	0.05	0.001	354	354	71	293	0.05	0.0013	339	10	275
M41121121	0.7 1	53	600	8 8	3 1	1.5	0.70	0.13	0.05	0.68	0.16	0.06	0.22	0.71 0.	71	0.04	0.001	437	437	124	326	0.04	0.0011	426	13	312
M41121122	0.7 1	53	600	8 8	3 1	2.5	0.77	0.14	0.05	0.68	0.16	0.06	0.20	0.70 0.	71	0.04	0.001	393	393	111	294	0.04	0.0012	381	12	279
M51121121	0.9 1	5 2	600	8 9	3 I 3 I	2.5	0.61	0.11	0.05	0.70	0.14	0.06	0.25	0.71 0.	71	0.03	0.001	504 455	455	184	335	0.03	0.0010	497	10	325
M11121211	0.1 1	5 3	600	8 10) 0.6	1.5	2.57	0.42	0.15	0.64	0.14	0.07	0.05	0.36 0.	69	0.11	0.001	110	110	4	106	0.11	0.0034	87	3	84
M11121212	0.1 1	53	600	8 10	0.6	2.5	2.84	0.46	0.17	0.64	0.19	0.07	0.04	0.33 0.	66	0.12	0.004	98	98	3	94	0.12	0.0036	75	2	73
M21121211	0.3 1	53	600	8 10	0.6	1.5	2.39	0.41	0.15	0.65	0.18	0.07	0.05	0.36 0.	69	0.11	0.003	116	116	13	107	0.11	0.0032	98	3	87
M21121212	0.3 1	53	600	8 10	0.6	2.5	2.64	0.45	0.16	0.65	0.18	0.07	0.04	0.34 0.	66	0.11	0.004	103	103	11	95	0.11	0.0033	84	3	75
M21121211	0.5 1	5 3	600	8 10	J 0.6	2.5	2.20	0.39	0.15	0.66	0.17	0.06	0.05	0.37 0.	70	0.10	0.003	128	128	25	109	0.10	0.0030	113	3	91
M41121211	0.7 1	53	600	8 10) 0.6	1.5	1.99	0.45	0.10	0.68	0.17	0.06	0.03	0.39 0	70	0.10	0.003	145	145	41	112	0.09	0.0028	133	4	97
M41121212	0.7 1	5 3	600	8 10	0.6	2.5	2.19	0.41	0.16	0.68	0.16	0.06	0.06	0.37 0.	68	0.10	0.003	126	126	36	99	0.10	0.0029	115	4	84
M51121211	0.9 1	53	600	8 10	0.6	1.5	1.75	0.33	0.13	0.70	0.14	0.06	0.08	0.43 0.	71	0.08	0.003	165	165	63	115	0.08	0.0026	156	5	102
M51121212	0.9 1	53	600	8 10	0.6	2.5	1.94	0.36	0.15	0.70	0.14	0.06	0.07	0.40 0.	70	0.09	0.003	149	149	57	104	0.09	0.0028	140	4	91
M11121221	0.1 1	5 3	600	8 10	1	1.5	2.48	0.40	0.14	0.64	0.19	0.07	0.05	0.37 0.	70	0.11	0.004	115	115	4	111	0.11	0.0034	91	3	89
M21121222	0.1 1	5 3	600	8 10	<u>ו 1</u> ו 1	2.5	2.75	0.45	0.16	0.64	0.19	0.07	0.04	0.34 0.	70	0.12	0.004	102	102	14	98	0.12	0.0035	102	2	/6
M21121222	0.3 1	5 3	600	8 10) 1	2.5	2.55	0.44	0.16	0.65	0.18	0.07	0.05	0.35 0.	67	0.11	0.003	107	107	14	99	0.11	0.0033	88	3	79
M31121221	0.5 1	53	600	8 10) 1	1.5	2.12	0.38	0.14	0.66	0.17	0.06	0.06	0.38 0.	70	0.10	0.003	134	134	26	114	0.10	0.0029	119	4	96
M31121222	0.5 1	53	600	8 10) 1	2.5	2.35	0.42	0.16	0.66	0.17	0.06	0.05	0.36 0.	68	0.11	0.003	117	117	23	100	0.11	0.0031	102	3	82
M41121221	0.7 1	5 3	600	8 10) 1	1.5	1.92	0.36	0.14	0.68	0.16	0.06	0.07	0.41 0.	71	0.09	0.003	150	150	43	117	0.09	0.0027	138	4	101

Description Description <thdescription< th=""> <thdescription< th=""></thdescription<></thdescription<>	Name	af	nf	hf m	f Ly	/ Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp [m]	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1 ISDR [-]	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MM
MELLIDIZI 69 16 1 1 16 10 100 100	M41121222	0.7	15	3 600	8 (10	1	2.5	2.12	0.39	0.15	0.68	0.16	0.06	0.06	0.37	0.69	0.10	0.003	132	132	38	103	0.10	0.0029	120	4	m 88
Sh11222 B B B B <td>M51121221</td> <td>0.9</td> <td>15</td> <td>3 600</td> <td>8</td> <td>10</td> <td>1</td> <td>1.5</td> <td>1.69</td> <td>0.32</td> <td>0.13</td> <td>0.70</td> <td>0.14</td> <td>0.06</td> <td>0.08</td> <td>0.44</td> <td>0.71</td> <td>0.08</td> <td>0.002</td> <td>172</td> <td>172</td> <td>65</td> <td>119</td> <td>0.08</td> <td>0.0025</td> <td>162</td> <td>5</td> <td>106</td>	M51121221	0.9	15	3 600	8	10	1	1.5	1.69	0.32	0.13	0.70	0.14	0.06	0.08	0.44	0.71	0.08	0.002	172	172	65	119	0.08	0.0025	162	5	106
M112121 0.1 5 0.00 0.0 0.0 0.00 0.	M51121222	0.9	15	3 600	8 (10	1	2.5	1.88	0.35	0.14	0.70	0.14	0.06	0.07	0.41	0.70	0.09	0.003	154	154	59	107	0.09	0.0027	145	5	95
0111010 0111010 0111010 01100 000 000 000 000 00000 00000 0000	M11122111	0.1	15	3 600	10	8	0.6	1.5	1.51	0.25	0.09	0.64	0.19	0.07	0.09	0.52	0.71	0.08	0.002	198	198	7	191	0.08	0.0024	174	5	169
	M11122112 M21122111	0.1	15	3 600	10	8	0.6	2.5	1.86	0.30	0.11	0.64	0.19	0.07	0.07	0.46	0.71	0.09	0.003	161	200	24	156	0.09	0.0029	138	4	134
Maillardi Gal 13 Gal 14 Gal 13 Gal 14 Gal 14 <thgal 14<="" th=""> <thgal 14<="" th=""> Gal 14</thgal></thgal>	M21122111 M21122112	0.3	15	3 600	0 10	8	0.6	2.5	1.40	0.24	0.09	0.65	0.18	0.07	0.08	0.33	0.71	0.07	0.002	169	169	24	155	0.09	0.0021	151	5	103
M112112 0 0 0 0 <td>M31122111</td> <td>0.5</td> <td>15</td> <td>3 600</td> <td>) 10</td> <td>8</td> <td>0.6</td> <td>1.5</td> <td>1.29</td> <td>0.23</td> <td>0.09</td> <td>0.66</td> <td>0.17</td> <td>0.06</td> <td>0.11</td> <td>0.55</td> <td>0.71</td> <td>0.07</td> <td>0.002</td> <td>228</td> <td>228</td> <td>45</td> <td>191</td> <td>0.07</td> <td>0.0019</td> <td>212</td> <td>6</td> <td>172</td>	M31122111	0.5	15	3 600) 10	8	0.6	1.5	1.29	0.23	0.09	0.66	0.17	0.06	0.11	0.55	0.71	0.07	0.002	228	228	45	191	0.07	0.0019	212	6	172
MAILLYILL 0.7 1 0.8 0.6 0.8 0.8 0.8 0.0 0.00 21 21 0.7 0.8 0.00 0.00	M31122112	0.5	15	3 600	10	8	0.6	2.5	1.59	0.28	0.11	0.66	0.17	0.06	0.09	0.47	0.71	0.08	0.002	183	183	36	154	0.08	0.0023	168	5	136
04112/11 0.1 3 0.0<	M41122111	0.7	15	3 600	10	8	0.6	1.5	1.17	0.22	0.08	0.68	0.16	0.06	0.12	0.57	0.71	0.06	0.002	254	254	72	193	0.06	0.0017	241	7	177
Constraint Constra	M41122112	0.7	15	3 600	10	8	0.6	2.5	1.44	0.27	0.10	0.68	0.16	0.06	0.10	0.49	0.71	0.07	0.002	203	203	58	155	0.07	0.0021	191	6	139
Dillizzizzi 0.1 0.1 0.1 0.1 0.0 <td< td=""><td>M51122111</td><td>0.9</td><td>15</td><td>3 600</td><td>10</td><td>8</td><td>0.6</td><td>2.5</td><td>1.03</td><td>0.19</td><td>0.08</td><td>0.70</td><td>0.14</td><td>0.06</td><td>0.14</td><td>0.51</td><td>0.71</td><td>0.05</td><td>0.002</td><td>294</td><td>294</td><td>110</td><td>199</td><td>0.05</td><td>0.0016</td><td>284</td><td>7</td><td>146</td></td<>	M51122111	0.9	15	3 600	10	8	0.6	2.5	1.03	0.19	0.08	0.70	0.14	0.06	0.14	0.51	0.71	0.05	0.002	294	294	110	199	0.05	0.0016	284	7	146
Initizizzi 0 1 1 1 1	M11122121	0.1	15	3 600) 10	8	1	1.5	1.37	0.24	0.08	0.64	0.19	0.07	0.10	0.54	0.71	0.07	0.002	219	219	8	212	0.07	0.0022	194	6	189
Name Name <th< td=""><td>M11122122</td><td>0.1</td><td>15</td><td>3 600</td><td>10</td><td>8</td><td>1</td><td>2.5</td><td>1.78</td><td>0.29</td><td>0.10</td><td>0.64</td><td>0.19</td><td>0.07</td><td>0.08</td><td>0.47</td><td>0.71</td><td>0.09</td><td>0.003</td><td>168</td><td>168</td><td>6</td><td>162</td><td>0.09</td><td>0.0027</td><td>144</td><td>4</td><td>140</td></th<>	M11122122	0.1	15	3 600	10	8	1	2.5	1.78	0.29	0.10	0.64	0.19	0.07	0.08	0.47	0.71	0.09	0.003	168	168	6	162	0.09	0.0027	144	4	140
Mail 12:22 0.1 1.5 1.60 1.5 1.5 1.6 0.00 0.00 1.70 1.70 1.70 1.70 1.70 1.80 0.001 0.00 0.00 0.00 0.000 0.001 0.001 0.000	M21122121	0.3	15	3 600	10	8	1	1.5	1.27	0.22	0.08	0.65	0.18	0.07	0.11	0.57	0.71	0.07	0.002	232	232	27	211	0.07	0.0020	212	6	189
M111221 0.5 3 400 0.8 1.5 1.0 0.0 </td <td>M21122122</td> <td>0.3</td> <td>15</td> <td>3 600</td> <td>) 10</td> <td>8</td> <td>1</td> <td>2.5</td> <td>1.66</td> <td>0.28</td> <td>0.10</td> <td>0.65</td> <td>0.18</td> <td>0.07</td> <td>0.08</td> <td>0.48</td> <td>0.71</td> <td>0.08</td> <td>0.003</td> <td>176</td> <td>176</td> <td>20</td> <td>161</td> <td>0.08</td> <td>0.0025</td> <td>158</td> <td>5</td> <td>141</td>	M21122122	0.3	15	3 600) 10	8	1	2.5	1.66	0.28	0.10	0.65	0.18	0.07	0.08	0.48	0.71	0.08	0.003	176	176	20	161	0.08	0.0025	158	5	141
International and another internatinternatintere international and another international and anothe	M31122121	0.5	15	3 600	10	8	1	1.5	1.1/	0.21	0.08	0.66	0.17	0.06	0.12	0.59	0.71	0.06	0.002	253	253	50	211	0.06	0.0018	237	/	192
Mail 2212 0.7 15 100 10 12 15 10 12 13 13 13 13 13 13 14 13 14	M41122122	0.3	15	3 600	10	8	1	1.5	1.06	0.27	0.10	0.68	0.17	0.06	0.09	0.49	0.71	0.08	0.002	283	283	81	214	0.08	0.0022	270	8	142
MAINTERNAM Unit B S	M41122122	0.7	15	3 600) 10	8	1	2.5	1.38	0.26	0.10	0.68	0.16	0.06	0.10	0.51	0.71	0.07	0.002	212	212	60	162	0.07	0.0020	200	6	146
MS112121 10 15 3 600 10 0 12 12 12 12 12 12 12 12 12 12 13 100 10 0 2 2 10 100 00 2 2 10 100 10 0 2 2 0	M51122121	0.9	15	3 600	10	8	1	1.5	0.93	0.18	0.07	0.70	0.14	0.06	0.16	0.64	0.71	0.05	0.001	326	326	121	220	0.05	0.0015	317	10	207
MI1122212 10.1 15 3 600 10 0.6 16 2 6 16 16 2 6 16 10	M51122122	0.9	15	3 600	10	8	1	2.5	1.22	0.23	0.09	0.70	0.14	0.06	0.12	0.55	0.71	0.06	0.002	245	245	92	167	0.06	0.0019	235	7	154
M11122121 0.1 1.5 3.6 0.1 0.0 0	M11122211	0.1	15	3 600	0 10	10	0.6	1.5	3.96	0.65	0.23	0.64	0.19	0.07	0.02	0.24	0.55	0.15	0.005	66	66	2	63	0.14	0.0044	46	1	45
Initizzi	M11122212	0.1	15	3 600	10	10	0.6	2.5	5.25	0.85	0.31	0.64	0.19	0.07	0.02	0.18	0.45	0.17	0.006	48	48	1	45	0.17	0.0051	31	1	30
Instructure	M21122211 M21122212	0.3	15	3 600	10	10	0.6	2.5	4.88	0.05	0.25	0.65	0.18	0.07	0.03	0.24	0.35	0.14	0.004	49	49	5	46	0.14	0.0040	35	1	40
M3112212 0.5 15 16 10 0.6 0.7 0.6 0.17 0.06 0.17 0.004 18 81 80 10 </td <td>M31122211</td> <td>0.5</td> <td>15</td> <td>3 600</td> <td>10</td> <td>10</td> <td>0.6</td> <td>1.5</td> <td>3.39</td> <td>0.60</td> <td>0.23</td> <td>0.66</td> <td>0.17</td> <td>0.06</td> <td>0.03</td> <td>0.26</td> <td>0.56</td> <td>0.13</td> <td>0.004</td> <td>73</td> <td>73</td> <td>14</td> <td>64</td> <td>0.13</td> <td>0.0037</td> <td>59</td> <td>2</td> <td>48</td>	M31122211	0.5	15	3 600	10	10	0.6	1.5	3.39	0.60	0.23	0.66	0.17	0.06	0.03	0.26	0.56	0.13	0.004	73	73	14	64	0.13	0.0037	59	2	48
MH112212 O. 15 S do 10 10 6 L 5 300 6 S o 10 10 6 L 5 300 10 0.05 0.05 0.00 11 0.00 55 56 10 10 0.00 55 56 10 10 0.00 15 36 0.10 10 0.5 55 0.00 10 0.00 15 56 10 10 0.00 15 56 10 10 0.00 15 56 10 10 0.00 10	M31122212	0.5	15	3 600	10	10	0.6	2.5	4.49	0.80	0.30	0.66	0.17	0.06	0.02	0.19	0.46	0.15	0.005	52	52	10	46	0.15	0.0044	40	1	32
M4112212 0.7 15 3 600 10 0.6 0.7 0.7 0.6 0.02 0.2 0.47 0.14 0.06 0.3 0.2 0.47 0.14 0.06 0.3 0.2 0.47 0.14 0.06 0.3 0.2 0.40 0.05 0.3 0.2 0.40 0.06 0.3 0.2 0.40 0.05 0.5 0.5 0.6 0.1 0.03 0.3 0.04 0.05 0.5 0.5 0.6 0.1 0.05 0.6 0.03 0.2 0.04 0.05 0.0 0.0 0.0 0.02 0.0 0.0 0.02 0.06 0.0	M41122211	0.7	15	3 600	10	10	0.6	1.5	3.06	0.57	0.22	0.68	0.16	0.06	0.04	0.27	0.57	0.12	0.004	81	81	23	65	0.12	0.0035	69	2	51
05112221 09 15 360 10 0 0.6 0.7 0.0 0.7 0.3 0.2 0.6 0.7 0.7 0.7 0.0 <td>M41122212</td> <td>0.7</td> <td>15</td> <td>3 600</td> <td>) 10</td> <td>10</td> <td>0.6</td> <td>2.5</td> <td>4.06</td> <td>0.75</td> <td>0.29</td> <td>0.68</td> <td>0.16</td> <td>0.06</td> <td>0.02</td> <td>0.20</td> <td>0.47</td> <td>0.14</td> <td>0.004</td> <td>56</td> <td>56</td> <td>16</td> <td>46</td> <td>0.14</td> <td>0.0041</td> <td>47</td> <td>1</td> <td>34</td>	M41122212	0.7	15	3 600) 10	10	0.6	2.5	4.06	0.75	0.29	0.68	0.16	0.06	0.02	0.20	0.47	0.14	0.004	56	56	16	46	0.14	0.0041	47	1	34
Millizzi	M51122211	0.9	15	3 600	10	10	0.6	2.5	3.58	0.51	0.20	0.70	0.14	0.06	0.04	0.31	0.60	0.11	0.003	94	94 65	37	08 47	0.11	0.0033	85 57	3	37
M112222 0.1 1.5 2.5 0.85 0.31 0.46 0.19 0.07 0.02 0.18 0.45 0.17 0.06 48 48 1 45 0.17 0.06 0.15 0.33 0.55 0.14 0.0001 70 7 65 0.14 0.004 35 2 47 M1122221 0.5 1.5 3.60 10 1 2.5 0.25 0.66 0.17 0.06 0.03 0.20 0.05 0.13 0.04 0.15 0.04 0.15 0.04 0.15 0.05 52 10 0.6 0.16 0.06 0.06 0.06 0.00 0.02 0.05 52 10 0.6 0.16 0.06 0.00 0.00 0.15 0.00 0.16 0.06 0.00 0.02 0.07 0.14 0.00 0.00 0.01	M111222212	0.1	15	3 600	10	10	1	1.5	3.89	0.63	0.23	0.64	0.19	0.07	0.03	0.23	0.55	0.12	0.005	67	67	20	65	0.12	0.0043	48	1	46
Mat122221 0.3 15 3 600 10 1 1.5 3.6 0.62 0.25 0.5 0.14 0.003 70 7 65 0.14 0.003 53 2 47 M31122221 0.5 15 3 0.001 1 1.5 3.32 0.59 0.22 0.66 0.17 0.06 0.02 0.19 0.46 0.15 0.004 75 75 14 65 0.13 0.004 40 1 32 M4112221 0.7 15 3.60 10 1 1.5 0.00 0.66 0.17 0.66 0.02 0.20 0.47 0.41 0.00 55 25 1.6 46 0.15 0.001 33 33 33 33 34 0.01 0.001 35 35 35 35 35 35 35 35 35 35 35 35 35 35 35 35 35 35 <td>M11122222</td> <td>0.1</td> <td>15</td> <td>3 600</td> <td>10</td> <td>10</td> <td>1</td> <td>2.5</td> <td>5.25</td> <td>0.85</td> <td>0.31</td> <td>0.64</td> <td>0.19</td> <td>0.07</td> <td>0.02</td> <td>0.18</td> <td>0.45</td> <td>0.17</td> <td>0.006</td> <td>48</td> <td>48</td> <td>1</td> <td>45</td> <td>0.17</td> <td>0.0051</td> <td>31</td> <td>1</td> <td>30</td>	M11122222	0.1	15	3 600	10	10	1	2.5	5.25	0.85	0.31	0.64	0.19	0.07	0.02	0.18	0.45	0.17	0.006	48	48	1	45	0.17	0.0051	31	1	30
M2112222 0.3 15 3 600 10 1 1.5 2.5 4.8 0.48 0.00 0.45 0.18 0.07 0.12 0.18 0.46 0.16 0.0004 25 5 3 60 10 1 1.5 3.2 0.55 0.15 0.001 10 1 1.5 0.00 0.05 0.21 0.004 0.05 0.21 0.004 4.6 0.003 0.65 0.15 0.0044 4.4 0.003 4.6 0.003 0.05 0.21 0.004 55 55 16 46 0.14 0.0041 4.66 0.01 0.004 55 55 16 46 0.14 0.0041 4.66 0.03 0.23 0.40 0.004 55 55 16 46 0.14 0.0041 35 35 13 33 33 33 33 33 33 33 33 33 33 33 33 33 33 33 33 34 34 0.01 35 36 0.10 0.01 35	M21122221	0.3	15	3 600	10	10	1	1.5	3.61	0.62	0.22	0.65	0.18	0.07	0.03	0.25	0.56	0.14	0.004	70	70	7	65	0.14	0.0039	53	2	47
Millizz221 0.5 15 3 600 10 1 15 3 20 0.05 0.17 0.06 0.17 0.06 0.01 0.06 0.01 0.06 0.01 0.06 0.01 0.06 0.01 0.06 0.01 0.06 0.01 0.06 0.02 0.09 0.06 0.01 0.06 0.02 0.00 0.04 0.01 0.01 0.05 0.01 0.05 0.01 0.06 0.02 0.00 0.04 0.01 0.01 0.01 1 5 0.06 0.01 0.01 0.05 0.04 0.05 0.02 0.04 0.01 0.03 97 73 8 69 0.10 0.003 88 3 57 M11211111 0.1 1 5 50 0.70 0.71 0.14 0.06 0.03 0.01 317 317 12 0.01 0.001 335 335 0.01 0.01 316 70 0.71 0.71 0.01 0.01 315 440 0.01 0.01 317 317 72 <t< td=""><td>M21122222</td><td>0.3</td><td>15</td><td>3 600</td><td>) 10</td><td>10</td><td>1</td><td>2.5</td><td>4.88</td><td>0.83</td><td>0.30</td><td>0.65</td><td>0.18</td><td>0.07</td><td>0.02</td><td>0.18</td><td>0.46</td><td>0.16</td><td>0.005</td><td>49</td><td>49</td><td>5</td><td>46</td><td>0.16</td><td>0.0048</td><td>35</td><td>1</td><td>31</td></t<>	M21122222	0.3	15	3 600) 10	10	1	2.5	4.88	0.83	0.30	0.65	0.18	0.07	0.02	0.18	0.46	0.16	0.005	49	49	5	46	0.16	0.0048	35	1	31
$ \begin{array}{c} 1112222 & 0.3 & 1.3 & 0.00 & 10 & 1 & 1.5 & 4.49 & 0.6 & 0.30 & 0.00 & 0.10 & 0.00 & 0.02 & 0.20 & 0.47 & 0.14 & 0.004 & 56 & 56 & 16 & 46 & 0.14 & 0.004 & 14 & 1 & 34 \\ \hline 01122222 & 0.7 & 15 & 3600 & 10 & 1 & 1.5 & 5.6 & 5.0 & 0.20 & 0.70 & 0.14 & 0.06 & 0.04 & 0.28 & 0.8 & 0.2 & 0.004 & 56 & 56 & 16 & 46 & 0.14 & 0.004 & 146 & 1 & 34 \\ \hline 01122222 & 0.9 & 15 & 3600 & 10 & 1 & 1.5 & 5.6 & 5.0 & 0.20 & 0.70 & 0.14 & 0.06 & 0.04 & 0.30 & 0.004 & 0.30 & 0.004 & 0.35 & 355 & 313 & 325 & 0.04 & 0.010 & 0.003 & 388 & 3 & 57 \\ \hline 011221111 & 0.1 & 15 & 4400 & 8 & 0.6 & 1.5 & 0.79 & 0.13 & 0.05 & 0.64 & 0.19 & 0.07 & 0.14 & 0.06 & 0.04 & 0.31 & 0.001 & 335 & 335 & 13 & 325 & 0.04 & 0.001 & 316 & 7 & 307 \\ \hline 011211112 & 0.1 & 15 & 4400 & 8 & 0.6 & 1.5 & 0.74 & 0.13 & 0.05 & 0.64 & 0.19 & 0.07 & 0.18 & 0.71 & 0.71 & 0.04 & 0.001 & 335 & 335 & 13 & 323 & 0.04 & 0.009 & 344 & 8 & 307 \\ \hline 011211112 & 0.1 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.74 & 0.13 & 0.05 & 0.65 & 0.18 & 0.07 & 0.21 & 0.71 & 0.71 & 0.04 & 0.001 & 335 & 335 & 13 & 323 & 0.04 & 0.009 & 344 & 8 & 307 \\ \hline 011211112 & 0.1 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.78 & 0.13 & 0.05 & 0.66 & 0.17 & 0.06 & 0.21 & 0.71 & 0.71 & 0.04 & 0.01 & 338 & 338 & 40 & 305 & 0.000 & 348 & 8 & 301 \\ \hline 011211112 & 0.5 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.66 & 0.17 & 0.06 & 0.21 & 0.71 & 0.71 & 0.01 & 0.01 & 371 & 371 & 75 & 306 & 0.04 & 0.000 & 358 & 82 & 91 \\ \hline 0131211112 & 0.7 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.66 & 0.17 & 0.06 & 0.23 & 0.71 & 0.71 & 0.01 & 0.01 & 371 & 371 & 75 & 306 & 0.04 & 0.000 & 358 & 82 & 91 \\ \hline 013121112 & 0.7 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.65 & 0.16 & 0.06 & 0.23 & 0.71 & 0.71 & 0.04 & 0.001 & 371 & 371 & 75 & 306 & 0.04 & 0.000 & 358 & 82 & 91 \\ \hline 01121112 & 0.7 & 15 & 4400 & 8 & 8 & 0.6 & 1.5 & 0.66 & 0.17 & 0.06 & 0.23 & 0.71 & 0.71 & 0.01 & 0.01 & 431 & 413 & 117 & 307 & 0.3 & 0.000 & 443 & 9 & 925 \\ \hline 0121121 & 0.7 & 15 & 4400 & 8 & 8 & 1 & 1.5 & 0.65 & 0.11 & 0.44 & 0.66 & 0.25 & 0.71 & 0.71 & 0.71 & 0.01 & 0.01 & 455 & 455 & 0.33 & 0.000 $	M31122221	0.5	15	3 600) 10	10	1	2.5	3.32	0.59	0.22	0.66	0.17	0.06	0.03	0.26	0.56	0.13	0.004	52	75 52	14	65	0.13	0.0037	61	2	49
M4112222 0.7 15 3 600 10 1 2.5 4.06 0.75 0.29 0.68 0.16 0.06 0.02 0.20 0.47 0.14 0.004 56 56 16 46 0.14 0.0014 46 1 34 M51122221 0.9 15 3 001 10 1 2.5 59 0.67 0.7 0.0 0.4 0.66 0.30 0.00 397 73 86 0.10 0.003 88 3 57 37 M11211112 0.15 4400 8 66 1.5 0.74 0.05 0.01 0.17 0.71<	M41122222	0.7	15	3 600	10	10	1	1.5	3.00	0.56	0.21	0.68	0.16	0.00	0.02	0.28	0.40	0.12	0.003	83	83	24	67	0.13	0.0034	71	2	52
MS1122221 0.9 15 3 600 10 1 1.5 2.65 0.50 0.20 0.70 0.14 0.06 0.03 0.23 0.49 0.12 0.0003 97 7 38 69 0.10 0.033 57 2 37 M11211111 0.1 15 4.00 8 0.6 1.5 0.79 0.13 0.05 0.64 0.19 0.07 0.16 0.01 335 13 32.5 0.04 0.001 316 33 13 32.5 0.04 0.001 316 33 13 32.5 0.04 0.001 316 31 10 10.05 0.011 317 17 12 307 0.05 0.011 38 38 40 305 0.04 0.0009 342 32 0.04 0.0009 322 7 287 M31211112 0.3 5 4.00 8 0.6 2.5 0.78 0.10 0.06 0.27 0.71 0.01 0.01 371 77 328 0.04 <t< td=""><td>M41122222</td><td>0.7</td><td>15</td><td>3 600</td><td>) 10</td><td>10</td><td>1</td><td>2.5</td><td>4.06</td><td>0.75</td><td>0.29</td><td>0.68</td><td>0.16</td><td>0.06</td><td>0.02</td><td>0.20</td><td>0.47</td><td>0.14</td><td>0.004</td><td>56</td><td>56</td><td>16</td><td>46</td><td>0.14</td><td>0.0041</td><td>46</td><td>1</td><td>34</td></t<>	M41122222	0.7	15	3 600) 10	10	1	2.5	4.06	0.75	0.29	0.68	0.16	0.06	0.02	0.20	0.47	0.14	0.004	56	56	16	46	0.14	0.0041	46	1	34
MS122222 0.9 15 600 10 1 2.5 3.5 0.67 0.27 0.70 0.14 0.06 0.23 0.12 0.00 65 65 26 47 0.12 0.00039 57 2 3.7 M1121111 0.1 15 4 0.08 8 0.6 0.5 0.70 0.10 0.07 0.11 0.07 0.11 0.01 317 12 307 0.05 0.001 316 7 208 0.001 316 40 0.00 0.00 317 0.17 0.10 0.001 338 303 0.00 0.000 327 7 27 288 0.01 230 0.01 338 40 0.00 0.000 338 40 0.00 0.000 338 40 0.00 0.000 338 40 0.00 0.00 338 40 0.00 0.000 338 40 0.00 0.000 338 40 0.00 0.00 0.01 341 11 12 0.00 0.00 0.00 0	M51122221	0.9	15	3 600	10	10	1	1.5	2.65	0.50	0.20	0.70	0.14	0.06	0.04	0.31	0.60	0.10	0.003	97	97	38	69	0.10	0.0033	88	3	57
M1121111 0.1 15 4 400 8 8.0.6 1.5 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.19 0.70 0.11 0.01 315 315 13 325 0.04 0.0001 316 7 307 M11211112 0.3 15 4 0.8 8 0.6 1.5 0.74 0.13 0.05 0.65 0.18 0.70 0.71 0.71 0.04 0.001 338 338 40 305 0.04 0.009 322 7 278 M31211111 0.5 15 4400 8 8.06 1.5 0.66 0.17 0.66 0.21 0.71 0.71 0.04 0.001 371 371 71 71 370 0.03 0.0007 433 10 371 71 0.03 0.011 411 413 117 75 360 0.04 0.000 382 9 372 M12	M51122222	0.9	15	3 600	10	10	1	2.5	3.59	0.67	0.27	0.70	0.14	0.06	0.03	0.23	0.49	0.12	0.004	65	65	26	47	0.12	0.0039	57	2	37
M11211112 0.1 15 4 400 8 8 0.6 2.5 0.64 0.19 0.07 0.18 0.17 0.07 0.15 4.10 0.05 0.0011 2.96 7 2.88 M21211111 0.3 15 4.400 8 8 0.6 2.5 0.78 0.13 0.05 0.65 0.18 0.07 0.19 0.71 0.14 0.001 338 338 40 305 0.04 0.0009 322 7 2.87 M31211112 0.5 15 4400 8 8 0.6 1.5 0.66 0.17 0.06 0.23 0.71 0.71 0.44 0.001 371 371 77 336 0.40 0.0009 328 8 2.91 M1211111 0.7 15 4.400 8 8 0.6 1.5 0.66 0.17 0.66 0.23 0.71 0.71 0.01 3.11 7.7 3.6 0.40 0.000 3.0007 7.7 1.2 3.23 0.03 0.0007 7.7	M11211111	0.1	15	4 400	8	8	0.6	1.5	0.79	0.13	0.05	0.64	0.19	0.07	0.19	0.71	0.71	0.04	0.001	335	335	13	325	0.04	0.0010	316	7	307
Mail Mail <th< td=""><td>M21211112</td><td>0.1</td><td>15</td><td>4 400</td><td>1 8</td><td>8</td><td>0.6</td><td>1.5</td><td>0.84</td><td>0.14</td><td>0.05</td><td>0.64</td><td>0.19</td><td>0.07</td><td>0.18</td><td>0.71</td><td>0.71</td><td>0.03</td><td>0.001</td><td>359</td><td>359</td><td>43</td><td>323</td><td>0.03</td><td>0.0011</td><td>344</td><td>8</td><td>307</td></th<>	M21211112	0.1	15	4 400	1 8	8	0.6	1.5	0.84	0.14	0.05	0.64	0.19	0.07	0.18	0.71	0.71	0.03	0.001	359	359	43	323	0.03	0.0011	344	8	307
M31211111 0.5 15 4 400 8 8 0.6 1.5 0.66 0.17 0.06 0.23 0.71 0.71 0.04 0.001 394 394 79 324 0.04 0.0008 382 9 310 M31211112 0.5 15 4 00 8 8 0.6 2.5 0.66 0.17 0.06 0.21 0.71 0.71 0.04 0.001 371 371 75 366 0.04 0.009 358 8 29 310 M41211112 0.7 15 4 400 8 8 0.6 1.5 0.65 0.16 0.66 0.25 0.71 0.71 0.03 0.001 411 413 117 307 0.3 0.0007 451 131 M121112 0.9 5 4 400 8 8 1.5 0.65 0.11 0.40 0.64 0.19 0.7 0.23 0.71 0.71 0.03 0.001 482 475 320 0.03 0.000	M21211111 M21211112	0.3	15	4 400	8	8	0.6	2.5	0.74	0.13	0.05	0.65	0.18	0.07	0.19	0.71	0.71	0.04	0.001	338	338	40	305	0.04	0.0009	322	7	287
M3121112 0.5 1 0.0 0.6 0.7 0.0 0.21 0.71 0.01 371 371 <	M31211111	0.5	15	4 400	8 (8	0.6	1.5	0.68	0.12	0.05	0.66	0.17	0.06	0.23	0.71	0.71	0.04	0.001	394	394	79	324	0.04	0.0008	382	9	310
M4121111 0.7 15 4 4 8 8 0.6 1.1 0.4 0.68 0.16 0.06 0.27 0.71 0.71 0.03 0.001 441 441 125 328 0.03 0.0007 433 10 317 M41211112 0.7 15 4 00 8 8 0.6 1.5 0.54 0.10 0.04 0.70 0.71 0.71 0.00 413 413 117 307 0.03 0.0007 476 11 312 M51211111 0.9 15 4 00 8 8 0.6 2.5 0.58 0.11 0.04 0.64 0.90 0.71 0.71 0.00 482 482 1.55 320 0.03 0.0007 476 11 312 M1121112 0.1 5 4 00 8 1 2.5 0.65 0.11 0.40 0.65 0.18 0.70 0.71 0.01 483 439 53 394 0.3 0.0008 495 357 <td>M31211112</td> <td>0.5</td> <td>15</td> <td>4 400</td> <td>) 8</td> <td>8</td> <td>0.6</td> <td>2.5</td> <td>0.72</td> <td>0.13</td> <td>0.05</td> <td>0.66</td> <td>0.17</td> <td>0.06</td> <td>0.21</td> <td>0.71</td> <td>0.71</td> <td>0.04</td> <td>0.001</td> <td>371</td> <td>371</td> <td>75</td> <td>306</td> <td>0.04</td> <td>0.0009</td> <td>358</td> <td>8</td> <td>291</td>	M31211112	0.5	15	4 400) 8	8	0.6	2.5	0.72	0.13	0.05	0.66	0.17	0.06	0.21	0.71	0.71	0.04	0.001	371	371	75	306	0.04	0.0009	358	8	291
MAILTITIZ 0.7 15 4 4 8 0.6 2.5 0.56 0.12 0.05 0.68 0.16 0.06 0.27 0.71 0.71 0.03 0.001 413	M41211111	0.7	15	4 400	8	8	0.6	1.5	0.61	0.11	0.04	0.68	0.16	0.06	0.25	0.71	0.71	0.03	0.001	441	441	125	328	0.03	0.0007	433	10	317
Instruint and and bit	M41211112	0.7	15	4 400	8	8	0.6	2.5	0.65	0.12	0.05	0.58	0.16	0.06	0.23	0.71	0.71	0.03	0.001	413 512	413 512	11/	240	0.03	0.0008	507	12	295
M11211121 0.1 15 4 400 8 8 1 15 0.65 0.11 0.04 0.64 0.19 0.07 0.22 0.71 0.01 405 405 16 393 0.04 0.0008 389 9 378 M11211122 0.1 15 4 400 8 8 1 1.5 0.61 0.10 0.04 0.64 0.19 0.07 0.22 0.71 0.71 0.04 0.001 381 381 15 370 0.04 0.0008 427 10 381 M21211122 0.3 15 4 400 8 8 1 1.5 0.66 0.10 0.06 0.87 0.27 0.71 0.71 0.01 489 498 49 367 0.04 0.008 427 10 381 M31211121 0.5 15 4 400 8 1 1.5 0.56 0.10 0.40 0.66 0.71 0.71 0.01 482 452 91 371 0.3 0.0007 442 10	M51211112	0.9	15	4 400	8	8	0.6	2.5	0.54	0.10	0.04	0.70	0.14	0.06	0.23	0.71	0.71	0.03	0.001	482	482	175	320	0.03	0.0007	476	11	312
M11211122 0.1 1 5 0.70 0.11 0.4 0.64 0.19 0.07 0.22 0.71 0.01 381 381 15 370 0.04 0.009 364 8 355 M1211121 0.3 15 4 400 8 8 1 1.5 0.61 0.10 0.04 0.65 0.18 0.07 0.22 0.71 0.71 0.04 0.001 489 439 53 394 0.30 0.008 427 10 381 M1211121 0.5 5 4 400 8 1 1.5 0.55 0.11 0.44 0.66 0.77 0.71 0.71 0.01 488 498 367 0.04 0.000 375 13 381 15 370 0.44 0.000 375 371 0.3 0.001 488 488 498 367 0.04 0.000 475 11 383 381 15 371 0.3 0.001 488 488 98 36 0.3 0.001 430 <t< td=""><td>M11211121</td><td>0.1</td><td>15</td><td>4 400</td><td>8</td><td>8</td><td>1</td><td>1.5</td><td>0.65</td><td>0.11</td><td>0.04</td><td>0.64</td><td>0.19</td><td>0.07</td><td>0.23</td><td>0.71</td><td>0.71</td><td>0.04</td><td>0.001</td><td>405</td><td>405</td><td>16</td><td>393</td><td>0.04</td><td>0.0008</td><td>389</td><td>9</td><td>378</td></t<>	M11211121	0.1	15	4 400	8	8	1	1.5	0.65	0.11	0.04	0.64	0.19	0.07	0.23	0.71	0.71	0.04	0.001	405	405	16	393	0.04	0.0008	389	9	378
M21211121 0.3 15 4 400 8 8 1 1.5 0.61 0.01 0.04 0.65 0.18 0.07 0.25 0.71 0.71 0.70 0.00 439 439 53 394 0.03 0.0008 427 10 3812 M21211121 0.3 15 4 400 8 1 2.5 0.65 0.11 0.04 0.65 0.18 0.07 0.24 0.71 0.71 0.01 408 49 367 0.04 0.0008 395 9 352 M31211121 0.5 15 4 400 8 8 1 2.5 0.59 0.11 0.04 0.66 0.17 0.06 0.28 0.71 0.71 0.01 0.44 484 98 396 0.03 0.0007 442 10 358 M41211121 0.7 15 4 400 8 1 1.5 0.44 0.68 0.61 0.70 0.71 0.71 0.01 686 220 402 0.02 0.000 507 14	M11211122	0.1	15	4 400) 8	8	1	2.5	0.70	0.11	0.04	0.64	0.19	0.07	0.22	0.71	0.71	0.04	0.001	381	381	15	370	0.04	0.0009	364	8	355
M21211122 0.3 15 4 400 8 8 1 2.5 0.65 0.11 0.40 0.65 0.18 0.07 0.71 0.71 0.70 0.40 0.001 408 408 498 367 0.40 0.0008 395 9 352 M31211121 0.5 15 4 400 8 1 1.5 0.56 0.10 0.44 0.66 0.17 0.06 0.28 0.71 0.71 0.10 0.001 484 484 98 396 0.03 0.0007 475 11 385 M41211121 0.7 15 4 400 8 8 1 2.5 0.59 0.11 0.04 0.66 0.17 0.06 0.26 0.71 0.71 0.30 0.001 452 452 91 371 0.3 0.0007 442 10 33 0.00 0.00 0.01 450 0.03 0.0006 530 12 385 M4121112 0.7 15 4400 8 8 1 2.5 0.48 <td>M21211121</td> <td>0.3</td> <td>15</td> <td>4 400</td> <td>8</td> <td>8</td> <td>1</td> <td>1.5</td> <td>0.61</td> <td>0.10</td> <td>0.04</td> <td>0.65</td> <td>0.18</td> <td>0.07</td> <td>0.25</td> <td>0.71</td> <td>0.71</td> <td>0.03</td> <td>0.001</td> <td>439</td> <td>439</td> <td>53</td> <td>394</td> <td>0.03</td> <td>0.0008</td> <td>427</td> <td>10</td> <td>381</td>	M21211121	0.3	15	4 400	8	8	1	1.5	0.61	0.10	0.04	0.65	0.18	0.07	0.25	0.71	0.71	0.03	0.001	439	439	53	394	0.03	0.0008	427	10	381
M31211121 0.3 1.4 0.0 0.3 0.10 0.00	M21211122	0.3	15	4 400	8	8	1	2.5	0.65	0.11	0.04	0.65	0.18	0.07	0.24	0.71	0.71	0.04	0.001	408	408	49	367	0.04	0.0008	395	11	352
Matrial 0.7 15 4 400 8 1 15 0.50 0.11 0.11 0.06 0.12 0.11 <th0.11< th=""> <th0.11< th=""> <th0.11<< td=""><td>M31211121</td><td>0.5</td><td>15</td><td>4 400</td><td>8</td><td>8</td><td>1</td><td>2.5</td><td>0.50</td><td>0.10</td><td>0.04</td><td>0.00</td><td>0.17</td><td>0.00</td><td>0.28</td><td>0.71</td><td>0.71</td><td>0.05</td><td>0.001</td><td>464</td><td>464</td><td>98</td><td>371</td><td>0.03</td><td>0.0007</td><td>473</td><td>10</td><td>358</td></th0.11<<></th0.11<></th0.11<>	M31211121	0.5	15	4 400	8	8	1	2.5	0.50	0.10	0.04	0.00	0.17	0.00	0.28	0.71	0.71	0.05	0.001	464	464	98	371	0.03	0.0007	473	10	358
M41211122 0.7 15 4 400 8 8 1 2.5 0.54 0.10 0.04 0.68 0.16 0.06 0.29 0.71 0.71 0.03 0.001 507 507 143 375 0.03 0.006 499 12 365 M51211121 0.9 15 4 0.08 8 1 1.5 0.44 0.88 0.03 0.70 0.14 0.06 0.24 0.71 0.71 0.02 0.001 507 577 143 375 0.03 0.006 499 12 365 M51211121 0.9 15 4 0.8 0.1 5.0 0.01 0.01 577 577 209 381 0.02 0.006 572 1.3 374 M11211211 0.1 15 4 0.8 10 6.5 2.0 0.33 0.12 0.64 0.19 0.07 0.71 0.10 0.002 133 133 5 128 0.10 0.0022 111 3 109	M41211121	0.7	15	4 400) 8	8	1	1.5	0.50	0.09	0.04	0.68	0.16	0.06	0.31	0.71	0.71	0.03	0.001	537	537	152	397	0.03	0.0006	530	12	388
MS1211121 0.9 15 4 0.0 8 8 1 1.5 0.44 0.88 0.03 0.70 0.14 0.06 0.24 0.71 0.71 0.02 0.001 608 608 220 402 0.20 0.0005 604 14 395 M51211122 0.9 15 4 00 8 0.7 0.71 0.71 0.71 0.02 0.001 677 577 209 381 0.02 0.0005 572 13 374 M11211211 0.1 15 4 0.8 10 6 1.5 1.88 0.31 0.11 0.64 0.9 0.07 0.70 0.71 0.10 0.002 133 133 5 128 0.10 0.0023 112 3 118 M1212121 0.3 15 4 0.8 10 6 1.5 1.80 0.70 0.70 0.44 0.71 0.00 0.002 149 149 147 136 0.9 0.002 112 3 110 1	M41211122	0.7	15	4 400	8 (8	1	2.5	0.54	0.10	0.04	0.68	0.16	0.06	0.29	0.71	0.71	0.03	0.001	507	507	143	375	0.03	0.0006	499	12	365
M51211122 0.9 15 4 4 8 8 1 2.5 0.48 0.99 0.40 0.70 0.14 0.60 0.27 0.71 0.71 0.71 0.70 0.77 577 209 381 0.02 0.000 572 13 374 M11211211 0.1 15 4 00 8 10 1.5 1.88 0.31 0.11 0.64 0.9 0.07 0.70 0.45 0.71 0.10 0.002 142 142 5 137 0.10 0.0022 121 3 118 M11211212 0.1 1.5 4 00 8 10 0.6 1.5 1.75 0.30 0.11 0.65 0.18 0.07 0.07 0.45 0.71 0.10 0.002 149 149 17 136 0.09 0.002 112 3 118 M11211212 0.3 1.5 4 0.6 1.5 1.66 0.07 0.07 0.44 0.71 0.09 0.02 159 139 <	M51211121	0.9	15	4 400	8	8	1	1.5	0.44	0.08	0.03	0.70	0.14	0.06	0.34	0.71	0.71	0.02	0.001	608	608	220	402	0.02	0.0005	604	14	395
M11211212 0.1 15 4 4 08 10 0.6 1.5 1.0 0.0 1.0 0.0	M51211122	0.9	15	4 400	8	8	1	2.5	0.48	0.09	0.04	0.70	0.14	0.06	0.32	0.71	0.71	0.02	0.001	577	577	209	381	0.02	0.0006	572	13	374
M121211211 0.3 15 4 4 0.6 1.5 1.75 0.10 0.07 0.04 0.71 0.09 0.002 139 155 155 5 128 0.09 0.0019 133 3 118 M21211212 0.3 5 4 0.6 1.5 1.60 0.29 0.11 0.66 0.07 0.07 0.44 0.17 0.09 0.002 139 16 128 0.09 0.0017 147 3 120 M31211211 0.7 5 4 400 8 10 6. 1.5 1.70 0.66 0.07 0.06 0.09 1.07 0.09 0.002	M11211211 M11211212	0.1	15	4 400	8	10	0.6	1.5	1.88	0.31	0.11	0.64	0.19	0.07	0.07	0.45	0.71	0.10	0.002	142	142	5	137	0.10	0.0022	121	3	118
M21211212 0.3 15 4 400 8 10 6.6 2.5 1.87 0.12 0.65 0.18 0.70 0.74 0.71 0.09 0.002 139 136 16 128 0.09 0.0021 123 3 110 M31211211 0.5 15 4 400 8 10 6.6 1.5 1.60 0.79 0.41 0.66 0.09 0.47 0.71 0.09 0.002 159 139 16 128 0.09 0.0021 147 3 120 M31211212 0.5 15 4 400 8 10 6.6 1.5 1.06 0.17 0.06 0.08 0.45 0.71 0.09 0.002 150 30 127 0.99 0.0019 137 3 111 M41211211 0.7 15 4 400 8 10 6.6 1.5 0.66 0.06 0.09 0.47 0.71 0.09 0.002 150 30 127 0.09 0.001 137 <	M21211212	0.1	15	4 400	8	10	0.6	1.5	1.75	0.30	0.12	0.65	0.19	0.07	0.07	0.45	0.71	0.09	0.002	149	149	17	136	0.09	0.0025	133	3	119
M31211211 0.5 15 4 400 8 10 6.6 1.5 1.60 0.79 0.11 0.66 0.07 0.06 0.90 0.47 0.71 0.08 0.002 161 161 32 136 0.08 0.0017 147 3 120 M31211212 0.5 15 4 400 8 10 6.2 1.72 0.31 0.11 0.66 0.17 0.06 0.08 0.47 0.71 0.09 0.002 150 30 127 0.9 0.0019 137 3 111 M4121121 0.7 15 4 400 8 10 6.6 1.55 0.66 0.16 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.66 0.06 0.049 0.71 0.09 0.001 178 178 137 0.7 0.06 0.06 0.040 0.7 0.06 0.002 166 166 47 128 0.0	M21211212	0.3	15	4 400	8	10	0.6	2.5	1.87	0.32	0.12	0.65	0.18	0.07	0.07	0.44	0.71	0.09	0.002	139	139	16	128	0.09	0.0021	123	3	110
M31211212 0.5 15 4 400 8 10 6.6 2.5 1.7 0.11 0.66 0.17 0.06 0.80 0.45 0.71 0.09 0.002 150 30 127 0.9 0.0019 137 3 111 M41211211 10.7 15 4 400 8 10 6.6 1.5 0.66 0.10 0.49 0.71 0.09 0.002 150 30 127 0.9 0.0019 137 3 111 M41211211 0.7 15 4 400 8 10 6.6 2.5 1.6 0.68 0.69 0.49 0.71 0.09 0.002 178 178 18 137 0.7 0.06 0.00 0.49 0.71 0.08 0.002 168 166 4 128 0.8 0.001 15 4 0.0 0.001 15 4 0.0 0.00 0.04 0.71 0.08 0.002 166 166 47 128 0.8 0.001 155 4 <td>M31211211</td> <td>0.5</td> <td>15</td> <td>4 400</td> <td>8</td> <td>10</td> <td>0.6</td> <td>1.5</td> <td>1.60</td> <td>0.29</td> <td>0.11</td> <td>0.66</td> <td>0.17</td> <td>0.06</td> <td>0.09</td> <td>0.47</td> <td>0.71</td> <td>0.08</td> <td>0.002</td> <td>161</td> <td>161</td> <td>32</td> <td>136</td> <td>0.08</td> <td>0.0017</td> <td>147</td> <td>3</td> <td>120</td>	M31211211	0.5	15	4 400	8	10	0.6	1.5	1.60	0.29	0.11	0.66	0.17	0.06	0.09	0.47	0.71	0.08	0.002	161	161	32	136	0.08	0.0017	147	3	120
M41211211 0.7 15 4 400 8 10 0.6 1.5 1.45 0.27 0.10 0.68 0.16 0.06 0.10 0.49 0.71 0.07 0.002 178 178 51 137 0.07 0.0016 168 4 123 M41211212 0.7 15 4 400 8 10 0.6 2.5 1.55 0.29 0.11 0.68 0.16 0.06 0.09 0.47 0.71 0.08 0.002 166 166 47 128 0.08 0.0017 155 4 114 M51211211 0.9 15 4 400 8 10 0.6 1.5 1.28 0.24 0.10 0.70 0.14 0.06 0.11 0.53 0.71 0.06 0.001 205 205 77 140 0.06 0.0015 197 5 129	M31211212	0.5	15	4 400	8	10	0.6	2.5	1.72	0.31	0.11	0.66	0.17	0.06	0.08	0.45	0.71	0.09	0.002	150	150	30	127	0.09	0.0019	137	3	111
M*1211211 0.7 13 4 400 8 10 0.6 2.3 1.55 0.29 0.11 0.68 0.10 0.09 0.49 0.71 0.08 0.002 100 100 47 128 0.08 0.0017 155 4 114 M*1211211 0.9 15 4 400 8 10 0.6 15 1.28 0.24 0.10 0.70 0.14 0.06 0.11 0.53 0.71 0.06 0.001 205 205 77 140 0.06 0.0015 197 5 129	M41211211	0.7	15	4 400	8	10	0.6	1.5	1.45	0.27	0.10	0.68	0.16	0.06	0.10	0.49	0.71	0.07	0.002	178	178	51	137	0.07	0.0016	168	4	123
	M51211212	0.7	15	4 400	8	10	0.0	2.5	1.35	0.29	0.11	0.08	0.10	0.06	0.09	0.47	0.71	0.08	0.002	205	205	47	128	0.08	0.0017	197	4	114

Name	af nf	hf	mf L	a Lx	tw	Lch	Tl [s]	T2 [s]	T3 [s]	MI FI	M2 FT	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Diso	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
Indito				.,		Lies	* * [9]	101				110			000	[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
M51211212	0.9 15	4	400	8 10	0.6	2.5	1.37	0.26	0.10	0.70	0.14	0.06	0.10	0.50	0.71	0.07	0.002	190	190	72	131	0.07	0.0015	182	4	119
M11211221 M11211222	0.1 1	4	400	8 10	1	2.5	1.72	0.28	0.10	0.64	0.19	0.07	0.08	0.48	0.71	0.09	0.002	155	145	5	140	0.09	0.0020	134	3	130
M21211222	0.1 1.	4	400	8 10	1	1.5	1.64	0.30	0.11	0.65	0.19	0.07	0.07	0.40	0.71	0.09	0.002	163	163	19	140	0.09	0.0021	124	3	120
M21211222	0.3 1	4	400	8 10	1	2.5	1.72	0.29	0.11	0.65	0.18	0.07	0.08	0.47	0.71	0.09	0.002	152	152	17	139	0.09	0.0019	135	3	121
M31211221	0.5 15	4	400	8 10	1	1.5	1.47	0.26	0.10	0.66	0.17	0.06	0.10	0.50	0.71	0.07	0.002	176	176	35	148	0.07	0.0016	163	4	132
M31211222	0.5 15	4	400	8 10	1	2.5	1.58	0.28	0.10	0.66	0.17	0.06	0.09	0.48	0.71	0.08	0.002	164	164	32	138	0.08	0.0017	150	3	122
M41211221	0.7 15	4	400	8 10	1	1.5	1.33	0.25	0.09	0.68	0.16	0.06	0.11	0.52	0.71	0.07	0.001	196	196	56	150	0.07	0.0015	185	4	135
M41211222	0.7 13	4	400	8 10	1	2.5	1.43	0.26	0.10	0.58	0.16	0.06	0.10	0.49	0.71	0.07	0.002	182	182	52	139	0.07	0.0016	1/1	4 r	142
M51211221 M51211222	0.9 1	4	400	8 10	1	2.5	1.26	0.22	0.09	0.70	0.14	0.00	0.12	0.54	0.71	0.00	0.001	209	209	79	143	0.00	0.0014	218	5	131
M11212111	0.1 15	4	400 1	0 8	0.6	1.5	1.11	0.18	0.06	0.64	0.19	0.07	0.13	0.64	0.71	0.06	0.001	241	241	9	233	0.06	0.0014	219	5	213
M11212112	0.1 15	4	400 1	0 8	0.6	2.5	1.26	0.21	0.07	0.64	0.19	0.07	0.11	0.59	0.71	0.07	0.002	211	211	8	204	0.07	0.0015	189	4	183
M21212111	0.3 15	4	400 1	0 8	0.6	1.5	1.03	0.18	0.06	0.65	0.18	0.07	0.14	0.64	0.71	0.06	0.001	257	257	30	233	0.06	0.0012	240	5	214
M21212112	0.3 1	4	400 1	0 8	0.6	2.5	1.1/	0.20	0.07	0.65	0.18	0.07	0.12	0.60	0.71	0.06	0.001	225	225	26	204	0.06	0.0014	207	5	185
M31212111 M31212112	0.5 1	4	400 1	0 8	0.0	2.5	1.08	0.17	0.00	0.00	0.17	0.00	0.10	0.00	0.71	0.05	0.001	201	201		205	0.03	0.0011	207	5	188
M41212111	0.7 15	4	400 1	0 8	0.6	1.5	0.86	0.16	0.06	0.68	0.16	0.06	0.17	0.67	0.71	0.05	0.001	312	312	89	235	0.05	0.0010	301	7	220
M41212112	0.7 15	4	400 1	0 8	0.6	2.5	0.98	0.18	0.07	0.68	0.16	0.06	0.15	0.64	0.71	0.05	0.001	274	274	78	207	0.05	0.0011	263	6	193
M51212111	0.9 15	4	400 1	0 8	0.6	1.5	0.76	0.14	0.06	0.70	0.14	0.06	0.20	0.70	0.71	0.04	0.001	357	357	132	240	0.04	0.0009	350	8	229
M51212112	0.9 15	4	400 1	0 8	0.6	2.5	0.86	0.16	0.06	0.70	0.14	0.06	0.17	0.67	0.71	0.04	0.001	315	315	117	212	0.04	0.0010	307	7	201
M1121212121 M11212122	0.1 15	4	400 1 400 1	08	1	1.5	0.95	0.15	0.06	0.64	0.19	0.07	0.16	0.68	0.71	0.05	0.001	283	283	11	275	0.05	0.0012	262	5	255
M2121212121	0.3 1	4	400 1	0 8	1	1.5	0.88	0.15	0.00	0.65	0.19	0.07	0.13	0.69	0.71	0.05	0.001	301	301	36	250	0.05	0.0014	285	6	210
M21212122	0.3 15	4	400 1	0 8	1	2.5	1.02	0.17	0.06	0.65	0.18	0.07	0.14	0.65	0.71	0.06	0.001	260	260	31	235	0.06	0.0012	243	6	216
M31212121	0.5 15	4	400 1	.0 8	1	1.5	0.81	0.14	0.05	0.66	0.17	0.06	0.19	0.70	0.71	0.04	0.001	330	330	66	273	0.04	0.0010	316	7	257
M31212122	0.5 15	4	400 1	0 8	1	2.5	0.94	0.17	0.06	0.66	0.17	0.06	0.16	0.66	0.71	0.05	0.001	284	284	57	236	0.05	0.0011	270	6	219
M41212121	0.7 15	4	400 1	0 8	1	1.5	0.73	0.14	0.05	0.68	0.16	0.06	0.21	0.71	0.71	0.04	0.001	368	368	104	275	0.04	0.0009	358	8	262
M51212122	0.7 1	4	400 1	0 8	1	1.5	0.65	0.10	0.00	0.08	0.10	0.00	0.18	0.08	0.71	0.04	0.001	422	422	154	257	0.04	0.0010	416	10	225
M51212122	0.9 15	4	400 1	0 8	1	2.5	0.75	0.14	0.06	0.70	0.14	0.06	0.20	0.71	0.71	0.04	0.001	362	362	133	243	0.04	0.0009	355	8	232
M11212211	0.1 15	4	400 1	0 10	0.6	1.5	2.78	0.45	0.16	0.64	0.19	0.07	0.04	0.34	0.67	0.12	0.003	89	89	3	86	0.12	0.0027	68	2	67
M11212212	0.1 15	4	400 1	0 10	0.6	2.5	3.28	0.53	0.19	0.64	0.19	0.07	0.03	0.29	0.62	0.13	0.003	73	73	2	71	0.13	0.0029	54	1	52
M21212211	0.3 15	4	400 1	0 10	0.6	1.5	2.59	0.44	0.16	0.65	0.18	0.07	0.05	0.34	0.67	0.11	0.003	93	93	10	86	0.11	0.0025	77	2	69
M312122212	0.5 1	4	400 1	0 10	0.6	1.5	2.38	0.32	0.19	0.65	0.18	0.07	0.04	0.30	0.62	0.12	0.005	102	102	20	88	0.12	0.0027	88	2	72
M31212212	0.5 15	4	400 1	0 10	0.6	2.5	2.80	0.50	0.19	0.66	0.17	0.06	0.04	0.31	0.62	0.12	0.003	83	83	16	72	0.12	0.0025	69	2	56
M41212211	0.7 15	4	400 1	0 10	0.6	1.5	2.15	0.40	0.15	0.68	0.16	0.06	0.06	0.37	0.68	0.10	0.002	115	115	33	90	0.10	0.0022	105	2	77
M41212212	0.7 15	4	400 1	0 10	0.6	2.5	2.53	0.47	0.18	0.68	0.16	0.06	0.05	0.33	0.63	0.11	0.002	92	92	26	73	0.11	0.0024	82	2	60
M51212211	0.9 15	4	400 1	0 10	0.6	1.5	1.90	0.36	0.14	0.70	0.14	0.06	0.07	0.41	0.70	0.09	0.002	135	135	52	94	0.09	0.0021	127	3	83
M11212212	0.9 1:	4	400 1	0 10	0.6	2.5	2.24	0.42	0.17	0.70	0.14	0.06	0.06	0.36	0.66	0.10	0.002	109	96	42	93	0.10	0.0023	75	2	73
M11212222	0.1 15	4	400 1	0 10	1	2.5	3.12	0.51	0.18	0.64	0.19	0.07	0.03	0.30	0.63	0.13	0.003	78	78	3	75	0.13	0.0028	58	1	56
M21212221	0.3 15	4	400 1	0 10	1	1.5	2.42	0.41	0.15	0.65	0.18	0.07	0.05	0.36	0.69	0.11	0.003	101	101	11	93	0.11	0.0024	85	2	76
M21212222	0.3 15	4	400 1	0 10	1	2.5	2.90	0.49	0.18	0.65	0.18	0.07	0.04	0.31	0.64	0.12	0.003	81	81	9	75	0.12	0.0026	65	1	58
M31212221	0.5 15	4	400 1	0 10	1	1.5	2.23	0.40	0.15	0.66	0.17	0.06	0.06	0.37	0.69	0.10	0.002	111	111	22	95	0.10	0.0022	98	2	79
M41212222	0.5 1	4 . A	400 1	0 10	1	2.5	2.67	0.48	0.18	0.66	0.17	0.06	0.04	0.32	0.64	0.11	0.003	126	126	36	98	0.11	0.0024	116	2	85
M41212222	0.7 15	4	400 1	0 10	1	2.5	2.41	0.45	0.17	0.68	0.16	0.06	0.05	0.34	0.65	0.11	0.002	99	99	28	78	0.10	0.0023	88	2	64
M51212221	0.9 15	4	400 1	0 10	1	1.5	1.78	0.33	0.13	0.70	0.14	0.06	0.08	0.43	0.71	0.08	0.002	145	145	55	101	0.08	0.0020	136	3	89
M51212222	0.9 15	4	400 1	0 10	1	2.5	2.13	0.40	0.16	0.70	0.14	0.06	0.06	0.37	0.67	0.09	0.002	117	117	45	82	0.09	0.0022	109	3	71
M11221111	0.1 15	4	600	8 8	0.6	1.5	0.97	0.16	0.06	0.64	0.19	0.07	0.15	0.67	0.71	0.05	0.001	414	414	16	401	0.05	0.0012	382	9	372
M21221112	0.1 15	4	600	88	0.6	2.5	0.90	0.17	0.06	0.65	0.19	0.07	0.14	0.68	0.71	0.06	0.001	389	389	52	3//	0.05	0.0013	357 415	8	34/
M21221112	0.3 1	4	600	8 8	0.6	2.5	0.96	0.16	0.06	0.65	0.18	0.07	0.16	0.67	0.71	0.05	0.001	416	416	49	376	0.05	0.0011	391	9	348
M31221111	0.5 15	4	600	8 8	0.6	1.5	0.83	0.15	0.06	0.66	0.17	0.06	0.18	0.69	0.71	0.05	0.001	482	482	97	399	0.05	0.0010	462	11	374
M31221112	0.5 15	4	600	8 8	0.6	2.5	0.88	0.16	0.06	0.66	0.17	0.06	0.17	0.68	0.71	0.05	0.001	451	451	90	374	0.05	0.0010	431	10	349
M41221111	0.7 15	4	600	8 8	0.6	1.5	0.75	0.14	0.05	0.68	0.16	0.06	0.20	0.71	0.71	0.04	0.001	536	536	152	401	0.04	0.0009	520	12	380
M51221112	0.7 15	4	600 600	88	0.6	2.5	0.80	0.15	0.05	0.68	0.16	0.06	0.19	0.69	0.71	0.04	0.001	619	610	226	3/8	0.04	0.0009	489	11	358
M51221112	0.9 1	4	600	8 8	0.6	2.5	0.70	0.12	0.05	0.70	0.14	0.06	0.22	0.71	0.71	0.04	0.001	583	583	214	390	0.04	0.0009	572	13	374
M11221121	0.1 1	4	600	8 8	1	1.5	0.80	0.13	0.05	0.64	0.19	0.07	0.19	0.71	0.71	0.04	0.001	500	500	20	485	0.04	0.0010	471	11	458
M11221122	0.1 15	4	600	8 8	1	2.5	0.85	0.14	0.05	0.64	0.19	0.07	0.18	0.71	0.71	0.05	0.001	470	470	18	456	0.05	0.0011	439	10	427
M21221121	0.3 15	4	600	8 8	1	1.5	0.74	0.13	0.05	0.65	0.18	0.07	0.20	0.71	0.71	0.04	0.001	535	535	64	482	0.04	0.0009	513	12	457
M31221122	0.3 15	4	600 600	8 8	1	2.5	0.79	0.13	0.05	0.65	0.18	0.07	0.19	0.71	0.71	0.04	0.001	502	502	60 119	453	0.04	0.0010	4/8 570	11	427
M31221121	0.5 1	4	600	8 8	1	2.5	0.73	0.12	0.05	0.66	0.17	0.06	0.22	0.71	0.71	0.04	0.001	550	550	110	453	0.04	0.0009	531	12	402
M41221121	0.7 15	4	600	8 8	1	1.5	0.62	0.11	0.04	0.68	0.16	0.06	0.25	0.71	0.71	0.03	0.001	657	657	186	488	0.03	0.0007	645	15	471
M41221122	0.7 15	4	600	8 8	1	2.5	0.66	0.12	0.05	0.68	0.16	0.06	0.23	0.71	0.71	0.04	0.001	613	613	174	457	0.04	0.0008	600	14	439
M51221121	0.9 15	4	600	8 8	1	1.5	0.54	0.10	0.04	0.70	0.14	0.06	0.29	0.71	0.71	0.03	0.001	764	764	277	506	0.03	0.0007	756	18	495
M51221122 M11221211	0.9 15	4	600 600	8 10	06	2.5	0.58	0.11	0.04	0.70	0.14	0.06	0.27	0.71	0.71	0.03	0.001	715	/15	260	475	0.03	0.0007	706	17	462
11111221211	0.1 1.	-	000	0 10	0.0	1.0	2.50	0.57	0.15	0.04	0.19	0.07	0.00	0.53	0.71	0.11	0.005	100	100	0	102	0.11	0.0024	1.57	2	roo

Name	af n	մհ	fmf	Lv Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2 Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
144004040	0.4.4	1.5		0.40	0.6	2.5	2.46	0.10	0.44	0.64	0.40	0.07	0.05	0.37 0.70		0.003	[MN-m]	[MIN]	[MN-m]	[MN-m]	[m]	SDR [-]	[MN-m]	(MIN)	m]
M11221212 M21221211	0.1 1	15	4 600	8 10	0.6	2.5	2.46	0.40	0.14	0.64	0.19	0.07	0.05	0.37 0.70	0.11	0.003	154	154	20	149	0.11	0.0025	124	3	120
M21221211 M21221212	0.3 1	15	4 600	8 10	0.6	2.5	2.29	0.39	0.13	0.65	0.18	0.07	0.06	0.37 0.70	0.10	0.002	163	163	18	150	0.10	0.0023	139	3	138
M31221211	0.5 1	15	4 600	8 10	0.6	1.5	1.97	0.35	0.13	0.66	0.17	0.06	0.07	0.41 0.71	0.10	0.002	197	197	39	167	0.10	0.0021	177	4	143
M31221212	0.5 1	15 -	4 600	8 10	0.6	2.5	2.10	0.37	0.14	0.66	0.17	0.06	0.06	0.39 0.71	0.10	0.002	180	180	35	154	0.10	0.0022	161	4	130
M41221211	0.7 1	15 -	4 600	8 10	0.6	1.5	1.78	0.33	0.13	0.68	0.16	0.06	0.08	0.43 0.71	0.09	0.002	217	217	62	168	0.09	0.0019	201	5	147
M51221212	0.7 1	15	4 600	8 10	0.6	2.5	1.90	0.35	0.14	0.68	0.16	0.06	0.07	0.41 0.71	0.09	0.002	202	202	58	157	0.09	0.0020	235	4	136
M51221211 M51221212	0.9 1	15	4 600	8 10	0.6	2.5	1.68	0.31	0.12	0.70	0.14	0.06	0.03	0.44 0.71	0.07	0.002	231	231	88	160	0.07	0.0019	233	5	143
M11221221	0.1 1	15 -	4 600	8 10	1	1.5	2.10	0.34	0.12	0.64	0.19	0.07	0.06	0.42 0.71	0.10	0.002	188	188	7	182	0.10	0.0023	157	4	153
M11221222	0.1 1	15 -	4 600	8 10	1	2.5	2.26	0.37	0.13	0.64	0.19	0.07	0.06	0.39 0.71	0.11	0.003	172	172	6	166	0.11	0.0024	141	3	137
M21221221	0.3 1	15	4 600	8 10	1	1.5	1.96	0.33	0.12	0.65	0.18	0.07	0.07	0.43 0.71	0.10	0.002	200	200	23	183	0.10	0.0021	175	4	156
M21221222 M31221221	0.3 1	15	4 600	8 10	1	2.5	2.10	0.36	0.13	0.65	0.18	0.07	0.06	0.40 0.71	0.10	0.002	215	215	42	168	0.10	0.0022	159	4	142
M31221222	0.5 1	15	4 600	8 10	1	2.5	1.93	0.34	0.13	0.66	0.17	0.06	0.07	0.42 0.71	0.10	0.002	200	200	39	170	0.10	0.0021	180	4	146
M41221221	0.7 1	15	4 600	8 10	1	1.5	1.63	0.30	0.12	0.68	0.16	0.06	0.09	0.46 0.71	0.08	0.002	237	237	68	183	0.08	0.0018	221	5	162
M41221222	0.7 1	15 -	4 600	8 10	1	2.5	1.75	0.32	0.12	0.68	0.16	0.06	0.08	0.44 0.71	0.09	0.002	221	221	63	171	0.08	0.0019	205	5	150
M51221221	0.9 1	15 -	4 600	8 10	1	1.5	1.44	0.27	0.11	0.70	0.14	0.06	0.10	0.49 0.71	0.07	0.002	272	272	103	187	0.07	0.0016	260	6	170
M112221222	0.9 1	15	4 600	10 8	0.6	2.5	1.54	0.29	0.12	0.70	0.14	0.06	0.09	0.47 0.71	0.07	0.002	252	252	96	284	0.07	0.0017	240	6	253
M11222112	0.1 1	15	4 600	10 8	0.6	2.5	1.54	0.25	0.09	0.64	0.19	0.07	0.09	0.51 0.71	0.08	0.002	257	257	10	249	0.08	0.0018	226	5	220
M21222111	0.3 1	15	4 600	10 8	0.6	1.5	1.27	0.22	0.08	0.65	0.18	0.07	0.11	0.57 0.71	0.07	0.001	311	311	36	282	0.07	0.0015	285	6	254
M21222112	0.3 1	15	4 600	10 8	0.6	2.5	1.44	0.24	0.09	0.65	0.18	0.07	0.10	0.52 0.71	0.07	0.002	272	272	32	248	0.07	0.0016	247	6	220
M31222111	0.5 1	15	4 600	10 8	0.6	1.5	1.16	0.21	0.08	0.66	0.17	0.06	0.12	0.59 0.71	0.06	0.001	339	339	68	283	0.06	0.0013	318	7	258
M41222112	0.5 1	15 15	4 600	10 8	0.6	2.5	1.32	0.24	0.09	0.68	0.17	0.06	0.11	0.54 0.71	0.07	0.002	296	380	108	248	0.07	0.0015	363	6 8	223
M41222112	0.7 1	15 -	4 600	10 8	0.6	2.5	1.19	0.22	0.09	0.68	0.16	0.06	0.12	0.56 0.71	0.06	0.001	330	330	94	251	0.06	0.0013	314	7	229
M51222111	0.9 1	15 -	4 600	10 8	0.6	1.5	0.93	0.17	0.07	0.70	0.14	0.06	0.16	0.65 0.71	0.05	0.001	437	437	162	295	0.05	0.0011	425	10	278
M51222112	0.9 1	15	4 600	10 8	0.6	2.5	1.06	0.20	0.08	0.70	0.14	0.06	0.14	0.61 0.71	0.05	0.001	382	382	143	259	0.05	0.0012	370	9	242
M11222121	0.1 1	15 -	4 600	10 8	1	1.5	1.16	0.19	0.07	0.64	0.19	0.07	0.13	0.62 0.71	0.06	0.001	346	346	13	335	0.06	0.0014	312	6	304
M21222122 M21222121	0.1 1	15	4 600	10 8	1	1.5	1.08	0.22	0.08	0.65	0.19	0.07	0.11	0.63 0.71	0.07	0.002	369	369	44	335	0.07	0.0010	343	8	306
M21222122	0.3 1	15	4 600	10 8	1	2.5	1.25	0.21	0.08	0.65	0.18	0.07	0.11	0.58 0.71	0.07	0.001	314	314	37	286	0.07	0.0014	288	7	257
M31222121	0.5 1	15 -	4 600	10 8	1	1.5	0.99	0.18	0.07	0.66	0.17	0.06	0.15	0.64 0.71	0.05	0.001	403	403	81	335	0.05	0.0011	382	9	309
M31222122	0.5 1	15 -	4 600	10 8	1	2.5	1.15	0.21	0.08	0.66	0.17	0.06	0.13	0.59 0.71	0.06	0.001	343	343	68	287	0.06	0.0013	321	7	261
M41222121	0.7 1	15	4 600	10 8	1	2.5	0.90	0.17	0.05	0.68	0.16	0.06	0.17	0.65 0.71	0.05	0.001	384	384	109	291	0.05	0.0010	431	8	315
M51222122	0.9 1	15	4 600	10 8	1	1.5	0.79	0.15	0.06	0.70	0.14	0.06	0.19	0.69 0.71	0.04	0.001	515	515	190	346	0.04	0.0009	504	12	330
M51222122	0.9 1	15 -	4 600	10 8	1	2.5	0.92	0.17	0.07	0.70	0.14	0.06	0.16	0.65 0.71	0.05	0.001	442	442	164	298	0.05	0.0011	430	10	281
M11222211	0.1 1	15 -	4 600	10 10	0.6	1.5	3.41	0.55	0.20	0.64	0.19	0.07	0.03	0.28 0.60	0.13	0.003	105	105	3	101	0.13	0.0030	76	2	74
M11222212	0.1 1	15 -	4 600	10 10	0.6	2.5	4.01	0.65	0.23	0.64	0.19	0.07	0.02	0.23 0.54	0.15	0.004	86	100	3	83	0.15	0.0033	61	1	59
M21222211 M21222212	0.5 1	15	4 600	10 10	0.0	2.5	3.17	0.54	0.20	0.65	0.18	0.07	0.03	0.29 0.01	0.15	0.003	109	109	12	83	0.15	0.0027	68	2	70
M31222211	0.5 1	15	4 600	10 10	0.6	1.5	2.91	0.52	0.19	0.66	0.17	0.06	0.04	0.30 0.61	0.12	0.003	118	118	23	102	0.12	0.0026	98	2	79
M31222212	0.5 1	15 -	4 600	10 10	0.6	2.5	3.43	0.61	0.23	0.66	0.17	0.06	0.03	0.25 0.55	0.13	0.003	96	96	18	84	0.13	0.0028	77	2	63
M41222211	0.7 1	15	4 600	10 10	0.6	1.5	2.63	0.49	0.19	0.68	0.16	0.06	0.04	0.32 0.62	0.11	0.002	131	131	38	105	0.11	0.0024	116	3	85
M41222212	0.7 1	15	4 600	10 10	0.6	2.5	3.10	0.57	0.22	0.68	0.16	0.06	0.04	0.27 0.56	0.12	0.003	106	106	30	110	0.12	0.0026	142	2	66
M51222211 M51222212	0.9 1	15	4 600	10 10	0.6	2.5	2.33	0.51	0.18	0.70	0.14	0.00	0.03	0.30 0.59	0.10	0.002	133	124	49	89	0.10	0.0025	143	3	73
M11222221	0.1 1	15	4 600	10 10	1	1.5	3.19	0.52	0.19	0.64	0.19	0.07	0.03	0.30 0.63	0.13	0.003	114	114	4	109	0.13	0.0029	84	2	82
M11222222	0.1 1	15	4 600	10 10	1	2.5	3.82	0.62	0.22	0.64	0.19	0.07	0.03	0.25 0.56	0.14	0.004	92	92	3	88	0.14	0.0032	65	1	63
M21222221	0.3 1	15	4 600	10 10	1	1.5	2.97	0.51	0.18	0.65	0.18	0.07	0.04	0.31 0.63	0.12	0.003	118	118	13	110	0.12	0.0027	94	2	84
M31222222	0.3 1	15	4 600	10 10	1	2.5	2.73	0.61	0.22	0.65	0.18	0.07	0.03	0.25 0.56	0.13	0.003	95 128	128	25	111	0.13	0.0029	108	2	88
M31222222	0.5 1	15	4 600	10 10	1	2.5	3.27	0.58	0.22	0.66	0.17	0.06	0.03	0.27 0.57	0.13	0.003	102	102	19	89	0.13	0.0027	83	2	67
M41222221	0.7 1	15	4 600	10 10	1	1.5	2.47	0.46	0.18	0.68	0.16	0.06	0.05	0.33 0.64	0.11	0.002	144	144	41	114	0.11	0.0023	128	3	93
M41222222	0.7 1	15 -	4 600	10 10	1	2.5	2.96	0.55	0.21	0.68	0.16	0.06	0.04	0.28 0.58	0.12	0.003	113	113	32	91	0.12	0.0026	98	2	71
M51222221 M51222222	0.9 1	15	4 600	10 10	1	1.5	2.18	0.41	0.16	0.70	0.14	0.06	0.06	0.36 0.66	0.10	0.002	170	170	50 50	119	0.10	0.0023	158	4	103
M12111111	0.9 1	30	3 400	8 8	0.6	1.5	1.01	0.49	0.20	0.63	0.14	0.00	0.03	0.66 0.71	0.10	0.002	767	767	29	743	0.10	0.00025	710	11	684
M12111112	0.1 3	30	3 400	8 8	0.6	2.5	1.05	0.17	0.06	0.63	0.19	0.07	0.14	0.65 0.71	0.06	0.001	739	739	28	717	0.06	0.0009	682	10	658
M22111111	0.3 3	30	3 400	8 8	0.6	1.5	0.95	0.16	0.06	0.64	0.18	0.06	0.16	0.67 0.71	0.05	0.001	825	825	95	741	0.05	0.0008	776	12	686
M22111112	0.3 3	30	3 400	8 8	0.6	2.5	0.98	0.17	0.06	0.64	0.18	0.06	0.15	0.66 0.71	0.05	0.001	794	794	92	714	0.05	0.0008	745	11	659
M32111111 M32111112	0.5 3	30	3 400	8 8	0.6	2.5	0.87	0.16	0.06	0.65	0.17	0.06	0.17	0.68 0.71	0.05	0.001	896	896	1/5	738	0.05	0.0007	856	13	690
M42111111	0.7 3	30	3 400	8 8	0.6	1.5	0.79	0.15	0.06	0.66	0.16	0.06	0.19	0.70 0.71	0.04	0.001	996	996	276	744	0.04	0.0006	965	15	703
M42111112	0.7 3	30	3 400	8 8	0.6	2.5	0.82	0.15	0.06	0.66	0.16	0.06	0.18	0.69 0.71	0.04	0.001	961	961	266	719	0.04	0.0006	930	14	678
M52111111	0.9 3	30	3 400	8 8	0.6	1.5	0.70	0.13	0.05	0.69	0.14	0.06	0.22	0.71 0.71	0.04	0.001	1147	1147	410	766	0.04	0.0006	1125	18	736
M1211112	0.9 3	30 30	3 400	8 8	0.6	2.5	0.73	0.14	0.05	0.69	0.14	0.06	0.21	0.71 0.71	0.04	0.001	1102	1102 077	395	/38	0.04	0.0006	1080	17	/06
M12111121 M12111122	0.1 3	30	3 400	8 8	1	2.5	0.82	0.13	0.05	0.63	0.19	0.07	0.19	0.71 0.71	0.05	0.001	943	943	36	913	0.05	0.0007	890	14	858
M22111121	0.3 3	30	3 400	8 8	1	1.5	0.74	0.13	0.05	0.64	0.18	0.06	0.20	0.71 0.71	0.04	0.001	1052	1052	123	940	0.04	0.0006	1009	15	893

Name	af	nf h	ıf mf	Ly	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2 Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
						~										[10]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	5DR [-]	[MN-m]	[MIN]	m]
M22111122	0.3	30	3 400	8	8	1	2.5	0.77	0.13	0.05	0.64	0.18	0.06	0.20	0.71 0.71	0.04	0.001	1008	1008	118	902	0.04	0.0006	964	15	852
M32111121 M32111122	0.5	30	3 400	8	8	1	2.5	0.08	0.12	0.05	0.65	0.17	0.00	0.22	0.71 0.71	0.04	0.001	1108	1108	220	942	0.04	0.0003	1073	1/	901
M42111121	0.7	30	3 400	8	8	1	1.5	0.62	0.11	0.04	0.66	0.16	0.06	0.25	0.71 0.71	0.03	0.000	1286	1286	355	952	0.03	0.0005	1261	20	919
M42111122	0.7	30	3 400	8 (8	1	2.5	0.64	0.12	0.05	0.66	0.16	0.06	0.24	0.71 0.71	0.03	0.001	1227	1227	339	910	0.03	0.0005	1201	19	875
M52111121	0.9	30	3 400	8 (8	1	1.5	0.55	0.10	0.04	0.69	0.14	0.06	0.28	0.71 0.71	0.03	0.000	1487	1487	527	985	0.03	0.0005	1470	23	961
M52111122	0.9	30	3 400	8	8	1	2.5	0.57	0.11	0.04	0.69	0.14	0.06	0.27	0.71 0.71	0.03	0.000	1430	1430	508	949	0.03	0.0005	1413	22	924
M12111211 M12111212	0.1	30	3 400	8 1	10	0.6	2.5	2.30	0.38	0.13	0.63	0.19	0.07	0.05	0.39 0.71	0.11	0.002	321	321	11	313	0.11	0.0017	267	4	257
M22111212	0.3	30	3 400	8	10	0.6	1.5	2.15	0.37	0.13	0.64	0.18	0.06	0.06	0.40 0.71	0.11	0.002	347	347	38	317	0.11	0.0015	300	5	265
M22111212	0.3	30	3 400	8	10	0.6	2.5	2.24	0.38	0.14	0.64	0.18	0.06	0.06	0.38 0.71	0.11	0.002	328	328	36	300	0.11	0.0016	282	4	249
M32111211	0.5	30	3 400) 8	10	0.6	1.5	1.98	0.35	0.13	0.65	0.17	0.06	0.07	0.41 0.71	0.10	0.001	382	382	73	323	0.10	0.0014	343	5	276
M32111212	0.5	30	3 400	8	10	0.6	2.5	2.06	0.37	0.14	0.65	0.17	0.06	0.07	0.40 0.71	0.10	0.002	363	363	69	307	0.10	0.0015	324	5	261
M42111211 M42111212	0.7	30	3 400	8	10	0.6	2.5	1.79	0.33	0.13	0.66	0.16	0.06	0.08	0.43 0.71	0.09	0.001	421	421	117	324	0.09	0.0013	388	6	283
M52111212 M52111211	0.9	30	3 400	8	10	0.6	1.5	1.59	0.30	0.13	0.69	0.10	0.06	0.09	0.46 0.71	0.09	0.001	403	403	177	330	0.09	0.0013	453	7	270
M52111212	0.9	30	3 400	8	10	0.6	2.5	1.66	0.31	0.12	0.69	0.14	0.06	0.08	0.45 0.71	0.08	0.001	458	458	170	316	0.08	0.0012	432	7	283
M12111221	0.1	30	3 400	8 (10	1	1.5	1.95	0.32	0.11	0.63	0.19	0.07	0.07	0.44 0.71	0.10	0.002	396	396	14	385	0.10	0.0015	342	5	329
M12111222	0.1	30	3 400	8	10	1	2.5	2.03	0.33	0.12	0.63	0.19	0.07	0.07	0.43 0.71	0.10	0.002	378	378	14	368	0.10	0.0016	324	5	312
M22111221	0.3	30	3 400	8	10	1	1.5	1.81	0.31	0.11	0.64	0.18	0.06	0.08	0.45 0.71	0.09	0.001	421	421	47	383	0.09	0.0014	373	6	330
M22111222	0.3	30	3 400	8	10	1	2.5	1.89	0.32	0.12	0.65	0.18	0.06	0.07	0.44 0.71	0.10	0.001	403	403	45	367	0.10	0.0014	356	5	314
M32111221 M32111222	0.5	30	3 400	8	10	1	2.5	1.74	0.30	0.11	0.65	0.17	0.00	0.08	0.45 0.71	0.09	0.001	434	434	84	365	0.09	0.0012	394	6	318
M42111221	0.7	30	3 400	8	10	1	1.5	1.51	0.28	0.11	0.66	0.16	0.06	0.09	0.48 0.71	0.08	0.001	500	500	139	382	0.08	0.0011	468	7	341
M42111222	0.7	30	3 400	8 (10	1	2.5	1.58	0.29	0.11	0.66	0.16	0.06	0.09	0.47 0.71	0.08	0.001	479	479	133	367	0.08	0.0012	447	7	325
M52111221	0.9	30	3 400	8	10	1	1.5	1.34	0.25	0.10	0.69	0.14	0.06	0.11	0.51 0.71	0.06	0.001	573	573	210	392	0.06	0.0010	548	9	358
M52111222	0.9	30	3 400	8	10	1	2.5	1.40	0.26	0.11	0.69	0.14	0.06	0.10	0.49 0.71	0.07	0.001	546	546	201	374	0.07	0.0011	521	8	341
M12112111 M12112112	0.1	30	3 400	10	8	0.6	2.5	1.38	0.23	0.08	0.63	0.19	0.07	0.10	0.56 0.71	0.07	0.001	557	518	21	503	0.07	0.0011	499	8	481
M22112112 M22112111	0.3	30	3 400) 10	8	0.6	1.5	1.29	0.24	0.05	0.64	0.18	0.07	0.11	0.57 0.71	0.07	0.001	596	596	68	539	0.07	0.0012	547	8	483
M22112112	0.3	30	3 400	0 10	8	0.6	2.5	1.38	0.24	0.09	0.64	0.18	0.06	0.10	0.54 0.71	0.07	0.001	553	553	63	500	0.07	0.0011	504	8	446
M32112111	0.5	30	3 400	10	8	0.6	1.5	1.19	0.21	0.08	0.65	0.17	0.06	0.12	0.58 0.71	0.06	0.001	650	650	126	541	0.06	0.0009	608	9	490
M32112112	0.5	30	3 400	10	8	0.6	2.5	1.27	0.23	0.08	0.65	0.17	0.06	0.11	0.55 0.71	0.07	0.001	601	601	117	501	0.07	0.0010	560	9	451
M42112111	0.7	30	3 400	10	8	0.6	1.5	1.07	0.20	0.08	0.66	0.16	0.06	0.14	0.60 0.71	0.06	0.001	725	725	201	548	0.06	0.0008	692	11	504
M52112112 M52112111	0.7	30	3 400) 10	8	0.6	1.5	0.95	0.21	0.08	0.60	0.10	0.06	0.15	0.58 0.71	0.00	0.001	832	832	302	562	0.00	0.0009	808	13	528
M52112112	0.9	30	3 400	0 10	8	0.6	2.5	1.02	0.19	0.08	0.69	0.14	0.06	0.14	0.62 0.71	0.05	0.001	771	771	280	522	0.05	0.0008	745	12	487
M12112121	0.1	30	3 400	0 10	8	1	1.5	1.10	0.18	0.06	0.63	0.19	0.07	0.13	0.64 0.71	0.06	0.001	704	704	27	683	0.06	0.0009	647	10	623
M12112122	0.1	30	3 400) 10	8	1	2.5	1.19	0.19	0.07	0.63	0.19	0.07	0.12	0.61 0.71	0.07	0.001	649	649	24	630	0.07	0.0010	591	9	570
M22112121	0.3	30	3 400	10	8	1	1.5	1.03	0.18	0.06	0.64	0.18	0.06	0.14	0.64 0.71	0.06	0.001	757	757	87	681	0.06	0.0008	707	11	626
M22112122 M32112121	0.3	30	3 400	10	8	1	2.5	1.11	0.19	0.07	0.64	0.18	0.06	0.13	0.62 0.71	0.06	0.001	875	825	161	628	0.05	0.0009	785	10	632
M32112121 M32112122	0.5	30	3 400	0 10	8	1	2.5	1.02	0.17	0.00	0.65	0.17	0.00	0.10	0.63 0.71	0.05	0.001	760	760	101	629	0.05	0.0007	719	11	579
M42112121	0.7	30	3 400	10	8	1	1.5	0.86	0.16	0.06	0.66	0.16	0.06	0.17	0.67 0.71	0.05	0.001	916	916	254	686	0.05	0.0007	884	14	644
M42112122	0.7	30	3 400	0 10	8	1	2.5	0.93	0.17	0.07	0.66	0.16	0.06	0.16	0.65 0.71	0.05	0.001	845	845	234	635	0.05	0.0007	813	13	592
M52112121	0.9	30	3 400	10	8	1	1.5	0.76	0.14	0.06	0.69	0.14	0.06	0.20	0.70 0.71	0.04	0.001	1044	1044	375	700	0.04	0.0006	1021	16	667
M52112122	0.9	30	3 400	10	10	1	2.5	0.82	0.15	0.06	0.69	0.14	0.06	0.18	0.68 0.71	0.04	0.001	968	968	349	651	0.04	0.0007	944	15	618
M12112211 M12112212	0.1	30	3 400	10	10	0.6	2.5	3.50	0.54	0.19	0.63	0.19	0.07	0.03	0.29 0.01	0.15	0.002	187	187		183	0.15	0.0020	138	2	133
M22112211	0.3	30	3 400	10	10	0.6	1.5	3.07	0.52	0.19	0.64	0.18	0.06	0.04	0.30 0.62	0.13	0.002	220	220	23	204	0.13	0.0018	175	3	155
M22112212	0.3	30	3 400	10	10	0.6	2.5	3.36	0.57	0.21	0.64	0.18	0.06	0.03	0.27 0.59	0.13	0.002	198	198	21	184	0.13	0.0019	154	2	136
M32112211	0.5	30	3 400	10	10	0.6	1.5	2.83	0.50	0.19	0.65	0.17	0.06	0.04	0.31 0.62	0.12	0.002	239	239	45	207	0.12	0.0017	200	3	161
M32112212	0.5	30	3 400	10	10	0.6	2.5	3.09	0.55	0.21	0.65	0.17	0.06	0.04	0.28 0.59	0.12	0.002	213	213	40	185	0.12	0.0018	176	3	142
M42112211 M42112212	0.7	30	3 400	10	10	0.6	2.5	2.50	0.47	0.18	0.66	0.16	0.06	0.05	0.32 0.63	0.11	0.002	268	268	/4	211	0.11	0.0016	235	4	1/2
M52112212	0.9	30	3 400	10	10	0.6	1.5	2.27	0.43	0.17	0.69	0.14	0.06	0.04	0.35 0.65	0.12	0.002	314	314	118	221	0.10	0.0016	288	5	189
M52112212	0.9	30	3 400	10	10	0.6	2.5	2.48	0.47	0.19	0.69	0.14	0.06	0.05	0.33 0.62	0.10	0.002	278	278	106	197	0.10	0.0016	252	4	165
M12112221	0.1	30	3 400) 10	10	1	1.5	2.85	0.46	0.17	0.63	0.19	0.07	0.04	0.33 0.66	0.12	0.002	248	248	8	243	0.12	0.0018	194	3	187
M12112222	0.1	30	3 400	10	10	1	2.5	3.14	0.51	0.18	0.63	0.19	0.07	0.03	0.30 0.63	0.13	0.002	221	221	7	216	0.13	0.0019	168	3	162
M22112221	0.3	30	3 400	10	10	1	1.5	2.65	0.45	0.17	0.64	0.18	0.06	0.04	0.34 0.66	0.12	0.002	265	265	29	244	0.12	0.0017	218	3	193
M32112222	0.5	30	3 400	0 10	10	1	2.5	2.92	0.50	0.18	0.65	0.18	0.06	0.04	0.35 0.67	0.12	0.002	234	234	25 55	21/	0.12	0.0018	250	3	201
M32112222	0.5	30	3 400	0 10	10	1	2.5	2.69	0.48	0.18	0.65	0.17	0.06	0.04	0.32 0.64	0.12	0.002	255	255	48	220	0.12	0.0017	235	3	174
M42112221	0.7	30	3 400) 10	10	1	1.5	2.21	0.41	0.16	0.66	0.16	0.06	0.06	0.36 0.67	0.10	0.002	327	327	91	254	0.10	0.0015	295	5	215
M42112222	0.7	30	3 400	10	10	1	2.5	2.44	0.45	0.17	0.66	0.16	0.06	0.05	0.34 0.65	0.11	0.002	286	286	80	225	0.11	0.0016	254	4	185
M52112221	0.9	30	3 400	10	10	1	1.5	1.96	0.37	0.15	0.69	0.14	0.06	0.07	0.39 0.69	0.09	0.001	384	384	143	266	0.09	0.0014	359	6	234
M12121111	0.9	30	3 400	101	10	1	2.5	2.16	0.41	0.16	0.69	0.14	0.05	0.06	0.60 0.71	0.10	0.002	337	337	126	236	0.10	0.0015	312	12	204
M12121111	0.1	30	3 600	8	8	0.6	2.5	1.24	0.20	0.08	0.63	0.19	0.07	0.12	0.58 0.71	0.07	0.001	897	897	34	871	0.07	0.0010	810	12	781
M22121111	0.3	30	3 600	8	8	0.6	1.5	1.16	0.20	0.07	0.64	0.18	0.06	0.13	0.61 0.71	0.06	0.001	1000	1000	115	902	0.06	0.0009	925	14	818
M22121112	0.3	30	3 600	8	8	0.6	2.5	1.20	0.20	0.07	0.64	0.18	0.06	0.12	0.59 0.71	0.07	0.001	963	963	110	869	0.06	0.0009	888	14	785
M32121111	0.5	30	3 600	8	8	0.6	1.5	1.07	0.19	0.07	0.65	0.17	0.06	0.14	0.62 0.71	0.06	0.001	1094	1094	213	906	0.06	0.0008	1032	16	831

Name	afi	nf]	hf mf	Lv	Lx	tw	Lch	T1 [s]	T2 [s]	T3 [s]	MI FI	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment (MN
				-0					- 101	[-]	11			0.00	(Contractor Contractor		[m]		[MN-m]	[MN]	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
M32121112	0.5 3	30	3 600	8	8	0.6	2.5	1.11	0.20	0.07	0.65	0.17	0.06	0.13	0.61 0	0.71	0.06	0.001	1050	1050	204	871	0.06	0.0009	987	15	796
M42121111 M42121112	0.7 3	30 20	3 600	8	8	0.6	2.5	1.00	0.18	0.07	0.66	0.16	0.06	0.15	0.63 0	1.71	0.05	0.001	1217	1217	338	916	0.05	0.0008	1169	18	851
M52121112	0.9 3	30	3 600	8	8	0.6	1.5	0.86	0.16	0.06	0.69	0.14	0.06	0.17	0.67 0	0.71	0.03	0.001	1391	1391	502	936	0.03	0.0007	1355	21	886
M52121112	0.9 3	30	3 600	8	8	0.6	2.5	0.89	0.17	0.07	0.69	0.14	0.06	0.17	0.66 0	0.71	0.05	0.001	1334	1334	482	899	0.04	0.0007	1297	20	848
M12121121	0.1 3	30	3 600	8	8	1	1.5	0.97	0.16	0.06	0.63	0.19	0.07	0.15	0.67 0).71	0.05	0.001	1205	1205	46	1167	0.05	0.0008	1121	17	1080
M12121122	0.1 3	30	3 600	8	8	1	2.5	1.01	0.16	0.06	0.63	0.19	0.07	0.15	0.66 0	0.71	0.06	0.001	1156	1156	44	1120	0.06	0.0008	1071	16	1032
M22121121 M22121122	0.3 3	30 30	3 600	8	8	1	2.5	0.90	0.15	0.06	0.64	0.18	0.06	0.16	0.68 0	1.71	0.05	0.001	1289	1289	149	1157	0.05	0.0007	1218	19	1077
M32121122	0.5 3	30	3 600	8	8	1	1.5	0.83	0.15	0.06	0.65	0.18	0.00	0.18	0.69 0	0.71	0.05	0.001	1243	1409	276	1159	0.05	0.0007	1350	21	1030
M32121122	0.5 3	30	3 600	8	8	1	2.5	0.87	0.15	0.06	0.65	0.17	0.06	0.17	0.68 0	.71	0.05	0.001	1351	1351	264	1113	0.05	0.0007	1292	20	1041
M42121121	0.7 3	30	3 600	8	8	1	1.5	0.75	0.14	0.05	0.66	0.16	0.06	0.20	0.71 0).71	0.04	0.001	1560	1560	432	1164	0.04	0.0006	1514	23	1103
M42121122	0.7 3	30	3 600	8	8	1	2.5	0.78	0.15	0.06	0.66	0.16	0.06	0.19	0.70 0	0.71	0.04	0.001	1502	1502	416	1122	0.04	0.0006	1456	23	1061
M52121121	0.9 3	30	3 600	8	8	1	1.5	0.67	0.13	0.05	0.69	0.14	0.06	0.23	0.71 0	0.71	0.03	0.001	1797	1797	642	1199	0.03	0.0005	1766	28	1155
M12121221	0.9 3	50 20	3 600	8	8 10	1	2.5	2.82	0.13	0.05	0.69	0.14	0.06	0.22	0.71 0	1.71	0.04	0.001	376	376	13	367	0.04	0.0006	297	21	284
M1212121211	0.1 3	30	3 600	8	10	0.6	2.5	2.94	0.48	0.17	0.63	0.19	0.07	0.04	0.32 0	0.65	0.12	0.002	358	358	12	350	0.12	0.0010	277	4	267
M22121211	0.3 3	30	3 600	8	10	0.6	1.5	2.63	0.45	0.16	0.64	0.18	0.06	0.04	0.34 0	.66	0.12	0.002	401	401	43	370	0.12	0.0017	331	5	292
M22121212	0.3 3	30	3 600	8	10	0.6	2.5	2.74	0.47	0.17	0.64	0.18	0.06	0.04	0.33 0	9.65	0.12	0.002	381	381	41	352	0.12	0.0017	311	5	275
M32121211	0.5 3	30	3 600	8	10	0.6	1.5	2.42	0.43	0.16	0.65	0.17	0.06	0.05	0.35 0	.67	0.11	0.002	438	438	83	375	0.11	0.0016	379	6	305
M32121212	0.5 3	30	3 600	8	10	0.6	2.5	2.52	0.45	0.1/	0.65	0.17	0.06	0.05	0.34 0	1.66	0.11	0.002	415	415	128	356	0.11	0.0016	356	5	28/
M42121211 M42121212	0.7 3	30 30	3 600	8	10	0.6	2.5	2.19	0.41	0.10	0.66	0.10	0.00	0.06	0.35 0	0.00	0.10	0.002	493	495	130	365	0.10	0.0015	448	6	306
M52121211	0.9 3	30	3 600	8	10	0.6	1.5	1.95	0.36	0.15	0.69	0.14	0.06	0.07	0.40 0	.69	0.09	0.001	581	581	216	403	0.09	0.0014	543	9	355
M52121212	0.9 3	30	3 600	8	10	0.6	2.5	2.03	0.38	0.15	0.69	0.14	0.06	0.07	0.38 0	.68	0.09	0.001	554	554	206	385	0.09	0.0015	516	8	337
M12121221	0.1 3	30	3 600	8	10	1	1.5	2.38	0.39	0.14	0.63	0.19	0.07	0.05	0.38 0	0.71	0.11	0.002	461	461	16	449	0.11	0.0017	380	6	366
M12121222	0.1 3	30	3 600	8	10	1	2.5	2.49	0.41	0.15	0.63	0.19	0.07	0.05	0.37 0	0.70	0.11	0.002	437	437	15	426	0.11	0.0017	356	5	343
M221212221	0.3 3	30	3 600	8	10	1	2.5	2.22	0.38	0.14	0.64	0.18	0.06	0.05	0.38 0	1.71	0.11	0.002	497	497	55	455	0.11	0.0016	428	/	3/8
M32121222	0.5 3	30	3 600	8	10	1	1.5	2.04	0.40	0.14	0.65	0.18	0.00	0.05	0.40 0	0.71	0.10	0.002	550	550	105	466	0.10	0.0015	400	8	396
M32121222	0.5 3	30	3 600	8	10	1	2.5	2.13	0.38	0.14	0.65	0.17	0.06	0.06	0.38 0	0.70	0.10	0.002	517	517	99	439	0.10	0.0015	459	7	370
M42121221	0.7 3	30	3 600	8	10	1	1.5	1.85	0.34	0.13	0.66	0.16	0.06	0.07	0.42 0).71	0.09	0.001	610	610	169	470	0.09	0.0013	562	9	409
M42121222	0.7 3	30	3 600	8	10	1	2.5	1.93	0.36	0.14	0.66	0.16	0.06	0.07	0.40 0	0.71	0.09	0.001	583	583	162	450	0.09	0.0014	535	8	390
M52121221	0.9 3	30	3 600	8	10	1	1.5	1.64	0.31	0.12	0.69	0.14	0.06	0.08	0.45 0	0.71	0.08	0.001	692	692	256	478	0.08	0.0012	654	10	427
M1212222	0.9 3	30 30	3 600	10	10	0.6	1.5	1.72	0.52	0.15	0.63	0.14	0.00	0.08	0.44 0	0.71	0.08	0.001	680	680	240	661	0.08	0.0013	598	9	577
M12122112	0.1 3	30	3 600	10	8	0.6	2.5	1.82	0.30	0.11	0.63	0.19	0.07	0.08	0.46 0	0.71	0.09	0.001	635	635	23	617	0.09	0.0014	553	8	533
M22122111	0.3 3	30	3 600	10	8	0.6	1.5	1.58	0.27	0.10	0.64	0.18	0.06	0.09	0.49 0	0.71	0.08	0.001	724	724	82	657	0.08	0.0012	653	10	578
M22122112	0.3 3	30	3 600	10	8	0.6	2.5	1.69	0.29	0.11	0.64	0.18	0.06	0.08	0.47 0).71	0.09	0.001	675	675	76	613	0.09	0.0013	604	9	534
M32122111	0.5 3	30	3 600	10	8	0.6	1.5	1.45	0.26	0.10	0.65	0.17	0.06	0.10	0.50 0	0.71	0.08	0.001	785	785	152	656	0.08	0.0011	725	11	584
M32122112 M42122111	0.5 3	30 20	3 600	10	8	0.6	2.5	1.56	0.28	0.10	0.65	0.17	0.06	0.09	0.48 0	1.71	0.08	0.001	871	871	242	663	0.08	0.0012	873	10	539
M42122112	0.7 3	30	3 600	10	8	0.6	2.5	1.41	0.26	0.10	0.66	0.16	0.06	0.10	0.50 0	0.71	0.07	0.001	806	806	224	614	0.07	0.0010	758	12	553
M52122111	0.9 3	30	3 600	10	8	0.6	1.5	1.17	0.22	0.09	0.69	0.14	0.06	0.12	0.57 0	0.71	0.06	0.001	1000	1000	366	681	0.06	0.0009	962	15	629
M52122112	0.9 3	30	3 600	10	8	0.6	2.5	1.25	0.23	0.09	0.69	0.14	0.06	0.11	0.54 0).71	0.06	0.001	924	924	339	631	0.06	0.0010	886	14	579
M12122121	0.1 3	30	3 600	10	8	1	1.5	1.35	0.22	0.08	0.63	0.19	0.07	0.11	0.57 0	0.71	0.07	0.001	856	856	32	831	0.07	0.0011	769	12	742
M12122122	0.1 3	30	3 600	10	8	1	2.5	1.46	0.24	0.09	0.63	0.19	0.07	0.10	0.54 0	1.71	0.08	0.001	/89	/89	105	/66	0.08	0.0012	704	11	6/9
M22122121 M22122122	0.3 3	30	3 600	10	8	1	2.5	1.36	0.21	0.08	0.64	0.18	0.00	0.10	0.55 0	0.71	0.07	0.001	843	843	96	763	0.07	0.0010	770	12	681
M32122121	0.5 3	30	3 600	10	8	1	1.5	1.16	0.21	0.08	0.65	0.17	0.06	0.13	0.59 0	0.71	0.06	0.001	998	998	194	829	0.06	0.0009	935	14	754
M32122122	0.5 3	30	3 600	10	8	1	2.5	1.25	0.22	0.08	0.65	0.17	0.06	0.11	0.56 0	0.71	0.07	0.001	916	916	178	763	0.07	0.0010	854	13	688
M42122121	0.7 3	30	3 600	10	8	1	1.5	1.05	0.19	0.08	0.66	0.16	0.06	0.14	0.61 0	0.71	0.06	0.001	1114	1114	309	841	0.06	0.0008	1065	16	776
M42122122	0.7 3	30	3 600	10	8	1	2.5	1.14	0.21	0.08	0.66	0.16	0.06	0.13	0.58 0	1.71	0.06	0.001	1019	1019	283	771	0.06	0.0009	969	15	706
M52122121	0.9 3	30	3 600	10	8	1	2.5	1.01	0.17	0.07	0.69	0.14	0.06	0.15	0.62 0	1.71	0.05	0.001	1174	1174	40Z 477	795	0.05	0.0007	1240	19	742
M12122211	0.1 3	30	3 600	10	10	0.6	1.5	4.04	0.66	0.24	0.63	0.19	0.07	0.02	0.23 0	.54	0.15	0.003	244	244	8	239	0.15	0.0022	176	3	170
M12122212	0.1 3	30	3 600	10	10	0.6	2.5	4.41	0.72	0.26	0.63	0.19	0.07	0.02	0.21 0	0.50	0.16	0.003	220	220	7	216	0.16	0.0024	156	2	150
M22122211	0.3 3	30	3 600	10	10	0.6	1.5	3.76	0.64	0.23	0.64	0.18	0.06	0.03	0.24 0	0.54	0.14	0.002	256	256	27	239	0.14	0.0021	197	3	174
M22122212	0.3 3	30	3 600	10	10	0.6	2.5	4.11	0.70	0.26	0.64	0.18	0.06	0.02	0.22 0	0.50	0.15	0.002	231	231	24	216	0.15	0.0022	174	3	154
M32122211 M32122212	0.5 3	20	3 600	10	10	0.6	2.5	3.40	0.62	0.23	0.65	0.17	0.06	0.03	0.25 0	1.55	0.13	0.002	2/6	2/6	51	241	0.13	0.0019	224	3	180
M42122211	0.7 3	30	3 600	10	10	0.6	1.5	3.14	0.58	0.23	0.66	0.16	0.06	0.03	0.27 0	0.56	0.12	0.002	307	307	85	246	0.14	0.0018	262	4	191
M42122212	0.7 3	30	3 600	10	10	0.6	2.5	3.43	0.64	0.24	0.66	0.16	0.06	0.03	0.24 0	0.52	0.13	0.002	273	273	76	219	0.13	0.0019	230	4	168
M52122211	0.9 3	30	3 600	10	10	0.6	1.5	2.78	0.52	0.21	0.69	0.14	0.06	0.04	0.30 0	.59	0.11	0.002	357	357	137	255	0.11	0.0017	319	5	208
M52122212	0.9 3	30	3 600	10	10	0.6	2.5	3.04	0.57	0.23	0.69	0.14	0.06	0.04	0.27 0	.55	0.11	0.002	317	317	122	228	0.11	0.0018	280	4	183
M12122221	0.1 3	30	3 600	10	10	1	1.5	3.48	0.57	0.20	0.63	0.19	0.07	0.03	0.27 0	1.59	0.14	0.002	292	292	10	286	0.14	0.0020	217	3	209
M22122222	0.1 3	50 30	3 600	10	10	1	2.5	3.84	0.63	0.22	0.63	0.19	0.07	0.03	0.24 0	1.50	0.15	0.002	259	309	22	254	0.14	0.0022	243	3	215
M22122222	0.3 3	30	3 600	10	10	1	2.5	3.58	0.61	0.20	0.64	0.18	0.06	0.03	0.25 0	0.56	0.14	0.002	273	273	29	255	0.13	0.0020	243	3	187
M32122221	0.5 3	30	3 600	10	10	1	1.5	2.99	0.53	0.20	0.65	0.17	0.06	0.04	0.29 0	0.60	0.12	0.002	334	334	62	290	0.12	0.0018	277	4	223
M32122222	0.5 3	30	3 600	10	10	1	2.5	3.29	0.59	0.22	0.65	0.17	0.06	0.03	0.26 0	0.57	0.13	0.002	295	295	55	257	0.13	0.0019	240	4	194
M42122221	0.7 3	30	3 600	10	10	1	1.5	2.71	0.50	0.19	0.66	0.16	0.06	0.04	0.31 0	0.61	0.11	0.002	373	373	104	295	0.11	0.0017	325	5	237

Name	af nf	hf mf	Ly L	x tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN
																	[MN-m]	[MIN]	[MN-m]	[MN-m]	[m]	EDK [-]	[MN-m]	[MN]	m]
M42122222	0.7 30	3 600	10 1	0 1	2.5	2.99	0.55	0.21	0.66	0.16	0.06	0.04	0.28 0	.58	0.12	0.002	328	328	91	262	0.12	0.0017	282	4	205
M52122221	0.9 30	3 600	10 1	0 1	2.5	2.40	0.45	0.18	0.69	0.14	0.00	0.03	0.34 0	.04 60	0.10	0.002	382	382	105	272	0.10	0.0010	343	5	200
M12211111	0.1 30	4 400	8	8 0.6	1.5	0.98	0.16	0.06	0.63	0.19	0.07	0.15	0.67 0	.71	0.06	0.001	1059	1059	40	1026	0.06	0.0006	984	11	948
M12211112	0.1 30	4 400	8	8 0.6	2.5	1.01	0.16	0.06	0.63	0.19	0.07	0.15	0.66 0	.71	0.06	0.001	1029	1029	39	997	0.06	0.0006	954	11	919
M22211111	0.3 30	4 400	8	8 0.6	1.5	0.91	0.16	0.06	0.64	0.18	0.06	0.16	0.68 0	.71	0.05	0.001	1135	1135	131	1019	0.05	0.0006	1071	12	947
M22211112	0.3 30	4 400	8	8 0.6	2.5	0.94	0.16	0.06	0.64	0.18	0.06	0.16	0.67 0	.71	0.05	0.001	1106	1106	128	994	0.05	0.0006	1042	12	922
M32211111 M32211112	0.5 30	4 400	8	8 0.6	2.5	0.84	0.15	0.06	0.65	0.17	0.06	0.18	0.69 0	./1	0.05	0.001	1238	1238	242	991	0.05	0.0005	1185	14	955
M42211111	0.7 30	4 400	8	8 0.6	1.5	0.76	0.14	0.05	0.66	0.16	0.06	0.20	0.70 0	.71	0.04	0.000	1369	1369	379	1022	0.04	0.0005	1328	15	968
M42211112	0.7 30	4 400	8	8 0.6	2.5	0.78	0.15	0.06	0.66	0.16	0.06	0.19	0.70 0	.71	0.04	0.000	1337	1337	370	998	0.04	0.0005	1295	15	944
M52211111	0.9 30	4 400	8	8 0.6	1.5	0.68	0.13	0.05	0.69	0.14	0.06	0.23	0.71 0	.71	0.04	0.000	1581	1581	565	1055	0.04	0.0004	1553	18	1015
M52211112	0.9 30	4 400	8	8 0.6	2.5	0.69	0.13	0.05	0.69	0.14	0.06	0.22	0.71 0	.71	0.04	0.000	1539	1539	551	1028	0.04	0.0004	1511	18	988
M12211121	0.1 30	4 400	8	8 1	2.5	0.75	0.12	0.04	0.63	0.19	0.07	0.20	0.71 0	./1	0.04	0.000	1378	1378	53	1332	0.04	0.0005	1314	14	1266
M22211122	0.3 30	4 400	8	8 1	1.5	0.70	0.12	0.04	0.64	0.18	0.06	0.22	0.71 0	.71	0.04	0.000	1491	1491	175	1331	0.04	0.0004	1438	16	1272
M22211122	0.3 30	4 400	8	8 1	2.5	0.72	0.12	0.04	0.64	0.18	0.06	0.21	0.71 0	.71	0.04	0.000	1450	1450	170	1295	0.04	0.0004	1395	16	1234
M32211121	0.5 30	4 400	8	8 1	1.5	0.64	0.11	0.04	0.65	0.17	0.06	0.24	0.71 0	.71	0.04	0.000	1619	1619	318	1323	0.04	0.0004	1577	18	1271
M32211122	0.5 30	4 400	8	8 1	2.5	0.66	0.12	0.04	0.65	0.17	0.06	0.23	0.71 0	.71	0.04	0.000	1581	1581	310	1292	0.04	0.0004	1537	18	1238
M42211121	0.7 30	4 400	8	8 1	2.5	0.58	0.11	0.04	0.66	0.16	0.06	0.27	0.71 0	./1	0.03	0.000	1833	1833	506	1354	0.03	0.0004	1802	21	1313
M52211122	0.9 30	4 400	8	8 1	1.5	0.52	0.10	0.04	0.69	0.10	0.00	0.30	0.71 0	.71	0.03	0.000	2091	2091	740	1313	0.03	0.0004	2070	20	1354
M52211122	0.9 30	4 400	8	8 1	2.5	0.53	0.10	0.04	0.69	0.14	0.06	0.29	0.71 0	.71	0.03	0.000	2041	2041	723	1351	0.03	0.0003	2019	24	1320
M12211211	0.1 30	4 400	81	0 0.6	1.5	2.18	0.36	0.13	0.63	0.19	0.07	0.06	0.41 0	.71	0.11	0.001	459	459	16	447	0.11	0.0012	386	4	372
M12211212	0.1 30	4 400	8 1	0 0.6	2.5	2.25	0.37	0.13	0.63	0.19	0.07	0.06	0.40 0	.71	0.11	0.001	441	441	16	430	0.11	0.0012	369	4	355
M22211211	0.3 30	4 400	8 1	0 0.6	1.5	2.03	0.35	0.13	0.64	0.18	0.06	0.07	0.41 0	.71	0.10	0.001	497	497	55	454	0.10	0.0011	435	5	385
M32211212 M32211211	0.5 30	4 400	8 1	0 0.6	1.5	1.87	0.33	0.13	0.65	0.18	0.00	0.07	0.43 0	.71	0.09	0.001	538	538	103	454	0.09	0.0011	410	6	308
M32211212	0.5 30	4 400	8 1	0 0.6	2.5	1.93	0.34	0.13	0.65	0.17	0.06	0.07	0.42 0	.71	0.10	0.001	522	522	100	441	0.10	0.0010	470	5	379
M42211211	0.7 30	4 400	81	0 0.6	1.5	1.70	0.31	0.12	0.66	0.16	0.06	0.08	0.45 0	.71	0.08	0.001	593	593	165	456	0.08	0.0009	550	б	401
M42211212	0.7 30	4 400	8 1	0 0.6	2.5	1.75	0.32	0.12	0.66	0.16	0.06	0.08	0.44 0	.71	0.09	0.001	575	575	160	442	0.09	0.0010	532	6	387
M52211211	0.9 30	4 400	8 1	0 0.6	1.5	1.51	0.28	0.11	0.69	0.14	0.06	0.09	0.48 0	.71	0.07	0.001	674	674	249	463	0.07	0.0009	641	8	419
M12211212	0.9 30	4 400	8 1	0 0.0	2.5	1.55	0.29	0.12	0.69	0.14	0.00	0.09	0.47 0	.71	0.07	0.001	581	581	242	564	0.07	0.0009	508	6	408
M12211222	0.1 30	4 400	8 1	0 1	2.5	1.82	0.30	0.11	0.63	0.19	0.07	0.08	0.46 0	.71	0.09	0.001	564	564	21	549	0.09	0.0011	491	6	474
M22211221	0.3 30	4 400	81	0 1	1.5	1.64	0.28	0.10	0.64	0.18	0.06	0.08	0.48 0	.71	0.09	0.001	617	617	70	560	0.09	0.0009	554	6	490
M22211222	0.3 30	4 400	8 1	0 1	2.5	1.69	0.29	0.11	0.64	0.18	0.06	0.08	0.47 0	.71	0.09	0.001	600	600	68	545	0.09	0.0010	536	6	474
M32211221	0.5 30	4 400	8 1	0 1	2.5	1.51	0.27	0.10	0.65	0.17	0.06	0.09	0.49 0	.71	0.08	0.001	667	667	129	559	0.08	0.0008	615 505	7	495
M42211222	0.7 30	4 400	8 1	0 1	1.5	1.37	0.25	0.10	0.66	0.16	0.00	0.09	0.51 0	.71	0.07	0.001	740	740	206	563	0.07	0.0009	697	8	508
M42211222	0.7 30	4 400	8 1	0 1	2.5	1.41	0.26	0.10	0.66	0.16	0.06	0.10	0.50 0	.71	0.07	0.001	716	716	199	546	0.07	0.0008	674	8	491
M52211221	0.9 30	4 400	81	.0 1	1.5	1.22	0.23	0.09	0.69	0.14	0.06	0.12	0.55 0	.71	0.06	0.001	851	851	311	580	0.06	0.0007	817	10	534
M52211222	0.9 30	4 400	8 1	0 1	2.5	1.25	0.24	0.09	0.69	0.14	0.06	0.11	0.54 0	.71	0.06	0.001	821	821	301	560	0.06	0.0007	787	9	515
M12212111	0.1 30	4 400	10	8 0.6	2.5	1.32	0.22	0.08	0.63	0.19	0.07	0.11	0.58 0	./1	0.07	0.001	780	780	29	757	0.07	0.0008	/03	8	6//
M22212112	0.3 30	4 400	10	8 0.6	1.5	1.33	0.23	0.08	0.64	0.19	0.07	0.12	0.59 0	.71	0.07	0.001	835	835	96	754	0.07	0.0007	769	9	680
M22212112	0.3 30	4 400	10	8 0.6	2.5	1.29	0.22	0.08	0.64	0.18	0.06	0.11	0.57 0	.71	0.07	0.001	793	793	90	716	0.07	0.0008	726	8	642
M32212111	0.5 30	4 400	10	8 0.6	1.5	1.13	0.20	0.08	0.65	0.17	0.06	0.13	0.60 0	.71	0.06	0.001	909	909	177	755	0.06	0.0007	853	10	687
M32212112	0.5 30	4 400	10	8 0.6	2.5	1.19	0.21	0.08	0.65	0.17	0.06	0.12	0.58 0	.71	0.06	0.001	864	864	168	718	0.06	0.0007	808	9	651
M42212111 M42212112	0.7 30	4 400	10	8 0.6 8 0.6	2.5	1.03	0.19	0.07	0.66	0.16	0.06	0.14	0.62 0	.71	0.05	0.001	1015	1015	282	765	0.05	0.0006	9/1	11	/07
M52212112	0.9 30	4 400	10	8 0.6	1.5	0.91	0.17	0.07	0.69	0.10	0.06	0.14	0.65 0	.71	0.05	0.001	1161	1161	420	783	0.05	0.0005	1129	13	738
M52212112	0.9 30	4 400	10	8 0.6	2.5	0.96	0.18	0.07	0.69	0.14	0.06	0.16	0.64 0	.71	0.05	0.001	1106	1106	401	747	0.05	0.0006	1073	13	702
M12212121	0.1 30	4 400	10	8 1	1.5	1.02	0.17	0.06	0.63	0.19	0.07	0.14	0.66 0	.71	0.06	0.001	1017	1017	39	985	0.06	0.0006	941	11	907
M12212122	0.1 30	4 400	10	8 1	2.5	1.08	0.18	0.06	0.63	0.19	0.07	0.14	0.64 0	.71	0.06	0.001	964	964	36	935	0.06	0.0007	888	10	856
M22212121 M22212122	0.3 30	4 400	10	8 1 8 1	2.5	0.95	0.16	0.06	0.64	0.18	0.06	0.16	0.67 0	./1	0.05	0.001	1093	1093	126	982	0.05	0.0006	1029	12	910
M32212121	0.5 30	4 400	10	8 1	1.5	0.88	0.16	0.06	0.65	0.17	0.06	0.17	0.68 0	.71	0.05	0.001	1186	1186	232	978	0.05	0.0005	1133	13	913
M32212122	0.5 30	4 400	10	8 1	2.5	0.92	0.16	0.06	0.65	0.17	0.06	0.16	0.66 0	.71	0.05	0.001	1127	1127	220	930	0.05	0.0005	1073	12	865
M42212121	0.7 30	4 400	10	8 1	1.5	0.79	0.15	0.06	0.66	0.16	0.06	0.19	0.69 0	.71	0.04	0.000	1321	1321	366	987	0.04	0.0005	1280	15	933
M42212122	0.7 30	4 400	10	8 1	2.5	0.84	0.15	0.06	0.66	0.16	0.06	0.18	0.68 0	.71	0.05	0.000	1254	1254	347	938	0.04	0.0005	1212	14	883
M52212121 M52212122	0.9 30	4 400	10	8 1 8 1	2.5	0.70	0.13	0.05	0.69	0.14	0.06	0.22	0.71 0	./1	0.04	0.000	1520	1520	544	1016	0.04	0.0004	1491	18	975
M12212221	0.9 50	4 400	10 1	0 0.6	1.5	3,07	0.14	0.00	0.63	0.14	0.00	0.20	0.31 0	.64	0.13	0.000	302	302	10	295	0.04	0.0005	231	3	223
M12212212	0.1 30	4 400	10 1	0 0.6	2.5	3.27	0.53	0.19	0.63	0.19	0.07	0.03	0.29 0	.62	0.13	0.002	281	281	9	274	0.13	0.0015	212	2	204
M22212211	0.3 30	4 400	10 1	0 0.6	1.5	2.86	0.49	0.18	0.64	0.18	0.06	0.04	0.32 0	.64	0.12	0.001	321	321	34	297	0.12	0.0013	259	3	229
M22212212	0.3 30	4 400	10 1	0 0.6	2.5	3.04	0.52	0.19	0.64	0.18	0.06	0.04	0.30 0	.62	0.13	0.001	297	297	32	276	0.12	0.0014	237	3	210
M32212211 M32212212	0.5 30	4 400	10 1	0.6	1.5	2.64	0.47	0.18	0.65	0.17	0.06	0.04	0.33 0	.64	0.11	0.001	349	349	66	300	0.11	0.0012	296	3	239
M42212212	0.7 30	4 400	10 1	0 0.6	2.3	2.30	0.30	0.19	0.65	0.17	0.06	0.04	0.34 0	.65	0.12	0.001	392	392	109	307	0.12	0.0013	349	3	218
M42212212	0.7 30	4 400	10 1	0 0.6	2.5	2.54	0.47	0.18	0.66	0.16	0.06	0.05	0.33 0	.63	0.11	0.001	361	361	100	285	0.11	0.0012	318	4	232
M52212211	0.9 30	4 400	10 1	0 0.6	1.5	2.12	0.40	0.16	0.69	0.14	0.06	0.06	0.37 0	.67	0.10	0.001	462	462	173	323	0.09	0.0011	429	5	280

Name	af nf	hf mf	Lv Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 F1	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN
															[m]		[MN-m]	(MN)	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m)
M52212212	0.9 30	4 400	10 10	0.6	2.5	2.25	0.42	0.17	0.69	0.14	0.06	0.06	0.36	0.65	0.10	0.001	425	425	160	298	0.10	0.0012	390	5	255
M12212221	0.1 30	4 400	10 10	1	2.5	2.53	0.41	0.15	0.63	0.19	0.07	0.05	0.30	0.69	0.12	0.001	382	382	13	3/2	0.11	0.0013	309	3	298
M22212222	0.3 30	4 400	10 10	1	1.5	2.35	0.40	0.15	0.64	0.18	0.06	0.04	0.37	0.69	0.12	0.001	410	410	45	376	0.12	0.0013	348	4	308
M22212222	0.3 30	4 400	10 10	1	2.5	2.51	0.43	0.16	0.64	0.18	0.06	0.05	0.35	0.68	0.11	0.001	377	377	41	347	0.11	0.0012	315	4	278
M32212221	0.5 30	4 400	10 10	1	1.5	2.17	0.39	0.14	0.65	0.17	0.06	0.06	0.38	0.70	0.10	0.001	451	451	86	383	0.10	0.0011	400	5	322
M32212222	0.5 30	4 400	10 10	1	2.5	2.31	0.41	0.15	0.65	0.17	0.06	0.05	0.36	0.68	0.11	0.001	414	414	79	353	0.11	0.0012	361	4	291
M42212221	0.7 30	4 400	10 10	1	1.5	1.96	0.36	0.14	0.66	0.16	0.06	0.07	0.40	0.71	0.10	0.001	510	510	142	394	0.10	0.0011	468	5	341
M42212222	0.7 30	4 400	10 10	1	2.5	2.10	0.39	0.15	0.66	0.16	0.06	0.06	0.38	0.69	0.10	0.001	469	469	130	363	0.10	0.0011	427	5	311
M52212221	0.9 30	4 400	10 10		2.5	1.74	0.33	0.13	0.69	0.14	0.06	0.08	0.43	0.71	0.08	0.001	580	542	215	276	0.08	0.0010	546	6	357
M122212222	0.1 30	4 600	8 8	0.6	1.5	1.20	0.20	0.07	0.63	0.19	0.07	0.12	0.61	0.71	0.07	0.001	1289	1289	48	1250	0.07	0.0007	1172	13	1130
M12221112	0.1 30	4 600	8 8	0.6	2.5	1.23	0.20	0.07	0.63	0.19	0.07	0.12	0.60	0.71	0.07	0.001	1252	1252	47	1215	0.07	0.0008	1136	13	1095
M22221111	0.3 30	4 600	8 8	0.6	1.5	1.12	0.19	0.07	0.64	0.18	0.06	0.13	0.62	0.71	0.06	0.001	1382	1382	159	1245	0.06	0.0007	1282	15	1134
M22221112	0.3 30	4 600	8 8	0.6	2.5	1.15	0.20	0.07	0.64	0.18	0.06	0.13	0.61	0.71	0.06	0.001	1342	1342	154	1210	0.06	0.0007	1242	14	1098
M32221111	0.5 30	4 600	8 8	0.6	1.5	1.03	0.18	0.07	0.65	0.17	0.06	0.14	0.63	0.71	0.06	0.001	1509	1509	294	1250	0.06	0.0006	1427	16	1150
M32221112	0.5 30	4 600	8 8	0.6	2.5	1.06	0.19	0.07	0.65	0.1/	0.06	0.14	0.62	0.71	0.06	0.001	1469	1469	286	121/	0.06	0.0006	1386	16	111/
M42221111	0.7 30	4 600	8 9	0.0	2.5	0.95	0.17	0.07	0.00	0.10	0.06	0.16	0.65	0.71	0.05	0.001	1634	1634	403	1201	0.05	0.0005	1014	19	11/0
M52221112	0.9 30	4 600	8 8	0.6	1.5	0.83	0.16	0.06	0.69	0.14	0.06	0.18	0.68	0.71	0.03	0.001	1922	1922	693	1292	0.03	0.0005	1875	22	1226
M52221112	0.9 30	4 600	8 8	0.6	2.5	0.85	0.16	0.06	0.69	0.14	0.06	0.18	0.67	0.71	0.04	0.001	1868	1868	674	1257	0.04	0.0005	1821	21	1190
M12221121	0.1 30	4 600	8 8	1	1.5	0.92	0.15	0.05	0.63	0.19	0.07	0.16	0.69	0.71	0.05	0.001	1700	1700	65	1646	0.05	0.0006	1589	18	1532
M12221122	0.1 30	4 600	8 8	1	2.5	0.94	0.15	0.06	0.63	0.19	0.07	0.16	0.68	0.71	0.05	0.001	1656	1656	63	1604	0.05	0.0006	1545	18	1489
M22221121	0.3 30	4 600	8 8	1	1.5	0.85	0.15	0.05	0.64	0.18	0.06	0.18	0.70	0.71	0.05	0.001	1826	1826	212	1637	0.05	0.0005	1732	20	1532
M32221122	0.3 30	4 600	88	1	2.5	0.88	0.15	0.05	0.64	0.18	0.06	0.17	0.69	0.71	0.05	0.001	1//2	1//2	206	1632	0.05	0.0005	10/7	19	1483
M32221121	0.5 30	4 600	8 8	1	2.5	0.78	0.14	0.05	0.65	0.17	0.06	0.19	0.71	0.71	0.04	0.000	1966	1900	379	1592	0.04	0.0005	1859	22	1340
M42221121	0.7 30	4 600	8 8	1	1.5	0.71	0.13	0.05	0.66	0.16	0.06	0.21	0.71	0.71	0.04	0.000	2222	2222	614	1654	0.04	0.0004	2164	25	1577
M42221122	0.7 30	4 600	8 8	: 1	2.5	0.73	0.14	0.05	0.66	0.16	0.06	0.21	0.71	0.71	0.04	0.000	2156	2156	597	1607	0.04	0.0004	2097	24	1528
M52221121	0.9 30	4 600	88	: 1	1.5	0.63	0.12	0.05	0.69	0.14	0.06	0.24	0.71	0.71	0.03	0.000	2535	2535	904	1688	0.03	0.0004	2496	29	1632
M52221122	0.9 30	4 600	8 8	1	2.5	0.65	0.12	0.05	0.69	0.14	0.06	0.23	0.71	0.71	0.03	0.000	2465	2465	880	1642	0.03	0.0004	2425	29	1585
M12221211	0.1 30	4 600	8 10	0.6	1.5	2.67	0.44	0.16	0.63	0.19	0.07	0.04	0.35	0.68	0.12	0.001	535	535	18	522	0.12	0.0013	426	5	410
M22221212	0.1 30	4 600	8 10	0.6	2.5	2.70	0.45	0.16	0.63	0.19	0.07	0.04	0.34	0.67	0.12	0.001	510	510	62	526	0.12	0.0014	407	5	392
M22221211	0.3 30	4 600	8 10	0.6	2.5	2.49	0.42	0.10	0.64	0.18	0.00	0.05	0.35	0.67	0.11	0.001	551	551	60	508	0.11	0.0012	457	5	42.5
M32221211	0.5 30	4 600	8 10	0.6	1.5	2.29	0.41	0.15	0.65	0.17	0.06	0.06	0.36	0.68	0.11	0.001	627	627	119	534	0.11	0.0012	549	6	442
M32221212	0.5 30	4 600	8 10	0.6	2.5	2.36	0.42	0.16	0.65	0.17	0.06	0.05	0.36	0.68	0.11	0.001	603	603	114	515	0.11	0.0012	524	6	423
M42221211	0.7 30	4 600	8 10	0.6	1.5	2.08	0.38	0.15	0.66	0.16	0.06	0.06	0.38	0.69	0.10	0.001	711	711	198	551	0.10	0.0011	649	8	473
M42221212	0.7 30	4 600	8 10	0.6	2.5	2.14	0.40	0.15	0.66	0.16	0.06	0.06	0.37	0.68	0.10	0.001	681	681	189	529	0.10	0.0011	619	7	451
M52221211	0.9 30	4 600	8 10	0.6	2.5	1.84	0.35	0.14	0.69	0.14	0.06	0.07	0.42	0.71	0.09	0.001	820	820	305	568	0.09	0.0010	769	9	503
M12221212	0.1 30	4 600	8 10	1	1.5	2.16	0.35	0.13	0.63	0.19	0.07	0.07	0.41	0.71	0.11	0.001	698	698	25	680	0.11	0.0012	589	7	568
M12221222	0.1 30	4 600	8 10	1	2.5	2.23	0.36	0.13	0.63	0.19	0.07	0.06	0.40	0.71	0.11	0.001	671	671	24	654	0.11	0.0012	562	6	542
M22221221	0.3 30	4 600	8 10	1	1.5	2.01	0.34	0.13	0.64	0.18	0.06	0.07	0.42	0.71	0.10	0.001	756	756	84	690	0.10	0.0011	663	8	587
M22221222	0.3 30	4 600	8 10	1	2.5	2.07	0.35	0.13	0.64	0.18	0.06	0.06	0.41	0.71	0.10	0.001	727	727	81	663	0.10	0.0011	634	7	561
M32221221	0.5 30	4 600	8 10	1	1.5	1.85	0.33	0.12	0.65	0.17	0.06	0.07	0.43	0.71	0.09	0.001	817	817	157	689	0.09	0.0010	738	8	595
M42221222	0.5 30	4 600	8 10		2.5	1.91	0.34	0.13	0.65	0.17	0.06	0.07	0.42	0.71	0.10	0.001	791	791	250	601	0.10	0.0010	925	10	574
M42221221	0.7 30	4 600	8 10	1	2.5	1.73	0.32	0.12	0.66	0.16	0.06	0.08	0.44	0.71	0.00	0.001	872	872	230	670	0.09	0.0009	807	9	588
M52221221	0.9 30	4 600	8 10	1	1.5	1.49	0.28	0.11	0.69	0.14	0.06	0.09	0.48	0.71	0.07	0.001	1024	1024	378	703	0.07	0.0008	974	11	637
M52221222	0.9 30	4 600	8 10	1	2.5	1.53	0.29	0.12	0.69	0.14	0.06	0.09	0.47	0.71	0.07	0.001	992	992	366	682	0.07	0.0009	941	11	615
M12222111	0.1 30	4 600	10 8	0.6	1.5	1.62	0.26	0.09	0.63	0.19	0.07	0.09	0.49	0.71	0.09	0.001	948	948	35	921	0.09	0.0010	839	10	809
M12222112	0.1 30	4 600	10 8	0.6	2.5	1.70	0.28	0.10	0.63	0.19	0.07	0.08	0.48	0.71	0.09	0.001	904	904	33	879	0.09	0.0010	795	9	766
M22222111	0.3 30	4 600	10 8	0.6	2.5	1.51	0.26	0.09	0.64	0.18	0.06	0.09	0.50	0.71	0.08	0.001	1011	1011	115	916	0.08	0.0009	916	10	760
M32222112	0.5 30	4 600	10 8	0.6	1.5	1.39	0.27	0.09	0.65	0.18	0.06	0.10	0.49	0.71	0.07	0.001	1097	1097	213	916	0.08	0.0009	1016	12	819
M32222112	0.5 30	4 600	10 8	0.6	2.5	1.46	0.26	0.10	0.65	0.17	0.06	0.10	0.50	0.71	0.08	0.001	1042	1042	202	871	0.08	0.0008	963	11	776
M42222111	0.7 30	4 600	10 8	0.6	1.5	1.26	0.23	0.09	0.66	0.16	0.06	0.11	0.54	0.71	0.06	0.001	1219	1219	339	926	0.06	0.0007	1153	13	840
M42222112	0.7 30	4 600	10 8	0.6	2.5	1.32	0.24	0.09	0.66	0.16	0.06	0.11	0.52	0.71	0.07	0.001	1158	1158	322	880	0.07	0.0007	1093	13	796
M52222111	0.9 30	4 600	10 8	0.6	1.5	1.11	0.21	0.08	0.69	0.14	0.06	0.13	0.59	0.71	0.06	0.001	1404	1404	512	954	0.06	0.0007	1353	16	885
M12222112	0.9 30	4 600	10 8	0.6	2.5	1.1/	0.22	0.09	0.69	0.14	0.06	0.12	0.57	0.71	0.06	0.001	1329	1329	486	905	0.06	0.0007	1110	15	1079
M12222121	0.1 30	4 600	10 8	1	2.5	1.32	0.20	0.07	0.63	0.19	0.07	0.11	0.58	0.71	0.07	0.001	1250	1250	40	1137	0.07	0.0008	1055	12	1078
M22222121	0.3 30	4 600	10 8	1	1.5	1.16	0.20	0.07	0.64	0.18	0.06	0.12	0.60	0.71	0.06	0.001	1326	1326	152	1196	0.06	0.0007	1226	14	1017
M22222122	0.3 30	4 600	10 8	1	2.5	1.23	0.21	0.08	0.64	0.18	0.06	0.12	0.59	0.71	0.07	0.001	1254	1254	143	1133	0.07	0.0007	1154	13	1021
M32222121	0.5 30	4 600	10 8	1	1.5	1.07	0.19	0.07	0.65	0.17	0.06	0.14	0.62	0.71	0.06	0.001	1450	1450	282	1202	0.06	0.0006	1367	16	1101
M32222122	0.5 30	4 600	10 8	1	2.5	1.13	0.20	0.08	0.65	0.17	0.06	0.13	0.60	0.71	0.06	0.001	1365	1365	265	1133	0.06	0.0007	1281	15	1032
M42222121	0.7 30	4 600	10 8	1	1.5	0.97	0.18	0.07	0.66	0.16	0.06	0.15	0.64	0.71	0.05	0.001	1614	1614	447	1214	0.05	0.0006	1548	18	1128
M52222122	0.7 30	4 600	10 8		2.5	0.86	0.19	0.07	0.00	0.10	0.06	0.14	0.62	0.71	0.05	0.001	19/2	18/2	423	1248	0.05	0.0006	1458	21	1062
M52222122	0.9 30	4 600	10 8	1	2.5	0.91	0.17	0.07	0.69	0.14	0.06	0.16	0.65	0.71	0.05	0.001	1743	1743	631	1176	0.05	0.0005	1694	20	1108
M12222211	0.1 30	4 600	10 10	0.6	1.5	3.76	0.61	0.22	0.63	0.19	0.07	0.03	0.25	0.57	0.14	0.002	354	354	12	347	0.14	0.0016	259	3	250

Name	af nf	hf mi	Lv	Lx	tw	Lch	T1 [s]	T2 [s]	T3 [s]	MI FI	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment IMN
																[m]		[MN-m]	(MN)	[MN-m]	[MN-m]	[m]	15DR [-]	[MN-m]	[MN]	m]
M12222212	0.1 30	4 60	0 10	10	0.6	2.5	4.00	0.65	0.23	0.63	0.19	0.07	0.02	0.23	0.54	0.15	0.002	329	329	11	322	0.15	0.0017	238	3	230
M222222211	0.3 30	4 60	0 10	10	0.6	2.5	3.51	0.60	0.22	0.64	0.18	0.06	0.03	0.26	0.57	0.14	0.002	374	3/4	39	348	0.14	0.0015	290	3	230
M32222211	0.5 30	4 60	0 10	10	0.6	1.5	3.23	0.58	0.22	0.65	0.17	0.06	0.03	0.27	0.58	0.13	0.001	404	404	75	351	0.13	0.0014	330	4	266
M32222212	0.5 30	4 60	0 10	10	0.6	2.5	3.43	0.61	0.23	0.65	0.17	0.06	0.03	0.25	0.55	0.13	0.002	373	373	69	326	0.13	0.0014	302	3	244
M42222211	0.7 30	4 60	0 10	10	0.6	1.5	2.93	0.54	0.21	0.66	0.16	0.06	0.04	0.29	0.59	0.12	0.001	449	449	125	358	0.12	0.0013	387	4	282
M42222212	0.7 30	4 60	0 10	10	0.6	2.5	3.11	0.58	0.22	0.66	0.16	0.06	0.03	0.27	0.56	0.12	0.001	415	415	115	332	0.12	0.0013	354	4	258
M52222211	0.9 30	4 60	0 10	10	0.0	2.5	2.39	0.49	0.20	0.69	0.14	0.00	0.03	0.32	0.01	0.11	0.001	482	482	185	344	0.10	0.0012	472	5	282
M12222221	0.1 30	4 60	0 10	10	1	1.5	3.09	0.50	0.18	0.63	0.19	0.07	0.04	0.31	0.63	0.13	0.002	449	449	15	439	0.13	0.0014	344	4	331
M12222222	0.1 30	4 60	0 10	10	1	2.5	3.30	0.54	0.19	0.63	0.19	0.07	0.03	0.29	0.61	0.13	0.002	416	416	14	407	0.13	0.0015	313	4	301
M22222221	0.3 30	4 60	0 10	10	1	1.5	2.88	0.49	0.18	0.64	0.18	0.06	0.04	0.31	0.64	0.12	0.001	477	477	51	441	0.12	0.0013	385	4	340
M22222222	0.3 30	4 60	0 10	10	1	2.5	3.08	0.52	0.19	0.64	0.18	0.06	0.04	0.30	0.62	0.13	0.001	440	440	47	408	0.13	0.0014	350	4	309
M32222221	0.5 30	4 60	0 10	10	1	2.5	2.65	0.47	0.18	0.65	0.17	0.06	0.04	0.33	0.64	0.12	0.001	477	477	98	447	0.11	0.0012	440	5	300
M422222221	0.7 30	4 60	0 10	10	1	1.5	2.40	0.45	0.17	0.66	0.16	0.06	0.05	0.34	0.65	0.11	0.001	583	583	162	457	0.11	0.0012	518	6	378
M42222222	0.7 30	4 60	0 10	10	1	2.5	2.57	0.48	0.18	0.66	0.16	0.06	0.05	0.32	0.63	0.11	0.001	534	534	148	421	0.11	0.0012	470	5	342
M52222221	0.9 30	4 60	0 10	10	1	1.5	2.13	0.40	0.16	0.69	0.14	0.06	0.06	0.37	0.67	0.10	0.001	687	687	257	480	0.10	0.0011	636	7	416
M52222222	0.9 30	4 60	0 10	10	1	2.5	2.28	0.43	0.17	0.69	0.14	0.06	0.06	0.35	0.65	0.10	0.001	627	627	236	440	0.10	0.0012	1220	7	376
M1311111 M13111112	0.1 45	3 40	08	8	0.6	2.5	1.18	0.19	0.07	0.62	0.19	0.07	0.12	0.62	0.71	0.07	0.001	1468	1468	53	1422	0.06	0.0006	1339	14	1288
M23111112	0.3 45	3 40	0 8	8	0.6	1.5	1.10	0.19	0.07	0.63	0.18	0.06	0.13	0.62	0.71	0.06	0.001	1578	1578	180	1420	0.06	0.0006	1467	15	1296
M23111112	0.3 45	3 40	08	8	0.6	2.5	1.12	0.19	0.07	0.63	0.18	0.06	0.13	0.62	0.71	0.06	0.001	1536	1536	175	1383	0.06	0.0006	1425	15	1259
M33111111	0.5 45	3 40	0 8	8	0.6	1.5	1.01	0.18	0.07	0.64	0.17	0.06	0.15	0.64	0.71	0.06	0.001	1718	1718	332	1420	0.06	0.0005	1626	17	1309
M33111112	0.5 45	3 40	0 8	8	0.6	2.5	1.03	0.18	0.07	0.64	0.17	0.06	0.14	0.63	0.71	0.06	0.001	1678	1678	325	1388	0.06	0.0005	1586	16	1277
M43111111 M43111112	0.7 45	3 40	08	8	0.6	2.5	0.92	0.17	0.07	0.66	0.16	0.06	0.16	0.65	0.71	0.05	0.000	1908	1908	525	1432	0.05	0.0005	1830	19	1338
M53111112	0.9 45	3 40	0 8	8	0.6	1.5	0.81	0.15	0.06	0.68	0.14	0.06	0.19	0.68	0.71	0.04	0.000	2183	2183	781	1468	0.04	0.0004	2130	22	1394
M53111112	0.9 45	3 40	08	8	0.6	2.5	0.83	0.16	0.06	0.68	0.14	0.06	0.18	0.68	0.71	0.04	0.000	2132	2132	763	1435	0.04	0.0005	2078	22	1360
M13111121	0.1 45	3 40	08	8	1	1.5	0.89	0.14	0.05	0.62	0.19	0.07	0.17	0.70	0.71	0.05	0.001	1945	1945	74	1881	0.05	0.0005	1824	18	1755
M13111122	0.1 45	3 40	08	8	1	2.5	0.91	0.15	0.05	0.62	0.19	0.07	0.16	0.69	0.71	0.05	0.001	1904	1904	72	1841	0.05	0.0005	1782	18	1715
M23111121	0.3 45	3 40	08	8	1	2.5	0.85	0.14	0.05	0.63	0.18	0.00	0.18	0.70	0.71	0.05	0.000	2099	2099	242	1833	0.05	0.0005	1995	20	1705
M33111121	0.5 45	3 40	0 8	8	1	1.5	0.76	0.14	0.05	0.64	0.17	0.06	0.20	0.71	0.71	0.04	0.000	2274	2274	442	1866	0.04	0.0004	2188	22	1762
M33111122	0.5 45	3 40	08	8	1	2.5	0.78	0.14	0.05	0.64	0.17	0.06	0.19	0.71	0.71	0.04	0.000	2225	2225	433	1827	0.04	0.0004	2140	22	1723
M43111121	0.7 45	3 40	08	8	1	1.5	0.69	0.13	0.05	0.66	0.16	0.06	0.22	0.71	0.71	0.04	0.000	2550	2550	700	1895	0.04	0.0004	2486	26	1812
M43111122	0.7 45	3 40	08	8	1	2.5	0.71	0.13	0.05	0.66	0.16	0.06	0.21	0.71	0.71	0.04	0.000	2487	2487	683	1851	0.04	0.0004	2423	25	1765
M53111121	0.9 45	3 40	08	8	1	2.5	0.61	0.12	0.05	0.68	0.14	0.06	0.23	0.71	0.71	0.03	0.000	2922	2922	1055	1943	0.03	0.0003	2791	29	1883
M13111211	0.1 45	3 40	0 8	10	0.6	1.5	2.60	0.42	0.15	0.62	0.19	0.07	0.05	0.35	0.68	0.12	0.001	616	616	21	601	0.12	0.0012	496	5	477
M13111212	0.1 45	3 40	08	10	0.6	2.5	2.67	0.44	0.16	0.62	0.19	0.07	0.04	0.35	0.68	0.12	0.001	597	597	21	583	0.12	0.0012	476	5	458
M23111211	0.3 45	3 40	0 8	10	0.6	1.5	2.42	0.41	0.15	0.63	0.18	0.06	0.05	0.36	0.69	0.11	0.001	661	661	72	606	0.11	0.0011	556	6	491
M23111212 M33111211	0.3 45	3 40	08	10	0.6	2.5	2.49	0.42	0.15	0.63	0.18	0.06	0.05	0.35	0.68	0.11	0.001	639 725	725	137	587	0.11	0.0011	638	7	4/2
M33111211 M33111212	0.5 45	3 40	08	10	0.6	2.5	2.29	0.40	0.15	0.64	0.17	0.06	0.06	0.36	0.68	0.11	0.001	701	701	132	596	0.11	0.0010	613	6	494
M43111211	0.7 45	3 40	08	10	0.6	1.5	2.02	0.37	0.14	0.66	0.16	0.06	0.07	0.39	0.70	0.10	0.001	823	823	227	636	0.10	0.0010	753	8	549
M43111212	0.7 45	3 40	08	10	0.6	2.5	2.08	0.38	0.15	0.66	0.16	0.06	0.06	0.38	0.69	0.10	0.001	794	794	219	615	0.10	0.0010	724	7	527
M53111211	0.9 45	3 40	0 8	10	0.6	1.5	1.80	0.34	0.14	0.68	0.14	0.06	0.08	0.42	0.71	0.08	0.001	939	939	346	650	0.08	0.0009	880	9	576
M13111212	0.9 45	3 40	0 8	10	0.6	2.5	1.84	0.35	0.14	0.68	0.14	0.06	0.07	0.42	0.71	0.09	0.001	914 874	914	337	034 801	0.09	0.0009	856 704	7	560
M13111222	0.1 45	3 40	0 8	10	1	2.5	2.12	0.35	0.12	0.62	0.19	0.07	0.06	0.41	0.71	0.11	0.001	795	795	28	773	0.11	0.0011	674	7	649
M23111221	0.3 45	3 40	08	10	1	1.5	1.93	0.33	0.12	0.63	0.18	0.06	0.07	0.43	0.71	0.10	0.001	882	882	98	803	0.10	0.0010	778	8	687
M23111222	0.3 45	3 40	0 8	10	1	2.5	1.98	0.34	0.12	0.63	0.18	0.06	0.07	0.42	0.71	0.10	0.001	858	858	95	781	0.10	0.0010	754	8	666
M33111221	0.5 45	3 40	08	10	1	1.5	1.77	0.32	0.12	0.64	0.17	0.06	0.08	0.44	0.71	0.09	0.001	950	950	181	799	0.09	0.0009	861	9	693
M43111222	0.3 45	3 40	0 8	10	1	1.5	1.61	0.32	0.12	0.66	0.17	0.06	0.08	0.44	0.71	0.09	0.001	920	1047	289	802	0.09	0.0009	974	10	710
M43111222	0.7 45	3 40	0 8	10	1	2.5	1.65	0.31	0.12	0.66	0.16	0.06	0.08	0.45	0.71	0.08	0.001	1018	1018	281	781	0.08	0.0008	945	10	689
M53111221	0.9 45	3 40	08	10	1	1.5	1.43	0.27	0.11	0.68	0.14	0.06	0.10	0.49	0.71	0.07	0.001	1192	1192	435	818	0.07	0.0007	1136	12	743
M53111222	0.9 45	3 40	0 8	10	1	2.5	1.47	0.28	0.11	0.68	0.14	0.06	0.10	0.48	0.71	0.07	0.001	1159	1159	424	796	0.07	0.0007	1102	12	721
M13112111 M12112112	0.1 45	3 40	0 10	8	0.6	1.5	1.58	0.26	0.09	0.62	0.19	0.07	0.09	0.50	0.71	0.08	0.001	1085	1085	40	1052	0.08	0.0008	963	10	927
M23112112	0.1 45	3 40	0 10	8	0.6	1.5	1.04	0.27	0.10	0.62	0.19	0.07	0.08	0.49	0.71	0.09	0.001	1038	1159	38 131	1008	0.09	0.0009	1052	11	929
M23112112	0.3 45	3 40	0 10	8	0.6	2.5	1.53	0.26	0.10	0.63	0.18	0.06	0.09	0.50	0.71	0.08	0.001	1107	1107	125	1001	0.08	0.0008	1002	10	885
M33112111	0.5 45	3 40	0 10	8	0.6	1.5	1.35	0.24	0.09	0.64	0.17	0.06	0.11	0.53	0.71	0.07	0.001	1258	1258	242	1049	0.07	0.0007	1167	12	940
M33112112	0.5 45	3 40	0 10	8	0.6	2.5	1.41	0.25	0.09	0.64	0.17	0.06	0.10	0.51	0.71	0.07	0.001	1201	1201	231	1002	0.07	0.0007	1111	11	895
M43112111 M42112112	0.7 45	3 40	0 10	8	0.6	1.5	1.23	0.23	0.09	0.66	0.16	0.06	0.12	0.55	0.71	0.07	0.001	1398	1398	385	1060	0.06	0.0006	1323	14	964
M53112112	0.9 45	3 40	0 10	8	0.6	1.5	1.20	0.24	0.09	0.68	0.10	0.06	0.11	0.54	0.71	0.07	0.001	1606	1606	581	1012	0.07	0.0006	1550	16	1014
M53112112	0.9 45	3 40	0 10	8	0.6	2.5	1.14	0.21	0.09	0.68	0.14	0.06	0.13	0.58	0.71	0.06	0.001	1526	1526	553	1039	0.06	0.0006	1469	15	961
M13112121	0.1 45	3 40	0 10	8	1	1.5	1.21	0.20	0.07	0.62	0.19	0.07	0.12	0.61	0.71	0.07	0.001	1432	1432	53	1387	0.07	0.0007	1303	13	1253
M13112122	0.1 45	3 40	0 10	8	1	2.5	1.26	0.21	0.07	0.62	0.19	0.07	0.11	0.59	0.71	0.07	0.001	1362	1362	51	1320	0.07	0.0007	1233	12	1186
WI23112121	0.3 45	3 40	0 10	8	1	1.5	1.12	0.19	0.07	0.63	0.18	0.06	0.13	0.62	0./1	0.06	0.001	1534	1534	1/5	1381	0.06	0.0006	1423	14	1257

	[MN] r	
1 1 2 2 4 2 2 4 2 1 4 2 1 4 2 1 4 2 2 4 2 2 2 2		m)
M23112122 U.3 45 3 400 10 8 1 2.5 1.18 U.20 U.07 U.63 U.18 U.06 U.12 U.60 U.71 U.66 U.001 1464 1464 167 1319 0.06 0.0006 13	2 14	1194
M33112121 0.5 45 3 400 10 8 1 1.5 1.04 0.18 0.07 0.64 0.17 0.06 0.14 0.65 0.71 0.06 0.001 1076 1076 324 1387 0.06 0.0000 13 M33112122 0.5 45 3 400 10 8 1 2.5 1.04 0.18 0.07 0.64 0.17 0.06 0.14 0.65 0.71 0.06 0.001 1598 1598 300 1324 0.06 0.0000 15	+ 10 5 15	1275
M331121 0.7 45 3 400 10 8 1 1.5 0.94 0.17 0.07 0.66 0.16 0.66 0.16 0.65 0.71 0.05 0.000 1863 1863 513 1400 0.05 0.0005 17) 19	1305
M43112122 0.7 45 3 400 10 8 1 2.5 0.98 0.18 0.07 0.66 0.16 0.06 0.15 0.63 0.71 0.05 0.001 1777 1777 489 1337 0.05 0.0005 17	1 18	1241
M53112121 0.9 45 3 400 10 8 1 1.5 0.83 0.16 0.06 0.68 0.14 0.06 0.18 0.68 0.71 0.04 0.000 2129 2129 762 1433 0.04 0.0005 20	5 22	1358
M53112122 0.9 45 3 400 10 8 1 2.5 0.87 0.16 0.07 0.68 0.14 0.06 0.17 0.66 0.71 0.04 0.000 2025 2025 726 1365 0.04 0.0005 19) 21	1290
MI3112211 0.1 45 3 400 10 10 0.6 15 3.64 0.59 0.21 0.62 0.19 0.07 0.03 0.26 0.58 0.14 0.002 411 411 13 402 0.14 0.0014 3 101311212 0.1 45 3 400 10 0.6 25 3.84 0.62 0.22 0.19 0.07 0.03 0.26 0.58 0.14 0.002 38 385 13 377 0.15 0.0015 3	3 3	292
	3 3	299
M23112212 0.3 45 3 400 10 10 0.6 2.5 3.57 0.61 0.22 0.63 0.18 0.06 0.03 0.25 0.56 0.14 0.001 407 407 42 379 0.14 0.0013 3	4 3	278
M33112211 0.5 45 3 400 10 10 0.6 1.5 3.12 0.56 0.21 0.64 0.17 0.06 0.03 0.28 0.59 0.13 0.001 470 470 87 407 0.13 0.0012 3	54	310
M33112212 0.5 45 3 400 10 10 0.6 2.5 3.29 0.59 0.22 0.64 0.17 0.06 0.03 0.26 0.57 0.13 0.001 439 439 81 382 0.13 0.0012 3	7 4	288
M43112211 0.7 45 3 400 10 10 0.6 25 200 055 011 066 016 0.06 0.04 0.30 0.60 0.12 0.001 523 523 144 415 0.12 0.0011 4 M4211211 0.7 45 3 400 10 10 0.6 25 200 055 011 066 016 0.06 0.04 0.30 0.50 012 0.001 498 499 124 290 0.12 0.0011	2 5	329
MM33112211 0.9 45 3 400 10 10 66 15 25 247 019 668 014 066 004 028 0136 012 0001 609 609 230 432 010 00011 5) 6	360
M53112212 0.9 45 3 400 10 10 0.6 2.5 2.65 0.50 0.20 0.68 0.14 0.06 0.04 0.31 0.60 0.11 0.001 567 567 215 404 0.11 0.0011 5) 5	333
M13112221 0.1 45 3 400 10 10 1 1.5 2.94 0.48 0.17 0.62 0.19 0.07 0.04 0.32 0.65 0.13 0.001 532 532 18 519 0.13 0.0013 4	3 4	397
M13112222 0.1 45 3 400 10 10 1 2.5 3.11 0.51 0.18 0.62 0.19 0.07 0.03 0.31 0.63 0.13 0.001 497 497 17 485 0.13 0.0013 3) 4	366
M23112221 0.3 45 3 400 10 10 1 1.5 2.74 0.47 0.17 0.63 0.18 0.06 0.04 0.33 0.65 0.12 0.001 556 566 61 522 0.12 0.0012 4	2 5	408
M23112222 0.5 45 5 400 10 10 1 1 5.5 25 0.45 0.17 0.66 0.17 0.66 0.13 0.064 0.12 0.001 526 526 50 56 468 0.12 0.0012 4 M3311222 0.5 45 3 400 10 10 1 1 5.5 52 0.45 0.17 0.66 0.17 0.06 0.13 0.010 1 617 617 116 529 0.11 0.011 15	3 4	426
	5 5	391
M43112221 0.7 45 3 400 10 10 1 1.5 2.29 0.42 0.16 0.66 0.16 0.06 0.06 0.35 0.67 0.10 0.001 695 695 191 542 0.10 0.0010 6	2 6	454
M43112222 0.7 45 3 400 10 10 1 2.5 2.42 0.45 0.17 0.66 0.16 0.06 0.05 0.34 0.65 0.11 0.001 644 644 178 505 0.11 0.001 5	2 6	416
M53112221 0.9 45 3 400 10 10 1 1.5 2.03 0.38 0.15 0.68 0.14 0.06 0.07 0.38 0.68 0.09 0.001 820 820 303 570 0.09 0.0010 7	3 8	500
1m1531122222 U.9 45 3 40U 1U U.U. 1 2.5 2.15 U.4U U.10 U.08 U.14 U.U0 U.06 U.37 U.07 U.10 U.UUI 7/57 7/57 281 529 0.10 0.0010 7 M1312111 0.1 45 50 0.8 8 0.6 1.5 1.44 0.23 0.08 0.62 0.10 0.07 0.10 0.54 0.71 0.08 0.011 7/56 1796 6.6 1723 0.08 0.000 0.12 0.10 0.10 0.10 0.10 0.10 0.1	7 16	458
MI3121112 0.1 45 3 600 8 8 06 25 148 0.24 0.09 0.62 0.19 0.07 0.10 0.53 0.71 0.08 0.001 1743 1743 64 1690 0.08 0.008 15	5 16	1496
M23121111 0.3 45 3 600 8 8 0.6 1.5 1.34 0.23 0.08 0.63 0.18 0.06 0.11 0.55 0.71 0.07 0.001 1910 1910 216 1725 0.07 0.007 17	5 18	1541
M23121112 0.3 45 3 600 8 8 0.6 2.5 1.37 0.23 0.09 0.63 0.18 0.06 0.10 0.54 0.71 0.07 0.001 1861 1861 210 1682 0.07 0.0007 16	7 17	1499
M33121111 0.5 45 3 600 8 8 0.6 1.5 1.24 0.22 0.08 0.64 0.17 0.06 0.12 0.57 0.71 0.07 0.001 2076 2076 400 1727 0.07 0.0006 19	5 20	1559
M33121112 0.5 45 3 600 8 8 0.6 2.5 1.27 0.23 0.08 0.64 0.17 0.06 0.11 0.56 0.71 0.07 0.001 2021 2021 389 1682 0.07 0.0000 18	2 19	1515
M4312111 0,7 45 3 600 8 8 0,6 1.5 1.12 0.21 0.08 0,66 0.16 0,66 0.13 0,58 0,71 0,66 0,001 2248 619 1702 0,06 0,0000 21	5 25 5 22	1557
M53121111 0.9 45 3 600 8 8 0.6 1.5 1.00 0.19 0.07 0.68 0.14 0.06 0.15 0.62 0.71 0.05 0.001 2652 2652 956 1796 0.05 0.005 25	3 27	1681
M53121112 0.9 45 3 600 8 8 0.6 2.5 1.02 0.19 0.08 0.68 0.14 0.06 0.14 0.62 0.71 0.05 0.001 2584 2584 932 1752 0.05 0.0005 25) 26	1636
M13121121 0.1 45 3 600 8 8 1 1.5 1.09 0.18 0.06 0.62 0.19 0.07 0.13 0.64 0.71 0.06 0.001 2392 2392 90 2315 0.06 0.0006 22) 22	2117
MI3121122 U.1 45 3 500 8 8 1 2.5 L11 0.18 0.07 0.52 0.19 0.07 0.13 0.53 0.71 0.06 0.001 2529 2329 87 2254 0.06 0.00005 24	22	2055
M23121121 0.3 45 3 000 8 8 1 1.5 101 0.17 0.00 0.65 0.18 0.06 0.14 0.64 0.71 0.06 0.001 2504 2304 234 2303 0.00 0.00005 24 M23121121 0.01 0.00 0.001 2504 2304 2304 2305 0.00 0.00005 24 M23121121 0.00 0.001 2504 2304 2305 0.00 0.00005 24 M23121121 0.000 0.001 2504 2504 2504 2505 0.00 0.00005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2504 2505 0.000 0.0005 2505 0.000 0.0005 2504 2505 0.000 0.0005 2505 0.000 0.0005 0.000 0.0005 0.000 000 0.0000 0.0000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.000 0.0000 0.0000 0.000 0.) 24	2067
M33121121 0.5 45 3 600 8 8 1 1.5 0.93 0.17 0.06 0.64 0.17 0.06 0.16 0.66 0.71 0.05 0.001 2796 2796 542 2307 0.05 0.0005 26) 27	2142
M33121122 0.5 45 3 600 8 8 1 2.5 0.96 0.17 0.06 0.64 0.17 0.06 0.16 0.65 0.71 0.05 0.001 2730 2730 529 2253 0.05 0.0005 25	3 27	2088
M43121121 0.7 45 3 600 8 8 8 1 1.5 0.85 0.16 0.06 0.66 0.16 0.06 0.18 0.68 0.71 0.05 0.000 3104 3104 853 2324 0.05 0.0004 29	7 31	2184
M43121122 U. / 45 3 600 8 8 1 2.5 0.87 0.16 0.06 0.66 0.16 0.06 0.17 0.67 0.71 0.05 0.000 3022 3022 831 2265 0.05 0.0005 29 14231111 0.0 45 3 600 8 8 1 2.5 0.87 0.16 0.06 0.66 0.16 0.06 0.17 0.67 0.71 0.05 0.000 3022 3022 831 2265 0.05 0.0005 29 14231111 0.0 45 3 600 8 14 0.06 0.17 0.07 0.07 0.07 0.07 0.07 0.07 0.07	o 30 o 36	2124
M5312112 0.9 45 3 60 8 8 1 2.5 0.7 0.14 0.06 0.68 0.14 0.06 0.20 0.70 0.71 0.04 0.000 346 346 12.0 231 0.4 0.04 0.0004 33	3 35	2204
M13121211 0.1 45 3 600 8 10 0.6 1.5 3.18 0.52 0.19 0.62 0.19 0.07 0.03 0.30 0.62 0.13 0.001 725 725 74 708 0.13 0.001 5	1 6	530
M13121212 0.1 45 3 600 8 10 0.6 2.5 3.27 0.53 0.19 0.62 0.19 0.07 0.03 0.29 0.62 0.13 0.001 702 702 23 687 0.13 0.0013 5) 5	510
M23121211 0.3 45 3 600 8 10 0.6 1.5 2.97 0.51 0.18 0.63 0.18 0.06 0.04 0.31 0.63 0.13 0.001 769 769 81 712 0.12 0.0012 6	7 6	545
M2212121 U.3 45 3 5000 810 U.5 2.5 3.05 U.52 U.19 U.53 U.8 U.06 0.40 0.30 0.52 0.13 0.001 745 745 79 690 0.13 0.0012 5	5 6 1 7	524
113312121 0.5 45 3 600 810 0.6 2.5 2.81 0.50 0.19 0.64 0.17 0.66 0.14 0.66 0.12 0.011 850 850 1.50 720 0.12 0.0011 7	+ / 3 7	546
M43121211 0.7 45 3 600 8 10 0.6 1.5 2.48 0.46 0.18 0.66 0.16 0.06 0.05 0.33 0.64 0.11 0.001 936 936 258 736 0.11 0.0011 8	3 9	603
M43121212 0.7 45 3 600 8 10 0.6 2.5 2.55 0.47 0.18 0.66 0.16 0.06 0.05 0.33 0.63 0.11 0.001 904 904 249 712 0.11 0.0011 7	5 8	580
M53121211 0.9 45 3 600 8 10 0.6 1.5 2.20 0.41 0.17 0.68 0.14 0.06 0.6 36 0.66 0.10 0.001 1100 1100 410 771 0.10 0.0010 10	3 11	663
ND5121212 U.9 45 3 5000 810 U.5 2.5 2.26 U.42 U.17 U.58 U.14 U.06 0.06 0.35 0.55 0.10 0.001 1050 1060 396 744 0.10 0.0010 9	2 10	636
M13121222 0.1 45 3 600 8 10 1 2.5 2.65 0.42 0.15 0.62 0.19 0.07 0.05 0.55 0.68 0.17 0.01 924 934 33 930 0.12 0.0012 7 M13121222 0.1 45 3 600 8 10 1 2.5 2.66 0.42 0.15 0.62 0.19 0.07 0.05 0.55 0.68 0.19 0.01 0.01 1.01 0.0011 7	3 8	744
M23121221 0.3 45 3 600 8 10 1 1.5 2.36 0.40 0.15 0.63 0.18 0.06 0.05 0.37 0.69 0.11 0.001 1024 1024 112 939 0.11 0.0011 8	9 9	767
M23121222 0.3 45 3 600 8 10 1 2.5 2.42 0.41 0.15 0.63 0.18 0.06 0.05 0.36 0.69 0.11 0.001 990 990 107 908 0.11 0.0011 8	1 8	736
M33121221 0.5 45 3 600 8 10 1 1.5 2.17 0.39 0.14 0.64 0.17 0.06 0.06 0.38 0.70 0.11 0.001 1127 1127 213 955 0.10 0.0010 9	7 10	803
M35121222 U.5 45 3 500 8 10 1 2.5 2.23 0.40 0.15 0.64 0.17 0.06 0.06 0.37 0.69 0.11 0.001 1087 1087 205 924 0.11 0.0010 9	o 10	770
044312122 07 45 5 000 8 10 1 1 5 1.97 0.36 0.14 0.60 0.16 0.06 0.07 0.40 0.70 0.10 0.001 1273 1273 351 983 0.10 0.0009 11	3 12	850
M53121221 0.9 45 3 600 8 10 1 1.5 1.75 0.33 0.13 0.68 0.14 0.06 0.08 0.3 0.71 0.08 0.001 147 147 533 1001 0.08 0.0009 13) 14	889
M53121222 0.9 45 3 600 8 10 1 2.5 1.80 0.34 0.14 0.68 0.14 0.06 0.08 0.42 0.71 0.08 0.001 1408 1408 519 975 0.08 0.0009 13) 14	864
M13122111 0.1 45 3 600 10 8 0.6 1.5 1.93 0.31 0.11 0.62 0.19 0.07 0.07 0.45 0.71 0.10 0.001 1335 1335 48 1297 0.10 0.0010 11	4 12	1110
M13122112 0.1 45 3 600 10 8 0.6 2.5 2.01 0.33 0.12 0.62 0.19 0.07 0.07 0.43 0.71 0.10 0.001 1277 1277 46 1242 0.10 0.0010 10	7 11	1056
10/23/22/11 0.5 45 5 000 10 8 0.6 1.5 1.80 0.51 0.11 0.65 0.18 0.06 0.72 0.40 0.71 0.09 0.001 1418 1418 158 1288 0.09 0.0009 12 10/23/22/12 0.3 45 5 0.01 0.8 0.6 1.5 1.88 0.32 0.12 0.63 0.18 0.06 0.72 0.44 0.71 0.10 0.011 1358 1350 1351 1325	1 12	1061
M33122111 0.5 45 3 600 10 8 0.6 1.5 1.66 0.30 0.11 0.64 0.17 0.06 0.8 8 0.6 0.71 0.09 0.01 1529 1529 129 1283 0.08 0.0008 13	1 12 1 14	1123

Name	af	nf	hf mf	Ly	Lx (w Lo	b T1	[S]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-] Sa	Sa2	Sa3	Disp [m]	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1 ISDR [-]	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN
M33122112	0.5	45	3 600	10	8 0.	6 2.5	1.7	3 (0.31	0.12	0.64	0.17	0.06	0.08	0.45	0.71	0.09	0.001	1465	1465	280	1231	0.09	0.0008	1331	14	1071
M43122111	0.7	45	3 600	10	8 0.	6 1.5	1.5	50 (0.28	0.11	0.66	0.16	0.06	0.09	0.48	0.71	0.08	0.001	1686	1686	465	1288	0.08	0.0007	1577	16	1149
M43122112	0.7	45	3 600	10	8 0.	6 2.5	1.5	57 (0.29	0.11	0.66	0.16	0.06	0.09	0.47	0.71	0.08	0.001	1614	1614	445	1235	0.08	0.0008	1505	16	1097
M53122111	0.9	45	3 600	10	8 0.	6 1.5	1.3	33 (0.25	0.10	0.68	0.14	0.06	0.11	0.51	0.71	0.06	0.001	1928	1928	702	1319	0.06	0.0007	1843	19	1206
M13122112	0.9	45	3 600	10	8 0.	1 1.5	1.3	18 (0.26	0.10	0.68	0.14	0.00	0.10	0.50	0.71	0.07	0.001	1740	1740	64	1688	0.07	0.0007	1553	16	140
M13122122	0.1	45	3 600	10	8	1 2.5	1.5	55 (0.25	0.09	0.62	0.19	0.07	0.09	0.51	0.71	0.08	0.001	1659	1659	61	1609	0.08	0.0008	1475	15	1419
M23122121	0.3	45	3 600	10	8	1 1.5	1.3	38 (D.23	0.09	0.63	0.18	0.06	0.10	0.54	0.71	0.07	0.001	1859	1859	210	1680	0.07	0.0007	1694	17	1497
M23122122	0.3	45	3 600	10	8	1 2.5	1.4	4 (0.25	0.09	0.63	0.18	0.06	0.10	0.52	0.71	0.08	0.001	1773	1773	200	1603	0.08	0.0007	1612	16	1424
M33122121 M33122122	0.5	45	3 600	10	8	1 2.5	1.2	33 (0.23	0.08	0.64	0.17	0.06	0.11	0.56	0.71	0.07	0.001	1926	1926	389	1605	0.07	0.0006	1789	19	1513
M43122121	0.7	45	3 600	10	8	1 1.5	1.1	15 (D.21	0.08	0.66	0.16	0.06	0.13	0.58	0.71	0.06	0.001	2245	2245	619	1700	0.06	0.0006	2134	22	1555
M43122122	0.7	45	3 600	10	8	1 2.5	1.2	0 0	0.22	0.09	0.66	0.16	0.06	0.12	0.56	0.71	0.06	0.001	2141	2141	590	1623	0.06	0.0006	2030	21	1479
M53122121	0.9	45	3 600	10	8	1 1.5	1.0)2 (0.19	0.08	0.68	0.14	0.06	0.14	0.62	0.71	0.05	0.001	2581	2581	931	1750	0.05	0.0005	2497	26	1634
M53122122	0.9	45	3 600	10	8	1 2.5	1.0	16 (0.20 n 73	0.08	0.68	0.14	0.06	0.14	0.50	0.71	0.05	0.001	2461	2461	889	1671	0.05	0.0006	2376	25	1555
M13122211 M13122212	0.1	45	3 600	10	10 0.	6 2.5	4.4	70 (0.77	0.20	0.62	0.19	0.07	0.02	0.21	0.48	0.10	0.002	455	455	15	4/4	0.10	0.0016	343	3	307
M23122211	0.3	45	3 600	10	10 0.	6 1.5	4.1	15 (0.71	0.26	0.63	0.18	0.06	0.02	0.22	0.50	0.15	0.002	508	508	52	474	0.15	0.0015	382	4	337
M23122212	0.3	45	3 600	10	10 0.	6 2.5	4.3	88 (0.75	0.27	0.63	0.18	0.06	0.02	0.20	0.48	0.16	0.002	477	477	49	446	0.16	0.0015	355	4	314
M33122211	0.5	45	3 600	10	10 0.	6 1.5	3.8	33 (0.68	0.25	0.64	0.17	0.06	0.03	0.22	0.51	0.14	0.001	545	545	99	477	0.14	0.0014	433	4	349
M33122212	0.5	45	3 600	10	10 0.	6 1 5	2.0	13 L	0.72	0.27	0.64	0.17	0.06	0.02	0.21	0.49	0.15	0.002	511	511	93	448	0.15	0.0014	402	5	324
M43122211 M43122212	0.7	45	3 600	10	10 0.	6 2.5	3.6	6 (0.68	0.25	0.66	0.16	0.06	0.03	0.24	0.50	0.13	0.001	562	562	154	453	0.13	0.0013	468	5	341
M53122211	0.9	45	3 600	10	10 0.	6 1.5	3.0)8 (0.58	0.23	0.68	0.14	0.06	0.04	0.27	0.55	0.12	0.001	696	696	266	501	0.11	0.0012	611	6	400
M53122212	0.9	45	3 600	10	10 0.	6 2.5	3.2	!5 (0.61	0.24	0.68	0.14	0.06	0.03	0.25	0.52	0.12	0.001	649	649	248	468	0.12	0.0012	566	6	371
M13122221	0.1	45	3 600	10	10	1 1.5	3.6	50 (0.59	0.21	0.62	0.19	0.07	0.03	0.26	0.58	0.14	0.002	625	625	20	612	0.14	0.0014	462	5	444
M13122222	0.1	45	3 600	10	10	1 2.5	3.8	SI (J.62	0.22	0.62	0.19	0.07	0.03	0.25	0.56	0.15	0.002	583	583	69	614	0.14	0.0014	516	5	411
M23122221 M23122222	0.3	45	3 600	10	10	1 2.5	3.5	55 (0.61	0.21	0.63	0.18	0.00	0.03	0.27	0.55	0.13	0.001	616	616	64	573	0.13	0.0013	476	5	433
M33122221	0.5	45	3 600	10	10	1 1.5	3.0	9 (0.55	0.21	0.64	0.17	0.06	0.04	0.28	0.59	0.13	0.001	714	714	132	619	0.12	0.0012	587	6	473
M33122222	0.5	45	3 600	10	10	1 2.5	3.2	17 (0.58	0.22	0.64	0.17	0.06	0.03	0.27	0.57	0.13	0.001	665	665	122	578	0.13	0.0012	541	6	436
M43122221	0.7	45	3 600	10	10	1 1.5	2.8	31 (0.52	0.20	0.66	0.16	0.06	0.04	0.30	0.60	0.12	0.001	795	795	219	631	0.11	0.0011	688	7	501
M43122222	0.7	45	3 600	10	10	1 2.5	2.9	97 (10 (0.55	0.21	0.66	0.16	0.06	0.04	0.28	0.58	0.12	0.001	/39	/39	203	588	0.12	0.0012	634	/	462
M53122221	0.9	45	3 600	10	10	1 1.3 1 2.5	2.4	i3 (0.47	0.19	0.68	0.14	0.06	0.03	0.35	0.62	0.10	0.001	859	859	325	611	0.10	0.0011	771	8	504
M13211111	0.1	45	4 400	8	8 0.	6 1.5	1.1	15 (0.19	0.07	0.62	0.19	0.07	0.13	0.62	0.71	0.06	0.000	2000	2000	75	1937	0.06	0.0005	1828	14	1759
M13211112	0.1	45	4 400	8	8 0.	6 2.5	1.1	L7 (0.19	0.07	0.62	0.19	0.07	0.12	0.62	0.71	0.06	0.001	1967	1967	73	1905	0.06	0.0005	1795	14	1727
M23211111	0.3	45	4 400	8	8 0.	6 1.5	1.0)7 (0.18	0.07	0.63	0.18	0.06	0.14	0.63	0.71	0.06	0.000	2154	2154	246	1937	0.06	0.0004	2007	15	1773
M23211112	0.3	45	4 400	8	8 0.	6 1 5	1.0	19 (10 (0.19 n 19	0.07	0.63	0.18	0.06	0.13	0.63	0.71	0.05	0.000	2115	2115	241	1020	0.05	0.0004	2224	15	1738
M33211111 M33211112	0.5	45	4 400	8	8 0.	6 2.5	1.0)1 (0.18	0.07	0.64	0.17	0.00	0.15	0.64	0.71	0.05	0.000	2340	2340	446	1939	0.05	0.0004	2180	17	1756
M43211111	0.7	45	4 400	8	8 0.	6 1.5	0.9	0 0	0.17	0.06	0.66	0.16	0.06	0.17	0.66	0.71	0.05	0.000	2596	2596	714	1948	0.05	0.0004	2500	19	1822
M43211112	0.7	45	4 400	8	8 0.	6 2.5	0.9	91 (0.17	0.07	0.66	0.16	0.06	0.16	0.66	0.71	0.05	0.000	2555	2555	703	1918	0.05	0.0004	2459	19	1792
M53211111	0.9	45	4 400	8	8 0.	6 1.5	0.8	30 (0.15	0.06	0.68	0.14	0.06	0.19	0.69	0.71	0.04	0.000	2974	2974	1062	1998	0.04	0.0003	2903	23	1900
M53211112	0.9	45	4 400	8	8 0.	b 2.5	0.8	SI (0.15	0.05	0.68	0.14	0.06	0.19	0.69	0.71	0.04	0.000	2925	2925	1046	2613	0.04	0.0003	2854	19	2448
M13211121 M13211122	0.1	45	4 400	8	8	1 2.5	0.8	37 (0.14	0.05	0.62	0.19	0.07	0.10	0.70	0.71	0.05	0.000	2654	2654	101	2565	0.05	0.0004	2493	19	2399
M23211121	0.3	45	4 400	8	8	1 1.5	0.8	30 (0.14	0.05	0.63	0.18	0.06	0.19	0.71	0.71	0.05	0.000	2904	2904	336	2597	0.05	0.0003	2768	21	2445
M23211122	0.3	45	4 400	8	8	1 2.5	0.8	31 (0.14	0.05	0.63	0.18	0.06	0.19	0.71	0.71	0.05	0.000	2856	2856	330	2556	0.05	0.0003	2720	21	2402
M33211121	0.5	45	4 400	8	8	1 1.5	0.7	3 (0.13	0.05	0.64	0.17	0.06	0.21	0.71	0.71	0.04	0.000	3168	3168	616	2596	0.04	0.0003	3059	24	2463
M43211122	0.5	45	4 400	8	8	1 2.5	0.7	5 (0.13	0.05	0.66	0.17	0.06	0.20	0.71	0.71	0.04	0.000	3106	3106	968	254/	0.04	0.0003	3446	23	2412
M43211122	0.7	45	4 400	8	8	1 2.5	0.6	58 (D.13	0.05	0.66	0.16	0.06	0.23	0.71	0.71	0.04	0.000	3470	3470	952	2577	0.04	0.0003	3387	26	2468
M53211121	0.9	45	4 400	8	8	1 1.5	0.5	i9 (0.11	0.04	0.68	0.14	0.06	0.26	0.71	0.71	0.03	0.000	4076	4076	1438	2709	0.03	0.0002	4021	32	2632
M53211122	0.9	45	4 400	8	8	1 2.5	0.6	60 (0.11	0.05	0.68	0.14	0.06	0.26	0.71	0.71	0.03	0.000	3995	3995	1411	2657	0.03	0.0003	3939	31	2578
M13211211	0.1	45	4 400	8	10 0.	6 1.5	2.5	01 (0.41	0.15	0.62	0.19	0.07	0.05	0.36	0.69	0.12	0.001	857	857	30	835	0.12	0.0009	696	5	670
M23211212	0.1	45	4 400	8	10 0.	6 1 5	2.3	14 (0.42	0.15	0.62	0.19	0.07	0.05	0.30	0.09	0.12	0.001	921	921	100	844	0.12	0.0009	783	6	692
M23211212	0.3	45	4 400	8	10 0.	6 2.5	2.3	38 (0.41	0.15	0.63	0.18	0.06	0.05	0.37	0.69	0.11	0.001	899	899	98	824	0.11	0.0008	760	6	671
M33211211	0.5	45	4 400	8	10 0.	6 1.5	2.1	15 (0.38	0.14	0.64	0.17	0.06	0.06	0.38	0.70	0.10	0.001	1013	1013	192	859	0.10	0.0008	898	7	723
M33211212	0.5	45	4 400	8	10 0.	6 2.5	2.2	0 0	0.39	0.15	0.64	0.17	0.06	0.06	0.37	0.70	0.11	0.001	988	988	187	839	0.11	0.0008	873	7	703
M43211211	0.7	45	4 400	8	10 0.	6 1.5	1.9	95 (U.36	0.14	0.66	0.16	0.06	0.07	0.40	0.71	0.10	0.001	1143	1143	315	882	0.10	0.0007	1048	8	764
M53211212 M53211211	0.7	45	4 400	8	10 0.	0 Z.5	1.9	13 (0.37	0.14	0.00	0.16	0.06	0.07	0.39	0.70	0.10	0.001	1298	1298	308	805	0.10	0.0007	1025	8 10	747
M53211212	0.9	45	4 400	8	10 0.	6 2.5	1.7	7 (0.33	0.13	0.68	0.14	0.06	0.08	0.43	0.71	0.08	0.001	1272	1272	468	881	0.08	0.0007	1194	9	781
M13211221	0.1	45	4 400	8	10	1 1.5	1.9)4 (0.32	0.11	0.62	0.19	0.07	0.07	0.44	0.71	0.10	0.001	1182	1182	43	1149	0.10	0.0008	1021	8	983
M13211222	0.1	45	4 400	8	10	1 2.5	1.9	97 (0.32	0.12	0.62	0.19	0.07	0.07	0.44	0.71	0.10	0.001	1159	1159	42	1126	0.10	0.0008	999	8	961
M23211221	0.3	45	4 400	8	10	1 1.5	1.8	30 (0.31	0.11	0.63	0.18	0.06	0.08	0.45	0.71	0.09	0.001	1256	1256	140	1141	0.09	0.0007	1115	9	985
M33211222	0.3	45	4 400	8	10	1 1.5	1.8	54 (16 (0.31	0.11	0.63	0.18	0.06	0.07	0.45	0.71	0.10	0.001	1232	1232	259	1120	0.10	0.0007	1092	8	964
M33211222	0.5	45	4 400	8	10	1 2.5	1.7	0 (0.30	0.11	0.64	0.17	0.06	0.08	0.46	0.71	0.09	0.001	1328	1328	254	1115	0.09	0.0006	1209	9	974
M43211221	0.7	45	4 400	8	10	1 1.5	1.5	51 (0.28	0.11	0.66	0.16	0.06	0.09	0.48	0.71	0.08	0.001	1492	1492	411	1140	0.08	0.0006	1395	11	1017

Name	af	nf l	մ ո	մեւ	Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 FI	M2 [-]	M3 [-]	Sa	Sa2	Sa3	Disp	ISDR [-]	Total Moment	Total Shear	Outrigger Moment	Wall Moment	Mode 1 Disp	Mode 1	Mode 1 Moment	Mode 1 Shear	Mode 1 wall Moment [MN-
75007465	12734				10		31.94322					57572 A B (B)		02-005	1025115		[m]		[MN-m]	(MN)	[MN-m]	[MN-m]	[m]	ISDR [-]	[MN-m]	[MN]	m]
M43211222	0.7	45	44	00	8 10	1	2.5	1.54	0.28	0.11	0.66	0.16	0.06	0.09	0.47	0.71	0.08	0.001	1464	1464	403	1119	0.08	0.0006	1367	11	996
M53211221	0.9	45	44	00	8 10	1	1.5	1.34	0.25	0.10	0.68	0.14	0.06	0.11	0.51	0.71	0.07	0.001	1706	1706	621	1167	0.06	0.0005	1631	13	1067
M53211222	0.9	45	4 4	00	8 10	1	2.5	1.37	0.26	0.10	0.68	0.14	0.06	0.10	0.50	0.71	0.07	0.001	1669	1669	608	1142	0.07	0.0005	1594	13	1043
M13212111	0.1	45	4 4	00 1	0 8	0.6	1.5	1.53	0.25	0.09	0.62	0.19	0.07	0.09	0.52	0.71	0.08	0.001	1491	1491	55	1446	0.08	0.0006	1326	10	1276
M13212112	0.1	45	4 4	00 1	0 8	0.6	2.5	1.58	0.26	0.09	0.62	0.19	0.07	0.09	0.50	0.71	0.08	0.001	1445	1445	190	1402	0.08	0.0006	1283	10	1234
M23212111	0.3	45	4 4	00 1	0 0	0.0	2.5	1.42	0.24	0.09	0.03	0.18	0.00	0.10	0.55	0.71	0.08	0.001	1543	15/3	174	1396	0.08	0.0000	1401	11	1237
M33212112	0.5	45	4 4	00 1	0 8	0.6	1.5	1.31	0.23	0.09	0.64	0.17	0.06	0.11	0.54	0.71	0.07	0.001	1732	1732	333	1443	0.07	0.0005	1610	12	1296
M33212112	0.5	45	4 4	00 1	0 8	0.6	2.5	1.35	0.24	0.09	0.64	0.17	0.06	0.10	0.53	0.71	0.07	0.001	1676	1676	322	1397	0.07	0.0005	1554	12	1252
M43212111	0.7	45	4 4	00 1	08	0.6	1.5	1.19	0.22	0.08	0.66	0.16	0.06	0.12	0.57	0.71	0.06	0.000	1925	1925	531	1459	0.06	0.0005	1826	14	1331
M43212112	0.7	45	4 4	00 1	0 8	0.6	2.5	1.23	0.23	0.09	0.66	0.16	0.06	0.12	0.55	0.71	0.06	0.000	1861	1861	513	1412	0.06	0.0005	1763	14	1284
M53212111	0.9	45	44	00 1	08	0.6	1.5	1.06	0.20	0.08	0.68	0.14	0.06	0.14	0.60	0.71	0.05	0.000	2213	2213	799	1502	0.05	0.0004	2138	17	1399
M53212112	0.9	45	4 4	00 1	0 8	0.6	2.5	1.09	0.20	0.08	0.68	0.14	0.06	0.13	0.59	0.71	0.05	0.000	2140	2140	774	1454	0.05	0.0004	2064	16	1351
M13212121	0.1	45	4 4	00 1	0 8	1	1.5	1.15	0.19	0.07	0.62	0.19	0.07	0.13	0.62	0.71	0.06	0.000	2007	2007	75	1943	0.06	0.0005	1835	14	1765
M13212122	0.1	45	4 4	00 1	0 8	1	2.5	1.18	0.19	0.07	0.62	0.19	0.07	0.12	0.61	0.71	0.07	0.001	1945	1945	/3	1883	0.07	0.0005	1//3	13	1/05
M23212121	0.3	45	4 4	00 1	0 8	i 1	1.5	1.07	0.18	0.07	0.63	0.18	0.06	0.14	0.63	0.71	0.06	0.000	2162	2162	24/	1944	0.06	0.0004	2015	15	1780
M33212122	0.5	45	4 4 4 4	00 1	0 8		1.5	0.98	0.19	0.07	0.05	0.18	0.00	0.15	0.62	0.71	0.00	0.000	2000	2000	456	1946	0.00	0.0004	2233	17	1714
M33212121 M33212122	0.5	45	4 4	00 1	0 0	1	2.5	1.02	0.10	0.07	0.64	0.17	0.00	0.15	0.63	0.71	0.05	0.000	2333	2335	430	1881	0.05	0.0004	2255	17	1733
M43212121	0.7	45	4 4	00 1	0 8	1	1.5	0.89	0.17	0.06	0.66	0.16	0.06	0.17	0.66	0.71	0.05	0.000	2604	2604	716	1954	0.05	0.0004	2508	19	1828
M43212122	0.7	45	44	00 1	0 8	: 1	2.5	0.92	0.17	0.07	0.66	0.16	0.06	0.16	0.65	0.71	0.05	0.000	2528	2528	696	1898	0.05	0.0004	2431	19	1772
M53212121	0.9	45	4 4	00 1	0 8	1	1.5	0.79	0.15	0.06	0.68	0.14	0.06	0.19	0.69	0.71	0.04	0.000	2984	2984	1066	2005	0.04	0.0003	2914	23	1907
M53212122	0.9	45	4 4	00 1	0 8	1	2.5	0.82	0.15	0.06	0.68	0.14	0.06	0.18	0.68	0.71	0.04	0.000	2891	2891	1034	1945	0.04	0.0003	2821	22	1846
M13212211	0.1	45	4 4	00 1	0 10	0.6	1.5	3.48	0.57	0.20	0.62	0.19	0.07	0.03	0.27	0.59	0.14	0.001	580	580	19	567	0.14	0.0010	431	3	415
M13212212	0.1	45	4 4	00 1	0 10	0.6	2.5	3.61	0.59	0.21	0.62	0.19	0.07	0.03	0.26	0.58	0.14	0.001	554	554	18	542	0.14	0.0011	409	3	394
M23212211	0.3	45	4 4	00 1	0 10	0.6	1.5	3.24	0.55	0.20	0.63	0.18	0.06	0.03	0.28	0.60	0.13	0.001	613	613	64	569	0.13	0.0009	482	4	426
M23212212	0.3	45	4 4	00 1	0 10	0.6	2.5	3.36	0.57	0.21	0.63	0.18	0.06	0.03	0.27	0.59	0.13	0.001	586	586	122	544	0.13	0.0010	457	3	403
M22212211	0.5	45	4 4	00 1	0 10	0.6	2.5	2.99	0.55	0.20	0.64	0.17	0.06	0.04	0.29	0.60	0.12	0.001	622	622	123	5/4	0.12	0.0009	520	4	442
M43212212	0.7	45	4 4	00 1	0 10	0.0	1.5	2.71	0.55	0.19	0.66	0.17	0.00	0.04	0.28	0.59	0.13	0.001	740	740	204	585	0.12	0.0009	644	4	419
M43212212	0.7	45	4 4	00 1	0 10	0.6	2.5	2.81	0.52	0.20	0.66	0.16	0.06	0.04	0.30	0.60	0.12	0.001	704	704	194	559	0.12	0.0008	609	5	444
M53212211	0.9	45	4 4	00 1	0 10	0.6	1.5	2.40	0.45	0.18	0.68	0.14	0.06	0.05	0.34	0.63	0.10	0.001	865	865	325	610	0.10	0.0008	786	6	514
M53212212	0.9	45	44	00 1	0 10	0.6	2.5	2.50	0.47	0.19	0.68	0.14	0.06	0.05	0.33	0.62	0.10	0.001	822	822	309	582	0.10	0.0008	743	6	486
M13212221	0.1	45	44	00 1	0 10	1	1.5	2.72	0.44	0.16	0.62	0.19	0.07	0.04	0.34	0.67	0.12	0.001	779	779	27	761	0.12	0.0009	619	5	595
M13212222	0.1	45	44	00 1	0 10	1	2.5	2.83	0.46	0.17	0.62	0.19	0.07	0.04	0.33	0.66	0.12	0.001	744	744	25	726	0.12	0.0009	584	4	562
M23212221	0.3	45	4 4	00 1	0 10	1	1.5	2.53	0.43	0.16	0.63	0.18	0.06	0.05	0.35	0.68	0.12	0.001	833	833	90	766	0.11	0.0008	694	5	613
M23212222	0.3	45	4 4	00 1	0 10	1	2.5	2.63	0.45	0.16	0.63	0.18	0.06	0.04	0.34	0.66	0.12	0.001	794	794	85	731	0.12	0.0008	654	5	578
M33212221	0.5	45	4 4	00 1	0 10	1	1.5	2.33	0.42	0.16	0.64	0.17	0.06	0.05	0.36	0.68	0.11	0.001	913	913	1/2	7/8	0.11	0.0008	796	6	641
NA42212222	0.5	45	4 4	00 1	0 10	1	2.5	2.43	0.43	0.15	0.64	0.17	0.06	0.05	0.35	0.67	0.11	0.001	1022	1022	103	741	0.11	0.0008	029	7	693
M43212221	0.7	45	4 4	00 1	0 10	1	2.5	2.12	0.59	0.15	0.00	0.10	0.00	0.00	0.37	0.69	0.10	0.001	979	979	284	761	0.10	0.0007	958	7	644
M53212222	0.9	45	4 4	00 1	0 10	1	1.5	1.88	0.35	0.14	0.68	0.10	0.06	0.07	0.41	0.70	0.09	0.001	1195	1195	441	829	0.09	0.0007	1118	9	732
M53212222	0.9	45	4 4	00 1	0 10	1	2.5	1.95	0.37	0.15	0.68	0.14	0.06	0.07	0.40	0.69	0.09	0.001	1147	1147	423	796	0.09	0.0007	1070	8	700
M13221111	0.1	45	4 6	00	8 8	0.6	1.5	1.41	0.23	0.08	0.62	0.19	0.07	0.10	0.55	0.71	0.08	0.001	2434	2434	90	2360	0.08	0.0006	2180	17	2097
M13221112	0.1	45	4 6	00	88	0.6	2.5	1.43	0.23	0.08	0.62	0.19	0.07	0.10	0.54	0.71	0.08	0.001	2393	2393	88	2320	0.08	0.0006	2140	16	2059
M23221111	0.3	45	46	00	88	0.6	1.5	1.31	0.22	0.08	0.63	0.18	0.06	0.11	0.56	0.71	0.07	0.001	2606	2606	295	2353	0.07	0.0005	2385	18	2107
M23221112	0.3	45	4 6	00	8 8	0.6	2.5	1.34	0.23	0.08	0.63	0.18	0.06	0.11	0.55	0.71	0.07	0.001	2560	2560	290	2312	0.07	0.0005	2339	18	2066
M33221111	0.5	45	4 6	00	8 8	0.6	1.5	1.21	0.22	0.08	0.64	0.17	0.06	0.12	0.57	0.71	0.06	0.000	2836	2836	547	2358	0.06	0.0005	2650	20	2134
M33221112	0.5	45	4 6	00	88	0.6	2.5	1.23	0.22	0.08	0.64	0.17	0.06	0.12	0.57	0.71	0.07	0.000	2783	2783	536	2315	0.07	0.0005	2597	20	2091
M43221111	0.7	45	4 0	00	0 8 9 0	0.6	2.5	1.10	0.20	0.08	0.66	0.16	0.06	0.13	0.00	0.71	0.06	0.000	3157	3005	870	2380	0.06	0.0004	2019	23	2193
M53221112	0.7	45	4 0	00	8 9	0.0	2.5	0.97	0.21	0.08	0.00	0.10	0.00	0.13	0.39	0.71	0.00	0.000	3622	3633	1204	2340	0.00	0.0004	2940	23	214/
M53221112	0.9	45	4 6	00	88	0.6	2.5	0.99	0.19	0.07	0.68	0.14	0.06	0.15	0.63	0.71	0.05	0.000	3555	3555	1281	2408	0.05	0.0004	3443	20	2258
M13221121	0.1	45	4 6	00	8 8	1	1.5	1.05	0.17	0.06	0.62	0.19	0.07	0.14	0.65	0.71	0.06	0.000	3318	3318	125	3211	0.06	0.0004	3065	23	2949
M13221122	0.1	45	4 6	00	8 8	1	2.5	1.07	0.17	0.06	0.62	0.19	0.07	0.14	0.65	0.71	0.06	0.000	3260	3260	123	3155	0.06	0.0004	3006	23	2892
M23221121	0.3	45	4 6	00	8 8	1	1.5	0.98	0.17	0.06	0.63	0.18	0.06	0.15	0.66	0.71	0.05	0.000	3564	3564	409	3199	0.05	0.0004	3347	26	2956
M23221122	0.3	45	46	00	88	: 1	2.5	0.99	0.17	0.06	0.63	0.18	0.06	0.15	0.66	0.71	0.06	0.000	3498	3498	401	3141	0.06	0.0004	3280	25	2897
M33221121	0.5	45	4 6	00	8 8	1	1.5	0.90	0.16	0.06	0.64	0.17	0.06	0.17	0.67	0.71	0.05	0.000	3865	3865	750	3185	0.05	0.0004	3685	28	2967
M33221122	0.5	45	4 6	00	8 8	1	2.5	0.91	0.16	0.06	0.64	0.17	0.06	0.16	0.67	0.71	0.05	0.000	3803	3803	737	3136	0.05	0.0004	3623	28	2917
M43221121	0.7	45	4 6	00	8 8	1	1.5	0.82	0.15	0.06	0.66	0.16	0.06	0.18	0.69	0.71	0.04	0.000	4306	4306	1183	3220	0.04	0.0003	4166	32	3036
M52221122	0.7	45	4 6	00	8 8	1	2.5	0.83	0.15	0.06	0.66	0.16	0.06	0.18	0.58	0.71	0.04	0.000	4230	4230	1163	3165	0.04	0.0003	4090	32	2980
M53221121	0.9	45	4 0	00 0	0 8 8 9	1	2.5	0.72	0.14	0.05	0.68	0.14	0.06	0.21	0.71	0.71	0.04	0.000	4931 4931	4931	1/56	3303	0.04	0.0003	4830	38	3101
M13221221	0.9	45	4 6	00 :	5 0 8 10	0.6	1.5	3.07	0.50	0.18	0.62	0.19	0.07	0.04	0.31	0.64	0.13	0.001	1008	1008	34	985	0.13	0.0010	774	57	744
M13221212	0.1	45	4 6	00	8 10	0.6	2.5	3.13	0.51	0.18	0.62	0.19	0.07	0.03	0.30	0.63	0.13	0.001	984	984	33	962	0.13	0.0010	752	6	724
M23221211	0.3	45	4 6	00	8 10	0.6	1.5	2.86	0.49	0.18	0.63	0.18	0.06	0.04	0.32	0.64	0.12	0.001	1072	1072	114	990	0.12	0.0009	866	7	765
M23221212	0.3	45	4 6	00	8 10	0.6	2.5	2.92	0.50	0.18	0.63	0.18	0.06	0.04	0.31	0.63	0.12	0.001	1047	1047	111	968	0.12	0.0009	842	6	744
M33221211	0.5	45	4 6	00	8 10	0.6	1.5	2.64	0.47	0.18	0.64	0.17	0.06	0.04	0.33	0.64	0.12	0.001	1166	1166	218	1002	0.11	0.0008	989	8	796
M33221212	0.5	45	4 6	00	8 10	0.6	2.5	2.69	0.48	0.18	0.64	0.17	0.06	0.04	0.32	0.64	0.12	0.001	1138	1138	212	979	0.12	0.0008	962	7	775
M43221211	0.7	45	4 6	00	8 10	0.6	1.5	2.39	0.44	0.17	0.66	0.16	0.06	0.05	0.34	0.65	0.11	0.001	1308	1308	360	1025	0.11	0.0008	1163	9	847
M43221212	0.7	45	4 6	00	8 10	0.6	2.5	2.44	0.45	0.17	0.66	0.16	0.06	0.05	0.34	0.65	0.11	0.001	1276	1276	352	1002	0.11	0.0008	1131	9	824
IVI53221211	0.9	45	4 6	00 1	8 IO	0.6	1.5	2.12	0.40	0.16	0.68	0.14	0.06	0.06	0.37	0.67	0.10	0.001	1540	1540	5/2	1075	0.10	0.0008	1425	11	933

														Disn		Total	Total	Outrigger	Wali	Mode	Mode 1	Mode 1	Mode 1	Mode 1 wall
Name	af nf	hf mf	Ly Lx	tw	Lcb	T1 [s]	T2 [s]	T3 [s]	M1 [-]	M2 [-]	M3 [-]	Sa	Sa2 Sa3	[m]	ISDR [-]	Moment	Shear	Moment	Moment	1 Disp	ISDR [-]	Moment	Shear	Moment [MN-
M53221212	09.4	4 600	9 8 10	1 06	2.5	2 17	0.41	0.16	0.68	0.14	0.06	0.06	0.37 0.67	0.10	0.001	[MR-m] 1500	1500	[AR-m]	10/19	0.10	0.0008	1385	(d)() 11	006
M13221212	0.1 /	4 600	1 8 10	1 1	1.5	2.27	0.11	0.14	0.62	0.10	0.00	0.05	0.38 0.71	0.11	0.001	1376	1376	18	13/1	0.11	0.0008	1138	0	1005
M13221221	0.1 4	4 600	8 10) 1	2.5	2.37	0.30	0.14	0.62	0.10	0.07	0.05	0.30 0.71	0.11	0.001	13/4	13/0	40	1310	0.11	0.0008	1105	8	1053
M32221222	0.2 4	4 600	0 10	<u>, 1</u>	1.5	2.42	0.35	0.14	0.62	0.19	0.07	0.05	0.20 0.71	0.11	0.001	1/99	1/99	162	1250	0.11	0.0000	1391	10	1121
M22221221	0.3 4.	4 000	$\frac{9}{9}$	$\frac{1}{1}$	2.5	2.21	0.38	0.14	0.03	0.10	0.00	0.00	0.39 0.71	0.11	0.001	1400	1400	159	1224	0.11	0.0008	1201	01	1007
M22221222	0.5 4.	4 000	0 9 10) <u>1</u>	1.5	2.23	0.36	0.14	0.03	0.10	0.00	0.00	0.38 0.71	0.11	0.001	1645	1645	212	1200	0.11	0.0008	1471	11	1197
IVI33221221	0.5 4.	4 000		<u> </u>	1.5	2.04	0.30	0.14	0.04	0.17	0.00	0.07	0.40 0.71	0.10	0.001	1045	1045	313	1350	0.10	0.0007	1471	11	1104
M42221222	0.5 4	4 000) <u>1</u>	1.5	1.00	0.37	0.14	0.64	0.17	0.00	0.00	0.59 0.71	0.10	0.001	1002	1002	504	1403	0.10	0.0007	1420	12	1130
IVI43221221	0.7 4:	4 600		1	2.5	1.85	0.34	0.13	0.66	0.10	0.06	0.07	0.42 0.71	0.09	0.001	1820	1820	502	1402	0.09	0.0007	1675	13	1220
1143221222	0.7 4:	4 600			2.5	1.88	0.35	0.13	0.66	0.16	0.06	0.07	0.41 0.71	0.09	0.001	1/82	1/82	491	1374	0.09	0.0007	1038	13	1194
IVI53221221	0.9 45	4 600		1 1	1.5	1.64	0.31	0.12	0.68	0.14	0.06	0.08	0.45 0.71	0.08	0.001	2062	2062	757	1423	0.08	0.0006	1946	15	12/4
IVI53221222	0.9 4:	4 600) 1	2.5	1.67	0.31	0.13	0.68	0.14	0.06	0.08	0.45 0.71	0.08	0.001	2023	2023	/44	1397	0.08	0.0006	1907	15	1248
M13222111	0.1 45	4 600) 10 8	3 0.6	1.5	1.8/	0.31	0.11	0.62	0.19	0.07	0.07	0.45 0.71	0.10	0.001	1832	1832	66	1/80	0.10	0.0007	1590	12	1530
M13222112	0.1 45	4 600) 10 8	3 0.6	2.5	1.93	0.31	0.11	0.62	0.19	0.07	0.07	0.44 0.71	0.10	0.001	1778	1778	64	1728	0.10	0.0008	1537	12	1478
M23222111	0.3 45	4 600) 10 8	3 0.6	1.5	1.75	0.30	0.11	0.63	0.18	0.06	0.08	0.46 0.71	0.09	0.001	1946	1946	217	1767	0.09	0.0007	1735	13	1532
M23222112	0.3 45	6 4 600) 10 8	3 0.6	2.5	1.80	0.31	0.11	0.63	0.18	0.06	0.08	0.45 0.71	0.09	0.001	1889	1889	211	1716	0.09	0.0007	1678	13	1482
M33222111	0.5 45	4 600) 10 8	3 0.6	1.5	1.61	0.29	0.11	0.64	0.17	0.06	0.09	0.47 0.71	0.08	0.001	2101	2101	402	1761	0.08	0.0006	1922	15	1548
M33222112	0.5 45	4 600) 10 8	3 0.6	2.5	1.66	0.30	0.11	0.64	0.17	0.06	0.08	0.46 0.71	0.09	0.001	2036	2036	390	1709	0.08	0.0006	1857	14	1496
M43222111	0.7 45	6 4 600) 10 8	3 0.6	1.5	1.46	0.27	0.10	0.66	0.16	0.06	0.10	0.49 0.71	0.07	0.001	2321	2321	640	1770	0.07	0.0005	2177	17	1586
M43222112	0.7 45	4 600) 10 8	3 0.6	2.5	1.50	0.28	0.11	0.66	0.16	0.06	0.09	0.48 0.71	0.08	0.001	2245	2245	619	1715	0.08	0.0006	2100	16	1530
M53222111	0.9 45	6 4 600) 10 8	3 0.6	1.5	1.29	0.24	0.10	0.68	0.14	0.06	0.11	0.53 0.71	0.06	0.001	2653	2653	965	1813	0.06	0.0005	2539	20	1662
M53222112	0.9 45	4 600) 10 8	3 0.6	2.5	1.33	0.25	0.10	0.68	0.14	0.06	0.11	0.51 0.71	0.06	0.001	2567	2567	935	1756	0.06	0.0005	2454	19	1606
M13222121	0.1 45	4 600) 10 8	31	1.5	1.40	0.23	0.08	0.62	0.19	0.07	0.10	0.55 0.71	0.08	0.001	2442	2442	90	2368	0.08	0.0006	2188	17	2105
M13222122	0.1 45	4 600) 10 8	31	2.5	1.45	0.24	0.08	0.62	0.19	0.07	0.10	0.54 0.71	0.08	0.001	2365	2365	87	2293	0.08	0.0006	2113	16	2033
M23222121	0.3 45	5 4 600) 10 8	31	1.5	1.31	0.22	0.08	0.63	0.18	0.06	0.11	0.56 0.71	0.07	0.001	2616	2616	296	2362	0.07	0.0005	2394	18	2115
M23222122	0.3 45	4 600) 10 8	31	2.5	1.35	0.23	0.08	0.63	0.18	0.06	0.11	0.55 0.71	0.07	0.001	2528	2528	286	2284	0.07	0.0005	2309	18	2039
M33222121	0.5 45	4 600) 10 8	31	1.5	1.21	0.21	0.08	0.64	0.17	0.06	0.12	0.58 0.71	0.06	0.000	2847	2847	549	2367	0.06	0.0005	2661	20	2143
M33222122	0.5 45	4 600) 10 8	31	2.5	1.25	0.22	0.08	0.64	0.17	0.06	0.12	0.56 0.71	0.07	0.000	2746	2746	529	2285	0.07	0.0005	2561	20	2062
M43222121	0.7 45	4 600) 10 8	31	1.5	1.09	0.20	0.08	0.66	0.16	0.06	0.13	0.60 0.71	0.06	0.000	3170	3170	873	2395	0.06	0.0004	3022	23	2202
M43222122	0.7 45	4 600) 10 8	31	2.5	1.13	0.21	0.08	0.66	0.16	0.06	0.13	0.59 0.71	0.06	0.000	3053	3053	841	2309	0.06	0.0004	2904	23	2116
M53222121	0.9 45	4 600) 10 8	31	1.5	0.97	0.18	0.07	0.68	0.14	0.06	0.15	0.63 0.71	0.05	0.000	3636	3636	1309	2461	0.05	0.0004	3525	28	2307
M53222122	0.9 45	4 600) 10 8	31	2.5	1.00	0.19	0.08	0.68	0.14	0.06	0.15	0.62 0.71	0.05	0.000	3509	3509	1265	2378	0.05	0.0004	3397	27	2223
M13222211	0.1 45	4 600) 10 10	0.6	1.5	4.26	0.69	0.25	0.62	0.19	0.07	0.02	0.22 0.52	0.16	0.001	681	681	22	667	0.15	0.0012	487	4	468
M13222212	0.1 45	4 600	0 10 10	0.6	2.5	4.42	0.72	0.26	0.62	0.19	0.07	0.02	0.21 0.50	0.16	0.001	652	652	21	639	0.16	0.0012	463	4	445
M23222211	0.3 45	4 600) 10 10	0.6	1.5	3.97	0.68	0.25	0.63	0.18	0.06	0.02	0.23 0.52	0.15	0.001	716	716	74	667	0.15	0.0011	542	4	479
M23222212	0.3 4	4 600	10 10	0.6	2.5	4.12	0.70	0.26	0.63	0.18	0.06	0.02	0.22 0.50	0.15	0.001	685	685	70	639	0.15	0.0011	515	4	455
M33222211	0.5 45	4 600) 10 10	0.6	1.5	3.66	0.65	0.24	0.64	0.17	0.06	0.03	0.23 0.53	0.14	0.001	768	768	140	670	0.14	0.0010	615	5	495
M33222212	0.5 4	4 600	0 10 10	0.6	2.5	3.80	0.68	0.25	0.64	0.17	0.06	0.03	0.23 0.51	0.14	0.001	734	734	134	641	0.14	0.0010	584	4	470
M43222211	0.7 4	4 60	10 10	0.6	1.5	3 32	0.61	0.24	0.66	0.16	0.06	0.03	0.25 0.54	0.13	0.001	850	850	233	681	0.13	0.0009	718	6	523
M43222212	0.7.4	4 60	10 10	0.6	2.5	3.44	0.64	0.25	0.66	0.16	0.06	0.03	0.24 0.52	0.13	0.001	809	809	222	649	0.13	0.0009	681	5	496
M53222211	09.4	4 600	10 10	0.6	15	2.94	0.55	0.22	0.68	0.14	0.06	0.04	0.28 0.56	0.11	0.001	986	986	376	707	0.11	0.0009	871	7	570
M53222211	0.9 4	4 600	10 10	0.0	2.5	3.06	0.55	0.22	0.68	0.14	0.06	0.04	0.20 0.50	0.11	0.001	030	930	358	675	0.11	0.0000	825	6	5/0
M132222212	0.1 /	4 600	10 10	1 1	1.5	3.33	0.57	0.25	0.60	0.14	0.00	0.04	0.27 0.55	0.11	0.001	917	917	30	807	0.13	0.0005	600	5	664
M12222221	0.1 4	4 600	10 10	1 1	2.5	2.46	0.54	0.10	0.62	0.10	0.07	0.05	0.22 0.01	0.14	0.001	975	975	20	856	0.13	0.0010	652	5	627
M23222222	0.1 4.	1 600	10 10	, <u>1</u>) 1	1.5	3.40	0.50	0.20	0.02	0.19	0.07	0.03	0.20 0.00	0.14	0.001	973	070	103	000	0.14	0.0010	771	ر ۵	691
M22222221	0.3 4	4 000	10 10	/ 1) 1	1.J	2.22	0.55	0.19	0.05	0.10	0.00	0.02	0.25 0.01	0.13	0.001	972	026	102	900	0.15	0.0009	772	6	642
M22222222	0.5 43	4 000	10 10	/ <u>1</u>	2.5	3.22	0.55	0.20	0.05	0.10	0.00	0.03	0.20 0.00	0.13	0.001	1053	1053	37	009	0.13	0.0009	120	7	700
N133222221	0.5 45	4 000) 10 IU	1 1	2.5	2.80	0.51	0.19	0.64	0.17	0.06	0.04	0.30 0.62	0.12	0.001	1003	1003	196	909	0.12	0.0009	880	/	708
10155222222	0.5 45	4 000	10 IU	/ 1) 1	2.3	2.97	0.55	0.20	0.64	0.17	0.00	0.04	0.29 0.01	0.12	0.001	1177	1177	180	807	0.12	0.0009	1033	0	200
1432222221	0.7 43	4 000	, 10 10		2.5	2.59	0.48	0.18	0.00	0.10	0.00	0.05	0.32 0.03	0.11	0.001	1110	11177	524	929	0.11	0.0008	1053	8	703
N45222222	0.7 45	4 000	10 IU	1 1	2.5	2.09	0.50	0.19	0.00	0.10	0.06	0.04	0.31 0.62	0.11	0.001	1118	1118	308	884	0.11	0.0008	1262	10	/10
IVID3222221	0.9 45	4 600	10 10		C.L	2.30	0.43	0.17	0.08	0.14	0.05	0.05	0.35 0.65	0.10	0.001	1380	1380	516	970	0.10	0.0008	1202	10	826
IVI53222222	0.9 45	4 600	10 10	, 1	2.5	2.39	0.45	0.18	0.68	0.14	0.06	0.05	0.34 0.64	0.10	0.001	1306	1306	490	922	0.10	0.0008	1188	9	111

Name	T1 [s]	Sa(T1)	Sa(T2)	Sa(T3)	DOC	Disp	ISDR [-]	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	Disp	ISDR [-]3	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	C1 [-]	c2[·]	C3 [·]	Mdy/
V11111111	0.73	0.55	0.88	0.89	0.70	0.11	0.0029	[MN·m]	[MN]	[MN]	[MIN] 115	[MN·m]	0.11	0.0029	[MN-m]4	[MN]5	[MN]6	[MNJ2	[MN·m]8	1.00	0.18	0.04	MST [•]
V111111112	0.73	0.53	0.88	0.89	0.74	0.11	0.0031	1073	1073	11	115	141	0.11	0.0031	1072	33	10	115	136	1.00	0.19	0.05	0.49
V21111111 V211111112	0.68	0.59	0.89	0.89	0.74	0.10	0.0028	1192	1192	12	129	134	0.10	0.0026	1190	37	12	129	155	1.00	0.18	0.04	0.52
V31111111 V31111112	0.62	0.63	0.89	0.89	0.77	0.09	0.0024	1265 1234	1265 1234	13 14	143	149 124	0.09	0.0024	1263 1232	39 38	13 13	143 142	145 120	1.00	0.19	0.05	0.56
V41111111	0.58	0.67	0.89	0.89	0.80	0.08	0.0022	1342	1342	15	157	138	0.08	0.0022	1339	41	15	157	134	1.00	0.20	0.05	0.60
V51111112	0.54	0.66	0.89	0.89	0.83	0.08	0.0025	1315	1315	15	130	113	0.07	0.0025	1402	40	15	130	111	1.00	0.21	0.05	0.65
V51111112 V11111121	0.55	0.70	0.89	0.89	0.85	0.08	0.0021	1382 1306	1382 1306	17	169 136	103 203	0.08	0.0021	1379 1304	42	16 12	169 136	100	1.00	0.22	0.06	0.65
V11111122	0.63	0.62	0.89	0.89	0.74	0.09	0.0024	1259	1259	12	136	169	0.09	0.0024	1258	39	12	136	165	1.00	0.16	0.04	0.49
V21111121 V21111122	0.58	0.69	0.89	0.89	0.75	0.08	0.0021	1395	1356	14	152	192	0.08	0.0021	1355	43	14	152	100	1.00	0.15	0.03	0.52
V31111121 V31111122	0.52	0.73	0.89	0.89	0.76	0.07	0.0019	1465 1435	1465 1435	16 16	166 167	179 150	0.07	0.0019	1463 1433	45 44	15 16	166 167	176	1.00	0.16	0.04	0.56
V41111121	0.48	0.76	0.89	0.89	0.79	0.06	0.0017	1514	1514	17	178	164	0.06	0.0017	1512	46	17	178	160	1.00	0.17	0.04	0.61
V51111121	0.30	0.78	0.89	0.89	0.81	0.06	0.0016	1552	1552	18	189	147	0.06	0.0016	1548	47	18	189	143	1.00	0.18	0.04	0.65
V51111122 V11111211	0.46	0.77	0.89	0.89	0.84	0.06	0.0016	1536 1221	1536 1221	18	189 127	123 170	0.06	0.0016	1533 1220	47	18	189	120	1.00	0.19	0.05	0.66
V11111212	0.68	0.59	0.88	0.89	0.77	0.10	0.0027	1192	1192	12	128	143	0.10	0.0027	1191	37	12	128	139	1.00	0.18	0.04	0.49
V21111212	0.63	0.63	0.89	0.89	0.79	0.09	0.0024	1278	1278	14	142	135	0.09	0.0023	1276	40	13	142	131	1.00	0.18	0.04	0.53
V31111211 V31111212	0.56	0.69	0.89	0.89	0.79	0.08	0.0021	1392 1366	1392 1366	15	157	151 126	0.08	0.0021	1391 1365	43	15	157 157	148	1.00	0.17	0.04	0.56
V41111211	0.52	0.73	0.89	0.89	0.81	0.07	0.0019	1460 1439	1460 1439	16 17	170 170	139	0.07	0.0019	1458 1437	45 44	16 16	170	136	1.00	0.18	0.04	0.61
V51111212	0.48	0.76	0.89	0.89	0.84	0.06	0.0017	1505	1505	18	180	124	0.06	0.0017	1502	46	10	180	121	1.00	0.19	0.05	0.65
V51111212 V11111221	0.49	0.75	0.89	0.89	0.86	0.07	0.0018	1492	1492 1431	18	181	104 205	0.07	0.0018	1489	46	17	181 150	201	1.00	0.20	0.05	0.66
V11111222	0.56	0.69	0.89	0.89	0.76	0.08	0.0021	1398	1398	14	152	173	0.08	0.0021	1397 1504	43 46	14	151 164	170	1.00	0.15	0.04	0.49
V21111221	0.52	0.73	0.89	0.89	0.78	0.07	0.0019	1483	1483	16	167	162	0.07	0.0019	1482	46	16	167	159	1.00	0.14	0.04	0.53
V31111221 V31111222	0.46	0.77	0.89	0.89	0.78	0.06	0.0016	1556 1540	1556 1540	17	176 178	177 150	0.06	0.0016	1555	48	16 17	176 178	174	1.00	0.15	0.04	0.56
V41111221	0.43	0.80	0.89	0.89	0.80	0.05	0.0014	1597 1584	1597 1584	18	187	162 137	0.05	0.0014	1595	49 49	18 18	187	159	1.00	0.16	0.04	0.61
V51111221	0.40	0.82	0.89	0.89	0.82	0.05	0.0013	1629	1629	19	196	146	0.05	0.0013	1627	50	19	196	143	1.00	0.17	0.04	0.66
V51111222 V11112111	0.41	0.81	0.89	0.89	0.85	0.05	0.0014	1619 1566	1519	19	197	123	0.05	0.0013	1616	48	19	197	277	1.00	0.18	0.04	0.66
V11112112 V21112111	0.46	0.77	0.89	0.89	0.68	0.06	0.0016	1550 1611	1550 1611	9	103	255 263	0.06	0.0016	1548 1610	48 49	9	103	251 260	1.00	0.12	0.03	0.49
V21112112	0.43	0.80	0.89	0.89	0.70	0.05	0.0015	1598	1598	10	111	239	0.05	0.0015	1596	49	10	111	235	1.00	0.12	0.03	0.52
V31112111 V31112112	0.40	0.82	0.89	0.89	0.70	0.05	0.0013	1647	1647	10	116	247 224	0.05	0.0013	1645	50	10	116	244 221	1.00	0.12	0.03	0.56
V41112111 V41112112	0.38	0.84	0.89	0.89	0.73	0.04	0.0011	1667 1667	1667 1667	11	121 123	231 210	0.04	0.0011	1666 1665	51 51	11	121 123	228 206	1.00	0.13	0.03	0.61
V51112111	0.36	0.84	0.89	0.89	0.75	0.04	0.0011	1677	1677	12	126	214	0.04	0.0011	1675	51	12	126	211	1.00	0.14	0.03	0.66
V11112121	0.37	0.84	0.89	0.89	0.63	0.04	0.0011	1666	1666	9	12/	313	0.04	0.0011	1664	51	9	12/	310	1.00	0.14	0.03	0.48
V11112122 V21112121	0.38	0.83	0.89	0.89	0.66	0.04	0.0012	1664 1683	1664 1683	9	109	288 293	0.04	0.0012	1663 1682	51 52	9	109	284 290	1.00	0.11	0.02	0.49
V21112122	0.36	0.84	0.89	0.89	0.68	0.04	0.0011	1683	1683	10	115	268	0.04	0.0011	1681	52	10	115	265	1.00	0.11	0.02	0.52
V31112121 V31112122	0.33	0.85	0.89	0.89	0.08	0.03	0.0010	1696	1696	10	120	270	0.03	0.0003	1695	52	10	120	248	1.00	0.11	0.02	0.56
V41112121 V41112122	0.32	0.85	0.89	0.89	0.70	0.03	0.0009	1707	1707	11	121	260 237	0.03	0.0008	1705	52 52	11	121	257	1.00	0.12	0.03	0.61
V51112121	0.31	0.86	0.89	0.89	0.72	0.03	0.0008	1714	1714	11	125	246	0.03	0.0008	1713	52	11	125	243	1.00	0.13	0.03	0.66
V11112211	0.41	0.81	0.89	0.89	0.66	0.05	0.0014	1621	1621	9	104	279	0.05	0.0014	1619	50	9	104	276	1.00	0.11	0.02	0.49
V11112212 V21112211	0.43	0.80	0.89	0.89	0.69	0.05	0.0014	1608 1661	1608	9 10	106	254 263	0.05	0.0014	1606	49 51	10	106	250	1.00	0.11	0.02	0.49
V21112212 V31112211	0.40	0.82	0.89	0.89	0.71	0.05	0.0013	1651 1679	1651	10	113	238 245	0.05	0.0013	1650 1677	51 52	10	113	235	1.00	0.12	0.03	0.52
V31112212	0.38	0.83	0.89	0.89	0.74	0.04	0.0012	1679	1679	11	119	223	0.04	0.0012	1677	52	11	119	220	1.00	0.12	0.03	0.56
V41112211 V41112212	0.35	0.84	0.89	0.89	0.76	0.04	0.0010	1690	1690	11	121	230	0.04	0.0010	1688	52	11	121	227	1.00	0.13	0.03	0.61
V51112211 V51112212	0.34	0.85	0.89	0.89	0.75	0.03	0.0009	1698 1699	1698 1699	12 12	124 126	215 195	0.03	0.0009	1697 1698	52 52	11	124 126	212 192	1.00	0.13	0.03	0.66
V11112221	0.34	0.85	0.89	0.89	0.64	0.04	0.0010	1688	1688	9	107	308	0.04	0.0010	1686	52	9	107	305	1.00	0.10	0.02	0.49
V21112221	0.33	0.85	0.89	0.89	0.66	0.04	0.0009	1704	1704	10	110	203	0.03	0.0001	1702	52	10	110	279	1.00	0.10	0.02	0.52
V21112222 V31112221	0.34	0.85	0.89	0.89	0.69	0.03	0.0009	1704 1715	1704 1715	10	115	265	0.03	0.0009	1703 1714	52	10	115 116	262	1.00	0.11	0.02	0.52
V31112222	0.32	0.85	0.89	0.89	0.71	0.03	0.0009	1716	1716	11	119	250 264	0.03	0.0009	1715	53	11	119	247	1.00	0.11	0.03	0.56
V41112222	0.31	0.86	0.89	0.89	0.73	0.03	0.0008	1726	1726	11	122	238	0.03	0.0008	1725	53	11	122	235	1.00	0.12	0.03	0.61
v51112221 V51112222	0.29	0.86	0.89	0.89	0.72	0.03	0.0007	1/32 1733	1/32 1733	11	123	249 226	0.03	0.0007	1/30 1732	53	11	123 125	246	1.00	0.12	0.03	0.66 0.66
V11121111	0.85	0.49	0.88	0.89	0.70	0.12	0.0035	1310	1310	12	135	200	0.12	0.0034	1307	40	12	135	193	1.00	0.20	0.04	0.49
V21121111	0.78	0.52	0.88	0.89	0.74	0.11	0.0031	1406	1406	14	152	189	0.11	0.0031	1403	43	14	152	183	1.00	0.20	0.05	0.52
V31121112	0.81	0.51	0.88	0.89	0.78	0.12	0.0032	1369	1369	14	152	159	0.12	0.0032	136/	42	14	152	153	1.00	0.22	0.05	0.53
V31121112 V41121111	0.75	0.54	0.88	0.89	0.80	0.11	0.0029	1462 1590	1462 1590	16 18	168 186	148 16.5	0.11	0.0029	1459 1586	45 49	16 18	168 186	143 159	1.00	0.22	0.06	0.56
V41121112	0.69	0.58	0.89	0.89	0.83	0.10	0.0027	1557	1557	18	185	137	0.10	0.0027	1554	48	18	185	132	1.00	0.23	0.06	0.61
v51121111 V51121112	0.62	0.63	0.89	0.89	0.83	0.09	0.0025	1672 1636	1672	20	203	149	0.09	0.0024	1667	51 50	19	203	143 118	1.00	0.24	0.06	0.65
V11121121 V11121122	0.70	0.58	0.88	0.89	0.69	0.10	0.0028	1549 1496	1549 1496	14	161 161	241 202	0.10	0.0028	1546 1494	48 46	14 15	161 161	235 196	1.00	0.16	0.04	0.49
V21121121	0.64	0.62	0.89	0.89	0.73	0.09	0.0025	1648	1648	16	180	228	0.09	0.0025	1646	51	16	180	222	1.00	0.17	0.04	0.52
V31121122	0.67	0.60	0.88	0.89	0.77	0.09	0.0028	1755	1755	1/	180	216	0.08	0.0026	1752	50	1/	180	210	1.00	0.18	0.04	0.52
V31121122 V41121121	0.62	0.64	0.89	0.89	0.79	0.09	0.0024	1706 1849	1706 1849	19 21	198 218	180 201	0.09	0.0024	1704 1845	53 56	19 20	198 218	175 196	1.00	0.19	0.05	0.56
V41121122	0.58	0.68	0.89	0.89	0.82	0.08	0.0022	1809	1809	21	217	167	0.08	0.0022	1806	55	21	217 225	162	1.00	0.20	0.05	0.61
V51121121	0.52	0.73	0.89	0.89	0.84	0.07	0.0020	1926	1926	23	233	163	0.07	0.0019	1922	58	22	233	1/8	1.00	0.20	0.05	0.66
V11121211	0.76	0.54	0.88	0.89	0.73	0.11	0.0030	1441	1441	14	150	202	0.11	0.0030	1439	44	14	150	196	1.00	0.18	0.04	0.49

Name	T1 [s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [•]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [+]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN-m]8	C1 [·]	œ[·]	C3 [·]	Mdy/ Mst [·]
V21121211	0.70	0.58	0.88	0.89	0.76	0.10	0.0027	1554	1554	16	169	192	0.10	0.0027	1552	48	16	169	187	1.00	0.19	0.05	0.52
V31121211	0.65	0.61	0.89	0.89	0.79	0.09	0.0025	1648	1648	18	185	180	0.09	0.0025	1645	51	17	185	175	1.00	0.19	0.05	0.55
V31121212 V41121211	0.67	0.60	0.88	0.89	0.82	0.09	0.0025	1618 1749	1618 1749	18 20	186 203	151	0.09	0.0025	1616 1745	50	18 19	186 203	146	1.00	0.20	0.05	0.57
V41121212	0.62	0.64	0.89	0.89	0.84	0.09	0.0023	1714	1714	20	202	140	0.09	0.0023	1711	53	19	202	135	1.00	0.21	0.05	0.61
V51121211 V51121212	0.56	0.69	0.89	0.89	0.84	0.08	0.0021	1837	1857	21	220	152	0.08	0.0021	1833	55	21	220	148	1.00	0.21	0.06	0.66
V11121221	0.62	0.63	0.89	0.89	0.72	0.09	0.0024	1701	1701	16	178	245	0.09	0.0024	1699	52	16	178	239	1.00	0.15	0.04	0.49
V21121221	0.57	0.68	0.89	0.89	0.75	0.08	0.0022	1821	1821	18	199	233	0.08	0.0022	1819	56	18	199	228	1.00	0.16	0.04	0.52
V21121222 V31121221	0.60	0.66	0.89	0.89	0.78	0.08	0.0023	1778 1920	1778	19 20	200	195 219	0.08	0.0023	1776	55	19 20	200	191 214	1.00	0.17	0.04	0.53
V31121222	0.55	0.70	0.89	0.89	0.81	0.08	0.0020	1885	1885	21	218	184	0.08	0.0020	1883	58	21	218	179	1.00	0.17	0.04	0.57
V41121221 V41121222	0.50	0.75	0.89	0.89	0.80	0.07	0.0019	1995	1995	22	233	202	0.07	0.0018	1992	61 61	22	233	198	1.00	0.17	0.04	0.61
V51121221	0.47	0.77	0.89	0.89	0.82	0.06	0.0016	2047	2047	24	246	184	0.06	0.0016	2043	62	23	246	179	1.00	0.18	0.04	0.66
V11122111	0.48	0.73	0.89	0.89	0.65	0.07	0.0019	1948	1948	11	126	349	0.07	0.0019	1945	60	11	126	344	1.00	0.13	0.03	0.00
V11122112	0.53	0.72	0.89	0.89	0.68	0.07	0.0020	1912 2018	2018	11	127	315	0.07	0.0020	1910 2015	59	11	127	310	1.00	0.13	0.03	0.49
V21122112	0.50	0.75	0.89	0.89	0.70	0.07	0.0018	1995	1995	12	139	299	0.07	0.0018	1993	61	12	139	294	1.00	0.13	0.03	0.52
V31122111 V31122112	0.46	0.78	0.89	0.89	0.70	0.06	0.0016	2073 2054	2073	13	146 148	312	0.06	0.0016	2070	63 63	13 13	146 148	307	1.00	0.13	0.03	0.56
V41122111	0.43	0.80	0.89	0.89	0.73	0.05	0.0015	2118	2118	14	154	294	0.05	0.0015	2115	65	14	154	289	1.00	0.14	0.03	0.61
V51122112	0.45	0.79	0.89	0.89	0.75	0.05	0.0015	2102	2102	14	156	265	0.05	0.0015	2099 21.52	65	14	156	260	1.00	0.14	0.03	0.66
V51122112	0.42	0.80	0.89	0.89	0.77	0.05	0.0014	2141	2141	15	163	248	0.05	0.0014	2138	66	15	163	244	1.00	0.15	0.04	0.66
V11122121 V11122122	0.45	0.80	0.89	0.89	0.66	0.06	0.0014	2124	2124	12	138	364	0.05	0.0014	2099	64	12	130	359	1.00	0.11	0.02	0.40
V21122121	0.40	0.82	0.89	0.89	0.65	0.05	0.0013	2175	2175	13	145	379 344	0.05	0.0013	2173	67 66	13	145 148	374	1.00	0.11	0.02	0.52
V31122121	0.38	0.83	0.89	0.89	0.68	0.04	0.0012	2215	2215	14	153	360	0.04	0.0012	2213	68	13	153	355	1.00	0.12	0.02	0.56
V31122122 V41122121	0.40	0.82	0.89	0.89	0.71	0.05	0.0013	2201	2201	14	155	327	0.05	0.0013	21.99	68 68	14	155	322	1.00	0.12	0.03	0.56
V41122122	0.38	0.83	0.89	0.89	0.73	0.04	0.0012	2229	2229	15	162	310	0.04	0.0012	2227	68	15	162	305	1.00	0.13	0.03	0.61
V51122121 V51122122	0.35	0.84	0.89	0.89	0.72	0.04	0.0010	2242 2242	2242	15	163	322 292	0.04	0.0010	2240	69 69	15	163 166	317 288	1.00	0.13	0.03	0.66
V11122211	0.48	0.76	0.89	0.89	0.66	0.06	0.0017	2032	2032	11	131	350	0.06	0.0017	2029	62	11	131	345	1.00	0.12	0.02	0.49
V21122212	0.45	0.73	0.89	0.89	0.69	0.06	0.0015	2010	2010	12	140	331	0.06	0.0018	2008	64	12	135	313	1.00	0.12	0.03	0.49
V21122212 V31122211	0.46	0.77	0.89	0.89	0.71	0.06	0.0016	2075	2075	13	143	300	0.06	0.0016	2073	64 66	13	143	295	1.00	0.13	0.03	0.52
V31122212	0.44	0.79	0.89	0.89	0.74	0.05	0.0015	2127	2127	14	151	284	0.05	0.0015	2125	66	14	151	279	1.00	0.13	0.03	0.56
V41122211 V41122212	0.41	0.82	0.89	0.89	0.73	0.05	0.0013	2182 2169	2182 2169	14	156 158	297 268	0.05	0.0013	21.80 21.68	67	14	156 158	293 264	1.00	0.13	0.03	0.61
V51122211	0.39	0.83	0.89	0.89	0.75	0.04	0.0012	2215	2215	15	162	281	0.04	0.0012	2213	68	15	162	277	1.00	0.14	0.03	0.66
V51122212 V11122221	0.40	0.82	0.89	0.89	0.77	0.05	0.0013	2204	2204	15	164	253 400	0.05	0.0013	2202 2187	67	15	164	395	1.00	0.14	0.03	0.49
V11122222	0.41	0.81	0.89	0.89	0.67	0.05	0.0014	2171	2171	12	141	364	0.05	0.0014	2169	67	12	141	359	1.00	0.11	0.02	0.49
V21122221	0.38	0.83	0.89	0.89	0.69	0.04	0.0012	2220	2220	13	140	346	0.04	0.0012	2223	68	13	140	341	1.00	0.11	0.02	0.52
V31122221	0.36	0.84	0.89	0.89	0.68	0.04	0.0011	2243	2243	13	152	359	0.04	0.0011	2241	69 69	13	152	354	1.00	0.11	0.02	0.56
V41122221	0.35	0.85	0.89	0.89	0.70	0.04	0.0010	2257	2257	14	157	341	0.04	0.0010	2255	69	14	157	337	1.00	0.12	0.03	0.61
V41122222 V51122221	0.36	0.84	0.89	0.89	0.73	0.04	0.0010	2257	2257	14	160	311	0.04	0.0010	2256	70	14	160	307	1.00	0.12	0.03	0.61
V51122222	0.34	0.85	0.89	0.89	0.74	0.04	0.0010	2269	2269	15	164	296	0.04	0.0010	2267	70	15	164	292	1.00	0.13	0.03	0.66
V11211111 V11211112	1.13	0.39	0.86	0.89	0.71	0.18	0.0037	1052	1052	10	109	162	0.18	0.0037	1049 1026	24	10	109	154	1.00	0.24	0.06	0.49
V21211111	1.03	0.41	0.87	0.89	0.74	0.16	0.0032	1110	1110	11	121	151	0.16	0.0032	1107	26	11	121	143	1.00	0.26	0.06	0.52
V31211112	0.95	0.40	0.88	0.89	0.78	0.14	0.0029	1090	1183	12	134	127	0.17	0.0034	1087	23	11	134	120	1.00	0.27	0.07	0.56
V31211112	0.99	0.43	0.87	0.89	0.81	0.15	0.0030	1152	1152	13	133	118	0.15	0.0030	1149	27	13	133	111	1.00	0.29	0.07	0.57
V41211112	0.91	0.46	0.88	0.89	0.83	0.14	0.0028	1227	1227	15	146	108	0.14	0.0028	1222	28	14	146	102	1.00	0.30	0.08	0.61
V51211111 V51211112	0.82	0.51	0.88	0.89	0.83	0.12	0.0026	1332 1304	1332 1304	16 16	162 160	117 97	0.12	0.0025	1325 1297	30 30	15 15	162 160	110 91	1.00	0.30	0.08	0.65
V11211121	0.92	0.45	0.88	0.89	0.70	0.14	0.0028	1218	1218	11	127	191	0.14	0.0028	1214	28	11	127	183	1.00	0.21	0.05	0.49
V21211122	0.97	0.44	0.88	0.89	0.74	0.14	0.0030	1308	1308	12	12/	180	0.13	0.0030	1175	30	12	127	153	1.00	0.23	0.05	0.49
V21211122	0.89	0.47	0.88	0.89	0.77	0.13	0.0027	1264	1264	13	142	151	0.13	0.0027	1262	29	13	142	144	1.00	0.23	0.06	0.53
V31211121	0.82	0.51	0.88	0.89	0.80	0.12	0.0024	1358	1358	15	159	142	0.12	0.0023	1355	31	15	159	136	1.00	0.24	0.06	0.56
V41211121	0.73	0.56	0.88	0.89	0.79	0.11	0.0022	1478	1478	17	175	158	0.11	0.0022	1474	34	16	175	152	1.00	0.23	0.06	0.61
V51211121	0.68	0.59	0.89	0.89	0.82	0.10	0.0021	1559	1559	19	191	144	0.10	0.0020	1553	35	18	191	138	1.00	0.25	0.06	0.65
V51211122 V11211211	0.70	0.58	0.89	0.89	0.85	0.10	0.0021	1526 1134	1526 1134	19	189	119 160	0.10	0.0021	1520	35	18	189	114	1.00	0.26	0.07	0.65
V11211212	1.05	0.41	0.87	0.89	0.77	0.16	0.0033	1113	1113	11	120	136	0.16	0.0033	1111	26	11	120	129	1.00	0.25	0.06	0.50
V21211211 V21211212	0.92	0.45	0.88	0.89	0.80	0.14	0.0028	1223	1223	13	133	152	0.14	0.0028	1221 1193	28	12	133	145	1.00	0.24	0.06	0.53
V31211211	0.85	0.49	0.88	0.89	0.79	0.13	0.0026	1309	1309	14	148	143	0.13	0.0025	1306	30	14	148	136	1.00	0.25	0.06	0.56
V41211212	0.88	0.52	0.88	0.89	0.82	0.13	0.0024	1390	1390	14	162	132	0.13	0.0023	1386	32	14	162	115	1.00	0.26	0.06	0.61
V41211212	0.81	0.51	0.88	0.89	0.84	0.12	0.0024	1366	1366	16	162	110	0.12	0.0024	1362	31	15	162	104	1.00	0.27	0.07	0.61
V51211212	0.75	0.54	0.88	0.89	0.87	0.11	0.0023	1444	1444	18	176	99	0.11	0.0022	1438	33	17	176	94	1.00	0.28	0.07	0.66
V11211221 V11211222	0.82	0.50	0.88	0.89	0.72	0.12	0.0025	1352	1352	13	142	194 163	0.12	0.0025	1349	31	13	142	187	1.00	0.19	0.04	0.49
V21211221	0.76	0.54	0.88	0.89	0.75	0.11	0.0022	1446	1446	15	159	184	0.11	0.0022	1444	33	15	159	178	1.00	0.20	0.05	0.52
V21211222 V31211221	0.78	0.52	0.88	0.89	0.79	0.11	0.0023	1413	1413	15	160 176	154	0.11	0.0023	1411 1543	33	15	159	149	1.00	0.21	0.05	0.53
V31211222	0.72	0.56	0.88	0.89	0.81	0.10	0.0021	1511	1511	17	176	145	0.10	0.0021	1509	35	17	176	140	1.00	0.22	0.05	0.57
V41211221 V41211222	0.65	0.61	0.89	0.89	0.81	0.10	0.0019	1629	1629	18	192	101	0.10	0.0019	1598	37	18	192	130	1.00	0.21	0.06	0.61
V51211221	0.61	0.65	0.89	0.89	0.83	0.09	0.0018	1717	1717	20	208	148	0.09	0.0018	1712	39	20	208	143	1.00	0.22	0.06	0.65
V11212111	0.68	0.59	0.89	0.89	0.65	0.10	0.0020	1578	1578	9	103	279	0.10	0.0020	1575	36	9	103	272	1.00	0.15	0.03	0.49
V11212112 V21212111	0.70	0.58	0.88	0.89	0.68	0.10	0.0021	1543 1666	1543 1666	9	104	250	0.10	0.0021	1540 1663	36 38	9	104	244	1.00	0.16	0.03	0.49
V21212112	0.65	0.61	0.89	0.89	0.71	0.09	0.0019	1634	1634	10	115	239	0.09	0.0019	1631	38	10	115	233	1.00	0.16	0.04	0.52

Name	T1 [s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [•]	Disp [m]	ISDR [-]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [•]3	Total Moment [MN-m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [·]	€2[•]	СЗ [•]	Mdy/ Mst [•]
V31212112	0.61	0.64	0.89	0.89	0.74	0.09	0.0018	1724	1724	12	126	228	0.09	0.0017	1722	40	11	126	223	1.00	0.17	0.04	0.56
V41212111	0.56	0.69	0.89	0.89	0.74	0.08	0.0016	1838	1838	13	136	244	0.08	0.0016	1835	42	13	136	238	1.00	0.17	0.04	0.61
V51212111	0.53	0.72	0.89	0.89	0.76	0.07	0.0015	1905	1905	14	146	230	0.07	0.0015	1902	44	14	146	224	1.00	0.17	0.04	0.66
V51212112 V11212121	0.54	0.71	0.89	0.89	0.79	0.08	0.0015	1879	1879	14	146	205	0.07	0.0015	1875	43	14	146	333	1.00	0.18	0.04	0.66
V11212122	0.58	0.67	0.89	0.89	0.67	0.08	0.0017	1802	1802	10	120	304	0.08	0.0017	1799	41	10	120	298	1.00	0.13	0.03	0.49
V2121212121 V212121222	0.52	0.73	0.89	0.89	0.07	0.07	0.0015	1930	1894	12	131	292	0.07	0.0015	1892	44	12	132	286	1.00	0.13	0.03	0.52
V31212121 V31212122	0.50	0.75	0.89	0.89	0.69	0.07	0.0014	1993 1969	1993 1969	13	141	309 279	0.07	0.0013	1990 1967	46	13	141	304 274	1.00	0.13	0.03	0.56
V41212121	0.47	0.77	0.89	0.89	0.72	0.06	0.0013	2040	2040	13	149	294	0.06	0.0013	2037	47	13	149	289	1.00	0.14	0.03	0.61
V51212122	0.49	0.76	0.89	0.89	0.74	0.06	0.0013	2020	2020	14	150	264	0.06	0.0013	2017	46	14	150	259	1.00	0.15	0.03	0.61
V51212122	0.46	0.77	0.89	0.89	0.76	0.06	0.0012	2061	2061	15	157	250	0.06	0.0012	2059	47	15	157	245	1.00	0.15	0.04	0.66
V11212212	0.64	0.62	0.89	0.89	0.70	0.09	0.0019	1650	1650	10	111	255	0.09	0.0019	1648	38	10	110	249	1.00	0.15	0.03	0.49
V21212211 V21212212	0.58	0.67	0.89	0.89	0.70	0.08	0.0017	1789 1755	1789 1755	11	122	274	0.08	0.0017	1787	41	11	122	269 240	1.00	0.15	0.03	0.52
V31212211	0.55	0.70	0.89	0.89	0.72	0.08	0.0015	1873	1873	12	132	263	0.08	0.0015	1871	43	12	132	258	1.00	0.15	0.03	0.56
V41212212	0.50	0.89	0.89	0.89	0.75	0.08	0.0018	1943	1943	12	155	253	0.07	0.0018	1940	45	12	135	230	1.00	0.16	0.04	0.61
V41212212	0.53	0.72	0.89	0.89	0.77	0.07	0.0015	1918	1918	13	142	224	0.07	0.0015	1916	44	13	142	219	1.00	0.16	0.04	0.61
V51212212	0.51	0.74	0.89	0.89	0.79	0.07	0.0014	1978	1978	14	150	212	0.07	0.0014	1976	46	14	150	207	1.00	0.17	0.04	0.66
V11212221 V11212222	0.52	0.73	0.89	0.89	0.65	0.07	0.0014	1951 1917	1951 1917	11	126 127	345 310	0.07	0.0014	1949 1915	45	11	126	339	1.00	0.12	0.02	0.49
V21212221	0.49	0.76	0.89	0.89	0.68	0.06	0.0013	2016	2016	12	135	328	0.06	0.0013	2014	46	12	135	323	1.00	0.12	0.03	0.52
V31212222	0.30	0.74	0.89	0.89	0.71	0.07	0.0014	2067	2067	12	137	313	0.06	0.0014	2065	40	12	137	308	1.00	0.13	0.03	0.52
V31212222 V41212221	0.48	0.76	0.89	0.89	0.73	0.06	0.0013	2048	2048	13	145	283	0.06	0.0013	2047	47	13	145	278	1.00	0.13	0.03	0.56
V41212222	0.45	0.78	0.89	0.89	0.75	0.06	0.0012	2092	2092	14	152	269	0.06	0.0012	2091	48	14	152	265	1.00	0.14	0.03	0.61
V51212221 V51212222	0.42	0.80	0.89	0.89	0.74	0.05	0.0011	2143	2143 2129	14	156	285	0.05	0.0011	2141 2127	49	14	156	280	1.00	0.14	0.03	0.66
V11221111	1.30	0.36	0.85	0.89	0.70	0.21	0.0044	1274	1274	12	132	198	0.21	0.0044	1270	29	12	132	186	1.00	0.26	0.06	0.49
V21221112	1.30	0.34	0.84	0.89	0.73	0.22	0.0039	1352	1352	14	133	185	0.19	0.0047	1347	31	12	133	130	1.00	0.28	0.07	0.52
V21221112 V31221111	1.24	0.37	0.85	0.89	0.78	0.20	0.0041	1322 1421	1322	14	147	156	0.20	0.0041	1318	31	14	147	146	1.00	0.29	0.07	0.53
V31221112	1.14	0.39	0.86	0.89	0.80	0.18	0.0037	1394	1394	16	161	144	0.18	0.0036	1390	32	15	161	134	1.00	0.31	0.08	0.57
V41221111 V41221112	1.02	0.42	0.88	0.89	0.80	0.16	0.0032	1482 1460	1482 1460	1/	1/4	155	0.16	0.0032	14/4	34	16	174	145	1.00	0.32	0.08	0.61
V51221111	0.94	0.45	0.88	0.89	0.83	0.15	0.0030	1571	1571	19	191	139	0.14	0.0030	1560	36	18	191	130	1.00	0.34	0.09	0.65
V11221112	1.07	0.41	0.87	0.89	0.80	0.15	0.0031	1358	1350	14	169	229	0.16	0.0031	1327	33	13	152	218	1.00	0.55	0.05	0.49
V11221122 V21221121	1.12	0.39	0.86	0.89	0.74	0.17	0.0036	1418	1418	14	153	215	0.17	0.0036	1415	33	14	153	184	1.00	0.25	0.06	0.49
V21221122	1.02	0.42	0.87	0.89	0.77	0.15	0.0032	1496	1496	16	169	180	0.15	0.0032	1493	35	16	168	171	1.00	0.26	0.06	0.53
V31221121 V31221122	0.91	0.46	0.88	0.89	0.80	0.14	0.0028	1641	1641	18	18/	202	0.14	0.0028	1535	38	1/	187	193	1.00	0.25	0.07	0.55
V41221121 V41221122	0.84	0.49	0.88	0.89	0.79	0.13	0.0026	1744	1744	20	207	188	0.13	0.0026	1738	40	19 19	207	179	1.00	0.26	0.06	0.61
V51221122	0.78	0.52	0.88	0.89	0.82	0.12	0.0024	1840	1840	20	226	171	0.12	0.0024	1831	42	21	204	163	1.00	0.28	0.07	0.65
V51221122 V11221211	0.81	0.51	0.88	0.89	0.85	0.12	0.0025	1801	1801	22	223	142 197	0.12	0.0025	1793	41 32	21	223	134	1.00	0.29	0.07	0.65
V11221212	1.21	0.37	0.85	0.89	0.77	0.19	0.0040	1353	1353	14	146	166	0.19	0.0040	1351	31	13	146	156	1.00	0.27	0.07	0.50
V21221211	1.10	0.41	0.87	0.89	0.70	0.10	0.0034	1435	1435	15	160	154	0.17	0.0034	1433	33	15	155	145	1.00	0.28	0.00	0.53
V31221211 V31221212	0.98	0.43	0.88	0.89	0.79	0.15	0.0030	1542	1542	17	174	170	0.15	0.0030	1538	36	16	174	160	1.00	0.28	0.07	0.57
V41221211	0.91	0.46	0.88	0.89	0.82	0.14	0.0028	1638	1638	19	191	157	0.14	0.0028	1632	38	18	191	148	1.00	0.29	0.07	0.61
V41221212 V51221211	0.93	0.45	0.88	0.89	0.84	0.14	0.0029	1609	1609	19	209	131	0.14	0.0028	1604	37	18	209	123	1.00	0.30	0.08	0.61
V51221212	0.86	0.48	0.88	0.89	0.87	0.13	0.0027	1704	1704	21	207	119	0.13	0.0027	1696	39	20	207	111	1.00	0.32	0.08	0.66
V11221222	0.99	0.43	0.87	0.89	0.76	0.15	0.0031	1545	1545	16	168	194	0.15	0.0031	1542	36	15	168	185	1.00	0.24	0.06	0.50
V21221221 V21221222	0.87	0.48	0.88	0.89	0.75	0.13	0.0026	1707	1707 1663	17	187 188	219 183	0.13	0.0026	1703	39	17	187 188	209 175	1.00	0.22	0.05	0.52
V31221221	0.81	0.51	0.88	0.89	0.78	0.12	0.0024	1824	1824	20	208	207	0.12	0.0024	1820	42	19	208	198	1.00	0.23	0.06	0.56
V41221222	0.84	0.54	0.88	0.89	0.81	0.12	0.0023	1928	1/05	20	208	1/3	0.11	0.0023	1923	41	20	208	183	1.00	0.24	0.06	0.61
V41221222 V51221221	0.77	0.53	0.88	0.89	0.84	0.11	0.0023	1888 2038	1888 2038	22	226	161	0.11	0.0023	1884 2031	44	22	226	153	1.00	0.25	0.06	0.61
V51221222	0.72	0.56	0.88	0.89	0.86	0.11	0.0022	2000	2000	24	245	148	0.11	0.0021	1993	46	23	245	141	1.00	0.26	0.07	0.66
V11222111 V11222112	0.78	0.52	0.88	0.89	0.65	0.11	0.0024	1862	1862 1820	11	122	331 297	0.11	0.0024	1857	43	11	122	321	1.00	0.17	0.04	0.49
V21222111	0.73	0.56	0.88	0.89	0.69	0.11	0.0022	1980	1980	12	136	318	0.11	0.0022	1976	45	12	136	309	1.00	0.17	0.04	0.52
V31222112	0.75	0.54	0.89	0.89	0.71	. 0.10	0.0022	2086	2086	12	130	305	0.10	0.0022	2082	44	12	130	276	1.00	0.18	0.04	0.52
V31222112 V41222111	0.70	0.57	0.88	0.89	0.74	0.10	0.0021	2046	2046	14	149	273	0.10	0.0021	2043	47	14	149	265	1.00	0.19	0.04	0.56
V41222112	0.66	0.60	0.89	0.89	0.76	0.09	0.0019	2142	2142	15	161	259	0.09	0.0019	2138	49	15	161	251	1.00	0.20	0.05	0.61
V51222111 V51222112	0.61	0.64	0.89	0.89	0.76	0.09	0.0018	2272	2272	16 16	174	275	0.09	0.0018	2266	52	16 16	174	268	1.00	0.19	0.05	0.66
V11222121	0.64	0.62	0.89	0.89	0.64	0.09	0.0019	2182	2182	12	142	402	0.09	0.0019	2177	50	12	142	393	1.00	0.14	0.03	0.49
V21222122	0.60	0.65	0.89	0.89	0.67	0.09	0.0018	2134	2134	12	143	389	0.09	0.0020	2303	53	12	142	381	1.00	0.15	0.03	0.49
V21222122 V31222121	0.63	0.63	0.89	0.89	0.70	0.09	0.0018	2249	2249	14	157	348	0.09	0.0018	2246	52	14	157	340	1.00	0.15	0.03	0.52
V31222122	0.59	0.66	0.89	0.89	0.72	0.08	0.0017	2364	2364	15	170	336	0.08	0.0017	2361	54	15	170	329	1.00	0.16	0.04	0.56
V41222121 V41222122	0.55	0.71	0.89	0.89	0.72	0.07	0.0015	2503 2460	2503 2460	17	182 183	361	0.07	0.0015	2500 2457	57	16 17	182 182	354	1.00	0.15	0.03	0.61
V51222121	0.52	0.73	0.89	0.89	0.74	0.07	0.0014	2579	2579	18	193	346	0.07	0.0014	2575	59	18	193	339	1.00	0.16	0.04	0.66
V112222122	0.54	0.71	0.89	0.89	0.76	0.10	0.0015	2003	2003	18	193	340	0.10	0.0015	2000	58 46	18	193	302	1.00	0.16	0.04	0.49
V11222212 V21222211	0.74	0.55	0.88	0.89	0.70	0.11	0.0022	1956 2120	1956 2120	12	131	304 327	0.11	0.0022	1953 2117	45 49	12	131 144	295 319	1.00	0.17	0.04	0.49
V21222212	0.69	0.58	0.88	0.89	0.72	0.10	0.0020	2081	2081	13	145	293	0.10	0.0020	2079	48	13	145	285	1.00	0.17	0.04	0.52
V31222211 V31222212	0.63	0.62	0.89	0.89	0.72	0.09	0.0019	2217	2217	14	157	313 280	0.09	0.0018	2214	51	14	157	305	1.00	0.17	0.04	0.56
V41222211	0.60	0.65	0.89	0.89	0.75	0.08	0.0017	2325	2325	16	169	301	0.08	0.0017	2322	53	16	169	294	1.00	0.17	0.04	0.61
v41222212 V51222211	0.57	0.68	0.89	0.89	0.77	0.09	0.0018	2285	2285	16	169	268	0.09	0.0018	2282	53	16	169	261	1.00	0.18	0.04	0.66

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [•]	Disp [m]	ISDR [•]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [•]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [-]	C2[·]	C3[·]	Mdy/ Mst [·]
V11222221	0.60	0.66	0.89	0.89	0.65	0.08	0.0017	2339	2339	13	3 151	414	0.08	0.0017	2335	54	13	151	407	1.00	0.13	0.03	0.49
V11222222	0.62	0.64	0.89	0.89	0.68	0.09	0.0018	2283	2283	13	151	371	0.09	0.0018	2280	53	13	151	363	1.00	0.14	0.03	0.49
V21222222	0.58	0.67	0.89	0.89	0.71	0.08	0.0017	2408	2408	15	5 166	359	0.08	0.0017	2405	56	15	166	352	1.00	0.14	0.03	0.52
V31222221	0.53	0.72	0.89	0.89	0.70	0.07	0.0015	2550	2550	16	5 177	387	0.07	0.0015	2547	59	16	177	380	1.00	0.14	0.03	0.56
V41222221	0.51	0.74	0.89	0.89	0.72	0.07	0.0013	2627	2627	17	7 187	373	0.07	0.0013	2624	60	10	187	366	1.00	0.13	0.03	0.61
V41222222	0.52	0.72	0.89	0.89	0.75	0.07	0.0015	2592	2592	17	7 188	334	0.07	0.0014	2590	60	17	188	328	1.00	0.15	0.03	0.61
V51222221	0.49	0.74	0.89	0.89	0.74	0.07	0.0013	2656	2656	18	3 190 3 197	321	0.07	0.0013	2653	61	18	190	315	1.00	0.13	0.03	0.66
V12111111	2.82	0.16	0.56	0.85	0.71	0.47	0.0065	1277	1277	6	5 132	209	0.47	0.0064	1265	20	6	132	183	1.00	0.38	0.13	0.45
V22111112	2.95	0.15	0.52	0.84	0.75	0.48	0.0067	1214	1432	6	3 150	204	0.44	0.0066	1205	22	7	130	150	1.00	0.39	0.15	0.46
V22111112	2.68	0.17	0.57	0.85	0.78	0.45	0.0062	1369	1369	8	3 153	168	0.45	0.0061	1360	21	7	153	146	1.00	0.42	0.15	0.48
V32111111 V321111112	2.30	0.20	0.62	0.87	0.78	0.43	0.0059	1597	1597	9	182	198	0.42	0.0056	1582	20	9	182	1/2	1.00	0.44	0.14	0.52
V42111111	2.17	0.23	0.72	0.88	0.81	0.40	0.0056	1778	1778	11	211	188	0.40	0.0054	1755	27	10	210	162	1.00	0.49	0.15	0.55
V42111112 V52111111	2.23	0.22	0.59	0.87	0.84	0.38	0.0056	1/1/ 1970	1/1/ 1970	14	2 <u>207</u> 1 243	154	0.38	0.0054	1697	26	10	206	130	0.99	0.51	0.16	0.56
V52111112	2.04	0.25	0.77	0.88	0.87	0.39	0.0055	1921	1921	14	1 239	138	0.39	0.0054	1882	29	11	237	115	0.99	0.57	0.17	0.60
V12111121 V12111122	2.30	0.21	0.66	0.87	0.70	0.41	0.0056	1655	1655	8	<u>3 174</u> 3 170	270	0.40	0.0056	1643	26	8	173	243	1.00	0.34	0.10	0.45
V22111121	2.11	0.24	0.71	0.88	0.74	0.38	0.0053	1860	1860	10	205	265	0.38	0.0052	1848	29	10	205	239	1.00	0.36	0.10	0.48
V22111122	2.19	0.23	0.67	0.87	0.78	0.39	0.0054	2039	2039	10	201	217	0.39	0.0053	2024	28	9	201	194	1.00	0.38	0.12	0.48
V32111121	2.01	0.25	0.72	0.88	0.81	0.37	0.0051	1983	1983	12	2 234	210	0.37	0.0050	1971	31	11	234	186	1.00	0.41	0.12	0.52
V42111121	1.78	0.28	0.79	0.88	0.80	0.33	0.0045	2174	2174	14	1 261 1 260	231	0.33	0.0044	2152	33	12	261	205	1.00	0.44	0.12	0.55
V52111122	1.64	0.30	0.84	0.89	0.84	0.30	0.0043	2306	2306	16	5 289	202	0.30	0.0040	2269	35	14	287	176	0.99	0.50	0.13	0.60
V52111122	1.68	0.29	0.83	0.88	0.86	0.31	0.0044	2264	2264	16	286	166	0.31	0.0043	2228	34	14	284	143	0.99	0.51	0.14	0.60
V12111211 V12111212	2.61	0.13	0.57	0.85	0.73	0.44	0.0061	1434	1484	7	7 154	180	0.43	0.0053	1474	23	7	155	155	1.00	0.30	0.12	0.46
V22111211	2.29	0.21	0.65	0.87	0.77	0.40	0.0056	1674	1674	9	9 184	215	0.40	0.0055	1664	26	9	183	191	1.00	0.38	0.12	0.48
V32111212	2.36	0.20	0.62	0.88	0.80	0.38	0.0058	1812	1878	11	214	208	0.38	0.0058	1865	25	10	214	136	1.00	0.39	0.14	0.48
V32111212	2.15	0.23	0.68	0.87	0.83	0.39	0.0053	1815	1815	11	211	171	0.39	0.0052	1804	28	10	211	150	1.00	0.43	0.14	0.52
V42111211 V42111212	1.92	0.26	0.76	0.88	0.83	0.37	0.0051	2050	2050	13	<u>3 242</u> 3 242	193	0.37	0.0048	2030	32	12	242	169	1.00	0.46	0.13	0.56
V52111211	1.76	0.28	0.82	0.88	0.86	0.33	0.0047	2185	2185	15	5 268	168	0.33	0.0045	21.48	33	13	266	144	0.99	0.52	0.15	0.60
V52111212 V12111221	2.04	0.28	0.81	0.88	0.88	0.34	0.0048	2155 1949	2155	16	5 <u>267</u>) 207	139	0.33	0.0046	21.20 1939	33	13	265	117 262	0.99	0.53	0.15	0.60
V12111222	2.12	0.24	0.68	0.87	0.77	0.38	0.0053	1859	1859	10) 204	236	0.38	0.0052	1852	29	10	204	214	1.00	0.34	0.11	0.45
V22111221	1.86	0.27	0.75	0.88	0.76	0.34	0.0046	2119	2119	11	236	273	0.34	0.0046	21.09	33	11	235	248	1.00	0.35	0.10	0.48
V32111221	1.71	0.29	0.79	0.88	0.79	0.31	0.0042	2266	2266	13	3 262	254	0.31	0.0041	2252	35	12	262	229	1.00	0.38	0.10	0.51
V32111222	1.76	0.28	0.77	0.88	0.82	0.32	0.0043	2221	2221	14	262	211	0.32	0.0043	2210	35	13	262	189	1.00	0.39	0.11	0.52
V42111222	1.61	0.30	0.81	0.88	0.85	0.29	0.0040	2368	2368	15	288	192	0.29	0.0039	2350	37	14	288	169	1.00	0.43	0.12	0.56
V52111221	1.44	0.33	0.85	0.89	0.85	0.26	0.0036	2545	2545	17	7 317	200	0.26	0.0035	2513	39	15	315	177	0.99	0.46	0.12	0.60
V12112111	1.65	0.30	0.81	0.88	0.67	0.29	0.0041	2313	2303	7	155	409	0.29	0.0030	2298	36	7	155	379	1.00	0.28	0.15	0.45
V12112112	1.71	0.29	0.79	0.88	0.70	0.30	0.0042	2264	2264	7	7 155	367	0.30	0.0042	2253	35	7	155	339	1.00	0.30	0.07	0.45
V22112111 V22112112	1.52	0.32	0.82	0.88	0.70	0.28	0.0038	2402	2402	6	3 174	346	0.27	0.0037	2446	38	8	174	319	1.00	0.30	0.08	0.48
V32112111	1.41	0.34	0.84	0.89	0.74	0.24	0.0033	2603	2603	0	193	361	0.24	0.0033	2587	40	9	193	334	1.00	0.32	0.08	0.51
V32112112 V42112111	1.45	0.33	0.84	0.89	0.76	0.22	0.0034	2563	2563	11	193 L 211	323	0.22	0.0034	2549	40	10	211	306	1.00	0.34	0.09	0.51
V42112112	1.34	0.35	0.85	0.89	0.79	0.23	0.0032	2699	2699	11	211	296	0.23	0.0031	2681	42	10	211	271	1.00	0.36	0.09	0.55
V52112111 V52112112	1.22	0.37	0.87	0.89	0.80	0.21	0.0029	2860	2860	12	2 230 2 229	297	0.21	0.0028	2833 2800	44	11	230	2/3	1.00	0.38	0.10	0.60
V12112121	1.34	0.35	0.85	0.89	0.66	0.23	0.0031	2702	2702	8	3 181	483	0.23	0.0031	2689	42	8	181	454	1.00	0.25	0.05	0.45
V12112122 V22112121	1.39	0.34	0.84	0.89	0.69	0.24	0.0033	2645 2854	2645	6	3 182 3 202	434	0.23	0.0033	2634 2841	41	8	182 202	406	1.00	0.26	0.06	0.45
V22112122	1.28	0.36	0.85	0.89	0.72	0.21	0.0029	2804	2804	9	203	408	0.21	0.0029	2793	44	9	203	382	1.00	0.28	0.07	0.48
V32112121 V32112122	1.16	0.39	0.86	0.89	0.73	0.19	0.0026	2992	2992	10	222	425	0.19	0.0026	2978	46	10	222	400	1.00	0.28	0.07	0.51
V42112121	1.08	0.40	0.87	0.89	0.76	0.17	0.0024	3118	3118	12	2 241	393	0.17	0.0023	3102	48	10	241	369	1.00	0.30	0.07	0.55
V42112122	1.11	0.40	0.87	0.89	0.78	0.18	0.0024	3079	3079	12	2 241	351	0.18	0.0024	3064	48	11	241	327	1.00	0.31	0.08	0.55
V52112121	1.01	0.41	0.88	0.89	0.81	0.16	0.0022	3195	3195	13	259	316	0.16	0.0022	3175	49	12	259	294	1.00	0.34	0.08	0.60
V12112211	1.51	0.32	0.83	0.89	0.69	0.26	0.0036	2485	2485	8	3 166	413	0.26	0.0036	2473	39	7	166	385	1.00	0.28	0.06	0.45
V22112211	1.39	0.34	0.84	0.89	0.72	0.24	0.0033	2641	2641	9	100	389	0.24	0.0032	2629	41	9	186	362	1.00	0.23	0.07	0.48
V22112212	1.43	0.33	0.84	0.89	0.75	0.24	0.0034	2603	2603	9	187	349	0.24	0.0033	2593	41	9	187	324	1.00	0.30	0.08	0.48
V32112211 V32112212	1.32	0.35	0.85	0.89	0.73	0.22	0.0030	2751	2751	10	203	325	0.22	0.0029	2775	43	10	205	301	1.00	0.31	0.08	0.51
V42112211	1.20	0.38	0.86	0.89	0.78	0.20	0.0027	2920	2920	11	223	334	0.20	0.0027	2904	45	11	223	310	1.00	0.33	0.08	0.55
V42112212 V52112211	1.22	0.37	0.85	0.89	0.80	0.18	0.0028	2888	2888	11	223	298	0.20	0.0027	28/3	45	11	223	275	1.00	0.34	0.09	0.56
V52112212	1.13	0.39	0.87	0.89	0.83	0.19	0.0026	3012	3012	13	3 241	267	0.19	0.0025	2990	46	12	240	245	1.00	0.37	0.09	0.60
V12112221 V12112222	1.22	0.37	0.86	0.89	0.68	0.20	0.0028	2888 2837	2888	9	9 194 9 196	486	0.20	0.0028	2876	45	9	194 195	459	1.00	0.24	0.05	0.45
V22112221	1.13	0.39	0.86	0.89	0.71	0.18	0.0025	3040	3040	10) 214	457	0.18	0.0025	3029	47	10	214	432	1.00	0.25	0.06	0.48
V22112222 V32112221	1.16	0.38	0.86	0.89	0.74	0.19	0.0026	2999	2999	10	216	410	0.19	0.0026	2990	47	10	216	386	1.00	0.26	0.06	0.48
V32112222	1.08	0.40	0.87	0.89	0.77	0.17	0.0023	3141	3141	11	235	382	0.17	0.0023	3131	49	11	235	360	1.00	0.28	0.07	0.51
V42112221	0.99	0.43	0.88	0.89	0.77	0.15	0.0021	3325	3325	12	2 253	398	0.15	0.0021	3312	52	12	253	376	1.00	0.28	0.07	0.55
V52112222	0.93	0.42	0.88	0.89	0.79	0.14	0.0021	3499	3499	14	200	369	0.14	0.0021	3482	51	12	235	348	1.00	0.30	0.07	0.56
V52112222	0.94	0.45	0.88	0.89	0.82	0.15	0.0020	3452	3452	14	275	327	0.15	0.0020	3436	53	13	275	307	1.00	0.31	0.08	0.60
V12121111 V12121112	3.26	0.14	0.49	0.84	0.70	0.53	0.0075	1423	1423	7	14/	237	0.53	0.0072	1342	22	7	147	204	1.00	0.40	0.15	0.46
V22121111	2.98	0.15	0.53	0.84	0.74	0.49	0.0068	1591	1591	9	173	231	0.49	0.0067	1576	25	8	173	199	1.00	0.43	0.16	0.48
V22121112 V32121111	3.09	0.15	0.50	0.83	0.78	0.46	0.0069	1523	1523	10	9 171) 201	190 224	0.50	0.0068	1511	24	8	201	162	1.00	0.44	0.18	0.48
V32121112	2.82	0.16	0.55	0.85	0.81	0.47	0.0065	1704	1704	11	198	183	0.47	0.0063	1688	26	9	198	155	1.00	0.49	0.18	0.52
V42121111 V42121112	2.51	0.19	0.64	0.87	0.81	0.44	0.0062	1967	1967 1901	13	3 233 3 228	212	0.44	0.0059	1937	30	11	232	178	1.00	0.53	0.18	0.56
V52121111	2.30	0.21	0.73	0.88	0.84	0.42	0.0061	2194	2194	17	270	194	0.42	0.0058	2138	33	13	267	159	0.99	0.62	0.19	0.60
V52121112	2.36	0.21	0.71	0.88	0.87	0.43	0.0062	2127	2127	17	264	158	0.43	0.0059	2075	32	13	261	127	0.99	0.64	0.20	0.60
V12121122	2.00	0.18	0.55	0.85	0.74	0.46	0.0064	1737	1737	9	132	250	0.46	0.0064	1725	27	9	188	219	1.00	0.38	0.14	0.46
V22121121	2,43	0.20	0.63	0.87	0.74	0.42	0.0058	2055	2055	11	227	298	0.42	0.0058	2039	32	10	226	263	1.00	0.39	0.12	0.48

Name	T1 [s]	Sa(T1)	Sa(T2)	Sa(T3)	DOC	Disp	ISDR [-]	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	Disp [m]2	ISDR [·]3	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	C1 [·]	C2[·]	C3 [·]	Mdy/
V32121121	2.23	0.22	0.68	0.87	0.77	0.40	0.0055	[MN·m] 2290	[MN] 2290	[MN] 13	264	[MN-m] 289	0.40	0.0054	[MN·m]4 2270	[MN]5 35	[MN]6 12	264	[MN·m]8 254	1.00	0.42	0.13	0.51
V32121122	2.32	0.21	0.65	0.87	0.81	0.41	0.0056	2194	2194	13	259	237	0.41	0.0055	2178	34	12	258	206	1.00	0.44	0.14	0.52
V42121121 V42121122	2.00	0.23	0.74	0.88	0.83	0.39	0.0054	2040	2449	16	299	275	0.39	0.0053	2423	38	14	298	193	1.00	0.49	0.14	0.56
V52121121 V52121122	1.89	0.27	0.80	0.88	0.84	0.36	0.0051	2747 2690	2747 2690	20	344 340	244 201	0.36	0.0049	2697 2642	41 41	16 16	341 337	210 170	0.99	0.53	0.15	0.60
V12121211	2.90	0.16	0.53	0.85	0.73	0.47	0.0066	1648 1579	1648	8	172	247	0.47	0.0066	1635	26	8	172	216	1.00	0.39	0.14	0.46
V22121212	2.64	0.18	0.58	0.85	0.77	0.45	0.0062	1853	1853	10	203	242	0.44	0.0061	1840	29	10	203	211	1.00	0.41	0.15	0.48
V22121212 V32121211	2.73	0.17	0.55	0.84	0.80	0.45	0.0063	1786 2073	2073	10	201	199 234	0.45	0.0062	2056	28	10	201	172	1.00	0.42	0.16	0.48
V32121212 V42121211	2.48	0.19	0.60	0.86	0.83	0.43	0.0059	2007	2007	12	233	192	0.43	0.0058	1993 2285	31	11	233	165	1.00	0.46	0.16	0.52
V42121212	2.27	0.22	0.67	0.87	0.85	0.41	0.0057	2245	2245	15	269	183	0.41	0.0055	2220	35	13	269	150	1.00	0.50	0.10	0.56
V52121211 V52121212	2.03	0.25	0.77	0.88	0.86	0.39	0.0056	2578 2517	25/8	19 19	316 311	202	0.39	0.0053	2529	39	15 15	313 308	1/0	0.99	0.56	0.16	0.60
V121212221	2.35	0.21	0.64	0.87	0.73	0.41	0.0057	2150 2053	2150	11	228	322	0.41	0.0057	2138	34	10	228	289	1.00	0.35	0.11	0.45
V22121221	2.15	0.23	0.69	0.87	0.76	0.39	0.0053	2422	2422	13	269	316	0.38	0.0053	2408	38	13	269	283	1.00	0.37	0.11	0.48
V22121222 V32121221	1.97	0.22	0.65	0.87	0.80	0.39	0.0050	2328 2684	2528	13	310	304	0.39	0.0054	2667	36 42	13	310	231	1.00	0.38	0.13	0.48
V32121222 V42121221	2.03	0.25	0.71	0.88	0.82	0.37	0.0051	2617 2869	2617 2869	16 18	309 344	252	0.37	0.0050	2603 2842	41	15	309 343	223	1.00	0.41	0.13	0.52
V42121222	1.86	0.27	0.76	0.88	0.85	0.34	0.0047	2818	2818	19	343	231	0.34	0.0046	2795	44	17	342	202	1.00	0.45	0.13	0.56
V52121221 V52121222	1.66	0.30	0.82	0.89	0.88	0.31	0.0043	3046	3046	21	370	244	0.31	0.0042	2957	46	18	376	172	0.99	0.50	0.14	0.60
V12122111 V12122112	1.91	0.27	0.76	0.88	0.67	0.35	0.0048	2749 2683	2749	8	184 184	490 438	0.35	0.0048	2730	42	8	184 184	450 401	1.00	0.30	0.07	0.45
V22122111	1.76	0.28	0.79	0.88	0.70	0.32	0.0044	2936	2936	10	207	465	0.32	0.0044	2917	45	9	207	426	1.00	0.32	0.08	0.48
V32122112	1.63	0.30	0.81	0.88	0.73	0.33	0.0040	3113	3113	11	208	437	0.29	0.0040	3092	43	10	208	399	1.00	0.35	0.09	0.48
V32122112 V42122111	1.67	0.29	0.80	0.88	0.76	0.30	0.0041	3062 3283	3062 3283	11	230	390 404	0.30	0.0041	3043 3255	47	11	230	354	1.00	0.36	0.09	0.51
V42122112	1.55	0.31	0.83	0.89	0.79	0.28	0.0038	3237	3237	13	253	360	0.27	0.0037	3212	50	12	253	325	1.00	0.39	0.10	0.55
V52122111 V52122112	1.44	0.33	0.85	0.89	0.82	0.25	0.0035	3405	3405	15	276	322	0.25	0.0035	3369	52	13	275	289	1.00	0.43	0.11	0.60
V12122121 V12122122	1.55	0.31	0.83	0.89	0.66	0.27	0.0038	3240 3169	3240 3169	10	217 218	584 524	0.27	0.0037	3221 3153	50 49	10	217	544 487	1.00	0.27	0.06	0.45
V22122121	1.43	0.33	0.84	0.89	0.70	0.25	0.0034	3436 3371	3436	11	243	553 496	0.25	0.0034	3418	53	11	243	515	1.00	0.29	0.07	0.48
V32122121	1.33	0.35	0.85	0.89	0.72	0.23	0.0031	3617	3617	13	268	519	0.23	0.0031	3598	56	12	268	483	1.00	0.31	0.07	0.51
V32122122 V42122121	1.37	0.34	0.84	0.89	0.75	0.23	0.0032	3557	3557	13	268	464 481	0.23	0.0032	3539	55	13	268	430	1.00	0.32	0.08	0.51
V42122122 V52122121	1.28	0.36	0.85	0.89	0.78	0.21	0.0029	3730 3940	3730 3940	15	292 316	429 438	0.21	0.0029	3708	58	14	292	396	1.00	0.34	0.08	0.55
V52122122	1.19	0.38	0.87	0.89	0.81	0.20	0.0027	3893	3893	16	315	389	0.20	0.0027	3864	60	15	315	358	1.00	0.36	0.09	0.60
V12122211 V12122212	1.74	0.29	0.79	0.88	0.69	0.32	0.0043	2966 2909	2966	9	200	497 445	0.32	0.0043	2949	46 45	9	198	459	1.00	0.30	0.08	0.45
V22122211 V22122212	1.60	0.30	0.81	0.88	0.72	0.28	0.0039	3164 3110	3164 3110	10	223	470	0.28	0.0039	31.47 30.96	49	10	223	434	1.00	0.31	0.08	0.48
V32122211	1.49	0.32	0.83	0.89	0.75	0.26	0.0036	3348	3348	12	246	441	0.26	0.0035	3329	52	11	246	405	1.00	0.34	0.08	0.51
V42122212	1.32	0.34	0.82	0.89	0.78	0.27	0.0033	3520	3520	13	240	407	0.24	0.0032	3496	54	12	240	373	1.00	0.35	0.09	0.55
V42122212 V52122211	1.41	0.34	0.84	0.89	0.80	0.24	0.0033	3479 3685	3479 3685	14	269	363	0.24	0.0033	3458 3653	54 56	13	269 291	331	1.00	0.37	0.10	0.56
V52122212	1.31	0.35	0.86	0.89	0.83	0.22	0.0031	3648 3477	3648	15	291	327	0.22	0.0031	3618 3461	56	14	291	296	1.00	0.40	0.10	0.60
V12122222	1.46	0.33	0.83	0.89	0.71	0.25	0.0035	3414	3414	11	235	530	0.25	0.0035	3401	53	11	235	495	1.00	0.28	0.07	0.45
V22122221 V22122222	1.31	0.35	0.85	0.89	0.71	0.23	0.0030	3680 3623	3623	12	259	558	0.22	0.0030	3665 3610	57	12	259	523	1.00	0.27	0.05	0.48
V32122221	1.22	0.37	0.85	0.89	0.74	0.20	0.0028	3862 3809	3862	14	284	523 468	0.20	0.0027	3846 3795	60 59	13	284	490	1.00	0.29	0.07	0.51
V42122221	1.14	0.39	0.87	0.89	0.77	0.18	0.0025	4025	4025	15	307	486	0.18	0.0025	4005	62	14	307	454	1.00	0.31	0.08	0.55
V52122222	1.16	0.38	0.86	0.89	0.79	0.17	0.0028	5982 4178	3982 4178	15	308	434	0.19	0.0028	4153	62	15	308	404	1.00	0.32	0.08	0.60
V52122222 V12211111	1.09 4.34	0.40	0.87	0.89	0.82	0.17	0.0024	4137	4137	17	330	397 174	0.17	0.0024	4114	64	16	329 104	367	1.00	0.34	0.09	0.60
V12211112	4.53	0.09	0.37	0.69	0.75	0.67	0.0071	960	960	5	103	144	0.67	0.0069	949	11	5	102	118	1.00	0.48	0.21	0.46
V22211111 V222111112	4.11	0.11	0.42	0.77	0.74	0.62	0.0065	1076	1076	6	122	169	0.62	0.0062	1064	13	6	122	159	1.00	0.46	0.21	0.49
V32211111 V32211112	3.63 3.75	0.12	0.45	0.80	0.77	0.57	0.0060	1244 1198	1244 1198	8	141	163 134	0.57	0.0058	1225 1183	14	7	141	133	1.00	0.53	0.23	0.52
V42211111	3.33	0.13	0.51	0.84	0.80	0.54	0.0058	1379	1379	9	163	155	0.54	0.0055	1351	16	8	162	124	1.00	0.60	0.25	0.56
V52211112	3.06	0.15	0.59	0.86	0.84	0.52	0.0057	1538	1538	12	189	142	0.51	0.0054	1484	17	9	195	100	0.98	0.72	0.27	0.60
V52211112 V12211121	3.13	0.14	0.57	0.85	0.86	0.52	0.0058	1492 1286	1492 1286	12 6	185	116 220	0.52	0.0055	1442	17	9	182 134	87	0.98	0.73	0.29	0.60
V12211122	3.70	0.12	0.43	0.78	0.74	0.57	0.0060	1220	1220	6	132	181	0.57	0.0059	1209	14	6	132	153	1.00	0.43	0.18	0.46
V22211122	3.37	0.13	0.47	0.81	0.77	0.53	0.0056	1369	1369	8	155	176	0.53	0.0054	1357	16	7	155	149	1.00	0.46	0.19	0.49
V32211121 V32211122	3.08	0.15	0.54	0.85	0.77	0.50	0.0051	1592	1592 1528	10	183 180	207	0.49	0.0050	1572	18	9	183	1/5	1.00	0.50	0.18	0.52
V42211121 V42211122	2.73	0.17	0.60	0.86	0.80	0.47	0.0049	1766 1702	1766 1702	12	212	198 162	0.47	0.0047	1736 1676	20	10	211 207	164	1.00	0.55	0.19	0.56
V52211121	2.51	0.19	0.69	0.88	0.83	0.45	0.0049	1965	1965	15	245	181	0.44	0.0046	1910	22	12	242	146	0.99	0.65	0.21	0.60
V12211122	3.86	0.18	0.67	0.87	0.86	0.45	0.0062	1902	1902	15	121	140	0.45	0.0047	1148	14	6	121	117	1.00	0.66	0.25	0.60
V12211212 V22211211	4.01	0.11	0.40	0.74	0.77	0.61	0.0064	1113 1299	1113	6	120 142	149	0.60	0.0063	1104 1286	13	6	120 142	124	1.00	0.45	0.20	0.46
V22211212	3.63	0.12	0.43	0.77	0.80	0.56	0.0059	1253	1253	7	141	145	0.56	0.0057	1243	15	7	141	120	1.00	0.47	0.21	0.49
V32211211	3.21	0.14	0.50	0.83	0.79	0.53	0.0055	1448	1448	9	163	139	0.52	0.0052	1431	1/	8	163	140	1.00	0.51	0.21	0.52
V42211211 V42211212	2.94 3.01	0.15	0.55	0.85	0.82	0.49	0.0052	1610 1565	1610 1565	11	190 187	161 132	0.49	0.0050	1583 1542	18 18	9	189 187	131 106	1.00	0.57	0.22	0.56
V52211211	2.69	0.17	0.64	0.87	0.85	0.47	0.0051	1798	1798	14	220	147	0.47	0.0049	1746	20	10	217	115	0.99	0.68	0.24	0.60
V12211221	3.13	0.14	0.50	0.84	0.73	0.50	0.0053	1497	1497	.14	159	231	0.50	0.0052	1484	17	7	158	200	1.00	0.40	0.15	0.46
V12211222 V22211221	3.26 2.85	0.14	0.47	0.81	0.77	0.51	0.0054	1432 1680	1432 1680	8	157	190 225	0.51	0.0053	1422 1666	17	7	157	164	1.00 <u>1.</u> 00	0.41	0.17	0.46
V22211222	2.95	0.15	0.51	0.83	0.80	0.48	0.0050	1616	1616	9	184	186	0.48	0.0049	1606	19	9	184	159	1.00	0.44	0.18	0.48
V32211222	2.69	0.17	0.57	0.85	0.82	0.45	0.0047	1811	1811	11	214	180	0.45	0.0046	1797	21	10	214	153	1.00	0.47	0.18	0.52

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [-]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [-]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [·]	c2[·]	C3 [·]	Mdy/ Mst [•]
V52211221 V52211222	2.20	0.22	0.74	0.88	0.85	0.41	0.0044	2324 2262	2324	17	289	190 156	0.41	0.0042	2274	26 26	14	286	157 126	0.99	0.59	0.18	0.60
V12212111	2.53	0.19	0.62	0.87	0.67	0.44	0.0045	1946	1946	6	130	354	0.43	0.0045	1927	22	6	130	315	1.00	0.35	0.10	0.45
V22212112	2.61	0.18	0.66	0.85	0.70	0.44	0.0048	2167	2167	7	129	313	0.41	0.0048	21.48	22	7	128	311	1.00	0.36	0.12	0.43
V22212112 V32212111	2.40	0.20	0.64	0.87	0.73	0.42	0.0043	2093	2093	7	151	307	0.42	0.0043	2077	24	7	151	273	1.00	0.38	0.12	0.48
V32212112	2.21	0.22	0.69	0.88	0.76	0.40	0.0041	2325	2325	g	175	299	0.40	0.0040	2306	27	8	175	264	1.00	0.41	0.13	0.51
V42212111 V42212112	2.00	0.25	0.75	0.88	0.77	0.37	0.0039	2631 2571	2631	11	204	324	0.37	0.0038	2601 2545	30	10	203	287	1.00	0.44	0.12	0.55
V52212111	1.85	0.27	0.80	0.88	0.80	0.35	0.0037	2796	2796	12	226	293	0.35	0.0036	2751	32	11	225	256	1.00	0.50	0.14	0.60
V52212112 V12212121	2.05	0.27	0.79	0.88	0.83	0.36	0.0038	2755 2562	2/55 2562	13	224	259 465	0.35	0.0037	2/12	31	11	223	425	1.00	0.51	0.14	0.60
V12212122	2.12	0.24	0.71	0.88	0.69	0.38	0.0040	2458	2458	8	170	410	0.38	0.0040	2442	29	8	170	373	1.00	0.32	0.09	0.45
V22212121 V222212122	1.89	0.27	0.76	0.88	0.70	0.35	0.0037	2767	2767 2706	9	197	399	0.35	0.0036	2748 2690	32	9	197	408 363	1.00	0.33	0.09	0.48
V32212121	1.76	0.28	0.79	0.88	0.73	0.32	0.0033	2936	2936	11	219	422	0.32	0.0033	2914	34	10	219	383	1.00	0.36	0.09	0.51
V42212122	1.64	0.30	0.82	0.88	0.76	0.33	0.0031	3098	3098	11	241	392	0.29	0.0030	3070	36	10	240	340	1.00	0.39	0.10	0.51
V42212122	1.67	0.29	0.81	0.88	0.79	0.30	0.0031	3049	3049	12	240	349	0.30	0.0031	3025	35	11	240	313	1.00	0.40	0.11	0.55
V52212122	1.56	0.31	0.84	0.89	0.82	0.28	0.0029	3212	3212	14	262	316	0.28	0.0029	3175	37	13	261	281	1.00	0.44	0.12	0.60
V12212211 V12212212	2.31	0.21	0.66	0.88	0.69	0.41	0.0042	2199	2199	7	148	374	0.40	0.0042	2182	26	7	147	336 296	1.00	0.34	0.10	0.45
√22212211	2.12	0.24	0.71	0.88	0.72	0.38	0.0040	2454	2454	8	173	368	0.38	0.0039	2437	29	8	173	331	1.00	0.36	0.10	0.48
V22212212 V32212211	2.18	0.23	0.68	0.87	0.75	0.39	0.0040	2381 2687	2381	10	1/2	325	0.39	0.0040	2367	28	9	172	291 319	1.00	0.37	0.11	0.48
V32212212	2.01	0.25	0.73	0.88	0.78	0.37	0.0038	2641	2641	10	198	315	0.37	0.0038	2624	31	9	198	282	1.00	0.39	0.11	0.51
V42212211 V42212212	1.82	0.28	0.78	0.88	0.79	0.34	0.0035	2855 2817	2855 2817	11	219	293	0.33	0.0034	2829	33	10	219	294	1.00	0.42	0.11	0.55
V52212211	1.69	0.29	0.82	0.88	0.82	0.31	0.0033	3016	3016	13	240	298	0.31	0.0032	2976	34	11	239	262	1.00	0.47	0.13	0.60
V12212212	1.72	0.29	0.81	0.88	0.69	0.34	0.0035	2981	2961	15	189	478	0.34	0.0032	2943	33	8	189	439	1.00	0.30	0.15	0.45
V12212222	1.92	0.26	0.74	0.88	0.71	0.35	0.0036	2745	2745	9	212	427	0.35	0.0036	2731	32	9	190	392	1.00	0.31	0.08	0.45
V22212221	1.72	0.28	0.75	0.88	0.72	0.32	0.0033	2938	2938	10	212	405	0.32	0.0032	2924	34	10	212	371	1.00	0.33	0.09	0.48
V32212221 V32212222	1.60	0.31	0.81	0.88	0.75	0.28	0.0029	3169	3169	11	234	427	0.28	0.0029	31.50	37	11	234	391	1.00	0.34	0.09	0.51
V42212221	1.49	0.32	0.84	0.89	0.78	0.26	0.0027	3335	3335	13	256	398	0.26	0.0027	3312	39	12	256	362	1.00	0.37	0.09	0.55
V42212222 V52212221	1.52	0.32	0.83	0.88	0.80	0.27	0.0028	3290 3491	3290 3491	13	256	354	0.27	0.0027	3269 3460	38	12	256	320	1.00	0.38	0.10	0.56
V52212222	1.42	0.33	0.85	0.89	0.83	0.25	0.0026	3453	3453	15	278	322	0.25	0.0025	3423	40	13	277	289	1.00	0.41	0.11	0.60
V12221111 V12221112	5.01	0.08	0.36	0.68	0.70	0.73	0.0078	1135 1064	1135	6	117	201	0.73	0.0075	1115	13	5	116	161	1.00	0.49	0.21	0.46
V22221111	4.57	0.09	0.38	0.71	0.74	0.68	0.0072	1263	1263	7	137	195	0.68	0.0070	1242	15	6	137	156	1.00	0.52	0.23	0.49
V32221112	4.75	0.10	0.30	0.66	0.78	0.64	0.0068	1397	1397	9	155	181	0.64	0.0071	1195	14	7	158	120	1.00	0.54	0.24	0.49
V32221112	4.33	0.10	0.39	0.72	0.81	0.66	0.0070	1346	1346	9	156	155	0.65	0.0066	1326	16	7	156	121	1.00	0.58	0.27	0.52
V42221111	3.96	0.11	0.43	0.77	0.83	0.62	0.0066	1495	1495	11	. 179	146	0.62	0.0062	1464	17	9	178	112	1.00	0.65	0.29	0.56
V52221111 V52221112	3.53	0.12	0.52	0.84	0.83	0.58	0.0064	1722	1722	14	211	163	0.57	0.0060	1654	19	10	207	122	0.98	0.76	0.32	0.60
V12221121	4.08	0.10	0.41	0.77	0.70	0.62	0.0065	1443	1443	7	150	252	0.62	0.0064	1421	17	7	150	210	1.00	0.44	0.18	0.46
V12221122 V22221121	4.28	0.10	0.39	0.73	0.74	0.64	0.0068	1370 1605	1370 1605	7	148	208 245	0.64	0.0066	1354	16 19	7	148	172 204	1.00	0.46	0.20	0.46
V22221122	3.89	0.11	0.41	0.76	0.77	0.60	0.0062	1533	1533	9	173	202	0.59	0.0061	1516	18	8	173	166	1.00	0.48	0.22	0.49
V32221121 V32221122	3.43	0.13	0.48	0.82	0.80	0.56	0.0059	1778	17/8	11	. 204	236	0.55	0.0055	1/52	20	10	204	195	1.00	0.51	0.21	0.52
V42221121	3.15	0.14	0.53	0.84	0.80	0.52	0.0055	1967	1967	13	235	225	0.52	0.0053	1929	22	11	234	182	1.00	0.58	0.23	0.56
V52221122	2.90	0.14	0.51	0.83	0.83	0.49	0.0056	2188	21897	14	231	207	0.49	0.0054	2118	22	13	230	147	0.99	0.69	0.25	0.60
V52221122	2.98	0.15	0.59	0.86	0.86	0.50	0.0055	2119	2119	18	266	169	0.50	0.0052	2053	24	13	263	130	0.99	0.71	0.27	0.60
V12221211 V12221212	4.43	0.09	0.36	0.72	0.73	0.68	0.0072	1251	1251	7	135	172	0.67	0.0070	1239	15	6	135	139	1.00	0.49	0.20	0.46
V22221211	4.03	0.02	0.14	0.32	0.75	0.14	0.0016	346	346	2	37	57	0.14	0.0014	335	4	2	37	38	1.00	0.73	0.40	0.51
V32221211	3.67	0.03	0.16	0.34	0.78	0.14	0.0016	393	393	3	44	56	0.13	0.0014	380	4	2	44	37	1.00	0.81	0.44	0.54
V32221212 V42221211	3.77	0.03	0.15	0.32	0.82	0.14	0.0016	378	378	3	43	46	0.14	0.0014	367	4	2	43	30	1.00	0.82	0.46	0.54
V42221212	3.43	0.03	0.17	0.36	0.84	0.13	0.0015	434	434	4	51	46	0.13	0.0013	415	5	2	51	28	0.99	0.95	0.51	0.57
V52221211 V52221212	3.06	0.04	0.23	0.45	0.85	0.13	0.0015	528	528	6	62	53 44	0.13	0.0013	481 468	5	3	59	30	0.95	1.14	0.62	0.63
V12221221	3.59	0.03	0.16	0.35	0.72	0.13	0.0015	407	407	2	43	75	0.13	0.0014	395	5	2	42	52	0.99	0.63	0.33	0.48
V12221222 V22221221	3.73	0.03	0.15	0.33	0.76	0.13	0.0015	464	385 464	3	51	74	0.13	0.0014	452	4	2	42	43	1.00	0.68	0.35	0.47
V22221222	3.37	0.03	0.16	0.35	0.79	0.13	0.0014	442	442	3	50	61	0.13	0.0013	433	5	2	50	42	1.00	0.69	0.37	0.50
V32221221 V32221222	3.06	0.04	0.20	0.41	0.78	0.12	0.0014	507	507	4	. 59	60	0.12	0.0012	495	6	3	59	41	1.00	0.76	0.39	0.53
V42221221	2.72	0.04	0.23	0.45	0.81	0.12	0.0013	606 584	606 584	5	72	72	0.12	0.0012	581	7	3	71	48	0.99	0.86	0.43	0.57
V52221222	2.48	0.05	0.22	0.52	0.84	0.11	0.0013	710	710	7	87	69	0.11	0.0012	656	8	4	83	42	0.95	1.06	0.51	0.62
V52221222	2.53	0.05	0.27	0.50	0.86	0.11	0.0013	688 557	688 557	8	85	57	0.11	0.0012	637	7	4	82	34	0.96	1.07	0.52	0.62
V12222111	2.97	0.04	0.20	0.42	0.69	0.12	0.0013	532	532	2	36	103	0.12	0.0012	516	6	2	36	75	1.00	0.58	0.28	0.47
V22222111	2.64	0.04	0.23	0.46	0.70	0.11	0.0012	631	631 607	2	44	116	0.11	0.0012	612 591	7	2	44	86 75	1.00	0.62	0.28	0.50
V322221112	2.42	0.05	0.22	0.49	0.73	0.11	0.0012	716	716	3	53	115	0.11	0.0011	693	8	2	53	84	1.00	0.69	0.31	0.53
V32222112 V42222111	2.48	0.05	0.24	0.47	0.76	0.11	0.0012	690 815	690 815	3	52	101	0.11	0.0011	671	8	2	52 62	73	1.00	0.69	0.32	0.53
V42222112	2.27	0.06	0.28	0.51	0.79	0.11	0.0012	789	789	4	62	100	0.11	0.0011	759	9	3	61	69	0.99	0.80	0.36	0.57
V52222111 V52222112	2.04	0.07	0.34	0.61	0.81	0.10	0.0012	943 918	943 91.9	6	76	106	0.10	0.0011	881	10	4	74	72 62	0.97	0.94	0.43	0.61
V12222121	2.31	0.05	0.27	0.51	0.66	0.11	0.0011	767	767	2	52	156	0.11	0.0011	746	9	2	51	121	1.00	0.52	0.21	0.46
V12222122 V22222121	2.39	0.05	0.25	0.49	0.69	0.11	0.0011	729	729	2	50	136	0.11	0.0011	713	8	2	50 62	105	1.00	0.53	0.23	0.46
V22222122	2.18	0.06	0.28	0.51	0.73	0.10	0.0011	837	837	3	61	137	0.10	0.0010	820	10	3	61	105	1.00	0.57	0.25	0.49
V32222121 V32222122	1.95	0.07	0.32	0.57	0.74	0.10	0.0011	981 953	981 953	4	74	152 135	0.10	0.0010	956 932	11	3	73 73	117	1.00	0.62	0.26	0.52
V42222121	1.79	0.08	0.35	0.61	0.77	0.09	0.0010	1077	1077	5	84	144	0.09	0.0009	1041	12	4	84	108	0.99	0.72	0.30	0.56
V42222122 V52222121	1.83	0.07	0.34	0.60	0.79	0.09	0.0010	1052	1052	5	98	128	0.09	0.0009	1020	12	4		94	0.99	0.73	0.31	0.56
V52222122	1.68	0.08	0.40	0.66	0.83	0.09	0.0010	1171	1171	7	97	115	0.09	0.0009	1107	13	5	94	81	0.97	0.87	0.38	0.61

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [-]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [·]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [-]	c2[·]	C3 [·]	Mdy/ Mst [·]
V22222211 V22222212	2.38	0.05	0.25	0.48	0.72	0.11	0.0012	732	732	3	3 52 3 51	123 108	0.11	0.0011	714	8		2 52 2 51	93 81	1.00	0.60	0.26	0.49
V32222211	2.19	0.06	0.28	0.51	0.76	0.10	0.0011	835	835	4	62	122	0.10	0.0011	814	9		3 62	91	1.00	0.66	0.29	0.53
V42222212	2.23	0.08	0.27	0.49	0.78	0.10	0.0011	952	952	5	5 73	108	0.10	0.0011	920	11		3 73	87	0.99	0.87	0.30	0.55
V42222212 V52222211	2.04	0.07	0.31	0.55	0.81	0.10	0.0011	929 1063	929 1063	6	5 72	105	0.10	0.0010	900	10	3	3 72 1 83	76	0.99	0.76	0.33	0.56
V52222212	1.86	0.07	0.36	0.63	0.84	0.10	0.0011	1046	1046	e	85	96	0.10	0.0010	987	11	4	82	65	0.97	0.90	0.41	0.61
V12222221 V12222222	2.09	0.06	0.29	0.54	0.69	0.10	0.0011	894 855	894	3	3 61 3 60	166	0.10	0.0010	8/6	10	3	<u> </u>	132	1.00	0.50	0.20	0.46
V222222221	1.91	0.07	0.32	0.57	0.72	0.10	0.0010	1003	1003	4	1 72 1 72	163	0.09	0.0010	984	12		3 72	129	1.00	0.54	0.22	0.49
V322222221	1.30	0.07	0.34	0.54	0.75	0.09	0.0009	1102	1102		+ 72 5 83	145	0.09	0.0010	1079	13	4	4 83	114	1.00	0.55	0.25	0.49
V32222222 V42222221	1.79	0.08	0.33	0.58	0.78	0.09	0.0010	1078 1212	1078	6	5 82 5 95	139	0.09	0.0009	1058	12	4	1 82 5 94	107	1.00	0.62	0.26	0.52
V42222222	1.64	0.08	0.37	0.63	0.81	0.08	0.0009	1187	1187	e	94	131	0.08	0.0009	1155	13	4	1 93	98	0.99	0.70	0.30	0.56
V52222221 V52222222	1.4/	0.10	0.44	0.70	0.83	0.08	0.0009	1348 1324	1348 1324	8	3 110 3 109	133	0.08	0.0008	1280	15		5 108 5 106	97	0.98	0.84	0.35	0.60
V13111111	6.26	0.01	0.09	0.21	0.68	0.18	0.0019	229	229	1	. 23	51	0.18	0.0016	216	2		L 23	31	0.99	0.76	0.42	0.47
V23111112	5.71	0.01	0.08	0.13	0.73	0.18	0.0019	213	215	1	. 22	42	0.18	0.0017	203	3	1	L 22 L 28	31	1.00	0.78	0.44	0.47
V23111112	5.92	0.01	0.09	0.21	0.76	0.18	0.0018	247	247	1	. 27	42	0.17	0.0016	238	2	1	L 27	25	1.00	0.83	0.48	0.49
V33111112	5.39	0.02	0.10	0.23	0.79	0.17	0.0018	287	287	2	2 32	42	0.17	0.0015	275	3		32	25	1.00	0.91	0.52	0.52
V43111111 V43111112	4.78 4.91	0.02	0.12	0.29	0.78	0.16	0.0017	350 335	350	2	2 40 2 39	51 42	0.16	0.0015	326	3	1	L 39 L 39	29	0.99	1.03	0.60	0.56
V53111111	4.36	0.02	0.16	0.35	0.81	0.16	0.0017	412	412	3	3 49	49	0.16	0.0015	363	4	1	L 46	26	0.94	1.27	0.75	0.61
V1311112	4.4/	0.02	0.15	0.34	0.84	0.17	0.0017	397	397		3 48 L 32	41 68	0.16	0.0015	353	3	1	L 45 L 32	44	0.94	0.68	0.37	0.61
V13111122	5.33	0.02	0.10	0.23	0.73	0.17	0.0017	292	292	1	. 31	55	0.17	0.0015	282	3	3	L 31	35	0.99	0.71	0.39	0.46
V23111122	4.84	0.02	0.11	0.26	0.76	0.16	0.0016	338	338	1	38	55	0.16	0.0014	327	3		L 37	35	1.00	0.76	0.43	0.49
V33111121 V33111122	4.26	0.02	0.14	0.31	0.75	0.15	0.0016	404	404	2	2 45	67 55	0.15	0.0013	385	4	1	L 45 L 44	42	1.00	0.84	0.47	0.52
V43111121	3.90	0.03	0.16	0.35	0.78	0.14	0.0015	459	459	3	53	66	0.14	0.0013	431	4		2 53	39	0.99	0.98	0.54	0.55
V43111122 V53111121	4.02 3.56	0.02	0.15	0.33	0.81	0.14	0.0015	439	439 538	2	3 52 1 65	54	0.14	0.0013	415	4	3	2 52 2 62	32	0.99	1.20	0.66	0.60
V53111122	3.66	0.03	0.19	0.39	0.84	0.14	0.0015	518	518	1	1 64	52	0.14	0.0013	466	5		2 60	27	0.95	1.21	0.68	0.60
V13111211	5.77	0.02	0.10	0.23	0.71	0.17	0.0018	257	274	1	. 20	44	0.17	0.0016	203	3		L 20 L 27	28	0.99	0.72	0.40	0.47
V23111211 V23111212	5.05	0.02	0.11	0.25	0.75	0.17	0.0017	319 302	319 302	1	34	54	0.16	0.0015	307 293	3	1	L 34 L 33	35	1.00	0.77	0.43	0.49
V33111211	4.60	0.02	0.12	0.28	0.78	0.16	0.0016	362	362	2	2 40	54	0.16	0.0014	347	4	1	L 40	33	1.00	0.86	0.49	0.52
V33111212 V43111211	4.73	0.02	0.11	0.26	0.81	0.15	0.0017	348 413	348 413	2	2 40 2 47	44 53	0.16	0.0014	337	4	1	L 40 2 47	27	0.99	1.01	0.50	0.52
V43111212	4.30	0.02	0.14	0.30	0.84	0.15	0.0016	399	399	3	3 47	44	0.15	0.0014	379	4	1	L 46	25	0.99	1.01	0.58	0.55
V53111211	3.90	0.03	0.18	0.38	0.85	0.15	0.0016	473	473	-	+ Jo 1 57	42	0.14	0.0013	430	4		2 54	27	0.94	1.23	0.71	0.60
V13111221 V13111222	4.50	0.02	0.12	0.29	0.71	0.15	0.0015	374	374	1	39	72	0.15	0.0014	360	4	1	L 39 L 38	48	0.99	0.67	0.37	0.46
V23111221	4.09	0.02	0.14	0.31	0.75	0.15	0.0015	425	425	2	2 46	71	0.14	0.0013	411	4		L 46	47	1.00	0.73	0.40	0.48
V23111222 V33111221	4.23	0.02	0.13	0.29	0.79	0.14	0.0015	405	405		2 46 2 55	58	0.15	0.0013	465	5	2	L 45 2 55	38 46	1.00	0.74	0.42	0.48
V33111222	3.84	0.03	0.15	0.32	0.81	0.14	0.0014	462	462		2 54	58	0.14	0.0012	449	5		2 54	37	1.00	0.83	0.46	0.51
V43111221	3.49	0.03	0.10	0.36	0.84	0.14	0.0014	531	531	3	5 65 3 64	57	0.13	0.0012	508	5		2 63	43 34	0.99	0.94	0.43	0.55
V53111221 V53111222	3.11 3.17	0.04	0.22	0.45	0.83	0.13	0.0014	648 628	648 628	() ()	5 79 5 77	66 54	0.13	0.0012	589	6		2 75 2 73	37	0.95	1.15	0.61	0.60
V13112111	3.61	0.03	0.16	0.37	0.65	0.14	0.0013	507	507	1	. 33	112	0.13	0.0012	485	5	1	L 33	78	0.99	0.62	0.30	0.46
V13112112 V23112111	3.73	0.03	0.15	0.35	0.69	0.13	0.0014	484 572	484 572	1	. 33 I. 40	99	0.14	0.0012	466 550	5	3	L 33 L 40	58	1.00	0.63	0.32	0.46
V23112112	3.40	0.03	0.17	0.37	0.72	0.13	0.0013	549	549	1	L 39	97	0.13	0.0012	531	6	1	L 39	67	1.00	0.68	0.35	0.48
V33112111	3.11	0.04	0.20	0.42	0.76	0.12	0.0012	622	622	2	2 46	96	0.12	0.0011	601	6		L 47 L 46	65	1.00	0.75	0.37	0.51
V43112111 V43112112	2.79	0.04	0.23	0.46	0.76	0.12	0.0012	733	733	3	3 56 3 55	<u>107</u> 94	0.12	0.0011	696 677	7		2 56 2 55	71 62	0.99	0.87	0.42	0.55
V53112111	2.56	0.05	0.28	0.52	0.80	0.12	0.0012	854	854	2	68	102	0.12	0.0011	782	8	-	2 66	64	0.96	1.07	0.51	0.59
V13112112	2.60	0.05	0.27	0.50	0.82	0.12	0.0012	689	689	- 1	+ 67 L 46	147	0.12	0.0011	664	7	1	2 64 L 46	108	1.00	0.57	0.52	0.45
V13112122	3.00	0.04	0.19	0.42	0.69	0.12	0.0012	656	656	1	45	129	0.12	0.0011	636	7	1	L 45	94	1.00	0.58	0.28	0.45
V23112121	2.00	0.04	0.23	0.44	0.03	0.12	0.0011	748	748	2	2 54	140	0.11	0.0010	728	8		2 54	93	1.00	0.63	0.30	0.48
V33112121 V33112122	2.45	0.05	0.25	0.48	0.73	0.11	0.0011	885	885	3	3 66 3 64	144	0.11	0.0010	856	9	1	2 66 2 64	105 91	1.00	0.69	0.31	0.51
V43112121	2.25	0.06	0.29	0.53	0.77	0.11	0.0011	1007	1007	3	3 78	141	0.11	0.0010	965	10		2 78	100	0.99	0.79	0.35	0.54
V43112122 V53112121	2.30	0.05	0.28	0.51	0.79	0.10	0.0011	974	9/4	5	5 95	124	0.11	0.0010	1089	10		2 <u>76</u> 393	8/	0.99	0.81	0.36	0.54
V53112122	2.10	0.06	0.33	0.59	0.82	0.11	0.0011	1134	1134	1	5 93	116	0.10	0.0010	1059	11		3 90	77	0.97	0.96	0.44	0.59
V13112211	3.37	0.03	0.18	0.33	0.03	0.13	0.0013	556	556	1	. 38	103	0.13	0.0012	540	6		L 38	74	1.00	0.61	0.23	0.45
V23112211 V23112212	2.99	0.04	0.20	0.42	0.72	0.12	0.0012	658 635	658 635	2	2 46 2 46	116 103	0.12	0.0011	638 618	7	1	L 46 L 45	83	1.00	0.65	0.32	0.48
V33112211	2.80	0.04	0.21	0.44	0.74	0.12	0.0012	724	724	2	2 53	116	0.12	0.0011	701	7	-	2 52	82	1.00	0.70	0.34	0.51
V33112212 V43112211	2.80	0.04	0.21	0.43	0.78	0.12	0.0012	852	852	3	2 54	102	0.12	0.0011	816	8		2 54 2 65	/1	0.99	0.73	0.36	0.51
V43112212	2.56	0.05	0.24	0.47	0.81	0.11	0.0011	830	830	3	3 64	99	0.11	0.0010	798	8		2 64	67	0.99	0.84	0.40	0.54
V53112211	2.30	0.05	0.30	0.55	0.82	0.11	0.0011	973	973	-	+ 79 1 78	94	0.11	0.0010	903	9		2 75	59	0.96	1.01	0.49	0.59
V13112221 V13112222	2.62	0.04	0.23	0.46	0.68	0.11	0.0011	797 764	797	2	2 54	155	0.11	0.0010	775	8		2 53	117	1.00	0.55	0.25	0.45
V23112221	2.40	0.05	0.25	0.48	0.72	0.11	0.0010	909	909	2	2 65	155	0.11	0.0010	887	9	1	2 65	117	1.00	0.59	0.26	0.47
V23112222 V33112221	2.46	0.05	0.24	0.46	0.75	0.11	0.0010	877 987	877 987	3	2 64 3 72	136	0.11	0.0010	859 963	9	1	2 64 2 72	102	1.00	0.60	0.28	0.47
V33112222	2.32	0.05	0.26	0.48	0.77	0.11	0.0010	955	955		3 71	135	0.11	0.0010	934	10	-	2 71	101	1.00	0.64	0.29	0.50
V43112221	2.10	0.06	0.30	0.54	0.77	0.10	0.0010	1077	1077	3	, 85 3 83	152	0.10	0.0009	1080	11	3	, 83 3 83	112	1.00	0.70	0.30	0.54
V53112221 V53112222	1.95	0.07	0.34	0.59	0.80	0.10	0.0010	1237	1237	1	1 98 1 97	145	0.10	0.0009	1187	12		3 97 3 96	104	0.99	0.80	0.35	0.58
V13121111	7.24	0.01	0.07	0.18	0.67	0.19	0.0021	245	245	1	24	57	0.19	0.0017	229	2		L 24	33	0.99	0.81	0.45	0.48
V13121112 V23121111	7.57 6.61	0.01	0.07	0.16	0.72	0.19	0.0021	227	227	1	24 29	46 57	0.19	0.0018	216	2	1	L 23 L 29	27	0.99	0.83	0.47	0.47
V23121112	6.86	0.01	0.07	0.18	0.76	0.19	0.0020	262	262	1	28	46	0.18	0.0017	251	3	1	28	27	1.00	0.89	0.51	0.50

Name	T1 [s]	Sa(T1)	Sa(T2)	Sa(T3)	DOC	Disp	ISDR [-]	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	Disp	ISDR [-13	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	C1 [-]	c2 [·]	C3 [-]	Mdy/
1/10104144	6.60	18	ß	8	0.77	(m)	0.004.0	[MN·m]	[MN]	[MN]	[MN]	[MN·m]	[m]2	0.0016	[MN·m]4	[MN]5	[MN]6	[MN]2	[MN-m]8		4.00	0.64	Mst [·]
V43121111 V43121112	5.67	0.02	0.11	0.25	0.77	0.18	0.0019	377 358	377 358	2	2 43 2 42	57	0.17	0.0016	349	3		1 42 1 41	3. 25	. 0.99 5 0.98	1.09	0.65	0.56
V53121111	5.04	0.02	0.13	0.31	0.80	0.17	0.0019	455	455	4	1 54 1 52	56	0.17	0.0016	398	4		2 50	28	0.94	1.31	0.80	0.62
V13121121	5.88	0.01	0.09	0.23	0.67	0.18	0.0019	337	337	1	. 34	75	0.17	0.0016	318	3		1 34	4	/ 0.99	0.73	0.40	0.47
V13121122 V23121121	6.16 5.37	0.01	0.08	0.20	0.72	0.18	0.0019	311 387	311 387	2	2 33	61 75	0.17	0.0016	298	3		<u>1 33</u> 1 41	3:	0.99 7 1.00	0.76	0.42	0.47
V23121122	5.59	0.02	0.09	0.22	0.76	0.17	0.0018	361	361	2	2 40	61	0.17	0.0015	348	4		1 40	38	3 1.00	0.81	0.46	0.49
V33121121 V33121122	4.91	0.02	0.12	0.27	0.74	0.17	0.0017	444 420	444	2	2 50 2 48	61	0.17	0.0015	422 403	4		2 49 2 48	33	7 1.00	0.87	0.49	0.52
V43121121	4.50	0.02	0.13	0.31	0.77	0.16	0.0017	505	505	3	58	74	0.16	0.0014	472	5		2 58	4	3 0.99	1.02	0.58	0.56
V53121122	4.04	0.02	0.15	0.29	0.81	0.15	0.0017	580	580	4	5 57 1 69	72	0.15	0.0014	519	ca c		2 56	39	0.99	1.05	0.39	0.56
V53121122	4.27 6.44	0.02	0.15	0.35	0.83	0.16	0.0017	558 291	558 291	4	1 68 29	59	0.15	0.0014	502	5		2 64 1 29	33	. 0.95	1.23	0.72	0.60
V13121212	6.69	0.01	0.08	0.18	0.75	0.18	0.0019	272	272	1	29	49	0.18	0.0017	263	3		1 29	29	0.99	0.79	0.45	0.47
V23121211 V23121212	5.86	0.01	0.09	0.21	0.74	0.18	0.0018	336 318	336 318	1	. 36 . 35	60 49	0.17	0.0016	322	3		1 <u>36</u> 135	30	1.00	0.82	0.47	0.49
V33121211	5.35	0.02	0.10	0.24	0.77	0.17	0.0018	388	388	2	2 43	60	0.17	0.0015	371	4		1 43	36	1.00	0.90	0.51	0.52
V33121212 V43121211	5.49 4.89	0.02	0.10	0.22	0.81	0.17	0.0018	450	450	4	2 42 3 51	49 60	0.17	0.0015	423	4		1 42 2 51	34	1.00	1.02	0.53	0.52
V43121212	5.00	0.02	0.11	0.26	0.83	0.17	0.0018	434	434	3	51	49	0.17	0.0015	411	4		2 50	28	3 0.99	1.03	0.60	0.55
V53121211 V53121212	4.4/	0.02	0.15	0.34	0.82	0.16	0.0018	511	527	4	+ 62 1 61	49	0.16	0.0015	4/1 460	5		2 59 2 58	24	1 0.95	1.25	0.74	0.60
V13121221	5.22	0.02	0.10	0.25	0.71	0.17	0.0017	404	404	1	L 42 41	80	0.16	0.0015	388	4		1 41 1 40	52	2 0.99	0.69	0.38	0.46
V23121221	4.72	0.02	0.10	0.22	0.74	0.16	0.0016	466	466	2	2 51	80	0.16	0.0013	450	5		2 51	52	2 1.00	0.75	0.43	0.49
V23121222	4.88	0.02	0.11	0.25	0.79	0.16	0.0016	444	444 528	2	2 50	65 79	0.16	0.0015	432	5		2 <u>50</u> 259	42	2 1.00	0.76	0.43	0.48
V33121222	4.43	0.02	0.12	0.28	0.81	0.15	0.0016	506	506	3	3 59	65	0.15	0.0014	490	5		2 59	40) 1.00	0.86	0.49	0.52
V43121221 V43121222	3.93	0.03	0.15	0.34	0.80	0.15	0.0015	603 581	603 581	4	1 70 1 69	78	0.14	0.0013	570	6		2 70 2 68	46	<u>) 0.99</u> 7 0.99	0.99	0.55	0.55
V53121221	3.58	0.03	0.19	0.40	0.83	0.14	0.0015	710	710	6	5 86	75	0.14	0.0013	638	7		3 81	4(0.95	1.20	0.67	0.60
V13122211	4.17	0.03	0.18	0.39	0.86	0.14	0.0015	556	556	1	> 84 L 36	126	0.14	0.0013	528	6		<u>3 80</u> 1 36		5 0.99	0.65	0.68	0.60
V13122112	4.30	0.02	0.13	0.31	0.68	0.15	0.0015	531	531	1	36	111	0.15	0.0014	509	5		1 35	74	0.99	0.66	0.35	0.46
V23122111 V23122112	3.82	0.03	0.15	0.33	0.69	0.14	0.0014	625	625	2	2 43 2 43	125	0.14	0.0013	596	6		1 43 1 42	7:	3 1.00	0.71	0.38	0.49
V33122111	3.51	0.03	0.17	0.37	0.72	0.13	0.0014	703 678	703	2	2 51	123	0.13	0.0012	671	7		2 51	8	1.00	0.79	0.41	0.52
V43122111	3.29	0.03	0.19	0.40	0.75	0.13	0.0013	771	771	-	58	121	0.13	0.0012	731	8		2 58	78	3 0.99	0.88	0.45	0.55
V43122112 V53122111	3.36	0.03	0.18	0.38	0.77	0.13	0.0014	747	747		3 57 1 69	107	0.13	0.0012	711 812	7		2 <u>57</u> 2 67	68	0.99	0.89	0.46	0.55
V53122112	3.10	0.04	0.22	0.44	0.80	0.13	0.0013	856	856	4	1 68	104	0.13	0.0012	792	8		2 66	6	0.97	1.06	0.55	0.59
V13122121 V13122122	3.40	0.03	0.18	0.39	0.64	0.13	0.0013	736	736	2	2 <u>48</u> 2_47	165	0.13	0.0012	675	7		1 48 1 47	11.	0.99	0.60	0.28	0.46
V23122121	3.13	0.03	0.19	0.41	0.68	0.13	0.0012	825	825	2	2 57	163	0.12	0.0011	794	8		2 57	110	<u>i 1.00</u>	0.64	0.31	0.48
V33122122 V33122121	2.89	0.03	0.18	0.39	0.71	0.13	0.0012	922	922	3	2 50 3 67	144	0.12	0.0011	765 888	g		2 56 2 67	10.	1.00	0.65	0.32	0.48
V33122122	2.97	0.04	0.20	0.42	0.75	0.12	0.0012	887	887	00.00	66	142	0.12	0.0011	859	9 10		2 66	99	1.00	0.71	0.35	0.51
V43122121 V43122122	2.08	0.04	0.23	0.40	0.73	0.12	0.0012	999	999	9	3 77	140	0.12	0.0010	960	10		2 77	95	5 1.00	0.80	0.39	0.54
V53122121 V53122122	2.48	0.05	0.27	0.50	0.78	0.11	0.0012	1172	1172	5	5 93 5 101	155	0.11	0.0010	1101	11		<u>3 91</u> 3 97	103	2 0.98	0.94	0.43	0.58
V13122211	3.78	0.03	0.15	0.35	0.68	0.14	0.0014	633	633	1	42	132	0.14	0.0013	607	6		1 42	9	0.99	0.63	0.32	0.46
V13122212 V23122211	3.89	0.03	0.14	0.33	0.71	0.14	0.0014	607 716	607 716	1	2 50	116	0.14	0.0013	587	7		1 41 2 50	80 90	0.99	0.64	0.34	0.46
V23122212	3.54	0.03	0.16	0.35	0.74	0.13	0.0013	691	691	2	2 49	115	0.13	0.0012	670	7		2 49	79	1.00	0.70	0.37	0.48
V33122211 V33122212	3.16	0.03	0.19	0.40	0.75	0.13	0.0013	786	786	2	2 58 2 58	129	0.13	0.0011	780	8		2 59 2 58	76	5 1.00	0.77	0.39	0.51
V43122211	2.90	0.04	0.22	0.44	0.78	0.12	0.0012	924	924	9	3 70	127	0.12	0.0011	879	01		2 70	83	0.99	0.89	0.44	0.55
V53122212	2.65	0.04	0.21	0.40	0.80	0.12	0.0012	1080	1080	5	5 86	112	0.12	0.0011	988	10		3 82	73	0.96	1.09	0.40	0.60
V53122212	2.69	0.04	0.26	0.49	0.83	0.12	0.0012	1056	1056	5	5 85 58	106	0.12	0.0011	968	10	(<u> </u>	3 81 2 58	63 1.26	0.96	1.09	0.54	0.59
V13122222	3.12	0.03	0.18	0.40	0.71	0.12	0.0012	827	827	2	2 57	153	0.12	0.0011	804	8		2 57	110) 1.00	0.60	0.29	0.45
V23122221 V23122222	2.84	0.04	0.21	0.44	0.71	0.12	0.0011	948 912	948 912	2	2 66 2 65	173	0.12	0.0011	920	10		2 66 2 65	120	<u>) 1.00</u>) 1.00	0.62	0.29	0.48
√33122221	2.62	0.05	0.23	0.46	0.74	0.11	0.0011	1063	1063	3	3 78	171	0.11	0.0010	1033	11		2 78	124	1.00	0.67	0.32	0.51
V33122222 V43122221	2.68	0.04	0.22	0.44	0.77	0.12	0.0011	1028	1028	4	3 // 7 91	151	0.11	0.0010	1002	10		<u>2 //</u> 3 90	10) <u>1.00</u>) <u>1.00</u>	0.68	0.33	0.51
V43122222	2.47	0.05	0.25	0.47	0.79	0.11	0.0011	1158	1158	4	1 89	148	0.11	0.0010	1123	12		3 89	104	1.00	0.75	0.36	0.54
V53122221 V53122222	2.25	0.06	0.29	0.53	0.80	0.11	0.0011	1348	1348	5	5 106	164	0.11	0.0010	1284	13		3 105 3 103	97	7 0.99 7 0.99	0.88	0.39	0.58
V13211111	9.64	0.01	0.05	0.13	0.66	0.22	0.0019	161	161	1	L 16	39	0.22	0.0015	149	1		0 16	22	. 0.99	0.83	0.51	0.49
V23211111	8.78	0.01	0.04	0.12	0.72	0.21	0.0018	183	183	1	L 19	39	0.21	0.0013	170	1		1 19	2.	L 1.00	0.93	0.56	0.51
V23211112 V33211111	9.12	0.01	0.05	0.13	0.75	0.22	0.0018	210	210	1	. 18	31	0.21	0.0014	162	1		1 18 1 22	2	1.00	0.93	0.58	0.50
V33211112	8.30	0.01	0.06	0.15	0.78	0.21	0.0018	197	197	1	22	32	0.20	0.0014	186	1		1 22	1	1.00	1.07	0.65	0.53
V43211111 V43211112	7.36	0.01	0.08	0.18	0.76	0.20	0.0017	244	244	2	2 27 2 26	40 33	0.19	0.0013	221	2		1 27 1 26	20	0.98 0.98 0.98	1.25	0.73	0.58
V53211111	6.72	0.01	0.10	0.23	0.79	0.19	0.0017	298	298		34	40	0.19	0.0013	250	2		1 32	18	3 0.92	1.48	0.93	0.64
V13211112	7.85	0.01	0.09	0.22	0.82	0.20	0.0017	285	285	1	s 34 L 22	53	0.19	0.0013	242	2		1 31	30	0.91	0.83	0.94	0.48
V13211122	8.21	0.01	0.06	0.15	0.71	0.20	0.0017	201	201	1	. 21	42	0.20	0.0014	190	2		1 21	24	0.99	0.83	0.49	0.48
V23211121 V23211122	7.45	0.01	0.07	0.16	0.75	0.19	0.0016	243	249	1	20	43	0.19	0.0013	235	2		1 25	24	1.00	0.90	0.51	0.50
V33211121	6.55	0.01	0.08	0.20	0.73	0.19	0.0015	285	285	1	31	53	0.18	0.0012	267	2		1 31 1 30	29	1.00	1.00	0.57	0.53
V43211121	6.00	0.01	0.10	0.23	0.76	0.18	0.0015	331	331	2	2 38	52	0.18	0.0012	304	2		1 37	28	3 0.98	1.14	0.66	0.57
V43211122 V53211121	6.18 5.49	0.01	0.09	0.21	0.80	0.18	0.0015	314	314 401	2	37	43	0.18	0.0012	291 346	2		1 <u>36</u> 144	22	0.98	1.15	0.68	0.56
V53211122	5.63	0.01	0.12	0.27	0.83	0.18	0.0015	383	383		3 46	42	0.18	0.0012	333	3		1 43	20) 0.93	1.37	0.86	0.62
V13211211 V13211212	8.56	0.01	0.06	0.14	0.70	0.21	0.0017	190 177	190 177	1	19	41	0.20	0.0014	178 170	1		1 <u>19</u> 1 18	23	0.99	0.83	0.49	0.48
V23211211	7.77	0.01	0.07	0.16	0.73	0.20	0.0017	218	218	1	23	42	0.19	0.0013	206	2		1 23	23	3 1.00	0.93	0.54	0.50
V23211212 V33211211	8.02	0.01	0.06	0.14	0.78	0.19	0.0017	206	206	1	23	34	0.19	0.0013	197	2		1 <u>22</u> 1 <u>27</u>	23	1.00 3 1.00	1.04	0.60	0.50
V33211212	7.28	0.01	0.07	0.16	0.80	0.20	0.0016	239	239	1	27	35	0.19	0.0013	228	2		1 27	18	3 1.00	1.05	0.61	0.53
v43211211 V43211212	6.62	0.01	0.09	0.20	0.79	0.19	0.0016	294	294	2	<u>. 33</u> 2 <u>3</u> 33	42	0.19	0.0012	2/1 262	2		1 <u>33</u> 1 <u>32</u>	2.	0.98 70.98	1.18	0.70	0.57
V53211211	5.89	0.01	0.11	0.26	0.81	0.18	0.0016	358	358	З	3 42	42	0.18	0.0013	309	2		1 39	19	0.93	1.40	0.88	0.63

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [-]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [-]3	Total Moment [MN∙m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN-m]8	C1 [·]	∞[·]	C3 [·]	Mdy/ Mst [·]
V13211221 V13211222	6.93 7.20	0.01	0.08	0.18	0.70	0.19	0.0015	261 244	261 244		L 27 L 26	56 45	0.18	0.0013	247	2		1 26 1 26	33 27	0.99	0.80	0.45	0.47
V23211221	6.29	0.01	0.08	0.20	0.74	0.18	0.0015	301	301	j.	L 32	56	0.18	0.0012	287	2		1 32	33	1.00	0.85	0.48	0.50
V33211222	5.74	0.01	0.09	0.18	0.78	0.18	0.0013	347	347		2 39	56	0.18	0.0012	: 330	3		1 32	32	1.00	0.94	0.54	0.43
V33211222 V43211221	5.91	0.01	0.09	0.20	0.80	0.18	0.0014	330 406	330 406		2 <u>38</u> 347	46 56	0.17	0.0012	<u>317</u> 379	3		1 <u>38</u> 146	26	0.99	0.95	0.55	0.52
V43211222	5.37	0.02	0.11	0.24	0.83	0.17	0.0014	389	389		3 46 1 59	46	0.17	0.0012	366	3		1 45	25	0.98	1.08	0.63	0.56
V53211221 V53211222	4.88	0.02	0.13	0.31	0.85	0.17	0.0014	472	472		4 57	45	0.17	0.0012	420	3		2 54	27	0.93	1.30	0.80	0.61
V13212111 V13212112	5.56 5.74	0.02	0.10	0.25	0.64	0.17	0.0014	371 350	371 350		L 24 L 23	89	0.17	0.0012	347	3		1 24 1 23	56 49	0.99	0.71	0.38	0.47
V23212111	5.09	0.02	0.11	0.27	0.68	0.17	0.0013	424	424	Ĵ	L 29	89	0.17	0.0011	401	3		1 29	56	1.00	0.76	0.42	0.50
V33212112	4.68	0.02	0.12	0.30	0.72	0.16	0.0013	476	476	-	L 34	88	0.16	0.0011	. 450	4		1 34	55	1.00	0.86	0.48	0.52
V33212112 V43212111	4.79	0.02	0.12	0.28	0.75	0.16	0.0013	460 539	460 539		L 34 2 40	78	0.16	0.0011	437	3		1 <u>34</u> 140	48	1.00	0.87	0.49	0.52
V43212112	4.39	0.02	0.14	0.32	0.77	0.16	0.0012	522	522		2 40	77	0.15	0.0010	488	4	_	1 39	45	0.99	1.02	0.57	0.56
V53212111	4.01	0.02	0.17	0.37	0.80	0.15	0.0012	613	613		3 49	73	0.15	0.0010	545	4		1 46	39	0.95	1.24	0.03	0.61
V13212121 V13212122	4.47	0.02	0.13	0.32	0.64	0.16	0.0012	507 483	507 483		L 33 L 33	117 103	0.15	0.0011	. 480	4		1 <u>33</u> 132	78	0.99	0.66	0.34	0.47
V23212121	4.10	0.02	0.14	0.34	0.68	0.15	0.0011	569	569		L 40	116	0.14	0.0010	542	4		1 40	77	1.00	0.72	0.38	0.49
V33212122	3.76	0.02	0.14	0.36	0.71	0.13	0.0011	638	638		2 47	102	0.13	0.0010	607	5		1 39	75	1.00	0.75	0.40	0.43
V33212122 V43212121	3.86 3.46	0.03	0.15	0.34	0.75	0.14	0.0011	615 723	615 723		2 46 3 55	101	0.14	0.0009	589 679	5		1 46 2 55	65 70	1.00	0.82	0.44	0.52
V43212122	3.54	0.03	0.17	0.37	0.78	0.14	0.0011	699	699		3 54	99	0.13	0.0009	660	5		2 54	61	0.99	0.95	0.50	0.55
V53212121 V53212122	3.17	0.03	0.22	0.45	0.79	0.13	0.0010	843 820	843		+ 68 4 66	107	0.13	0.0009	739	6		2 64	54	0.95	1.15	0.62	0.60
V13212211 V13212212	5.04	0.02	0.11	0.27	0.67	0.17	0.0013	430	430		L 28	94	0.16	0.0011	. 408	3		1 <u>28</u> 1 27	61 53	0.99	0.68	0.37	0.47
V23212211	4.61	0.02	0.12	0.29	0.71	0.16	0.0012	484	484		L 33	93	0.15	0.0011	462	4		1 33	60	1.00	0.75	0.41	0.49
V23212212 V33212211	4.72	0.02	0.12	0.27	0.74	0.15	0.0012	467 545	467 545		L 33 2 39	82 92	0.15	0.0011	. 449 1 519	4		1 <u>33</u> 139	53	1.00	0.75	0.42	0.49
V33212212 V43212211	4.31	0.02	0.13	0.30	0.77	0.15	0.0012	528	528		2 39 2 47	81	0.15	0.0010	507	4		1 <u>39</u> 1 <u>4</u> 5	51	1.00	0.85	0.47	0.52
V43212211	3.94	0.02	0.15	0.34	0.79	0.15	0.0011	603	603		2 46	80	0.14	0.0010	569	4		1 46	48	0.99	0.99	0.55	0.55
V53212211 V53212212	3.54	0.03	0.19	0.42	0.80	0.14	0.0011	725	725	4	4 57 4 56	87	0.14	0.0010	649 637	5		2 54 2 53	48	0.95	1.19	0.66	0.60
V13212221	4.04	0.02	0.14	0.34	0.67	0.14	0.0011	579	579		L 38	123	0.14	0.0010	554	4		1 38	84	0.99	0.65	0.33	0.46
V23212221	3.69	0.03	0.16	0.35	0.71	0.14	0.0010	654	654		2 46	122	0.14	0.0009	629	5		1 46	83	1.00	0.70	0.37	0.48
V23212222 V33212221	3.78	0.03	0.15	0.34	0.74	0.14	0.0010	630 738	630 738		2 45 2 54	107 120	0.14	0.0009	609 709	5		1 45 2 54	72	1.00	0.71	0.38	0.48
V33212222	3.45	0.03	0.17	0.36	0.77	0.13	0.0010	715	715		2 54	106	0.13	0.0009	690	5		2 54	70	1.00	0.79	0.42	0.51
V43212221 V43212222	3.16	0.04	0.20	0.42	0.78	0.13	0.0010	817	817		5 04 3 64	110	0.13	0.0009	737	6		2 63	66	0.99	0.90	0.40	0.55
V53212221 V53212222	2.84	0.04	0.25	0.48	0.81	0.12	0.0010	982 958	982 958	2	5 79 5 77	112 99	0.12	0.0009	894	7		2 75 2 74	67	0.96	1.11	0.56	0.60
V13221111	11.13	0.01	0.04	0.11	0.67	0.25	0.0021	182	182		L 18	43	0.25	0.0017	168	1		1 18	24	0.99	0.79	0.50	0.49
V13221112 V23221111	10.14	0.01	0.03	0.10	0.72	0.28	0.0021	1/2	1/2		L 18 L 20	43	0.25	0.0018	162	1		1 18	20	1.00	0.92	0.52	0.48
V23221112	10.53 9.28	0.01	0.04	0.11	0.75	0.24	0.0020	189	189 226		L 20	35	0.23	0.0016	208	1		1 20 1 24	19	1.00	0.90	0.59	0.50
V33221112	9.58	0.01	0.05	0.12	0.77	0.22	0.0019	213	213		L 24	35	0.22	0.0015	200	2		1 24	18	1.00	1.07	0.68	0.53
V43221111 V43221112	8.49	0.01	0.06	0.16	0.75	0.22	0.0019	264 250	264		2 29 2 28	45	0.21	0.0014	236	2		1 <u>28</u> 1 28	21	0.98	1.31	0.80	0.58
V53221111 V53221112	7.76	0.01	0.08	0.20	0.78	0.21	0.0018	324	324		3 37 3 36	45	0.20	0.0014	267	2		1 34	19	0.91	1.56	0.98	0.65
V13221121	9.06	0.01	0.05	0.14	0.66	0.22	0.0018	235	235	-	L 23	58	0.21	0.0014	217	2		1 23	32	0.99	0.83	0.49	0.49
V13221122 V23221121	9,48	0.01	0.05	0.13	0.71	0.22	0.0018	217	217		L 23 L 28	46 58	0.21	0.0015	204	2		1 22	26	1.00	0.83	0.51	0.48
V23221122	8.60	0.01	0.06	0.14	0.75	0.21	0.0017	249	249		L 27	47	0.20	0.0014	236	2		1 27 1 33	26	1.00	0.93	0.57	0.50
V33221122	7.83	0.01	0.07	0.16	0.77	0.20	0.0017	288	288		2 32	48	0.20	0.0013	271	2		1 32	25	1.00	1.08	0.63	0.53
V43221121 V43221122	6.93 7.14	0.01	0.08	0.20	0.75	0.19	0.0016	356 337	356 337		2 40 2 39	59 48	0.19	0.0013	323 309	3		1 40 1 38	30 24	0.98	1.21	0.71	0.57
V53221121	6.33	0.01	0.10	0.25	0.79	0.19	0.0016	432	432		4 51 1 50	58	0.19	0.0013	366	3		2 47	26	0.92	1.44	0.90	0.63
V13221211	9.88	0.01	0.05	0.12	0.70	0.22	0.0019	205	205		L 20	45	0.22	0.0015	192	2		1 20	25	0.99	0.83	0.52	0.48
V13221212 V23221211	10.26	0.01	0.04	0.11	0.75	0.23	0.0019	194 235	194 235		L 20 L 25	36	0.22	0.0016	185 221	2		1 20	21	1.00	0.81	0.53	0.48
V23221212	9.27	0.01	0.05	0.12	0.78	0.22	0.0018	221	221		L 24	37	0.21	0.0014	212	2		1 24	20	1.00	0.92	0.59	0.50
V33221211	8.41	0.01	0.06	0.13	0.80	0.21	0.0018	257	257		2 29	38	0.20	0.0014	235	2		1 29	20	1.00	1.00	0.66	0.53
V43221211 V43221212	7.47	0.01	0.07	0.18	0.78	0.20	0.0017	316	316		2 <u>35</u> 235	48	0.20	0.0013	288	2		1 <u>35</u> 134	23	0.98	1.26	0.74	0.57
V53221211	6.81	0.01	0.09	0.22	0.81	0.20	0.0017	387	387	4	4 45	47	0.19	0.0013	328	3		1 41	20	0.92	1.48	0.94	0.64
V13221222	8.00	0.01	0.05	0.16	0.70	0.20	0.0017	280	280	-	L 28	62	0.20	0.0014	263	2		1 28	35	0.99	0.83	0.48	0.48
V13221222 V23221221	8.31	0.01	0.05	0.14	0.75	0.20	0.0017	261 322	261 322		L 28 L 34	50	0.20	0.0014	250	2		1 28 1 34	28	0.99	0.82	0.50	0.47
V23221222	7.51	0.01	0.07	0.16	0.78	0.20	0.0016	304	304		2 34	51	0.19	0.0013	291	2		1 34	28	1.00	0.93	0.54	0.50
V33221221	6.82	0.01	0.08	0.18	0.80	0.19	0.0015	353	353		2 40	51	0.18	0.0012	337	3		1 40	28	1.00	1.02	0.59	0.53
V43221221 V43221222	6.06 6.20	0.01	0.09	0.22	0.79	0.18	0.0015	433 415	433 415		3 50 3 49	62 51	0.18	0.0012	401 387	3		2 49 2 48	33	0.98	1.14	0.67	0.57
V53221221	5.52	0.02	0.12	0.28	0.81	0.18	0.0015	527	527		5 63	62	0.18	0.0012	457	4		2 58	29	0.93	1.36	0.85	0.62
v53221222 V13222111	5.63	0.01	0.11	0.27	0.84	0.18	0.0015	508 396	508 396		> 61 L 25	51 99	0.18	0.0012	444 367	3		2 57 1 25	23	0.93	1.37 0.76	0.86	0.62
V13222112 V23222111	6.62 5.88	0.01	0.08	0.20	0.67	0.19	0.0015	375 451	375		L 25	86 98	0.18	0.0013	351	3		1 25	51	0.99	0.78	0.43	0.48
V23222112	6.04	0.01	0.09	0.22	0.71	0.18	0.0014	430	430		L 30	86	0.18	0.0012	406	3		1 30	52	1.00	0.83	0.47	0.50
V33222111 V33222112	5.40 5.53	0.02	0.11	0.25	0.71	0.17	0.0014	515 493	515 493		2 37 2 36	98	0.17	0.0012	483	4		1 37 1 36	59	1.00	0.90	0.51	0.53
V43222111 V43222112	4.96	0.02	0.12	0.29	0.74	0.17	0.0014	594 574	594 574		2 44 2 44	98	0.17	0.0011	549	4		1 44 1 43	56 49	0.99	1.04	0.60	0.56
V53222111	4.55	0.02	0.15	0.35	0.78	0.16	0.0013	697	697		4 54	95	0.16	0.0011	609	5		2 51	50	0.94	1.28	0.76	0.62
v53222112 V13222121	4.63 5.16	0.02	0.15	0.34	0.80	0.17	0.0013	556	556		+ 54 L 36	84 132	0.16 0.17	0.0011	596	4		2 50 1 36	43	0.94	0.68	0.37	0.61
V13222122	5.33	0.02	0.10	0.25	0.67	0.17	0.0013	524	524		L 35	115	0.17	0.0012	497	4		1 35	74	0.99	0.70	0.38	0.47

Name	T1 [s]	Sa(T1)	Sa(T2) [g]	Sa(T3) [g]	DOC [-]	Disp [m]	ISDR [-]	Total Moment	Total Shear	Max CB Shear	Max Pp [MN]	Max wall moment	Disp [m]2	ISDR [-]3	Total Moment	Total Shear	Max CB Shear	Max Pp [MN]2	Max wall moment	C1 [·]	C2[·]	C3 [·]	Mdy/ Mst [-]
V33222121	4.35	0.02	0.14	0.32	0.72	0.15	0.0012	[MN•m] 701	[MN] 701	[MN] 2	2 51	[MN-m] 129	0.15	0.0010	[MN-m]4 663	[MN]5 5	[MN]6	2 51	[MN·m]8 81	. 1.00	0.85	0.46	0.52
V33222122 V43222121	4.46	0.02	0.13	0.30	0.74	0.16	0.0012	675 793	675 793	2	2 50 3 60	114 127	0.15	0.0010	643 739	5		2 <u>50</u> 260	71	. 1.00	0.85	0.48	0.52
V43222122	4.08	0.02	0.15	0.34	0.77	0.15	0.0012	767	767	3	3 59 5 74	112	0.15	0.0010	719	6		2 59	67	0.99	1.00	0.55	0.56
V53222121	3.73	0.03	0.13	0.40	0.80	0.14	0.0012	900	900	1	5 72	107	0.14	0.0010	803	6		2 69	58	0.95	1.22	0.68	0.61
V13222211 V13222212	5.82	0.01	0.09	0.23	0.66	0.18	0.0014	457	457 435	1	. 30 . 29	104 91	0.17	0.0012	431 414	3		1 29 1 29	56	0.99 0.99	0.73	0.40	0.47
V23222211	5.32	0.02	0.10	0.25	0.70	0.17	0.0013	525 503	525 503	1	36	104	0.17	0.0011	499 482	4		<u>1 36</u> 1 35	65	5 <u>1.00</u> 7 <u>1.00</u>	0.78	0.44	0.49
V33222211	4.87	0.02	0.12	0.28	0.74	0.17	0.0013	600	600	2	2 43	104	0.16	0.0011	569	4		1 43	64	1.00	0.87	0.49	0.52
V33222212 V43222211	4.97	0.02	0.11	0.26	0.77	0.17	0.0013	581 681	581	3	2 43 3 51	92	0.16	0.0011	635	4		1 43 2 50	56) 1.00) 0.99	0.87	0.50	0.52
V43222212 V53222211	4.55	0.02	0.13	0.30	0.79	0.16	0.0013	662 798	662 798	3	3 50 1 62	91 98	0.16	0.0011	622 707	5		2 <u>50</u> 2 59	52	2 0.99	1.02	0.59	0.56
V53222212	4.15	0.02	0.16	0.36	0.82	0.15	0.0012	781	781	4	1 61	87	0.15	0.0011	694	5		2 58	45	0.94	1.25	0.73	0.61
V13222221 V13222222	4.66	0.02	0.12	0.30	0.87	0.16	0.0012	637 610	637	1	L 42 L 42	139	0.16	0.0011	586	5		1 42 1 41	94	0.99	0.67	0.36	0.46
V23222221 V23222222	4.26	0.02	0.13	0.32	0.71	0.15	0.0011	717 690	717 690	2	2 50 2 50	138 121	0.15	0.0010	686 665	5		2 50 2 49	90 79) <u>1.00</u>) <u>1.00</u>	0.73	0.40	0.49
V33222221	3.91	0.03	0.15	0.34	0.74	0.14	0.0011	807 782	807 782	0	3 59 2 58	136	0.14	0.0010	771	6		2 59	87	1.00	0.82	0.44	0.52
V43222221	3.58	0.02	0.17	0.37	0.77	0.14	0.0011	917	917	3	3 70	133	0.14	0.0010	864	7		2 69	82	2 0.99	0.95	0.40	0.55
V43222222 V53222221	3.65 3.28	0.03	0.17	0.36	0.80	0.14	0.0011	891 1073	891 1073		3 69 5 85	117 127	0.14	0.0009	844 966	7		2 <u>68</u> 381	71	. 0.99	0.96	0.52	0.55
V53222222 M11111111	3.33	0.03	0.21	0.43	0.82	0.13	0.0011	1048 426	1048 426	5	5 84 1 44	112	0.13	0.0009	947	7		3 80 4 44	63	3 0.95	1.17	0.64	0.60
M11111112	0.75	0.20	0.60	0.71	0.75	0.04	0.0011	406	406	2	1 44	55	0.04	0.0011	404	13		4 44	50	1.00	0.35	0.10	0.50
M21111111 M21111112	0.68	0.23	0.63	0.71	0.75	0.04	0.0010	467 452	467 452	20 20	> 51 5 51	64 53	0.04	0.0010	464 450	14		5 51 5 51	58 48	3 1.00 3 1.00	0.35	0.10	0.53
MB1111111 MB1111112	0.60	0.26	0.68	0.71	0.78	0.03	0.0009	514 495	514 495	6	59	61 50	0.03	0.0009	510 492	16 15		6 <u>59</u> 5 58	55	5 1.00 5 1.00	0.38	0.09	0.57
M41111111	0.55	0.28	0.71	0.71	0.81	0.03	0.0009	561	561	3	7 67	57	0.03	0.0008	556	17		6 67	51	1.00	0.41	0.10	0.61
M51111112	0.57	0.27	0.70	0.71	0.84	0.03	0.0009	601	601	8	3 75	47 49	0.03	0.0009	593	17		7 74	42	0.99	0.42	0.11	0.65
M51111112 M11111121	0.52	0.30	0.71	0.71	0.87	0.03	0.0008	589 532	589 532	5	3 73 5 56	41 83	0.03	0.0008	581 530	18 16		7 <u>73</u> 5556	36	0.99 1.00	0.45	0.12	0.65
M11111122	0.61	0.25	0.66	0.71	0.75	0.03	0.0009	507	507	e e	5 55	69	0.03	0.0009	505	16		5 <u>55</u> 6 65	64	1.00	0.31	0.08	0.50
M21111122	0.56	0.28	0.69	0.71	0.79	0.03	0.0008	563	563	6	5 64	66	0.03	0.0008	561	17		<u>6 64</u>	61	. 1.00	0.32	0.08	0.53
MB1111121 MB1111122	0.49	0.31	0.71	0.71	0.78	0.03	0.0007	63U 611	63U 611	1	7 73 7 72	/4 62	0.03	0.0007	608	19 19	-	7 73 7 72	57	1.00 1.00	0.32	0.08	0.57
M41111121 M41111122	0.45	0.34	0.71	0.71	0.81	0.03	0.0007	673 658	673 658	8	81 8 81	67 56	0.03	0.0007	668 654	20 20		8 81 8 81	63 52	1.00 1.00	0.34	0.08	0.61
M51111121	0.41	0.36	0.71	0.71	0.84	0.02	0.0006	708	708	2	90	57	0.02	0.0006	702	21	1	8 89	53	1.00	0.37	0.09	0.65
M1111122	0.45	0.35	0.66	0.71	0.87	0.02	0.0010	481	481		5 51	40 68	0.02	0.0007	479	15	1	o oo 5 51	63	1.00	0.30	0.10	0.50
M11111212 M21111211	0.66	0.23	0.63	0.71	0.77	0.04	0.0010	466 538	466 538	6	5 51 5 59	57	0.04	0.0010	465 536	14		5 <u>51</u> 6 59	52	<u>1.00</u> 1.00	0.33	0.09	0.50
M21111212	0.60	0.26	0.67	0.71	0.80	0.03	0.0009	520 587	520	6	5 59	55	0.03	0.0009	519 584	16 18		6 <u>59</u> 6 67	50) 1.00	0.34	0.09	0.53
MB1111212	0.55	0.28	0.70	0.71	0.83	0.03	0.0008	573	573	-	7 67	52	0.03	0.0008	571	18		6 67	47	1.00	0.36	0.09	0.57
M41111211 M41111212	0.49	0.32	0.71	0.71	0.83	0.03	0.0008	632 619	632 619	2	s 75 3 74	57	0.03	0.0007	616	19 19		7 75 7 74	52 43	2 1.00 3 1.00	0.37	0.10	0.61
M51111211 M51111212	0.45	0.34	0.71	0.71	0.86	0.02	0.0007	673 663	673 663	0	83	49 41	0.02	0.0007	666 656	20 20	1	8 83 8 82	44	1.00 1.00	0.39	0.10	0.65
M11111221	0.52	0.30	0.71	0.71	0.73	0.03	0.0008	604	604	6	65	85	0.03	0.0008	602	19		6 65	80	1.00	0.27	0.06	0.49
M21111222	0.34	0.33	0.89	0.71	0.77	0.03	0.0008	658	658	i i	7 74	81	0.03	0.0008	656	20		7 74	76	5 1.00	0.29	0.07	0.53
M21111222 M31111221	0.49	0.32	0.71	0.71	0.80	0.03	0.0007	641 703	641 703	5	7 74 3 82	68 74	0.03	0.0007	640 701	20	1	7 74 8 82	63 70	3 1.00) 1.00	0.30	0.07	0.53
MB1111222	0.44	0.34	0.71	0.71	0.83	0.02	0.0007	690 741	690 741	8	3 82 90	62	0.02	0.0006	688	21	1	8 82 8 89	58	3 1.00 2 1.00	0.31	0.08	0.57
M41111222	0.41	0.37	0.71	0.71	0.85	0.02	0.0006	731	731	4	89	56	0.02	0.0006	727	22		9 89	52	1.00	0.32	0.08	0.61
M51111221 M51111222	0.36	0.40	0.71	0.71	0.86	0.02	0.0005	787	787	10) 99	48	0.02	0.0006	781	24		9 98 9 97	53	1.00 I.00	0.33	0.09	0.65
M11112111 M11112112	0.42	0.36	0.71	0.71	0.68	0.02	0.0006	720	720	4	49 1 49	120 108	0.02	0.0006	717	22		4 49 4 49	115	5 <u>1.00</u> 3 <u>1.00</u>	0.21	0.05	0.49
M21112111	0.38	0.38	0.71	0.71	0.72	0.02	0.0005	764	764		5 55	111	0.02	0.0005	762	23		5 55	107	1.00	0.23	0.05	0.52
MB1112111	0.35	0.41	0.71	0.71	0.75	0.02	0.0005	819	819	6	5 62	100	0.02	0.0005	816	25	1	6 62	99	1.00	0.24	0.06	0.56
M31112112 M41112111	0.36	0.40	0.71	0.71	0.78	0.02	0.0005	805	805	ب ت	5 62 7 69	92	0.02	0.0005	802	25		6 68	88) 1.00	0.25	0.06	0.56
M41112112 M51112111	0.33	0.43	0.71	0.71	0.81	0.02	0.0005	854 903	854 903	5	7 <u>68</u> 775	83	0.02	0.0005	851 898	26 27		6 <u>68</u> 775	79	1.00 1.00	0.27	0.07	0.60
M51112112	0.30	0.46	0.71	0.71	0.85	0.02	0.0004	894	894		7 75	71	0.02	0.0004	889	27		7 74	67	1.00	0.29	0.08	0.65
M11112121	0.35	0.43	0.71	0.71	0.68	0.02	0.0005	835	835	5	5 59	142	0.02	0.0005	833	26		5 59	137	3 1.00	0.18	0.04	0.49
M21112121 M21112122	0.31	0.45	0.71	0.71	0.72	0.02	0.0004	904 891	904 891	6	5 66 5 66	132 118	0.02	0.0004	902	28		6 <u>66</u> 666	128	3 1.00 1 1.00	0.19	0.04	0.52
MB1112121 MB1112122	0.28	0.48	0.71	0.71	0.75	0.01	0.0004	947	947	3	7 72	120	0.01	0.0004	945	29		7 72 7 73	117	1.00 L	0.20	0.05	0.56
M41112121	0.26	0.50	0.71	0.71	0.79	0.01	0.0003	983	983	ŧ	3 79	100	0.01	0.0003	980	30		7 79	103	3 1.00	0.22	0.05	0.60
M41112122 M51112121	0.27	0.49	0.71	0.71	0.81	0.01	0.0003	974 1041	9/4	2	3 79 9 88	95	0.01	0.0003	9/1 1037	30	1	7 <u>79</u> 887	92 89	2 1.00 9 1.00	0.23	0.06	0.60
M51112122 M11112211	0.24	0.52	0.71	0.71	0.84	0.01	0.0003	1027 774	1027 774	5	87	81	0.01	0.0003	1022	31	1	8 <u>87</u> 5553	78	<u>1.00</u>	0.25	0.07	0.64
M11112212	0.39	0.38	0.71	0.71	0.73	0.02	0.0006	758	758	ţ	5 53	107	0.02	0.0006	756	23		5 53	103	3 1.00	0.22	0.05	0.49
M21112212	0.34	0.42	0.71	0.71	0.74	0.02	0.0005	835	835	6	5 60	113	0.02	0.0005	833	25		6 60	105	2 1.00	0.21	0.05	0.52
M31112211 M31112212	0.32	0.44	0.71	0.71	0.77	0.02	0.0004	885 875	885	6	67 67	104 93	0.02	0.0004	883 873	27	6	6 67 6 67	100) <u>1.00</u>) <u>1.00</u>	0.22	0.05	0.56
M41112211	0.29	0.47	0.71	0.71	0.81	0.01	0.0004	927	927	1	7 73	92	0.01	0.0004	923	28		7 73	89	1.00	0.24	0.06	0.60
M51112211	0.27	0.49	0.71	0.71	0.84	0.01	0.0004	959	959	5	3 79	78	0.01	0.0004	954	29		7 79	75	5 1.00	0.23	0.07	0.65
M11112212	0.27	0.49	0.71	0.71	0.86 0.70	0.01	0.0004	953 916	953 916	6	5 63	69 142	0.01	0.0004	948 914	29 28		78 6 63	66 138	5 1.00 3 1.00	0.27	0.07	0.65
M11112222 M21112221	0.31	0.45	0.71	0.71	0.73	0.02	0.0004	903 964	903 964	e	5 64 5 70	127	0.02	0.0004	902 963	28		6 64 6 70	124	1.00	0.18	0.04	0.49
M21112222	0.28	0.47	0.71	0.71	0.76	0.01	0.0004	955	955		7 71	117	0.01	0.0004	953	29		7 71	114	1.00	0.19	0.05	0.52
MB1112222	0.25	0.51	0.71	0.71	0.79	0.01	0.0003	998	998	1	77	119	0.01	0.0003	996	31		7 77	102	2 1.00	0.20	0.05	0.56

Name M511112221	T1 [s]	Sa(T1) [g] 0.58	Sa(T2) [g]	Sa(T3) [g] 0.71	DOC [•]	Disp [m]	ISDR [+]	Total Moment [MN·m] 111.28	Total Shear [MN] 11128	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m] 91	Disp [m]2	ISDR [•]3	Total Moment [MN·m]4 11123	Total Shear [MN]5 34	Max CB Shear [MN]6	Max Pp [MN]2 94	Max wall moment [MN-m]8	C1[-]	C2 [·]	C3 [•]	Mdy/ Mst [·]
M51112222	0.22	0.57	0.71	0.71	0.86	0.01	0.0003	1117	1117	9	9 93	81	0.01	0.0003	1113	34	9	98	78	3 1.00	0.23	0.06	0.65
M11121111 M11121112	0.83	0.18	0.58	0.71	0.71	0.05	0.0012	487 466	487 466	5	5 50	64	0.04	0.0012	483 463	15		5 51 5 50	57	· 1.00	0.36	0.10	0.50
M21121111 M21121112	0.76	0.20	0.61	0.71	0.75	0.04	0.0011	532 515	532	6	5 58	74	0.04	0.0011	529 513	16	1	5 58	66	5 1.00	0.38	0.11	0.53
MB1121111	0.70	0.22	0.64	0.71	0.78	0.04	0.0011	587	587	7	7 67	71	0.04	0.0010	582	18	6	67	63	3 1.00	0.41	0.11	0.57
MB1121112 M41121111	0.72	0.21	0.62	0.71	0.81	0.04	0.0011	568 634	568 634	7	7 66 3 75	59	0.04	0.0011	564 627	17	6	5 <u>66</u> 7 75	52	2 1.00 7 1.00	0.43	0.12	0.57
M41121112	0.66	0.23	0.66	0.71	0.84	0.04	0.0010	619	619	8	3 75	55	0.04	0.0010	613	19		7 74	47	1.00	0.47	0.13	0.61
M51121111 M51121112	0.59	0.26	0.71	0.71	0.84	0.03	0.0010	698	679	10) 86) 85	49	0.03	0.0009	667	21	2	<u>s 86</u> 3 84	42	2 0.99	0.50	0.13	0.66
M11121121	0.68	0.23	0.65	0.71	0.70	0.04	0.0010	605	605	6	5 64	96	0.04	0.0010	602	19	6	5 64	88	3 1.00	0.32	0.08	0.49
M21121122	0.71	0.21	0.62	0.71	0.75	0.04	0.0009	665	665	7	7 74	92	0.03	0.0011	661	20	1	7 74	. 84	1.00	0.34	0.08	0.50
M21121122	0.64	0.24	0.65	0.71	0.78	0.03	0.0010	637	637	7	7 73	76	0.03	0.0009	634	20		7 73	69	1.00	0.36	0.10	0.53
MB1121121	0.62	0.25	0.66	0.71	0.79	0.03	0.0009	666	666	5	3 77	75	0.03	0.0009	663	20		, <u></u> 777	68	3 1.00	0.36	0.10	0.57
M41121121 M41121122	0.56	0.28	0.71	0.71	0.78	0.03	0.0009	748	748	9	9 88 9 86	86	0.03	0.0008	743	23	8	3 88 3 86	- 79	1.00	0.36	0.09	0.61
M51121121	0.52	0.30	0.71	0.71	0.81	0.03	0.0008	793	793	10) 96	80	0.03	0.0008	787	24	ç	96	73	3 1.00	0.38	0.09	0.66
M51121122 M11121211	0.54	0.29	0.71	0.71	0.84	0.03	0.0008	534	534	10) <u>95</u> 5 55	67	0.03	0.0008	768	23	3	9 <u>95</u> 5 <u>5</u> 5	72	1.00 2 1.00	0.41	0.10	0.66
M11121212	0.79	0.19	0.57	0.71	0.76	0.04	0.0012	517	517	5	5 55	67	0.04	0.0012	515	16	5	5 55	60	1.00	0.35	0.11	0.50
M21121211 M21121212	0.70	0.22	0.63	0.71	0.76	0.04	0.0011	589	589	6	5 63	65	0.04	0.0010	586	18	6	5 63	58	1.00	0.35	0.10	0.53
MB1121211	0.61	0.25	0.67	0.71	0.80	0.03	0.0009	671	671	8	3 77	72	0.03	0.0009	667	20	1	7777	65	5 1.00	0.39	0.10	0.57
M41121212	0.56	0.24	0.71	0.71	0.83	0.03	0.0009	736	736	9	87	67	0.03	0.0009	730	20	{	87	60) 1.00	0.40	0.10	0.61
M41121212	0.58	0.27	0.70	0.71	0.86	0.03	0.0009	719	719	11	9 86	56	0.03	0.0009	714	22	8	3 86	50	1.00	0.43	0.11	0.61
M51121212	0.53	0.29	0.71	0.71	0.88	0.03	0.0009	779	779	11	1 96	49	0.03	0.0008	769	23	9	9 96	43	0.99	0.46	0.12	0.65
M11121221 M11121222	0.60	0.26	0.68	0.71	0.73	0.03	0.0009	696 667	696 667	7	7 74	100	0.03	0.0009	694 665	21		7 74	. 92 : 76	2 <u>1.00</u> 5 <u>1.00</u>	0.30	0.07	0.50
M21121221	0.54	0.29	0.70	0.71	0.77	0.03	0.0008	768	768	8	3 86	95	0.03	0.0008	766	24	8	3 86	88	3 1.00	0.31	0.08	0.53
M21121222 MB1121221	0.56	0.28	0.68	0.71	0.80	0.03	0.0008	746 832	746	10	3 86) 97	80	0.03	0.0008	744	23	6	3 86 9 97	73	3 1.00 2 1.00	0.33	0.09	0.53
MB1121222	0.51	0.30	0.71	0.71	0.83	0.03	0.0008	811	811	10) 96	75	0.03	0.0008	808	25	\$	96	68	3 1.00	0.35	0.09	0.57
M41121221 M41121222	0.46	0.34	0.71	0.71	0.83	0.03	0.0007	892	892	11	108	68	0.03	0.0007	886 870	27	10) 108) 107	62	1.00 2 1.00	0.35	0.09	0.61
M51121221	0.42	0.36	0.71	0.71	0.86	0.02	0.0007	941	941	12	2 118	69	0.02	0.0006	932	28	11	118	63	3 1.00 1.00	0.37	0.10	0.65
M11122111	0.43	0.35	0.71	0.71	0.88	0.02	0.0007	928	928	5	5 58	144	0.02	0.0007	855	28	, L	5 58	136	5 1.00	0.38	0.05	0.65
M11122112	0.50	0.31	0.71	0.71	0.71	0.03	0.0008	837	837	5	5 58	129	0.03	0.0007	834 917	26	5	5 58	122	2 1.00	0.25	0.06	0.49
M21122112	0.45	0.34	0.71	0.71	0.74	0.02	0.0007	905	905	6	5 66	121	0.02	0.0007	902	28	e	5 66	115	5 1.00	0.26	0.06	0.52
MB1122111 MB1122112	0.41	0.37	0.71	0.71	0.75	0.02	0.0006	973 877	973 877	7	7 74	124	0.02	0.0006	970	30		7 74	118	3 1.00 3 1.00	0.27	0.06	0.56
M41122111	0.37	0.39	0.71	0.71	0.79	0.02	0.0005	1029	1029	8	81	112	0.02	0.0005	1024	31	8	3 81	106	5 1.00	0.29	0.07	0.60
M41122112 M51122111	0.38	0.38	0.71	0.71	0.81	0.02	0.0006	1011 1091	1011 1091		3 81 9 91	100	0.02	0.0005	1006	31		3 81 9 90	92	1.00 2 1.00	0.30	0.07	0.60
M51122112	0.35	0.41	0.71	0.71	0.84	0.02	0.0005	1077	1077	9	90	86	0.02	0.0005	1069	32	8	3 89	81	. 1.00	0.32	0.08	0.65
M11122121 M11122122	0.39	0.30	0.71	0.71	0.68	0.02	0.0006	9901	990	6	5 70	150	0.02	0.0006	988	30	6	5 69 5 70	162	5 1.00	0.20	0.04	0.49
M21122121	0.35	0.41	0.71	0.71	0.71	0.02	0.0005	1087	1087	7	7 79	159	0.02	0.0005	1084	33		7 79 7 79	153	3 <u>1.00</u> 7 <u>1.00</u>	0.21	0.05	0.52
MB1122121	0.33	0.43	0.71	0.71	0.75	0.02	0.0005	1154	1154	8	3 88	147	0.02	0.0005	1150	35	{	3 88	142	1.00	0.22	0.05	0.56
M31122122 M41122121	0.33	0.43	0.71	0.71	0.78	0.02	0.0005	1136	1136	6	3 <u>88</u> 9 97	131	0.02	0.0005	1133	35	8	3 88 9 97	126	5 <u>1.00</u> 7 <u>1.00</u>	0.23	0.06	0.56
M41122122	0.31	0.45	0.71	0.71	0.81	0.02	0.0004	1196	1196	<u>c</u>	9 97	117	0.02	0.0004	1191	36	3	9 97	113	3 1.00	0.25	0.06	0.60
M51122121 M51122122	0.28	0.48	0.71	0.71	0.83	0.01	0.0004	1255	1255	10) 105) 105	112 99	0.01	0.0004	1248	38	10) 105) 105	10)	1.00 5 1.00	0.27	0.07	0.64
M11122211	0.44	0.35	0.71	0.71	0.70	0.02	0.0006	931	931	6	5 64	145	0.02	0.0006	928	29	6	5 64	139	9 1.00	0.23	0.05	0.49
M21122212	0.45	0.34	0.71	0.71	0.73	0.02	0.0005	915	915	7	7 71	131	0.02	0.0007	913 987	28	6	5 71	124	1.00	0.24	0.06	0.49
M21122212	0.41	0.36	0.71	0.71	0.76	0.02	0.0006	977	977	7	7 72	121	0.02	0.0006	975	30		7 72 7 80	115	5 1.00	0.25	0.06	0.52
MB1122212	0.37	0.39	0.71	0.71	0.79	0.02	0.0005	1040	1040	6	3 79	111	0.02	0.0005	1035	32		7 79	106	5 1.00	0.26	0.06	0.56
M41122211 M41122212	0.34	0.42	0.71	0.71	0.81	0.02	0.0005	1123	1123	9	88 88	113	0.02	0.0005	1119	34		<u> </u>	107	<u>1.00</u>	0.27	0.07	0.60
M51122211	0.39	0.38	0.71	0.71	0.75	0.02	0.0006	1005	1005	7	7 73	131	0.02	0.0005	1002	31	į.	7 73	125	5 1.00	0.24	0.06	0.66
M111222212	0.31	0.45	0.71	0.71	0.86	0.02	0.0005	1167	1167	10) 96 7 76	171	0.02	0.0005	1160	35		<u>96</u> 7 76	166	<u>1.00</u> 5 1.00	0.30	0.08	0.65
M11122222	0.36	0.40	0.71	0.71	0.72	0.02	0.0005	1082	1082	7	7 76	153	0.02	0.0005	1080	33		7 76	148	3 1.00	0.20	0.05	0.49
M21122221	0.32	0.43	0.71	0.71	0.74	0.02	0.0004	11/8	11/8	6	s ac 3 86	143	0.02	0.0004	1175	36	8		130	3 1.00	0.20	0.05	0.52
MB1122221	0.29	0.47	0.71	0.71	0.77	0.01	0.0004	1239	1239	9	9 94	146	0.01	0.0004	1236	38	9	9 94	141	1.00	0.21	0.05	0.56
M41122221	0.27	0.49	0.71	0.71	0.80	0.01	0.0004	1289	1289	10) 103	129	0.01	0.0004	1285	39	10	103	125	5 1.00	0.23	0.06	0.60
M41122222 M51122221	0.27	0.48	0.71	0.71	0.82	0.01	0.0004	1279	1279	10	103	115	0.01	0.0004	1275	39	10	0 103	111	1.00	0.24	0.06	0.60
M51122222	0.25	0.51	0.71	0.71	0.86	0.01	0.0003	1337	1337	11	112	98	0.01	0.0003	1331	40	11	111	94	1.00	0.26	0.07	0.65
M11211111 M11211112	1.11	0.13	0.47	0.71	0.71	0.06	0.0012	356	356 339	3	3 37 3 36	58	0.06	0.0012	352	8		3 <u>37</u> 336	51	. <u>1.00</u> 2 <u>1.00</u>	0.40	0.14	0.50
M21211111	1.01	0.15	0.50	0.71	0.74	0.05	0.0011	393	393	4	43	56	0.05	0.0011	390	9	4	43	49	1.00	0.42	0.14	0.53
M312111112 M31211111	0.93	0.14	0.48	0.71	0.78	0.05	0.0011	432	432	5	4 43 5 49	54	0.05	0.0011	427	10	4	<u>42</u> 5 49	40	5 1.00	0.44	0.16	0.54
MB1211112	0.96	0.15	0.51	0.71	0.81	0.05	0.0011	418	418	5	5 49	45	0.05	0.0010	414	10		5 49	38	3 1.00	0.48	0.17	0.57
M41211111 M41211112	0.85	0.18	0.59	0.71	0.81	0.05	0.0010	471 456	4/1 456	6	56 5 <u>5</u> 55	51 42	0.05	0.0009	464	11		5 55	43	5 <u>1.00</u> 5 <u>1.00</u>	0.54	0.16	0.62
M51211111	0.78	0.19	0.65	0.71	0.84	0.04	0.0009	513	513	8	63	45	0.04	0.0009	500	11	6	5 62	37	0.99	0.62	0.18	0.66
M1121112	0.80	0.19	0.54	0.71	0.86	0.04	0.0010	501 445	501 445	4	, 62 1 47		0.04	0.0009	488 441	11	4	61 1 47	64	, 0.99 1 <u>1.0</u> 0	0.63	0.11	0.50
M11211122	0.95	0.16	0.51	0.71	0.74	0.05	0.0010	427	427	4	47	60	0.05	0.0010	424	10	4	46	53	3 1.00	0.38	0.12	0.50
M21211121 M21211122	0.85	0.18	0.58	0.71	0.74	0.05	0.0009	491 471	491 471		5 54	/U 58	0.05	0.0009	487	11		2 54 5 54	51	. 1.00	0.40	0.13	0.53
MB1211121	0.76	0.20	0.62	0.71	0.77	0.04	0.0008	533	533	6	5 62	66	0.04	0.0008	528	12	6	62	58	3 1.00	0.43	0.12	0.57
M41211121	0.70	0.22	0.66	0.71	0.80	0.04	0.0008	585	585	6	3 71	62	0.04	0.0009	578	12		7 70	54	1.00	0.48	0.13	0.61
M41211122 M51211121	0.76	0.20	0.61	0.71	0.82	0.04	0.0008	535	535	7	7 64	54	0.04	0.0008	530	12	6	5 64 7 79	46	5 1.00	0.47	0.14	0.62
M51211122	0.66	0.24	0.69	0.71	0.86	0.04	0.0008	617	617	9	78	46	0.04	0.0008	604	14		77	39	0.99	0.56	0.15	0.66

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [-]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [·]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [-]	C2 [·]	C3 [·]	Mdy/ Mst [·]
M21211211 M21211212	0.90	0.17	0.54	0.71	0.77	0.05	0.0010	449 436	449 436	5	49 49	58 48	0.05	0.0010	446	10	0	5 49 5 49	51 42	1.00	0.41	0.13	0.53
M31211211	0.82	0.18	0.58	0.71	0.80	0.04	0.0009	494	494	6	56	55	0.04	0.0009	490	11	u) 0	56	48	1.00	0.45	0.14	0.57
M41211212	0.75	0.18	0.63	0.71	0.83	0.03	0.0009	538	538	7	64	51	0.03	0.0008	531	12	6	63	40	1.00	0.40	0.13	0.61
M41211212 M51211211	0.77	0.20	0.61	0.71	0.85	0.04	0.0009	524 590	524 590	7	63	43	0.04	0.0008	518 578	12	e 7	63 72	36	1.00 0.99	0.52	0.15	0.62
M51211212	0.70	0.22	0.67	0.71	0.88	0.04	0.0009	577	577	9	9 7 <u>1</u>	39	0.04	0.0008	566	13	7	70	32	0.99	0.58	0.16	0.66
M11211222	0.83	0.13	0.56	0.71	0.73	0.04	0.0009	492	492	5	54	63	0.04	0.0009	490	11	-	54	56	1.00	0.37	0.11	0.50
M21211221 M21211222	0.73	0.21	0.62	0.71	0.77	0.04	0.0008	563 543	563 543	6	63	72	0.04	0.0008	560 541	13	6	63 62	65 53	1.00	0.38	0.10	0.53
M31211221	0.66	0.23	0.65	0.71	0.80	0.04	0.0007	617	617	7	72	68 57	0.04	0.0007	613	14	7	72	61 50	1.00	0.41	0.11	0.57
M41211222	0.61	0.22	0.69	0.71	0.83	0.04	0.0007	675	675	9	81	64	0.03	0.0007	669	15	6	8 81	56	1.00	0.44	0.11	0.61
M41211222 M51211221	0.62	0.24	0.68	0.71	0.85	0.03	0.0007	655	655	9	80 92	53	0.03	0.0007	649	15 16	8	3 80 9 92	46	1.00	0.46	0.12	0.61
M51211222	0.57	0.27	0.71	0.71	0.88	0.03	0.0007	721	721	10	91	47	0.03	0.0007	711	16	9	90	41	0.99	0.49	0.13	0.65
M11212111	0.66	0.24	0.65	0.71	0.08	. 0.04	0.0007	620	620	4	- 45 - 43	98	0.04	0.0007	617	14	2	43	90	1.00	0.30	0.07	0.49
M212121111 M21212112	0.59	0.26	0.69	0.71	0.72	0.03	0.0007	707 686	707 686	5	51	107 95	0.03	0.0007	702	16 16		50	<u>98</u> 87	1.00	0.32	0.07	0.52
MB1212111	0.54	0.29	0.71	0.71	0.75	0.03	0.0006	768	768	6	58	101	0.03	0.0006	763	17	5	58	93	1.00	0.34	0.08	0.56
M41212111	0.50	0.20	0.71	0.71	0.78	0.03	0.0006	823	823	7	65	90	0.03	0.0006	817	19	6	5 65	85	1.00	0.35	0.09	0.57
M41212112 M51212111	0.51	0.30	0.71	0.71	0.81	0.03	0.0006	808	808 877	7	64	82	0.03	0.0006	802	18	6	64	75	1.00	0.37	0.09	0.61
M51212112	0.47	0.33	0.71	0.71	0.84	0.03	0.0006	865	865	8	72	71	0.03	0.0006	855	19	7	72	65	1.00	0.40	0.10	0.65
M11212121 M11212122	0.51	0.30	0.71	0.71	0.67	0.03	0.0006	804 784	804 784	5	55	137	0.03	0.0006	800 781	18	5	55	129	1.00	0.20	0.06	0.49
M21212121 M21212122	0.47	0.33	0.71	0.71	0.71	0.03	0.0005	870	870	6	63	130	0.03	0.0005	867	20	6	63	123	1.00	0.26	0.06	0.52
M31212121	0.50	0.31	0.71	0.71	0.69	0.03	0.0006	833	833	5	59	134	0.03	0.0006	829	19		59	127	1.00	0.26	0.06	0.56
M31212122 M41212121	0.45	0.34	0.71	0.71	0.77	0.02	0.0005	912	912	8	71	108	0.02	0.0005	908	21	7	71 /1	101	1.00	0.30	0.07	0.60
M41212122	0.41	0.36	0.71	0.71	0.81	0.02	0.0005	963 1029	963 1029	8	8 78	96 93	0.02	0.0005	957	22	7	78	90 88	1.00	0.31	0.08	0.60
M51212122	0.38	0.39	0.71	0.71	0.84	0.02	0.0004	1013	1013	9	85	82	0.02	0.0004	1021	23	6	85	77	1.00	0.34	0.09	0.65
M11212211 M11212212	0.58	0.27	0.69	0.71	0.70	0.03	0.0007	697	717 697	4	49 49	115	0.03	0.0007	7 <u>14</u> 694	16 16	2	49	<u>107</u> 94	1.00	0.29	0.07	0.49
M21212211	0.53	0.29	0.71	0.71	0.74	0.03	0.0006	784	784	5	56	109	0.03	0.0006	780	18	0	56	102	1.00	0.30	0.07	0.52
M312122212	0.49	0.32	0.71	0.71	0.70	0.03	0.0006	846	846	6	64	102	0.03	0.0005	841	19	6	5 64	95	1.00	0.31	0.07	0.55
MB1212212 M41212211	0.50	0.31	0.71	0.71	0.79	0.03	0.0006	831 902	831 902	6	63	91	0.03	0.0006	828	19 20	6	63 71	84	1.00	0.33	0.08	0.56
M41212212	0.46	0.34	0.71	0.71	0.82	0.03	0.0005	891	891	7	71	83	0.03	0.0005	885	20	5	70	76	1.00	0.34	0.09	0.61
M51212211	0.41	0.36	0.71	0.71	0.84	0.02	0.0005	940	949	8	10	73	0.02	0.0005	931	21	7	77	65	1.00	0.30	0.10	0.65
M11212221 M11212222	0.46	0.33	0.71	0.71	0.70	0.03	0.0005	884	884 866	5	61	140	0.03	0.0005	881	20	6	61	133	1.00	0.24	0.05	0.49
M21212221	0.43	0.35	0.71	0.71	0.73	0.02	0.0005	946	946	6	69	131	0.02	0.0005	943	22	6	69	124	1.00	0.25	0.06	0.52
M312122222	0.44	0.35	0.71	0.71	0.76	0.02	0.0003	932	932 998	7	76	117	0.02	0.0003	930	21	7	76	111	1.00	0.26	0.06	0.52
M31212222 M41212221	0.40	0.37	0.71	0.71	0.79	0.02	0.0004	987 1066	987	7	76	107	0.02	0.0004	984	23	7	76	101	1.00	0.27	0.07	0.56
M41212222	0.37	0.40	0.71	0.71	0.82	0.02	0.0004	1050	1050	8	84	96	0.02	0.0004	1046	24	8	84	91	1.00	0.29	0.07	0.60
M51212221 M51212222	0.33	0.43	0.71	0.71	0.84	0.02	0.0004	1125	1125	9	93	93	0.02	0.0004	1118	25	9	93	88	1.00	0.31	0.08	0.65
M11221111	1.28	0.11	0.43	0.69	0.71	0.07	0.0014	403	403	4	42	67	0.06	0.0013	398	9	4	42	57	1.00	0.43	0.16	0.50
M21221111	1.17	0.12	0.46	0.71	0.74	0.06	0.0013	446	446	5	49	65	0.06	0.0012	441	10	2	49	55	1.00	0.46	0.17	0.54
M21221112 M31221111	1.22	0.12	0.44	0.69	0.78	0.06	0.0013	429	429	5	9 48 9 56	54	0.06	0.0013	425	10	5	48 5 56	45	1.00	0.48	0.18	0.54
M31221112	1.11	0.13	0.47	0.71	0.81	0.06	0.0012	475 540	475	6	55	52	0.06	0.0012	: 470 530	11	e F	55	43	1.00	0.52	0.19	0.58
M41221112	1.01	0.15	0.51	0.71	0.83	0.05	0.0012	522	522	7	63	49	0.05	0.0011	514	12	6	62	40	1.00	0.57	0.20	0.62
M51221111 M51221112	0.90	0.16	0.60	0.71	0.84	0.05	0.0011	590 575	590 575	9	9 72 9 71	53 45	0.05	0.0010	572	13 13	7	71	42	0.99	0.67	0.21	0.66
M11221121	1.04	0.14	0.49	0.71	0.70	0.05	0.0011	509	509	5	54	84	0.05	0.0011	. 504	12		53	74	1.00	0.38	0.13	0.50
M21221121	0.95	0.16	0.52	0.71	0.74	0.05	0.0011	561	561	6	62	81	0.05	0.0010	556	13	6	62	71	1.00	0.42	0.13	0.53
M21221122 M31221121	0.99	0.15	0.49	0.71	0.78	0.05	0.0011	539 612	539 612	6	61 71	67	0.05	0.0011	535	12	6	<u>61</u> 71	58	1.00	0.43	0.15	0.53
M31221122	0.91	0.16	0.54	0.71	0.81	0.05	0.0010	591	591	7	70	64	0.05	0.0010	586	14	7	70	55	1.00	0.47	0.16	0.57
M41221121	0.87	0.13	0.56	0.71	0.82	0.04	0.0010	614	614	8	74	63	0.05	0.0009	608	14	7	73	53	1.00	0.50	0.16	0.61
M51221121 M51221122	0.74	0.21	0.67	0.71	0.84	0.04	0.0009	729	729	11	. 91 . 89	65 54	0.04	0.0009	1 711 1 689	16 16	8) <u>90</u> ; 88	54 44	0.99	0.60	0.17	0.66
M11221211	1.14	0.13	0.46	0.71	0.73	0.06	0.0012	461	461	5	48	69 50	0.06	0.0012	457	11	2	48	60 50	1.00	0.41	0.15	0.50
M21221212	1.04	0.12	0.48	0.71	0.77	0.05	0.0011	514	514	6	56	67	0.05	0.0011	. 510	12	5	56	58	1.00	0.44	0.15	0.54
M21221212 M31221211	1.07 0.95	0.14	0.47	0.71	0.80	0.06	0.0012	497 566	497 566	6	56 64	56	0.06	0.0011	494	11	6	5 56 5 64	48	1.00	0.45	0.17	0.54
M31221212	0.97	0.15	0.50	0.71	0.83	0.05	0.0011	550	550	7	64	53	0.05	0.0010	546	13	6	64	45	1.00	0.48	0.17	0.58
M41221211	0.89	0.17	0.56	0.71	0.85	0.05	0.0010	601	601	8	, 73 1 72	50	0.05	0.0010	593	14		72	41	1.00	0.55	0.18	0.62
M51221211 M51221212	0.79	0.19	0.64	0.71	0.85	0.04	0.0010	676 662	676 662	10	83	54 46	0.04	0.0009	658 646	15 15	8	3 82 3 80	44	0.99	0.63	0.18	0.66
M11221221	0.92	0.16	0.53	0.71	0.73	0.05	0.0010	583	583	6	62	87	0.05	0.0010	580	13	6	62	77	1.00	0.37	0.12	0.50
M21221222	0.96	0.16	0.50	0.71	0.77	0.04	0.00010	563 645	563 645	7	62 72	84	0.04	0.0010	641 b	13	7	72	64 74	1.00	0.40	0.13	0.50
M21221222 MB1221221	0.87	0.17	0.54	0.71	0.80	0.05	0.0009	623 703	623 703	7	71	70	0.05	0.0009	620 697	14	7	71	61 69	1.00	0.41	0.14	0.53
MB1221222	0.79	0.19	0.59	0.71	0.83	0.04	0.0009	685	685	8	81	66	0.04	0.0009	681	16	8	81	58	1.00	0.45	0.14	0.57
M41221221 M41221222	0.70	0.22	0.65	0.71	0.82	0.04	0.0008	753	753	10 10	93	62	0.04	0.0008	764	17	01	93) 93	64 53	1.00	0.48	0.13	0.61
M51221221 M51221222	0.64	0.24	0.70	0.71	0.85	0.04	0.0008	837 821	837 821	12	104	66 56	0.04	0.0008	821	19	10	103	56 46	0.99	0.55	0.15	0.66
M11222111	0.74	0.20	0.63	0.71	0.68	0.04	0.0008	732	732	4	50	128	0.04	0.0008	726	17	4	50	116	1.00	0.33	0.08	0.49
M21222112 M21222111	0.76	0.20	0.65	0.71	0.70	0.04	0.0008	707 804	804	4	49	113	0.04	0.0008	702	16 18	5	49 58	102	1.00	0.34	0.09	0.53
M21222112	0.70	0.22	0.64	0.71	0.74	0.04	0.0008	784	784	5	57	110	0.04	0.0008	779	18	5	57	99	1.00	0.36	0.09	0.53

Name M41222111	T1[s]	Sa(T1) [g] 0.27	Sa(T2) [g]	Sa(T3) [g] 0.71	DOC [•]	Disp [m]	ISDR [+]	Total Moment [MN·m] 957	Total Shear [MN] 957	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m] 109	Disp [m]2	ISDR [•]3 0.0007	Total Moment [MN·m]4 948	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2 75	Max wall moment [MN-m]8	C1 []	C2 [·]	C3[•]	Mdy/ Mst [•]
M41222112	0.59	0.26	0.70	0.71	0.81	0.03	0.0007	937	937	8	3 75	97	0.03	0.0007	928	21	7	74	87	1.00	0.42	0.11	0.61
M51222111 M51222112	0.53	0.29	0.71	0.71	0.82	0.03	0.0007	1030	1030	9	9 85 9 84	95	0.03	0.0006	1016	23	8	85	86	0.99	0.44	0.11	0.65
M11222121	0.59	0.26	0.69	0.71	0.67	0.03	0.0007	929	929 896	6	5 64	160	0.03	0.0007	923	21	6	64	149	1.00	0.28	0.06	0.49
M21222122	0.54	0.29	0.71	0.71	0.70	0.03	0.0006	1017	1017	ā	7 74	154	0.03	0.0006	1012	23	7	74	143	1.00	0.30	0.07	0.43
M21222122 M31222121	0.56	0.28	0.70	0.71	0.74	0.03	0.0006	991 1095	991 1095		7 73	137	0.03	0.0006	987	23	7	73	127	1.00	0.31	0.07	0.53
M31222122	0.51	0.30	0.71	0.71	0.77	0.03	0.0006	1072	1072	Ę	3 83	128	0.03	0.0006	1066	24	8	83	119	1.00	0.33	0.08	0.56
M41222121 M41222122	0.46	0.33	0.71	0.71	0.78	0.03	0.0005	11/1 1151	11/1 1151		9 94 9 93	131	0.03	0.0005	1162	27	9	93	122	1.00	0.34	0.08	0.61
M51222121	0.43	0.35	0.71	0.71	0.82	0.02	0.0005	1235	1235	11	L 104	113	0.02	0.0005	1223	28	10	103	105	1.00	0.37	0.09	0.65
M112222122	0.43	0.35	0.71	0.71	0.84	0.02	0.0008	815	815		5 55	132	0.02	0.0008	810	19	5	55	121	1.00	0.38	0.10	0.65
M11222212 M21222211	0.69	0.22	0.64	0.71	0.73	0.04	0.0008	795	795	6	5 55	118	0.04	0.0008	791	18	5	55	108	1.00	0.33	0.08	0.50
M21222212	0.63	0.24	0.66	0.71	0.76	0.03	0.0007	872	872	E	5 64	113	0.03	0.0007	868	20	6	64	103	1.00	0.35	0.09	0.53
M31222211 M31222212	0.56	0.28	0.70	0.71	0.77	0.03	0.0006	985	985 965		/ /4 7 73	121	0.03	0.0006	979	22	7	73	<u>111</u> 98	1.00	0.35	0.09	0.56
M41222211	0.52	0.30	0.71	0.71	0.80	0.03	0.0006	1060	1060	8	83	111	0.03	0.0006	1052	24	8	83	101	1.00	0.38	0.09	0.61
M51222212	0.35	0.32	0.71	0.71	0.82	0.03	0.0006	1045	1135	10) 93) 93	99	0.03	0.0006	1037	24	9	92	88	1.00	0.59	0.10	0.65
M51222212	0.48	0.32	0.71	0.71	0.85	0.03	0.0006	1121	1121	1() <u>92</u> 5 71	85	0.03	0.0006	1108	25	9	92	77	0.99	0.42	0.11	0.65
M11222222	0.55	0.28	0.70	0.71	0.72	0.03	0.0006	1010	1010	-	7 71	148	0.03	0.0006	1006	23	6	71	138	1.00	0.28	0.07	0.49
M21222221 M21222222	0.49	0.31	0.71	0.71	0.73	0.03	0.0006	1123 1100	1123	8	3 82 3 81	157	0.03	0.0006	1118	26	7	82	147	1.00	0.28	0.06	0.52
M31222221	0.45	0.34	0.71	0.71	0.77	0.02	0.0005	1204	1204	4	9 92	146	0.02	0.0005	1199	28	9	92	137	1.00	0.29	0.07	0.56
M41222221	0.46	0.35	0.71	0.71	0.80	0.02	0.0005	1272	1272	10) 101	130	0.02	0.0005	1181	27	10	101	121	1.00	0.31	0.08	0.60
M41222222	0.52	0.30	0.71	0.71	0.74	0.03	0.0006	1062	1062	11	7 77	143	0.03	0.0006	1058	24	7	77	134	1.00	0.29	0.07	0.61
M51222222	0.39	0.38	0.71	0.71	0.86	0.02	0.0004	1316	1316	11	L 110	99	0.02	0.0004	1305	30	10	109	92	1.00	0.35	0.09	0.65
M12111111 M12111112	2.81	0.04	0.21	0.44	0.70	0.12	0.0017	324	324	2	2 <u>33</u> 232	61 50	0.11	0.0016	316 298	5	1	. 33	45	1.00	0.57	0.27	0.47
M22111111	2.56	0.05	0.23	0.46	0.73	0.11	0.0016	371	371	2	2 40	61	0.11	0.0015	362	6	2	40	45	1.00	0.61	0.29	0.50
M32111112	2.66	0.04	0.21	0.43	0.77	0.11	0.0016	423	423	4 100	2 <u>39</u> 348	50	0.11	0.0015	412	5	2	48	30 44	1.00	0.68	0.31	0.50
M32111112	2.42	0.05	0.24	0.46	0.80	0.11	0.0016	403	403	3	3 46 1 57	49	0.11	0.0015	394	6	2	46	36	1.00	0.69	0.33	0.53
M42111112	2.21	0.06	0.28	0.50	0.83	0.11	0.0016	464	464	2	4 55	48	0.10	0.0014	450	7	3	55	34	0.99	0.79	0.36	0.57
M52111111 M52111112	1.96 2.01	0.07	0.35	0.61	0.83	0.10	0.0015	557 541	557 541		5 68 5 67	55 45	0.10	0.0014	523	8	3	66	37	0.97	0.92	0.42	0.61
M12111121	2.28	0.06	0.26	0.50	0.69	0.11	0.0015	439	439	1	2 46	81	0.10	0.0014	430	7	2	46	63	1.00	0.53	0.22	0.46
MI2111122 M22111121	2.39	0.05	0.24	0.47	0.74	0.10	0.0015	410 505	505	4	2 44	81	0.10	0.0015	403	8	3	55	63	1.00	0.53	0.25	0.46
M22111122	2.17	0.06	0.27	0.49	0.77	0.10	0.0015	475	475	3	3 54 1 65	66	0.10	0.0014	468	7	3	54	51	1.00	0.58	0.26	0.49
M32111122	1.98	0.07	0.30	0.53	0.80	0.10	0.0014	543	543	4	4 64	65	0.10	0.0013	534	8	3	64	49	1.00	0.63	0.28	0.52
M42111121 M42111122	1.75	0.08	0.35	0.61	0.80	0.09	0.0013	622 602	622 602		5 74 5 73	. 74 61	0.09	0.0012	<u>603</u> 586	9	3	. 74	55 45	0.99	0.72	0.30	0.56
M52111121	1.60	0.09	0.41	0.68	0.83	0.08	0.0013	692	692	e	5 86	67	0.08	0.0012	656	10	4	. 84	48	0.97	0.86	0.37	0.61
M1211122	2.49	0.08	0.40	0.66	0.86	0.11	0.0013	386	386	2	5 84 2 40	65	0.11	0.0012	379	6	2	40	50	1.00	0.87	0.38	0.61
M12111212 M22111211	2.59	0.05	0.22	0.43	0.77	0.11	0.0016	366	366	2	2 39	53	0.11	0.0015	360	6	2	39	40	1.00	0.56	0.28	0.47
M22111212	2.34	0.05	0.24	0.46	0.80	0.11	0.0015	425	425	3	3 48	53	0.10	0.0014	419	7	2	48	40	1.00	0.59	0.28	0.49
M32111211 M32111212	2.07	0.06	0.29	0.52	0.79	0.10	0.0015	491	512 491		3 58 3 57	64 53	0.10	0.0014	502 483	8	3	58	48	1.00	0.64	0.28	0.52
M42111211	1.89	0.07	0.33	0.57	0.82	0.10	0.0014	573	573	2	4 67	62	0.09	0.0013	556	9	3	67	45	0.99	0.74	0.32	0.56
M52111212	1.95	0.07	0.38	0.55	0.85	0.09	0.0014	640	640	6	+ 07	55	0.09	0.0013	: 545 : 606	° 9	4	. 76	38	0.99	0.75	0.39	0.61
M52111212	2.02	0.08	0.37	0.64	0.88	0.09	0.0014	626 531	626 531	6	5 77 3 56	46	0.09	0.0013	594	9	4	75	31	0.97	0.89	0.40	0.61
M12111222	2.10	0.06	0.27	0.49	0.77	0.10	0.0014	502	502	3	3 55	71	0.10	0.0014	497	8	3	55	57	1.00	0.51	0.23	0.46
M22111221 M22111222	1.83	0.07	0.32	0.56	0.76	0.09	0.0013	591 571	591 571	3	3 66 3 65	84	0.09	0.0012	583	9	3	65	67 55	1.00	0.54	0.23	0.49
M32111221	1.67	0.08	0.35	0.60	0.79	0.08	0.0012	652	652	4	4 75	81	0.08	0.0011	641	10	4	- 75	63	1.00	0.60	0.25	0.52
M42111222	1.72	0.08	0.39	0.65	0.82	0.08	0.0012	720	720		5 86	76	0.08	0.0012	. 702	11	4	. 75	57	0.99	0.62	0.27	0.56
M42111222 M52111221	1.57	0.09	0.37	0.63	0.85	0.08	0.0012	702	702	5	5 85 7 100	63	0.08	0.0011	686	11	4	85	47	0.99	0.69	0.30	0.56
M52111222	1.42	0.10	0.44	0.70	0.88	0.08	0.0011	784	784	-	7 98	57	0.08	0.0011	749	12	5	96	40	0.98	0.83	0.35	0.60
M12112111 M12112112	1.62	0.09	0.36	0.64	0.67	0.08	0.0012	676	676 654	2	2 <u>45</u> 245	129 115	0.08	0.0011	. 663	10	2	45	106 94	1.00	0.45	0.17	0.46
M22112111	1.48	0.09	0.39	0.66	0.71	0.08	0.0011	744	744	0	3 53	125	0.08	0.0010	731	11	2	53	102	1.00	0.49	0.19	0.49
M32112112	1.32	0.09	0.37	0.68	0.74	0.08	0.0010	817	817	1	5 52 3 61	110	0.07	0.0011	801	12	3	61	90	1.00	0.55	0.20	0.43
M32112112	1.40	0.10	0.41	0.67	0.77	0.07	0.0010	795	795	3	3 60 1 70	106	0.07	0.0010	873	12	3	60 70	85	1.00	0.56	0.22	0.52
M42112112	1.28	0.11	0.45	0.70	0.80	0.07	0.0010	876	876	2	4 69	100	0.07	0.0009	856	13	3	69	78	1.00	0.64	0.25	0.56
M52112111 M52112112	1.15	0.13	0.52	0.71	0.82	0.06	0.0009	990 972	990 972		5 81 5 80	100	0.06	0.0009	950	15	4	80 - 80 - 79	78	0.98	0.74	0.26	0.60
M12112121	1.30	0.11	0.43	0.70	0.67	0.07	0.0009	860	860		3 58	162	0.07	0.0009	847	13	3	58	137	1.00	0.42	0.15	0.46
M22112122 M22112121	1.34	0.11	0.46	0.68	0.70	0.07	0.0009	950	950		s se 3 68	145	0.06	0.0010	936	15	3	68	121	1.00	0.45	0.16	0.48
M22112122	1.23	0.12	0.44	0.70	0.73	0.06	0.0009	921	921	3	3 68 1 70	139	0.06	0.0009	909	14	3	68	117	1.00	0.47	0.17	0.48
MB2112121	1.10	0.13	0.48	0.71	0.74	0.06	0.0008	1043	1043	4	4 78	149	0.06	0.0008	1028	16	4	- 78	120	1.00	0.51	0.18	0.52
M42112121 M42112122	1.01	0.15	0.53	0.71	0.78	0.06	0.0008	1142 1117	1142 1117		5 91 5 an	140	0.06	0.0008	1119	17	4	. 90 	116	1.00	0.57	0.18	0.56
M52112121	0.93	0.16	0.60	0.71	0.82	0.05	0.0008	1255	1255		7 105	124	0.05	0.0007	1213	19	5	103	101	0.99	0.68	0.21	0.60
M52112122 M12112211	0.94	0.16	0.59	0.71	0.84	0.05	0.0008	1232	1232		/ 103 2 51	110	0.05	0.0007	1191	18	5	102	87	0.98	0.69	0.22	0.60
M12112212	1.51	0.09	0.37	0.64	0.72	0.08	0.0011	731	731	2	2 51	118	0.08	0.0010	722	11	2	50	98	1.00	0.45	0.18	0.46
M22112211 M22112212	1.34 1.37	0.11	0.42	0.68	0.73	0.07	0.0010	833 811	833	3	> 59 3 <u>5</u> 9	129	0.07	0.0009	822	13 13	3	59	107 94	1.00	0.48	0.19	0.49
MB2112211 MB2112212	1.23	0.12	0.45	0.70	0.76	0.06	0.0009	917	917	4	4 68 1 69	123	0.06	0.0009	903	14	3	68 69	101	1.00	0.53	0.20	0.52
M42112212	1.13	0.13	0.44	0.69	0.80	0.06	0.0009	1008	1008	5	5 78	109	0.06	0.0009	987	14	3	- 78	93	1.00	0.54	0.21	0.56
M42112212	1.15	0.13	0.48	0.71	0.82	0.06	0.0009	989	989	5	5 78	102	0.06	0.0008	970	15	4	78	81	1.00	0.61	0.23	0.56

Name	T1 [s]	Sa(T1)	Sa(T2)	Sa(T3)	DOC	Disp	ISDR [-]	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	Disp [m]2	ISDR [•]3	Total Moment	Total Shear	Max CB Shear	Max Pp	Max wall moment	C1 [·]	C2 [·]	C3 [·]	Mdy/
M12112221	1.18	0.12	0.46	0.71	0.69	0.06	0.0009	[MN•m] 966	[MN] 966	[MN]	66	[MN-m] 167	0.06	0.0008	[MN-m]4 954	[MN]5 15	[MN]6	[IVIIV]2 66	[MN•m]8 144	1.00	0.41	0.14	0.45
M12112222	1.21	0.12	0.44	0.70	0.72	0.06	0.0009	937	937	3	66	149	0.06	0.0009	928	15	3	66	127	1.00	0.42	0.15	0.46
M22112221 M22112222	1.07	0.14	0.48	0.71	0.73	0.06	0.0008	1071	10/1	4	i // i 77	161	0.06	0.0008	1059	1/	4	77	139	1.00	0.43	0.15	0.48
MB2112221	0.99	0.15	0.52	0.71	0.76	0.05	0.0007	1176	1176	5	89	154	0.05	0.0007	1162	18	4	89	132	1.00	0.48	0.16	0.52
M42112221	0.90	0.16	0.57	0.71	0.80	0.05	0.0007	1284	1284	Ē	101	143	0.05	0.0007	1262	20	5	101	120	1.00	0.55	0.17	0.55
M42112222 M52112221	0.92	0.16	0.56	0.71	0.82	0.05	0.0007	1262	1262	6	101	127	0.05	0.0007	1242	19 21		101	105	1.00	0.56	0.18	0.56
M52112222	0.84	0.18	0.63	0.71	0.85	0.05	0.0007	1388	1388	7	116	111	0.05	0.0007	1348	21		114	90	0.99	0.65	0.19	0.60
M12121111 M12121112	3.24	0.03	0.18	0.39	0.69	0.13	0.0018	352	352	2	2 <u>36</u> 235	56	0.12	0.0017	341 323	5		2 <u>30</u> 235	49	1.00	0.60	0.29	0.47
M22121111	2.95	0.04	0.20	0.41	0.73	0.12	0.0018	401	401	2	2 43	68	0.12	0.0016	390	6	4	2 43	49	1.00	0.65	0.32	0.50
M321211112	2.70	0.04	0.18	0.38	0.76	0.12	0.0018	456	456	3	51	67	0.11	0.0015	443	7		2 51	47	1.00	0.07	0.35	0.53
M32121112	2.79	0.04	0.21	0.42	0.80	0.12	0.0017	434	434	3	50	55	0.12	0.0016	424	7		2 50	38	1.00	0.74	0.37	0.53
M42121112	2.55	0.05	0.24	0.46	0.83	0.11	0.0017	499	499	2	59	54	0.11	0.0015	482	7		59	36	0.99	0.84	0.41	0.57
M52121111 M52121112	2.27	0.06	0.31	0.56	0.83	0.11	0.0017	608 586	608 586	6	74	63 52	0.11	0.0015	564	9		8 71 8 69	40	0.97	1.01	0.47	0.62
M12121121	2.64	0.04	0.22	0.46	0.69	0.11	0.0016	473	473	2	2 49	90	0.11	0.0015	461	7		2 49	67	1.00	0.56	0.25	0.47
M12121122 M22121121	2.76	0.04	0.21	0.42	0.74	0.11	0.0016	442 541	442 541	2	: 48 59	/3	0.11	0.0016	433 529	8		<u>47</u> 3 59	54	1.00	0.57	0.28	0.47
M22121122	2.51	0.05	0.23	0.45	0.77	0.11	0.0016	510	510	9	58	73	0.11	0.0015	501	8	-	3 57	54	1.00	0.61	0.29	0.49
M32121121 M32121122	2.21	0.06	0.26	0.51	0.76	0.11	0.0015	586	586	4	F 71 F 69	69 72	0.10	0.0014	574	9		5 71 3 69	53	1.00	0.68	0.31	0.53
M42121121	2.02	0.07	0.32	0.56	0.79	0.10	0.0015	707	707	5	84	86	0.10	0.0014	683	11	4	84	63	0.99	0.75	0.33	0.56
M52121122	1.85	0.07	0.37	0.63	0.83	0.10	0.0014	791	791	7	98	78	0.09	0.0013	746	11	4	5 96	54	0.97	0.90	0.40	0.61
M52121122 M12121211	2.88	0.07	0.36	0.62	0.86	0.10	0.0015	769 417	769	2	96	65	0.10	0.0014	725	<u>11</u> 6	4	94	43	0.97	0.91	0.42	0.61
M12121212	2.99	0.04	0.18	0.38	0.77	0.12	0.0017	395	395	2	43	59	0.12	0.0016	388	6		42	43	1.00	0.59	0.30	0.47
M22121211 M22121212	2.61	0.05	0.22	0.44	0.76	0.11	0.0016	4/9	4/9	3	52		0.11	0.0015	469 450	7		2 <u>52</u> 251	43	1.00	0.62	0.30	0.50
M32121211	2.39	0.05	0.25	0.47	0.79	0.11	0.0016	548	548	4	62	71	0.11	0.0014	536	8		62	52	1.00	0.69	0.32	0.53
M32121212 M42121211	2.45	0.05	0.23	0.45	0.82	0.11	0.0016	631	631	5	74	58	0.10	0.0015	610	8	4	5 <u>61</u> F 73	42	0.99	0.69	0.34	0.53
M42121212	2.23	0.06	0.28	0.49	0.85	0.11	0.0016	609 721	609 721	5	73	57	0.11	0.0014	591	9		3 72	39	0.99	0.90	0.36	0.57
M52121211	2.03	0.07	0.34	0.59	0.83	0.10	0.0015	713	713	7	· 88	54	0.10	0.0014	673	10	4	- <u>85</u>	35	0.97	0.93	0.45	0.61
M121212221 M12121222	2.33	0.05	0.25	0.48	0.72	0.11	0.0015	570	570	3	60 59	96 78	0.10	0.0014	559	9		60 3 59	74	1.00	0.53	0.24	0.46
M22121221	2.12	0.06	0.28	0.50	0.76	0.10	0.0014	657	657	4	73	96	0.10	0.0014	647	10		3 73	74	1.00	0.57	0.25	0.49
M22121222 M32121221	2.19	0.06	0.26	0.48	0.80	0.10	0.0015	626 743	626 743	4	5 71 5 86	78	0.10	0.0014	618 729	10	4	8 71 F 86	60 72	1.00	0.58	0.27	0.49
MB2121222	1.99	0.07	0.30	0.52	0.82	0.10	0.0014	720	720	5	85	77	0.10	0.0013	709	11	4	85	59	1.00	0.64	0.28	0.52
M42121221 M42121222	1.77	0.08	0.35	0.58	0.82	0.09	0.0013	820	820	6	98	74	0.09	0.0012	797 780	12		<u>97</u> 96	53	0.99	0.72	0.31	0.56
M52121221	1.61	0.09	0.41	0.67	0.85	0.08	0.0013	916	916	8	114	80	0.08	0.0012	869	13		111	56	0.97	0.87	0.38	0.61
M12122111	1.87	0.07	0.33	0.59	0.67	0.09	0.0013	771	771	2	52	149	0.09	0.0012	755	12		2 51	121	1.00	0.48	0.19	0.46
M12122112 M22122111	1.93	0.07	0.31	0.57	0.70	0.10	0.0013	745	745	2	<u>51</u>	133	0.09	0.0013	732	11		2 51 3 60	107	1.00	0.49	0.20	0.46
M22122112	1.76	0.08	0.34	0.59	0.73	0.09	0.0012	823	823	3	60	128	0.09	0.0012	809	13		3 60	103	1.00	0.53	0.22	0.49
M32122111 M32122112	1.57	0.09	0.37	0.64	0.74	0.08	0.0011	929 906	929 906	4	69 69	138 123	0.08	0.0011	910 889	14		69 69	110 97	1.00	0.57	0.23	0.52
M42122111	1.45	0.10	0.42	0.68	0.78	0.08	0.0011	1021	1021	5	80	131	0.08	0.0010	992	15	4	79	102	1.00	0.67	0.26	0.56
M42122112 M52122111	1.48	0.10	0.40	0.66	0.80	0.08	0.0011	998	998 1130	6	92	116	0.08	0.0010	9/1 1078	15		+ <u>78</u> + 90	89	0.98	0.68	0.27	0.56
M52122112	1.35	0.11	0.46	0.71	0.83	0.07	0.0011	1108	1108	e	91	104	0.07	0.0010	1057	16	4	89	77	0.98	0.81	0.32	0.60
M12122121	1.55	0.09	0.33	0.64	0.07	0.08	0.0011	945	945	9	66	186	0.08	0.0011	931	15		, <u>00</u> 3 66	133	1.00	0.45	0.10	0.46
M22122121 M22122122	1.38	0.10	0.41	0.68	0.71	0.07	0.0010	1076	1076	4	77	181	0.07	0.0010	1058	16	4	<u> </u>	150	1.00	0.48	0.18	0.48
M32122121	1.27	0.11	0.44	0.70	0.74	0.07	0.0009	1180	1180	5	89	173	0.07	0.0009	1159	18	4	89	142	1.00	0.54	0.20	0.52
M32122122 M42122121	1.30	0.11	0.43	0.69	0.77	0.07	0.0010	1151 1297	1151 1297	6	88	154	0.07	0.0009	1132	18		88 5 102	125	1.00	0.55	0.21	0.52
M42122122	1.19	0.12	0.47	0.71	0.80	0.06	0.0009	1268	1268	6	101	144	0.06	0.0009	1241	19	5	101	115	1.00	0.61	0.23	0.56
M52122121 M52122122	1.07	0.14	0.55	0.71	0.82	0.06	0.0009	1436	1436	8	119	145	0.06	0.0008	1381	21	6	5 117	99	0.98	0.72	0.24	0.60
M12122211 M12122212	1.70	0.08	0.35	0.62	0.69	0.09	0.0012	858	858	3	58	154	0.08	0.0012	843	13	1	58	126	1.00	0.47	0.18	0.46
M22122211	1.55	0.09	0.37	0.64	0.73	0.08	0.0011	947	947	3	67	149	0.08	0.0011	932	15		67	121	1.00	0.51	0.20	0.49
M22122212 M32122211	1.59	0.09	0.36	0.62	0.76	0.08	0.0011	924 1041	924 1041	3	67	132	0.08	0.0011	911 1023	14	3	67 77	107	1.00	0.52	0.21	0.49
M32122212	1.45	0.10	0.39	0.65	0.79	0.07	0.0011	1019	1019	4	77	127	0.07	0.0010	1003	16	4	77	101	1.00	0.57	0.23	0.52
M42122211 M42122212	1.38	0.10	0.42	0.68	0.77	0.07	0.0010	1071 1049	10/1 1049		81	140	0.07	0.0010	1050	16	4	81	112	1.00	0.59	0.23	0.56
M52122211	1.19	0.12	0.50	0.71	0.83	0.06	0.0010	1270	1270	7	103	119	0.06	0.0009	1219	19		101	91	0.98	0.75	0.28	0.60
M121222212	1.36	0.10	0.41	0.68	0.69	0.07	0.0010	1095	1095	4	5 30	118	0.07	0.0009	1080	17		3 75	163	1.00	0.43	0.16	0.80
M12122222 M22122221	1.46	0.10	0.38	0.66	0.70	0.07	0.0010	1015	1015	3	69	174	0.07	0.0010	1003	16		69 1 80	146	1.00	0.43	0.16	0.46
M22122222	1.27	0.11	0.43	0.69	0.75	0.07	0.0009	1178	1178	2	87	166	0.07	0.0009	1165	18	4	87	138	1.00	0.48	0.18	0.48
MB2122221 MB2122222	1.22	0.12	0.45	0.71	0.74	0.06	0.0009	1238	1238	5	90 89	185	0.06	0.0009	1221	19	4	90 89	156	1.00	0.48	0.17	0.52
M42122221	1.04	0.14	0.51	0.71	0.80	0.06	0.0008	1469	1469	7	116	167	0.06	0.0008	1441	22	6	116	137	1.00	0.57	0.20	0.56
M42122222 M52122221	1.16 0.96	0.12	0.46	0.71	0.79	0.06	0.0009	1302 1620	1302 1620	5	100	158 148	0.06	0.0008	1285 1566	20	6	5 <u>100</u> 5 <u>132</u>	131	1.00	0.52	0.20	0.56
M52122222	1.09	0.13	0.49	0.71	0.81	0.06	0.0008	1404	1404	e	111	151	0.06	0.0008	1381	21		111	123	1.00	0.56	0.20	0.60
M12211111 M12211112	4.32 4.51	0.02	0.13	0.31	0.59	0.15	0.0017	237 223	237 223	1	24 23	49	0.15	0.0015	227	3	1	24 23	33	0.99	0.66	0.35	0.48
M22211111	3.94	0.02	0.14	0.33	0.72	0.14	0.0016	268	268	2	29	48	0.14	0.0014	258	3	1	. 28	32	1.00	0.72	0.39	0.51
MB2211112	3.61	0.02	0.13	0.30	0.77	0.14	0.0015	∠54 303	303	2	28	48	0.13	0.0015	246	3	1	28	26	1.00	0.73	0.43	0.51
MB2211112	3.72	0.03	0.15	0.34	0.80	0.14	0.0016	289	289	2	33	39	0.14	0.0014	280	3	2	2 33	25	1.00	0.82	0.45	0.54
M42211112	3.40	0.03	0.18	0.37	0.82	0.13	0.0015	331	331	3	39	38	0.13	0.0013	315	4	-	39	23	0.99	0.94	0.50	0.58
M52211111 M52211112	3.02 3.09	0.04	0.23	0.46	0.82	0.13	0.0015	404 389	404 38.9	4	48 47	45	0.12	0.0013	367 355	4		46 2 45	26 21	0.96	1.13	0.59	0.63
M12211121	3.52	0.03	0.16	0.37	0.69	0.13	0.0015	314	314	2	32	63	0.13	0.0013	303	4	1	. 32	44	0.99	0.62	0.31	0.48
NIL2211122	3.68	0.03	0.15	0.34	0.73	0.13	0.0015	294	294	2	: 31	52	0.13	0.0014	286	3	1	. 31	36	0.99	0.64	0.34	0.48

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [•]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [•]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN-m]8	C1 [·]	¢2[·]	C3 [·]	Mdy/ Mst [·]
MB2211121 MB2211122	2.94	0.04	0.20	0.42	0.76	0.12	0.0013	404 384	404 384	3	3 46 3 45	62 51	0.12	0.0012	391 373	5		2 46 2 45	43	3 1.00 4 1.00	0.74	0.37	0.53
M42211121	2.69	0.04	0.23	0.46	0.79	0.12	0.0013	461	461	L	1 54	61	0.12	0.0012	441	5		3 54	4(0.99	0.86	0.42	0.57
M52211122	2.62	0.04	0.22	0.43	0.82	0.12	0.0013	539	539	6	+ 32 5 66	50	0.12	0.0012	415	6		2 51 3 64	36	5 0.95	1.05	0.43	0.57
M52211122 M12211211	2.53	0.05	0.28	0.51	0.85	0.11	0.0013	517 276	517 276	6	5 64	48	0.11	0.0012	478	5		3 62 1 28	25	0.96	1.07	0.51	0.62
M12211212	3.98	0.02	0.13	0.30	0.76	0.14	0.0016	263	263	2	2 28	42	0.14	0.0014	257	3		1 28	25	0.99	0.65	0.36	0.48
M22211211 M22211212	3.51	0.03	0.15	0.35	0.79	0.13	0.0015	299	313 299		2 34 2 33	51 41	0.13	0.0013	293	3		2 <u> </u>		5 1.00 3 1.00	0.69	0.37	0.50
MB2211211 MB2211212	3.18	0.03	0.18	0.38	0.78	0.13	0.0014	361	361 341	3	3 40 3 30	50 41	0.12	0.0013	350	4		2 40 2 39	34	1 1.00 7 1.00	0.77	0.40	0.53
M42211211	2.91	0.04	0.21	0.43	0.81	0.12	0.0014	413	413		3 48	49	0.12	0.0013	395	5		2 47	32	2 0.99	0.89	0.45	0.57
M42211212 M52211211	2.97	0.04	0.20	0.41	0.84	0.12	0.0014	399 469	399 469	3	3 47 5 56	40	0.12	0.0012	384 437	4		2 <u>47</u> 3 54	26	5 0.99 9 0.97	0.90	0.46	0.57
M52211212	2.70	0.04	0.25	0.48	0.87	0.12	0.0014	470	470	5	5 57	39	0.12	0.0012	434	5		3 55	22	2 0.96	1.09	0.55	0.62
M12211221 M12211222	3.10	0.04	0.18	0.39	0.72	0.12	0.0013	374 349	374 349	4	2 35	55	0.12	0.0013	364 342	4		2 <u>39</u> 238	44	3 1.00 9 0.99	0.60	0.32	0.47
M22211221 M22211222	2.82	0.04	0.20	0.42	0.75	0.12	0.0013	428	428	3	3 47 3 46	66	0.12	0.0012	418	5		2 47 2 46	4	3 1.00 9 1.00	0.64	0.32	0.50
M32211221	2.61	0.05	0.23	0.45	0.78	0.11	0.0012	479	479	3	3 55	66	0.11	0.0011	467	5		3 54	4	7 1.00	0.70	0.34	0.53
MB2211222 M42211221	2.65	0.04	0.22	0.43	0.82	0.11	0.0012	469 562	469	3	5 55 5 67	54	0.11	0.0011	459	6		<u>3 55</u> 3 66	38	3 1.00 4 0.99	0.72	0.36	0.53
M42211222	2.46	0.05	0.24	0.46	0.84	0.11	0.0012	525	525	4	1 63 7 91	53	0.11	0.0011	510	6		3 63 1 79	36	5 0.99	0.80	0.38	0.57
M52211221	2.15	0.06	0.32	0.53	0.84	0.11	0.0012	607	607	6	5 75	52	0.11	0.0011	575	7		+ 73 4 73	33	3 0.97 3 0.98	0.96	0.44	0.61
M12212111 M12212112	2.53	0.05	0.24	0.48	0.66	0.11	0.0012	503 492	503 492	2	2 33	105	0.11	0.0011	488	6		1 <u>33</u> 233	80	0 <u>1.00</u> 0 <u>1.00</u>	0.53	0.23	0.47
M22212111	2.33	0.05	0.26	0.50	0.69	0.11	0.0012	569	569	2	2 40	105	0.11	0.0011	554	6		2 40	80	1.00	0.57	0.24	0.49
M32212112 M32212111	2.40	0.05	0.25	0.48	0.72	0.10	0.0012	667	545		2 39	104	0.10	0.0011	532 649	8		2 <u>39</u> 249	75	9 1.00	0.58	0.26	0.49
M32212112	2.21	0.06	0.28	0.50	0.75	0.10	0.0011	616 716	616 716	0	3 46	92	0.10	0.0011	602	7		2 46 2 54	69	9 1.00	0.63	0.27	0.53
M42212111	1.97	0.07	0.32	0.57	0.80	0.10	0.0011	729	729	ž) 5- 1 57	89	0.10	0.0010	705	8		3 57	65	5 0.99	0.74	0.32	0.56
M52212111 M52212112	1.85	0.07	0.35	0.61	0.79	0.10	0.0011	783	783	4	1 62 1 61	96	0.09	0.0010	749	9		3 61 3 60	70	0.99	0.80	0.34	0.60
M12212121	2.00	0.07	0.31	0.57	0.66	0.10	0.0011	713	713	2	2 48	141	0.10	0.0010	697	8		2 48	113	3 1.00	0.48	0.19	0.46
M22212122 M222212121	1.89	0.06	0.29	0.58	0.68	0.09	0.0010	760	760	4 10	2 43 3 53	124	0.09	0.0010	744	g		2 45 2 53	110	1.00	0.49	0.20	0.46
M22212122	1.89	0.07	0.32	0.57	0.73	0.09	0.0010	762	762	0	3 56 3 60	121	0.09	0.0010	748	01 0		3 56 3 60	96	5 1.00	0.54	0.23	0.49
M32212121	1.80	0.08	0.33	0.59	0.75	0.09	0.0010	801	801	3	3 60	119	0.09	0.0009	786	9		3 60	93	3 1.00	0.57	0.24	0.52
M42212121 M42212122	1.55	0.09	0.39	0.66	0.77	0.08	0.0009	944 921	944 921	5	1 74 5 73	124	0.08	0.0008	916 896	11		4 74 3 73	95	5 1.00 3 0.99	0.68	0.28	0.56
M52212121	1.52	0.09	0.40	0.67	0.78	0.08	0.0009	964	964	, s	5 77	122	0.08	0.0008	932	11		4 76	93	8 0.99	0.71	0.29	0.60
MI2212122 M12212211	2.26	0.10	0.44	0.70	0.83	0.08	0.0011	595	595	2	2 85 2 40	112	0.10	0.0008	975 581	7		+ 83 2 40	87	2 0.98 7 1.00	0.83	0.34	0.60
M12212212 M22212211	2.37	0.05	0.25	0.48	0.71	0.11	0.0011	553	553	2	2 <u>38</u> 2 <u>4</u> 9	98	0.11	0.0011	542	6		2 <u>37</u> 2 <u>4</u> 8	76	5 1.00 7 1.00	0.52	0.23	0.46
M22212212	2.12	0.06	0.28	0.51	0.75	0.10	0.0011	658	658	-	3 48	98	0.10	0.0010	647	8		2 48	76	5 1.00	0.57	0.25	0.49
M32212211 M32212212	1.96	0.07	0.31	0.56	0.75	0.10	0.0010	731	731	3	3 53 3 56	110	0.10	0.0010	716	8		2 <u>53</u> 356	86	5 1.00 3 1.00	0.59	0.25	0.52
M42212211	1.74	0.08	0.36	0.62	0.79	0.09	0.0010	836	836	4	1 65 1 60	102	0.09	0.0009	811	9		3 64	76	5 0.99	0.71	0.30	0.56
M52212212	1.69	0.08	0.33	0.63	0.80	0.09	0.0010	864	864	1	+ 00 1 68	100	0.09	0.0009	834	10		3 67	7:	3 0.99	0.87	0.32	0.60
M52212212 M12212221	1.61	0.09	0.41	0.67	0.85	0.08	0.0010	915 798	915 798	6	5 74	82	0.08	0.0009	867	10	1	<u>3 72</u> 2 54	51	7 0.97	0.87	0.37	0.61
M12212222	1.92	0.07	0.31	0.56	0.70	0.09	0.0010	748	748	2	2 51	130	0.09	0.0010	736	9		2 51	100	5 1.00	0.47	0.19	0.46
M22212221 M22212222	1.65	0.08	0.35	0.62	0.73	0.08	0.0009	880	858		3 63 3 63	141	0.08	0.0009	865	10		3 63 3 63	114	+ 1.00) 1.00	0.52	0.21	0.49
M32212221	1.60	0.09	0.36	0.63	0.74	0.08	0.0009	914	914 945	3	3 67 1 71	139	0.08	0.0008	898	11		3 67 3 72	112	2 1.00	0.54	0.22	0.52
M42212221	1.39	0.10	0.42	0.68	0.79	0.07	0.0008	1063	1063	e.	5 83	127	0.07	0.0007	1036	12		4 83	99	9 1.00	0.66	0.26	0.56
M42212222 M52212221	1.52	0.09	0.38	0.64	0.79	0.08	0.0008	964 1065	964 1065	2	1 74 5 84	118	0.08	0.0008	948	11		4 74 4 83	93	3 1.00 3 1.00	0.60	0.25	0.56
M52212222	1.29	0.11	0.47	0.71	0.85	0.07	0.0008	1160	1160		7 96 De	100	0.07	0.0007	1110	13		4 <u>94</u>	74	1 0.98	0.79	0.31	0.60
M12221111	5.23	0.02	0.10	0.24	0.03	0.17	0.0019	201	201	1	20	44	0.16	0.0017	249	3		1 25	25	0.93	0.00	0.39	0.43
M22221111 M22221112	4.55	0.02	0.12	0.29	0.72	0.16	0.0018	294	294 278		2 <u>31</u> 231	. 54	0.15	0.0016	282	3		1 <u>31</u> 131	35	5 <u>1.00</u> 9 <u>1.00</u>	0.75	0.41	0.51
M32221111	4.19	0.02	0.14	0.32	0.75	0.15	0.0017	329	329	4	2 36	54	0.15	0.0015	315	4		2 36	34	1.00	0.83	0.46	0.54
M42221112	3.81	0.02	0.15	0.35	0.79	0.14	0.0017	378	378	3	3 43	53	0.13	0.0013	356	4		2 33 2 43	20	2 0.99	0.98	0.48	0.54
M42221112 M52221111	3.92	0.03	0.15	0.34	0.82	0.14	0.0017	362	362	3	<u>3 42</u> 5 51	44	0.14	0.0014	343	4		2 42 2 49	26	0.99	0.99	0.55	0.58
M52221112	3.57	0.03	0.19	0.40	0.84	0.14	0.0017	426	426		5 51	42	0.14	0.0014	385	4		2 49	22	2 0.95	1.20	0.67	0.63
M12221121 M12221122	4.06	0.02	0.14	0.33	0.68	0.14	0.0016	344 319	344 319	2	2 <u>35</u> 234	71	0.14	0.0015	330	4		2 <u>35</u> 234	4	3 0.99 9 0.99	0.65	0.34	0.48
M22221121	3.73	0.03	0.15	0.35	0.72	0.14	0.0015	385	385	2	2 41	. 71	0.14	0.0014	371	4		2 41	4	3 1.00 0 1.00	0.70	0.37	0.51
M32221122	3.39	0.03	0.14	0.32	0.75	0.13	0.0015	439	439		3 50	. 50 I 69	0.13	0.0014	423	4		2 49		5 1.00	0.72	0.41	0.54
M32221122	3.55	0.03	0.16	0.35	0.79	0.13	0.0015	411 490	411	3	3 47 1 57	57	0.13	0.0013	399	5		2 47	31	7 1.00	0.79	0.43	0.54
M42221122	3.21	0.03	0.19	0.39	0.82	0.13	0.0015	478	478	2	1 57	56	0.13	0.0013	457	5		3 57	3	5 0.99	0.92	0.48	0.57
M52221121 M52221122	2.85	0.04	0.25	0.48	0.81	0.12	0.0014	585 543	585	6	5 67	55	0.12	0.0013	533	6		<u>3 68</u> 3 64	35	0.96 2 0.97	1.11	0.56	0.63
M12221211	4.43	0.02	0.12	0.29	0.72	0.15	0.0017	305	305	2	2 31	. 57	0.15	0.0015	294	3		1 31	38	3 0.99	0.67	0.37	0.48
M22221212	4.03	0.02	0.11	0.32	0.75	0.14	0.0015	346	346	2	2 37	57	0.14	0.0010	335	4		2 37	3	3 1.00	0.73	0.40	0.43
M22221212 M32221211	4.19	0.02	0.13	0.29	0.79	0.15	0.0016	327 398	327	2	2 <u>36</u> 3 <u>4</u> 4	47	0.14	0.0015	319	4		2 36 2 44	3.	L 1.00	0.73	0.41	0.50
MB2221212	3.77	0.03	0.15	0.32	0.82	0.14	0.0016	378	378		3 43	46	0.14	0.0014	367	4		2 43	30	0 1.00	0.82	0.46	0.54
M42221211 M42221212	3.35 3.48	0.03	0.18	0.38	0.81	. 0.13 0.13	0.0015	449 425	449 425		+ 52 150	55 46	0.13	0.0013	428 408	5		2 51 2 49	34	+ 0.99 3 <u>0.9</u> 9	0.94	0.49	0.58
M52221211	3.06	0.04	0.23	0.45	0.83	0.13	0.0015	528	528	6	63	53	0.13	0.0013	481	6		3 60	30	0.95	1.14	0.60	0.63
M12221221	3.59	0.03	0.22	0.35	0.72	0.13	0.0015	407	407	2	2 43	75	0.13	0.0013	395	5		2 42	52	2 0.99	0.63	0.33	0.48
M12221222 M22221221	3.76 3.26	0.03	0.14	0.32	0.76	0.14	0.0015	380 464	380 464	2	2 41 3 51	61 74	0.13	0.0014	372 452	4		2 41 2 51	43	0.99 2 1.00	0.64	0.35	0.47
M22221222	3.37	0.03	0.16	0.35	0.79	0.13	0.0014	442	442	3	3 50	61	0.13	0.0013	433	5		2 50	4	2 1.00	0.69	0.37	0.50
MB2221221 MB2221222	3.11	0.04	0.20	0.41	0.78	0.12	0.0013	529 496	529 496	1	1 58	61	0.12	0.0012	485	6		, 0 0 3 58	4	L 1.00	0.75	0.38	0.53

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [•]	Disp [m]	ISDR [-]	Total Moment [MN∙m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [-]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN-m]8	C1 [-]	c2[·]	C3 [•]	Mdy/ Mst [·]
M52221221 M52221222	2.48	0.05	0.28	0.52	0.84	0.11	0.0013	710 656	710 656	5	7 87 7 81	69 58	0.11	0.0012	656 617	8	2	83 78	42	0.96	1.06	0.51	0.62
M12222111	2.92	0.04	0.21	0.44	0.65	0.12	0.0013	545	545	2	2 36	117	0.12	0.0012	526	6	2	2 35	86	1.00	0.57	0.26	0.47
M22222112	2.64	0.04	0.23	0.42	0.03	0.12	0.0013	631	631	2	2 44	105	0.11	0.0012	612	7	2	2 44	86	1.00	0.58	0.28	0.47
M22222112 M32222111	2.77	0.04	0.21	0.44	0.72	0.12	0.0013	588 687	588 687	2	2 42 3 50	103 116	0.11	0.0012	572 666	7	2	2 42 2 50	75	1.00	0.62	0.29	0.50
M32222112	2.48	0.05	0.24	0.47	0.76	0.11	0.0012	690 915	690 915	3	3 52 1 63	101	0.11	0.0011	671	8	2	2 52	73	1.00	0.69	0.32	0.53
M42222111	2.28	0.06	0.28	0.51	0.79	0.11	0.0012	789	789	4	+ 62	110	0.11	0.0011	759	9	-	61	69	0.99	0.80	0.36	0.57
M52222111 M52222112	2.14	0.06	0.31	0.56	0.78	0.11	0.0012	870 917	870 917	6	5 68 5 75	<u>111</u> 94	0.10	0.0011	827	10		68 8 72	77 62	0.99	0.85	0.38	0.61
M12222121	2.31	0.05	0.27	0.51	0.66	0.11	0.0011	767	767	4	2 52	156	0.11	0.0011	746	9	2	2 51	121	1.00	0.52	0.21	0.46
M22222121	2.19	0.05	0.28	0.53	0.69	0.10	0.0011	836	836		3 58	156	0.10	0.0011	815	10	5	58	103	1.00	0.55	0.23	0.49
M22222122 M32222121	2.25	0.06	0.27	0.50	0.72	0.10	0.0011	798	798 981	3	3 <u>57</u> 1 74	137	0.10	0.0011	781	9		3 57 3 73	105	1.00	0.56	0.24	0.49
MB2222122	2.00	0.07	0.31	0.55	0.76	0.10	0.0011	953 1076	953 1076	2	1 73	135	0.10	0.0010	932	11	3	3 73	103	1.00	0.63	0.27	0.52
M42222121	1.73	0.08	0.32	0.51	0.77	0.10	0.0011	990	990	-) 04 1 77	133	0.10	0.0009	966	12	2	r 04 I 77	100	1.00	0.71	0.30	0.56
M52222121 M52222122	1.76	0.08	0.36	0.63	0.78	0.09	0.0010	1100 1171	1100	6	5 <u>87</u> 7 <u>9</u> 7	142	0.09	0.0009	1060	12	4	- 86 5 94	105	0.99	0.74	0.31	0.60
M12222211	2.61	0.05	0.23	0.46	0.68	0.11	0.0012	641	641 506	2	2 43	124	0.11	0.0012	624	7	2	2 43	93	1.00	0.55	0.25	0.47
M22222212	2.74	0.04	0.21	0.44	0.71	0.11	0.0012	704	704	2	2 40	103	0.11	0.0012	687	8	2	2 49	93	1.00	0.58	0.26	0.47
M22222212 M32222211	2.44	0.05	0.24	0.47	0.75	0.11	0.0012	706	706	3	3 51 4 62	108	0.11	0.0011	692 814	8		2 51 3 62	81 91	1.00	0.60	0.28	0.49
MB2222212	2.23	0.06	0.27	0.49	0.78	0.11	0.0011	809	809	2	1 61	108	0.10	0.0011	791	9		61	80	1.00	0.67	0.30	0.53
M42222211 M422222212	2.10	0.06	0.30	0.54	0.77	0.10	0.0011	861	861	4	+ 67 1 66	121	0.10	0.0010	804	10		s 67 3 66	90 78	1.00	0.69	0.30	0.56
M52222211 M52222212	1.84	0.07	0.37	0.64	0.82	0.09	0.0011	1063 1046	1063 1046	6	5 86 5 85	108 96	0.09	0.0010	1002 987	12	4	83	75	0.97	0.90	0.40	0.61
M12222221	2.09	0.06	0.29	0.54	0.69	0.10	0.0011	894	894	3	61	166	0.10	0.0010	875	10		60	132	1.00	0.50	0.20	0.46
M122222222 M222222221	1.99	0.06	0.27	0.50	0.70	0.10	0.0011	959	959	3	s 56 3 67	146	0.10	0.0011	941	9 11	3	s 56 3 67	115	1.00	0.50	0.21	0.46
M22222222 M32222222	1.96	0.07	0.30	0.54	0.75	0.10	0.0010	977 1102	977 1102	4	1 72 5 83	145	0.10	0.0010	961	11	3	8 7 <u>1</u> 83	114	1.00	0.55	0.23	0.49
M32222222	1.79	0.08	0.33	0.58	0.78	0.09	0.0010	1077	1077		82	139	0.09	0.0009	1057	12	4	82	107	1.00	0.61	0.26	0.52
M422222221 M422222222	1.72	0.08	0.35	0.61	0.76	0.09	0.0010	1125	1125	5	5 85 5 85	135	0.09	0.0009	1100	13	4	- 85 - 84	120	1.00	0.62	0.26	0.56
M52222221 M52222222	1.47	0.10	0.44	0.70	0.83	0.08	0.0009	1348 1324	1348	8	3 110 3 109	133	0.08	0.0008	1280 1259	15	0 0	108	97 84	0.98	0.84	0.35	0.60
M13111111	6.26	0.01	0.09	0.21	0.68	0.18	0.0019	229	229	1	. 23	51	0.18	0.0016	216	2	1	. 23	31	0.99	0.76	0.42	0.47
M13111112 M23111111	5.71	0.01	0.08	0.19	0.73	0.18	0.0019	213	213	1	. 22	42	0.18	0.0017	203	3	1	. 22	25	1.00	0.78	0.44	0.47
M23111112 M33111111	5.92	0.01	0.09	0.21	0.76	0.18	0.0018	247	247 304	1	27	42	0.17	0.0016	238	2	1	. 27	25	1.00	0.83	0.48	0.49
M33111112	5.39	0.02	0.10	0.23	0.79	0.17	0.0018	287	287		2 32	42	0.17	0.0015	275	3	1	. 32	25	1.00	0.91	0.52	0.52
M43111111 M43111112	4.78	0.02	0.12	0.29	0.78	0.16	0.0017	350	350	2	2 40 2 39	42	0.16	0.0015	326	3	1	. 39 . 39	29	0.99	1.03	0.60	0.56
M53111111	4.36	0.02	0.16	0.35	0.81	0.16	0.0017	412	412	3	3 49 2 49	49	0.16	0.0015	363	4	1	. 46	26	0.94	1.27	0.75	0.61
M13111121	5.10	0.02	0.11	0.27	0.68	0.17	0.0017	317	317	1	32	68	0.16	0.0015	301	3	1	. 32	44	0.99	0.68	0.37	0.01
M13111122 M23111121	4.65	0.02	0.10	0.23	0.73	0.17	0.0017	292 358	358	1	. 31 . 38	55	0.17	0.0015	343	3	1	. 31	35	0.99	0.71	0.39	0.46
M23111122	4.84	0.02	0.11	0.26	0.76	0.16	0.0016	338	338	1	38	55	0.16	0.0014	327	3	1	. 37	35	1.00	0.76	0.43	0.49
MB3111122	4.41	0.02	0.13	0.29	0.79	0.15	0.0016	383	383	2	2 44	55	0.15	0.0013	369	4	1	. 44	34	1.00	0.85	0.48	0.52
M43111121 M43111122	3.90	0.03	0.16	0.35	0.78	0.14	0.0015	459	459 439		3 53 3 52	66 54	0.14	0.0013	431	4	2	2 53 2 52	39	0.99	0.98	0.54	0.55
M53111121	3.56	0.03	0.19	0.41	0.81	0.14	0.0015	538	538	4	1 65 1 64	63	0.14	0.0013	482	5	2	2 62	35	0.95	1.20	0.66	0.60
M13111211	5.56	0.02	0.10	0.23	0.71	0.17	0.0018	274	274	1	28	54	0.17	0.0015	263	3	1	. 28	34	0.99	0.72	0.40	0.00
M13111212 M23111211	5.77	0.01	0.09	0.21	0.76	0.17	0.0018	257 319	257 319	1	L 27 L 34	44 54	0.17	0.0016	249	3	1	. 27 . 34	28	0.99	0.74	0.42	0.46
M23111212	5.21	0.02	0.10	0.23	0.79	0.17	0.0017	302	302	1	33	44	0.16	0.0015	293	3	1	. 33	28	1.00	0.78	0.45	0.49
M33111211	4.73	0.02	0.12	0.26	0.75	0.16	0.0017	348	348	2	2 40	44	0.16	0.0014	337	4	1	. 40 . 40	27	1.00	0.87	0.43	0.52
M43111211 M43111212	4.20	0.02	0.14	0.32	0.80	0.15	0.0016	413	413 399	2	2 47 3 47	53	0.15	0.0013	390	4	2	<u>2 47</u> . 46	31	0.99	1.01	0.57	0.55
M53111211	3.83	0.03	0.18	0.38	0.83	0.15	0.0016	488	488	4	1 58 1 57	51	0.14	0.0013	436	4	2	2 55	27	0.94	1.23	0.69	0.61
M13111212	4.50	0.03	0.12	0.29	0.80	0.15	0.0015	374	374	1	+ 37 L 39	72	0.15	0.0014	360	4	1	. 39	48	0.99	0.67	0.37	0.00
M13111222 M23111221	4.68	0.02	0.11	0.26	0.76	0.16	0.0016	353	353 425	2	2 38 2 46	59	0.15	0.0014	344	4	1	. 38 . 46	39	0.99	0.67	0.38	0.46
M23111222	4.23	0.02	0.13	0.29	0.79	0.15	0.0015	405	405	2	2 46	58	0.15	0.0013	394	4	1	. 45	38	1.00	0.74	0.42	0.48
M33111221	3.84	0.03	0.15	0.34	0.78	0.14	0.0014	462	462	1	2 54	58	0.14	0.0012	405	5	2	2 54	37	1.00	0.82	0.44	0.51
M43111221 M43111222	3.41	0.03	0.18	0.38	0.80	0.13	0.0014	551 531	551 531	3	3 65 3 64	69 57	0.13	0.0012	524 508	5	2	2 64	43	0.99	0.94	0.49	0.55
M53111221	3.11	0.04	0.22	0.45	0.83	0.13	0.0014	648	648	5	5 79	66	0.13	0.0012	589	6	2	2 75	37	0.95	1.15	0.61	0.60
M13112111	3.61	0.03	0.22	0.44	0.86	0.14	0.0013	507	507	1	33	112	0.13	0.0012	485	5	1	. 33	78	0.95	0.62	0.62	0.80
M13112112 M23112111	3.73	0.03	0.15	0.35	0.68	0.14	0.0014	484	484 572	1	33	99 111	0.14	0.0012	466	5	1	. 33	68 77	0.99	0.63	0.32	0.46
M23112112	3.40	0.03	0.17	0.37	0.72	0.13	0.0013	549	549	1	39	97	0.13	0.0012	531	6	1	. 39	67	1.00	0.68	0.35	0.48
MB3112111 MB3112112	3.04	0.04	0.20	0.42	0.76	0.12	0.0012	645	645	2	2 47 2 46	109	0.12	0.0011	601	6	1	. 47	/5 65	1.00	0.75	0.38	0.51
M43112111 M43112112	2.79	0.04	0.23	0.46	0.76	0.12	0.0012	733	733	3	3 55	107	0.12	0.0011	696	7	2	2 56	71	0.99	0.87	0.42	0.55
M53112111	2.56	0.05	0.28	0.52	0.80	0.12	0.0012	854	854	2	1 68	102	0.12	0.0011	782	8	2	2 66	64	0.96	1.07	0.51	0.59
M53112112 M13112121	2.60	0.05	0.27	0.50	0.82	0.12	0.0012	831	831 689	- 1	+ 67 L 46	90	0.12	0.0011	762	8	1	<u>64</u> . 46	108	0.96	1.08	0.52	0.59
M13112122 M23112121	3.00	0.04	0.19	0.42	0.69	0.12	0.0012	656 781	656 781	1	45	129	0.12	0.0011	636 756	7	1	. 45	94	1.00	0.58	0.28	0.45
M23112122	2.00	0.04	0.23	0.44	0.09	0.12	0.0011	748	748	2	2 54	140	0.11	0.0010	738	8	2	2 54	93	1.00	0.63	0.30	0.48
MB3112121 MB3112122	2.45	0.05	0.25	0.48	0.73	0.11	0.0011	885	885	3	s 66 3 64	144	0.11	0.0010	856	9	2	<u>66</u> 64	105	1.00	0.69	0.31	0.51
M43112121	2.25	0.06	0.29	0.53	0.77	0.11	0.0011	1007	1007 974		3 78	141	0.11	0.0010	965	10	2	78	100	0.99	0.79	0.35	0.54
M53112122	2.06	0.06	0.34	0.60	0.80	0.10	0.0011	1166	1166		5 95	132	0.10	0.0010	1089	11	3	3 93	90	0.93	0.94	0.43	0.54
M53112122	2.10	0.06	0.33	0.59	0.82	0.11	0.0011	1134	1134	1	93	116	0.10	0.0010	1059	11		3 90	77	0.97	0.96	0.44	0.59

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [·]	Disp [m]	ISDR [·]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [·]3	Total Moment [MN·m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN-m]8	C1 [·]	c2[·]	C3 [·]	Mdy/ Mst [·]
M23112211 M23112212	2.99 3.06	0.04	0.20	0.42	0.72	0.12	0.0012	658 635	658 635	2	2 46 2 46	116 103	0.12	0.0011	638 618	7	1	L 46 L 45	83 73	1.00	0.65	0.32	0.48
M33112211	2.74	0.04	0.22	0.44	0.75	0.12	0.0012	747	747	2	55	115	0.12	0.0010	723	8		2 55	81	1.00	0.72	0.35	0.51
M33112212 M43112211	2.80	0.04	0.21	0.43	0.78	0.11	0.0012	852	852	4	: 54 65	102	0.12	0.0011	816	8		2 <u>54</u> 265	71	0.99	0.73	0.39	0.54
M43112212	2.56	0.05	0.24	0.47	0.81	0.11	0.0011	830	830 994	3	64 G4	99	0.11	0.0010	798	8		2 64	67	0.99	0.84	0.40	0.54
M53112212	2.33	0.05	0.30	0.55	0.84	0.11	0.0011	972	972	4	78	94	0.11	0.0010	902	9	1	2 75	59	0.96	1.02	0.48	0.59
M13112221 M13112222	2.62	0.04	0.23	0.46	0.68	0.11	0.0011	796	796	2	54 53	155 137	0.11	0.0010	774	8	1	2 53 2 53	117	1.00	0.55	0.25	0.45
M23112221	2.40	0.05	0.25	0.48	0.72	0.11	0.0010	909	909	2	65	155	0.11	0.0010	887	9		2 65	117	1.00	0.59	0.26	0.47
M23112222 M33112221	2.46	0.05	0.24	0.46	0.75	0.11	0.0010	1038	1038	3	2 64 3 77	136	0.11	0.0010	1011	9	1	2 <u>64</u> 277	102	1.00	0.60	0.28	0.47
M33112222	2.24	0.06	0.27	0.49	0.78	0.11	0.0010	1004	1004	3	76	135	0.10	0.0009	981	10		2 76	100	1.00	0.67	0.30	0.50
M43112221	2.01	0.07	0.32	0.55	0.73	0.10	0.0010	1154	1154	4	91	143	0.10	0.0009	1145	12	-	3 <u>32</u> 3 90	95	0.99	0.76	0.33	0.54
M53112221 M53112222	1.84	0.07	0.37	0.64	0.82	0.10	0.0010	1325	1325	5	108	135	0.10	0.0009	1249	13		3 105	94	0.97	0.90	0.40	0.58
M13121111	7.23	0.01	0.07	0.18	0.67	0.19	0.0021	246	246	1	. 24	57	0.19	0.0017	230	2	1	L 24	33	0.99	0.81	0.45	0.48
M13121112 M23121111	6.59	0.01	0.07	0.16	0.72	0.20	0.0021	228	228	1	. 24	46	0.19	0.0018	217	2	1	L 23 L 29	27	0.99	0.83	0.47	0.47
M23121112	6.84	0.01	0.08	0.18	0.76	0.19	0.0020	264	264	1	. 29	46	0.18	0.0017	252	3	1	1 29	27	1.00	0.89	0.51	0.50
MB3121111 MB3121112	6.22	0.01	0.09	0.22	0.74	0.18	0.0019	324	324	2	2 35	46	0.18	0.0016	291	3		L 35 L 34	33 26	1.00	0.96	0.56	0.52
M43121111	5.52	0.02	0.11	0.25	0.77	0.18	0.0019	376	376	2	2 43	57	0.17	0.0016	349	4	1	L 42	31	0.99	1.09	0.64	0.56
M53121112	5.04	0.01	0.10	0.25	0.81	0.18	0.0019	455	455	4	54	56	0.17	0.0016	398	4		2 50	23	0.98	1.31	0.80	0.62
M53121112	5.16	0.02	0.13	0.30	0.83	0.17	0.0019	436	436	4	52	46	0.17	0.0016	383	4		2 49	22	0.93	1.32	0.82	0.61
M13121122	6.16	0.01	0.08	0.20	0.72	0.18	0.0019	311	311	1	. 33	61	0.17	0.0016	298	3		L 33	37	0.99	0.76	0.42	0.47
M23121121 M23121122	5.37	0.02	0.10	0.24	0.71	0.17	0.0018	387	387 361	2	2 41 9 40	75	0.17	0.0015	369	4	1	L 41 40	47	1.00	0.79	0.44	0.49
M33121121	4.91	0.02	0.11	0.27	0.74	0.17	0.0017	444	444	2	50	75	0.16	0.0015	422	4	4	2 49	46	1.00	0.87	0.49	0.52
MB3121122 M43121121	5.09 4.50	0.02	0.11	0.25	0.79	0.17	0.0017	420 505	420 505	2	2 48 58	61 74	0.17	0.0015	403	4		2 48 2 58	37	1.00	0.88	0.50	0.52
M43121122	4.64	0.02	0.13	0.29	0.81	0.16	0.0017	482	482	3	57	61	0.16	0.0014	454	5	1	2 56	35	0.99	1.03	0.59	0.56
M53121121 M53121122	4.11	0.02	0.17	0.37	0.80	0.15	0.0015	593	593	5	5 70	59	0.15	0.0014	525	5		2 67	38	0.94	1.25	0.72	0.61
M13121211	6.42	0.01	0.08	0.20	0.71	0.18	0.0019	292	292	1	. 30	60	0.18	0.0016	278	3	11	L 29	36	0.99	0.77	0.43	0.47
M23121212	5.83	0.01	0.08	0.18	0.76	0.18	0.0013	339	339	1	. 23	60	0.17	0.0017	324	3	1	L 36	37	1.00	0.82	0.47	0.49
M23121212 M33121211	6.02 5.32	0.01	0.08	0.19	0.79	0.18	0.0019	320	320	1	35	49	0.17	0.0016	310	3		L 35 43	30	1.00	0.84	0.48	0.49
M33121212	5.46	0.02	0.10	0.22	0.81	0.17	0.0018	374	374	2	2 43	49	0.17	0.0015	360	4	1	L 42	29	1.00	0.92	0.53	0.52
M43121211 M43121212	4.85	0.02	0.12	0.28	0.80	0.17	0.0018	455	455 439		52	60 49	0.16	0.0015	427	4		2 <u>51</u> 2 51	34	0.99	1.04	0.60	0.56
M53121211	4.42	0.02	0.15	0.34	0.83	0.16	0.0017	538	538	4	64	59	0.16	0.0015	477	5		2 60	30	0.94	1.28	0.76	0.61
M53121212 M13121221	4.50	0.02	0.15	0.33	0.85	0.15	0.0018	523 407	523 407	1	. 42	48	0.16	0.0015	391	5	1	2 <u>59</u> L 42	 52	0.94	0.69	0.38	0.61
M13121222	5.40	0.02	0.10	0.22	0.75	0.17	0.0017	381	381	2	2 41	65	0.17	0.0015	370	4	1	L 41	42	0.99	0.71	0.41	0.46
M23121221	4.88	0.02	0.12	0.25	0.79	0.16	0.0016	400	400	2	2 50	65	0.16	0.0014	443	5		2 50	42	1.00	0.76	0.43	0.43
M33121221 M33121222	4.31	0.02	0.13	0.30	0.77	0.15	0.0016	528 506	528 506	3	60 59	79	0.15	0.0013	507	5	1	2 <u>59</u> 2 59	50 40	1.00	0.85	0.48	0.52
M43121221	3,93	0.03	0.15	0.34	0.80	0.15	0.0015	603	603	4	70	78	0.14	0.0013	570	6	1	2 70	46	0.99	0.99	0.55	0.55
M43121222 M53121221	4.03	0.02	0.15	0.32	0.83	0.15	0.0015	581	581	6	69	64	0.15	0.0013	553 638	6		2 68 3 81	37	0.99	1.00	0.56	0.55
M53121222	3.66	0.03	0.18	0.39	0.86	0.14	0.0015	688	688	e	84	62	0.14	0.0013	621	6	3	3 80	32	0.94	1.21	0.68	0.60
M13122111 M13122112	4.17	0.02	0.14	0.34	0.68	0.15	0.0015	531	531	1	. 36	126	0.15	0.0013	528	5	1	L 30 L 35	74	0.99	0.65	0.33	0.46
M23122111	3.82	0.03	0.15	0.35	0.69	0.14	0.0014	625	625	2	2 43	125	0.14	0.0013	597	6	100	L 43	84	1.00	0.71	0.37	0.49
MB3122112	3.51	0.03	0.17	0.37	0.72	0.13	0.0014	703	703	2	2 51	123	0.13	0.0013	671	7		2 51	81	1.00	0.72	0.40	0.43
M33122112	3.59	0.03	0.16	0.36	0.75	0.14	0.0014	678 797	678 797	2	50	108	0.13	0.0012	651	7		2 50	71	1.00	0.80	0.42	0.52
M43122112	3.29	0.03	0.19	0.40	0.78	0.13	0.0013	773	773	3	60	106	0.13	0.0012	732	8		2 59	67	0.99	0.93	0.48	0.55
M53122111 M53122112	2.96	0.04	0.24	0.47	0.79	0.13	0.0013	929 904	929 904	4	74	114 101	0.12	0.0012	842	9		2 71 2 69	68 59	0.96	1.12	0.57	0.60
M13122121	3.35	0.03	0.18	0.40	0.65	0.13	0.0013	750	750	2	2 50	165	0.13	0.0012	718	7	1	L 49	117	1.00	0.60	0.29	0.46
M13122122 M23122121	3.46	0.03	0.17	0.37	0.69	0.13	0.0013	847	/14 847	2	2 49	144	0.13	0.0012	816	9	1	L 49 2 59	102	1.00	0.61	0.30	0.46
M23122122	3.16	0.03	0.18	0.40	0.72	0.13	0.0012	811	811	2	58	143	0.12	0.0011	785	8		2 58	101	1.00	0.67	0.33	0.48
MB3122121 MB3122122	2.82	0.04	0.22	0.44	0.75	0.12	0.0012	920	930	3	5 71 5 69	101	0.12	0.0011	889	9	-	2 69	98	1.00	0.73	0.37	0.51
M43122121	2.60	0.05	0.25	0.48	0.76	0.12	0.0012	1086	1086	4	84	158	0.11	0.0010	1035	11		3 84 3 82	107	0.99	0.84	0.39	0.55
M53122122	2.38	0.05	0.30	0.55	0.80	0.11	0.0012	1262	1262	6	102	150	0.11	0.0010	1162	10	3	3 99	96	0.96	1.03	0.48	0.59
M53122122 M13122211	2.43	0.05	0.29	0.54	0.82	0.11	0.0012	1227 633	1227 633	1	42	132	0.11	0.0011	1132 607	12	1	<u> </u>	83	0.96	1.04	0.50	0.59
M13122212	3.89	0.03	0.14	0.33	0.71	0.14	0.0014	607	607	1	41	116	0.14	0.0013	587	6		L 41	80	0.99	0.64	0.34	0.46
M23122211 M23122212	3.45	0.03	0.17	0.37	0.71	0.13	0.0013	716	716 691	2	2 50 2 49	131	0.13	0.0012	691	7		2 50 2 49	90 79	1.00	0.68	0.35	0.48
M33122211	3.17	0.03	0.19	0.40	0.75	0.13	0.0013	810	810	2	: 59	129	0.12	0.0011	780	8		2 59	88	1.00	0.76	0.39	0.51
M33122212 M43122211	2.90	0.03	0.18	0.38	0.77	0.13	0.0013	923	923	3	58	114	0.13	0.0011	878	8		2 58 2 70	83	0.99	0.78	0.40	0.51
M43122212	2.95	0.04	0.21	0.43	0.80	0.12	0.0012	899	899	3	69	112	0.12	0.0011	859	9		2 69	72	0.99	0.89	0.45	0.55
M53122211	2.69	0.04	0.27	0.30	0.81	0.12	0.0012	1075	1075		5 <u>84</u>	106	0.12	0.0011	968	10		3 81	63	0.96	1.09	0.54	0.59
M13122221	3.03	0.04	0.19	0.42	0.68	0.12	0.0012	863	863	2	58	173	0.12	0.0011	835	9		2 58	126	1.00	0.58	0.28	0.45
M23122221	2.77	0.03	0.18	0.40	0.71	0.12	0.0011	981	981	2	2 70	133	0.12	0.0011	953	° 10	-	2 70	125	1.00	0.63	0.29	0.43
M23122222 M33122221	2.84	0.04	0.20	0.42	0.75	0.12	0.0011	946 1115	946 1115	3	69	152	0.12	0.0011	923	10		2 69	110	1.00	0.64	0.32	0.48
MB3122222	2.59	0.05	0.23	0.45	0.78	0.11	0.0011	1080	1080	3	82	150	0.11	0.0010	1052	11		3 82	107	1.00	0.71	0.34	0.51
M43122221 M43122222	2.33	0.05	0.28	0.50	0.78	0.11	0.0011	1275 1239	1275 1239	4	99	167 147	0.11	0.0010	1224 1193	13 12		3 <u>98</u> 3 97	116 101	0.99	0.81	0.36	0.54
M53122221	2.13	0.06	0.33	0.59	0.82	0.11	0.0011	1481	1481	6	120	157	0.11	0.0010	1381	14	4	1 116	104	0.97	0.97	0.45	0.59
M13211111	2.16 9.64	0.06	0.32	0.58	0.84	0.11	0.0011	1447 161	1447 161	e1	<u>118</u>	138	0.11	0.0010	1351	14	(+ 114) <u>1</u> 6	89 21	0.97	0.98	0.46	0.59
M13211112	10.06	0.01	0.04	0.12	0.72	0.23	0.0019	149	149	1	15	31	0.22	0.0015	141	1	(15	17	0.99	0.82	0.53	0.48
M23211112	9.12	0.01	0.05	0.13	0.75	0.22	0.0018	171	171	1	. 18	39	0.21	0.0014	162	1	1	13	17	1.00	0.93	0.58	0.50

Name	T1[s]	Sa(T1) [g]	Sa(T2) [g]	Sa(T3) [g]	DOC [•]	Disp [m]	ISDR [•]	Total Moment [MN·m]	Total Shear [MN]	Max CB Shear [MN]	Max Pp [MN]	Max wall moment [MN-m]	Disp [m]2	ISDR [-]3	Total Moment [MN∙m]4	Total Shear [MN]5	Max CB Shear [MN]6	Max Pp [MN]2	Max wall moment [MN·m]8	C1 [-]	[.]	C3 [·]	Mdy/ Mst [•]
M43211111 M43211112	7.36	0.01	0.08	0.18	0.76	0.20	0.0017	244 232	244 232	2	2 27 2 26	40	0.19	0.0013	221	2		1 27 1 26	20 16	0.98	1.25	0.73 0.75	0.58
M53211111 M53211112	6.72	0.01	0.10	0.23	0.79	0.19	0.0017	298 285	298 285	3	3 34 3 34	40	0.19	0.0013	250	2		1 <u>32</u> 1 31	18	0.92	1.48	0.93	0.64
M13211121	7.85	0.01	0.07	0.17	0.66	0.20	0.0017	218	218	1	L 22	53	0.20	0.0014	202	2		1 21	30	0.99	0.83	0.46	0.48
M13211122 M23211121	8.21 7.16	0.01	0.05	0.15	0.71	0.19	0.0017	201 249	201	1	L 21 L 26	42	0.20	0.0014	233	2		1 21	24	1.00	0.83	0.49	0.48
M23211122 M83211121	7.45	0.01	0.07	0.16	0.75	0.20	0.0016	232	232	1	L 25	43	0.19	0.0013	221	2		1 25 1 31	24	1.00	0.92	0.53	0.50
MB3211122	6.78	0.01	0.08	0.18	0.78	0.19	0.0016	268	268		2 30	43	0.18	0.0012	254	2		1 30	23	1.00	1.02	0.59	0.53
M43211121 M43211122	6.18	0.01	0.10	0.23	0.76	0.18	0.0015	331 314	331 314		2 38 2 37	52 43	0.18	0.0012	291	2		1 37 1 36	28	0.98	1.14	0.68	0.57
M53211121 M53211122	5.49 5.63	0.02	0.12	0.29	0.79	0.18	0.0015	401	401	3	3 48 3 46	52 42	0.18	0.0012	346	3		1 44 1 43	25	0.93	1.36	0.85	0.62
M13211211	8.56	0.01	0.06	0.14	0.70	0.21	0.0017	190	190	1	L 19	41	0.20	0.0014	178	1		1 19	23	0.99	0.83	0.49	0.48
M13211212 M23211211	7.77	0.01	0.05	0.13	0.75	0.20	0.0017	218	218	1	L 19 L 23	42	0.19	0.0014	206	2		1 18	23	1.00	0.82	0.54	0.47
M23211212 M33211211	8.02	0.01	0.06	0.14	0.78	0.20	0.0017	206 251	206 251	1	L 23	34	0.20	0.0013	197	2		1 22 1 27	19	1.00	0.92	0.56	0.50
MB3211212	7.28	0.01	0.07	0.16	0.80	0.20	0.0016	239	239	1	L 27	35	0.19	0.0013	228	2		1 27	18	1.00	1.05	0.61	0.53
M43211211 M43211212	6.62	0.01	0.09	0.20	0.79	0.19	0.0016	294	294		2 33	35	0.18	0.0012	262	2		1 33	17	0.98	1.18	0.69	0.56
M53211211 M53211212	5.89 6.01	0.01	0.11	0.26	0.81	0.18	0.0016	358 346	358 346	3	<u>3 42</u> 3 41	42	0.18	0.0013	309	2		1 <u>39</u> 138	19	0.93	1.40	0.88	0.63
M13211221	6.93	0.01	0.08	0.18	0.70	0.19	0.0015	261	261	1	L 27	56	0.18	0.0013	247	2		1 26	33	0.99	0.80	0.45	0.47
M23211222	6.29	0.01	0.07	0.18	0.75	0.19	0.0015	301	301	1	L 20 L 32	45 56	0.19	0.0013	234	2		1 20	33	1.00	0.82	0.47	0.47
M23211222 M33211221	6.51 5.74	0.01	0.08	0.18	0.78	0.18	0.0015	284 347	284 347	1	L 32 2 39	45	0.18	0.0012	273	2		1 <u>32</u> 139	27	1.00	0.87	0.50	0.49
M33211222	5.91	0.01	0.09	0.20	0.80	0.18	0.0014	330	330	2	2 38	46	0.17	0.0012	317	2		1 38	26	1.00	0.95	0.55	0.52
M43211221	5.37	0.02	0.11	0.26	0.79	0.17	0.0014	389	389	3	> 47 3 46	46	0.17	0.0011	366	9		1 45	25	0.99	1.08	0.63	0.56
M53211221 M53211222	4.78	0.02	0.14	0.32	0.82	0.17	0.0014	487	487	4	4 58 4 57	55 45	0.16	0.0012	428	3		2 <u>55</u> 2 54	27	0.94	1.30	0.79	0.61
M13212111	5.56	0.02	0.10	0.25	0.64	0.17	0.0014	371	371	1	L 24	89	0.17	0.0012	347	3		1 24	56	0.99	0.71	0.38	0.47
M13212112 M23212111	5.74	0.01	0.09	0.23	0.67	0.18	0.0014	424	424	1	L 23 L 29	/8 89	0.17	0.0012	401	3		1 23	49	1.00	0.73	0.40	0.47
M23212112 M33212111	5.23 4.68	0.02	0.11	0.25	0.71	0.17	0.0013	403	403	1	L 28 L 34	78	0.17	0.0011	385	3		1 28 1 34	49	1.00	0.78	0.43	0.49
M33212112	4.79	0.02	0.12	0.28	0.75	0.16	0.0013	460	460	1	L 34	78	0.16	0.0011	437	3		1 34	48	1.00	0.87	0.49	0.52
M43212111 M43212112	4.30	0.02	0.14	0.33	0.75	0.16	0.0012	539	522		2 40 2 40	87	0.15	0.0010	488	4		1 40 1 39	51 45	0.99	1.01	0.57	0.56
M53212111 M53212112	3.94	0.02	0.18	0.38	0.78	0.15	0.0012	630 613	630	3	3 49 3 49	83	0.15	0.0010	557	4		<u>1 47</u> 1 46	45	0.95	1.23	0.69	0.61
M13212121	4.47	0.02	0.13	0.32	0.64	0.16	0.0012	507	507	1	L 33	117	0.15	0.0011	480	4		1 33	78	0.99	0.66	0.34	0.47
M13212122 M23212121	4.62	0.02	0.12	0.30	0.68	0.15	0.0012	483 569	483	1	L 33 L 40	103	0.14	0.0011	542	4		1 <u>32</u> 1 40	77	1.00	0.67	0.38	0.47
M23212122 M33212121	4.21	0.02	0.14	0.32	0.71	0.15	0.0011	545 638	545 638	1	L 39 2 47	102	0.15	0.0010	523	4		1 <u>39</u> 147	67	1.00	0.73	0.40	0.49
MB3212122	3.86	0.03	0.15	0.34	0.75	0.14	0.0011	615	615	1	2 46	101	0.14	0.0009	589	5		1 46	65	1.00	0.82	0.44	0.52
M43212121 M43212122	3.46	0.03	0.18	0.39	0.75	0.14	0.0010	699	699		s 55 3 54	99	0.13	0.0009	660	5		2 55 2 54	70 61	0.99	0.94	0.49	0.55
M53212121 M53212122	3.17	0.03	0.22	0.45	0.79	0.13	0.0010	843 820	843 820	4	4 68 4 66	107 95	0.13	0.0009	758	6		2 64 2 63	63 54	0.95	1.15	0.61	0.60
M13212211	5.04	0.02	0.11	0.27	0.67	0.17	0.0013	430	430	1	L 28	94	0.16	0.0011	408	3		1 28	61	0.99	0.68	0.37	0.47
M13212212 M23212211	4.61	0.02	0.10	0.25	0.70	0.16	0.0013	409	409	1	L 28 L 33	93	0.15	0.0011	462	4		1 27	53 60	1.00	0.69	0.41	0.49
M23212212 M33212211	4.72	0.02	0.12	0.27	0.74	0.16	0.0012	467 545	467 545	1	L 33 2 39	82 92	0.16	0.0011	449 519	4		1 <u>33</u> 139	53	1.00	0.75	0.42	0.49
MB3212212	4.31	0.02	0.13	0.30	0.77	0.15	0.0012	528	528	1	2 39	81	0.15	0.0010	507	4		1 39	51	1.00	0.85	0.47	0.52
M43212211 M43212212	3.87	0.03	0.15	0.35	0.77	0.15	0.0011	603	603		2 47 2 46	80	0.14	0.0010	569	4		1 46 1 46	55 48	0.99	0.98	0.55	0.55
M53212211 M53212212	3.54 3.59	0.03	0.19	0.42	0.80	0.14	0.0011	725	725	4	4 57 4 56	87	0.14	0.0010	649	5		2 <u>54</u> 2 53	48	0.95	1.19	0.66	0.60
M13212221	4.04	0.02	0.14	0.34	0.67	0.14	0.0011	579	579	1	L 38	123	0.14	0.0010	554	4		1 38	84	0.99	0.65	0.33	0.46
M23212222	3.69	0.02	0.15	0.35	0.70	0.13	0.0011	654	654	1	2 46	108	0.14	0.0010	629	4		1 46	83	1.00	0.83	0.37	0.48
M23212222 M33212221	3.78	0.03	0.15	0.34	0.74	0.14	0.0010	630 738	630 738	2	2 45 2 54	107 120	0.14	0.0009	609 709	5		1 45 2 54	72	1.00	0.71	0.38	0.48
M33212222	3.45	0.03	0.17	0.36	0.77	0.13	0.0010	715	715	2	2 54	106	0.13	0.0009	690	5		2 54	70	1.00	0.79	0.42	0.51
M43212222	3.16	0.04	0.19	0.40	0.80	0.13	0.0010	817	817	3	3 64	113	0.13	0.0009	778	6		2 63	66	0.99	0.91	0.40	0.55
M53212221 M53212222	2.84	0.04	0.25	0.48	0.81	0.12	0.0010	982 958	982 958		5 79 5 77	<u>112</u> 99	0.12	0.0009	894	7		2 75 2 74	67 58	0.96	1.11	0.56	0.60
M13221111	11.13	0.01	0.04	0.11	0.67	0.25	0.0021	182	182	1	L 18	43	0.25	0.0017	168	1		1 18 1 18	24	0.99	0.79	0.50	0.49
M23221112	10.14	0.01	0.04	0.12	0.72	0.23	0.0021	199	199	1	L 20	43	0.22	0.0015	185	1		1 10	23	1.00	0.92	0.52	0.40
M23221112 M33221111	10.53 9.28	0.01	0.04	0.11	0.75	0.24	0.0020	189 226	189 226	1	L 20 L 24	35	0.23	0.0016	208	2		1 <u>20</u> 124	19	1.00	0.90	0.59	0.50
M33221112	9.58	0.01	0.05	0.12	0.77	0.22	0.0019	213	213	1	L 24	35	0.22	0.0015	200	2		1 24	18	1.00	1.07	0.68	0.53
M43221111	8.73	0.01	0.06	0.15	0.79	0.21	0.0019	250	204	4	2 23	36	0.21	0.0014	230	2		1 28	17	0.98	1.31	0.80	0.58
M53221111 M53221112	7.76	0.01	0.08	0.20	0.78	0.21	0.0018	324 310	324 310	3	3 37 3 36	45	0.20	0.0014	267	2		1 <u>34</u> 133	19 15	0.91	1.56	0.98	0.65
M13221121	9.06	0.01	0.05	0.14	0.66	0.22	0.0018	235	235	1	L 23	58	0.21	0.0014	217	2		1 23	32	0.99	0.83	0.49	0.49
M23221122	8.26	0.01	0.05	0.15	0.71	0.22	0.0013	268	268	1	L 23	58	0.21	0.0013	249	2		1 22	32	1.00	0.93	0.55	0.43
M23221122 M33221121	8.60	0.01	0.06	0.14	0.75	0.21	0.0017	249 307	249 307	1	L 27 2 33	47	0.20	0.0014	236	2		1 27 1 33	26 31	1.00	0.93	0.57	0.50
MB3221122	7.83	0.01	0.07	0.16	0.77	0.20	0.0017	288	288	1	2 32	48	0.20	0.0013	271	2		1 32	25	1.00	1.08	0.63	0.53
M43221121	7.14	0.01	0.08	0.20	0.75	0.20	0.0016	335	337	-	2 40 2 39	59 48	0.19	0.0013	323	2		1 40 1 38	30	0.98	1.21	0.71	0.57
M53221121 M53221122	6.33 6.50	0.01	0.10	0.25	0.79	0.19	0.0016	432 413	432	4	4 51 4 50	58 48	0.19	0.0013	366	3		2 47 1 45	26	0.92	1.44	0.90	0.63
M13221211	9.88	0.01	0.05	0.12	0.70	0.22	0.0019	205	205	1	L 20	45	0.22	0.0015	192	2		1 20	25	0.99	0.83	0.52	0.48
M23221212 M23221211	8.97	0.01	0.04	0.11	0.75	0.23	0.0019	235	235	1	L 20 L 25	36	0.22	0.0016	221	2		1 20 1 24	21	1.00	0.81	0.53	0.48
M23221212 M33221211	9.27 8.18	0.01	0.05	0.12	0.78	0.22	0.0018	221 270	221	1	L 24 2 29	37 47	0.21	0.0014	212	2		1 24	20	1.00	0.92 1.08	0.59	0.50
MB3221212	8.41	0.01	0.06	0.14	0.80	0.21	0.0018	257	257	-	2 29	38	0.20	0.0014	244	2		1 29	20	1.00	1.07	0.66	0.53
M43221211 M43221212	7.64	0.01	0.07	0.18	0.78	0.20	0.0017	316	303		2 35 2 35	48	0.20	0.0013	288	2		1 35 1 34	23	0.98	1.26	0.74	0.57

		Co/T1)	C=/T2)	C_(TO)	DOC	Diam		Total	Total	Max CB	May De	Max wall	Diam		Total	Total	Max CB	May De	Max wall				Mahal
Name	T1 [s]	50(11)	58(12)	3a(13) [#]	г.1	[m]	ISDR [-]	Moment	Shear	Shear	IMAN1	moment	Im12	ISDR [-]3	Moment	Shear	Shear	IMAN12	moment	C1 [-]	C2[·]	C3 [·]	Met [.]
		15.	151	15.	191	[111]		[MN·m]	[MN]	[MN]	[inita]	[MN-m]	[m]2		[MN·m]4	[MN]5	[MN]6	[IVIIV]2	[MN-m]8				INISC [-]
M13221221	8.00	0.01	0.06	0.16	5 0.70	0.20	0.0017	280	280	1	. 28	62	0.20	0.0014	263	2	1	. 28	35	0.99	0.83	0.48	0.48
M13221222	8.31	0.01	0.06	0.14	4 0.75	5 0.20	0.0017	261	261	1	. 28	50	0.20	0.0014	250	2	1	. 28	28	0.99	0.82	0.50	0.47
M23221221	7.27	0.01	0.07	0.1	7 0.73	3 0.19	0.0016	322	322	1	. 34	62	0.19	0.0013	305	2	1	. 34	35	1.00	0.91	0.52	0.50
M23221222	7.51	0.01	0.07	0.16	5 0.78	8 0.20	0.0016	304	304	2	34	51	0.19	0.0013	291	2	1	. 34	28	1.00	0.93	0.54	0.50
M33221221	6.63	0.01	0.08	0.1	9 0.76	5 0.19	0.0015	371	371	2	41	62	0.18	0.0012	350	3	1	41	34	1.00	1.01	0.58	0.53
M33221222	6.82	0.01	0.08	0.1	3 0.80	0.19	0.0016	353	353	2	40	51	0.18	0.0012	337	3	1	. 40	28	1.00	1.02	0.59	0.53
M43221221	6.06	0.01	0.09	0.2	2 0.79	0.18	0.0015	433	433	3	50	62	0.18	0.0012	401	3	2	49	33	0.98	1.14	0.67	0.57
M43221222	6.20	0.01	0.09	0.2	1 0.82	2 0.18	0.0015	415	415	3	49	51	0.18	0.0012	387	3	2	48	26	0.98	1.16	0.68	0.56
M53221221	5.52	0.02	0.12	0.2	3 0.81	0.18	0.0015	527	527	5	63	62	0.18	0.0012	457	4	- 2	58	29	0.93	1.36	0.85	0.62
M53221222	5.63	0.01	0.11	0.2	7 0.84	0.18	0.0015	508	508	5	61	51	0.18	0.0012	444	3	2	57	23	0.93	1.37	0.86	0.62
M13222111	6.42	0.01	0.09	0.2	2 0.63	3 0.18	0.0015	396	396	1	. 25	99	0.18	0.0012	367	3	1	. 25	59	0.99	0.76	0.41	0.48
M13222112	6.62	0.01	0.08	0.20	0.67	0.19	0.0015	375	375	1	25	86	0.18	0.0013	351	3	1	. 25	51	0.99	0.78	0.43	0.48
M23222111	5.88	0.01	0.09	0.23	3 0.68	3 0.18	0.0014	451	451	1	31	98	0.17	0.0012	423	3	1	. 31	59	1.00	0.82	0.45	0.50
M23222112	6.04	0.01	0.09	0.2	2 0.71	0.18	0.0014	430	430	1	30	86	0.18	0.0012	406	3	1	. 30	52	1.00	0.83	0.47	0.50
MB3222111	5.40	0.02	0.11	0.2	5 0,71	0.17	0.0014	515	515	2	37	98	0.17	0.0012	483	4	1	. 37	59	1.00	0.90	0.51	0.53
M33222112	5.53	0.02	0.10	0.24	4 0.74	0.17	0.0014	493	493	2	36	86	0.17	0.0012	467	4	1	. 36	51	1.00	0.92	0.52	0.53
M43222111	4.96	0.02	0.12	0.2	9 0.74	0.17	0.0014	594	594	2	44	98	0.17	0.0011	549	4	1	. 44	56	0.99	1.04	0.60	0.56
M43222112	5.07	0.02	0.12	0.2	3 0.77	0.17	0.0014	574	574	2	44	86	0.17	0.0011	534	4	1	. 43	49	0.99	1.05	0.61	0.56
M53222111	4.55	0.02	0.15	0.3	5 0.78	3 0.16	0.0013	697	697	4	54	95	0.16	0.0011	609	5	2	51	50	0.94	1.28	0.76	0.62
M53222112	4.63	0.02	0.15	0.34	4 0.80	0.16	0.0013	678	678	4	. 54	84	0.16	0.0011	596	5	2	50	43	0.94	1.29	0.77	0.61
M13222121	5.16	0.02	0.11	0.2	3 0.64	0.17	0.0013	556	556	1	. 36	132	0.17	0.0011	522	4	1	. 36	85	0.99	0.68	0.37	0.47
M13222122	5.33	0.02	0.10	0.2	5 0.67	0.17	0.0013	524	524	1	35	115	0.17	0.0012	497	4	1	. 35	74	0.99	0.70	0.38	0.47
M23222121	4.73	0.02	0.12	0.2	9 0.68	8 0.16	0.0012	626	626	2	43	131	0.16	0.0011	593	5	1	. 43	84	1.00	0.75	0.41	0.49
M23222122	4.87	0.02	0.11	0.2	3 0.71	0.16	0.0013	600	600	2	43	115	0.16	0.0011	573	4	1	43	74	1.00	0.75	0.42	0.49
MB3222121	4.35	0.02	0.14	0.3	2 0.72	2 0.15	0.0012	701	701	2	51	129	0.15	0.0010	663	5	2	51	81	1.00	0.85	0.46	0.52
MB3222122	4.46	0.02	0.13	0.30	0.74	0.16	0.0012	675	675	2	50	114	0.15	0.0010	643	5	2	50	71	1.00	0.85	0.48	0.52
M43222121	4.00	0.02	0.16	0.3	5 0.75	5 0.15	0.0012	793	793	3	60	127	0.15	0.0010	739	6	2	60	77	0.99	0.99	0.54	0.56
M43222122	4.08	0.02	0.15	0.34	4 0.77	0.15	0.0012	767	767	3	59	112	0.15	0.0010	719	6	2	59	67	0.99	1.00	0.55	0.56
M53222121	3.66	0.03	0.19	0.4	1 0.78	3 0.14	0.0011	925	925	5	. 74	122	0.14	0.0010	823	6	4	70	68	0.95	1.21	0.67	0.61
M53222122	3.73	0.03	0.18	0.4	0.80	0.14	0.0012	900	900	5	72	107	0.14	0.0010	803	6	- 2	69	58	0.95	1.22	0.68	0.61
M13222211	5.82	0.01	0.09	0.2	3 0.66	> 0.18	0.0014	457	457	1	30	104	0.17	0.0012	431	3	3	. 29	65	0.99	0.73	0.40	0.47
M13222212	5.99	0.01	0.09	0.2	2 0.70	0.18	0.0014	435	435	1	. 29	91	0.17	0.0012	414	3	1	. 29	56	0.99	0.74	0.41	0.47
M23222211	5.32	0.02	0.10	0.2	5 0.70	0.1/	0.0013	525	525	1	36	104	0.17	0.0011	499	4	1	. 36	65	1.00	0.78	0.44	0.49
MI23222212	5.45	0.02	0.10	0.2	3 0.75	s u17	0.0013	503	503	1		91	0.17	0.0011	482	4		. 35	5/	1.00	0.79	0.45	0.49
M33222211	4.87	0.02	0.12	0.2	3 0.74	1 0.17	0.0013	600	600	2	43	104	0.16	0.0011	569	4	1	. 43	64	1.00	0.87	0.49	0.52
MB3222212	4.97	0.02	0.11	0.2	5 0.7	U17	0.0013	581	581	2	43	92	0.16	0.0011	555	4	1	. 43	56	1.00	0.87	0.50	0.52
IVI43222211	4.47	0.02	0.14	0.3	L U.71	0.16	0.0013	681	681	5	51	103	0.16	0.0011	635	5		50	60	0.99	1.02	0.58	0.56
M43222212	4.55	0.02	0.13	0.3	1 0.75	0.15	0.0013	662	562	3	50	91	0.16	0.0011	622	5	4	50	52	0.99	1.02	0.59	0.56
NI53222211	4.09	0.02	0.17	0.3	/ 0.80	0.15	0.0012	798	798	4	. 62	98	0.15	0.0010	707	5	4	59	53	0.95	1.24	0.71	0.61
M53222212	4.15	0.02	0.16	0.3	5 0.82	2 0.15	0.0012	/81	/81	4	. 61	87	0.15	0.0011	694	5		58	45	0.94	1.25	0.73	0.61
MIL3222221	4.66	0.02	0.12	0.3	0.61	0.16	0.0012	63/	037	1	. 42	139	0.16	0.0011	606	5	1	. 42	92	0.99	0.67	0.36	0.47
ML 3222222	4.80	0.02	0.11	0.2	3 0.70	0.15	0.0012	610	510	1	42	122	0.16	0.0011	586	5		41	80	0.99	0.57	0.37	0.46
IVI23222221	4.20	0.02	0.13	0.3.	2 0.71	0.15	0.0011	/1/	/1/	4	50	138	0.15	0.0010	686	5	4	50	90	1.00	0.73	0.40	0.49
NIZ3222222	4.37	0.02	0.13	0.3	J 0.74	1 0.15	0.0011	690	690	2	50	121	0.15	0.0010	665	5	4	49	/9	1.00	0.74	0.41	0.49
11/133222221	3.91	0.03	0.15	0.34	+ 0.74	+ 0.14	0.0011	807	807	3	59	136	0.14	0.0010	//1	6	4	59	8/	1.00	0.82	0.44	0.52
11/153222222	3.99	0.02	0.14	0.3	2 0.7	0.12	0.0011	/82	/82	3	58	120	0.14	0.0010	/51	6	4	58	/b	T.00	0.65	0.46	0.52
N442222221	3.58	0.03	0.17	0.3	0.71	0.14	0.0011	917	917	3	/0	133	0.14	0.0009	864	/	4	69	82	0.99	0.95	0.50	0.55
1/143222222	3.05	0.03	0.1/	0.3	> U.80	0.14	0.0011	891	891	5	69	117	0.14	0.0009	844	/	4	68	/1	0.99	0.96	0.52	0.55
N4E2222222	3.28	0.03	0.21	0.4	+ 0.80	0.13	0.0011	10/3	10/3	5	85	14.0	0.12	0.0009	966	/	-	81	/3	0.95	1.17	0.63	0.60
IVIDSZZZZZZ	3.33	0.03	U.21	0.4	5 U.82	: U.13	0.0011	1048	1048	- 5	84	112	0.13	0.0009	94/	- /		. 80	63	0.95	1.1/	- 0.64	0.00

Appendix B: Ground motion de-aggregation

The following figure shows the De-aggregation for the Vancouver site for the 50% in 30 year hazard.

B.1 - Vancouver LH



Figure: De-aggregation for Vancouver 50% in 30 year hazard: (a) T = 0.3s; (b) T = 0.5s; (c) T = 1s; (d) T = 2s; (e) T = 3s; (f) and T = 5s



Figure: De-aggregation for Vancouver 50% in 30 year hazard
B.2 - Vancouver MH

The following figure shows the De-aggregation for the Vancouver site for the 20% in 50 year hazard.



Figure: De-aggregation for Vancouver 20% in 50 year hazard: (a) T = 0.3s; (b) T = 0.5s; (c) T = 1s; (d) T = 2s; (e) T = 3s; (f) and T = 5s



Figure: De-aggregation for Vancouver 20% in 50 year hazard

B.3 - Vancouver HH

The following figure shows the De-aggregation for the Vancouver site for the 2% in 50 year hazard.



Figure: De-aggregation for Vancouver 2% in 50 year hazard



Figure: De-aggregation for Vancouver 2% in 50 year hazard

B.4 - Victoria LH

The following figure shows the De-aggregation for the Victoria site for the 50% in 30 year hazard.



Figure: De-aggregation for Victoria 50% in 30 year hazard: (a) T = 0.3s; (b) T = 0.5s; (c) T = 1s; (d) T = 2s; (e) T = 3s; (f) and T = 5s



Figure: De-aggregation for Victoria 50% in 30 year hazard

B.5 - Victoria MH

The following figure shows the De-aggregation for the Victoria site for the 20% in 50 year hazard.



Figure: De-aggregation for Victoria 20% in 50 year hazard: (a) T = 0.3s; (b) T = 0.5s; (c) T = 1s; (d) T = 2s; (e) T = 3s; (f) and T = 5s



Figure: De-aggregation for Victoria 20% in 50 year hazard

B.5 - Victoria HH

The following figure shows the De-aggregation for the Victoria site for the 2% in 50 year hazard.



Figure: De-aggregation for Victoria 2% in 50 year hazard: (a) T = 0.3s; (b) T = 0.5s; (c) T = 1s; (d) T = 2s; (e) T = 3s; (f) and T = 5s



Figure: 2D De-aggregation for Victoria 2% in 50 year hazard

Appendix C: Ground motion scaling for unidirectional





Figure: Crustal LH Vancouver



Figure: Subcrustal LH Vancouver





Figure: Crustal MH Vancouver



Figure: Subduction intraslab MH Vancouver



Figure: Subduction interface MH Vancouver



Figure: Crustal HH Vancouver



Figure: Subduction intraslab HH Vancouver



Figure: Subduction interface HH Vancouver



Figure: Crustal LH Victoria



Figure: Subcrustal LH Victoria



Figure: Crustal MH Victoria



Figure: Subduction intraslab MH Victoria



Figure: Subduction interface MH Victoria



Figure: Crustal HH Victoria



Figure: Subduction intraslab HH Victoria



Figure: Subduction interface HH Victoria

Appendix D: Detailed construction drawings

The following shows the detailed construction drawings for the experimental set-up.




















Appendix E: Remaining construction steps

The specimen is currently partially assembled in the laboratory in the Tongji University. The following describes the steps required to prepare the specimen for shake-table testing.

Step 1: Secure and level the 10m wall to the foundation

We will level and grout the 10m wall to the foundation. The 10m wall should be vertically straight prior to grouting. A rigid non-yielding "dummy damper" will be placed in lieu of the damper at this stage.



Step 2: Secure the outrigger beam on the 5m wall as shown in the figure below.

Secure the outrigger beam to the 5m wall through bolt connection using 32 embedded rods. Approximately 50mm of grout between the wall and the outrigger beam. We will ensure beam is level and adjust any necessary misalignments.





Step 3: Connect the 5m wall onto the 10m wall

We will line up the 5m wall to the 10m wall. The bolt connection can be used to align the wall segments. Make sure the wall segments are fully aligned. Welded the 5 m and 10 m wall segment together using a 19mm first-grade fillet weld. Bolt connection to line up the



<image>

wall

Weld around the steel plate

Step 4: Cut out section of steel braces

The steel braces within the wall must be cut out as the drawing shows. A total of 32 cuts are needed.



Step 5: Install the steel coupling beam dampers

We will assemble and install the coupling beams to the specimen. It should be noted that the damper-to-beam connections are slip critical connections. All bolts need to be pretensioned. Operation should be supervised by research team.



Step 6: Install the steel mass frames

We will assemble and install all 8 mass block frames. We will make sure they are leveled. Any gaps between frame and wall should be filled with grout.



Step 7: Post-tension tendons

Post-tension the wall segments using the 8 tendons provided. Each tendon need to be tensioned to a force of 150kN. Please see drawings in Appendix D for location of PT tendon.



Step 8: Install the outrigger columns, dampers, column lateral support

We will install the outrigger columns, and column lateral supports. The columns will be bolted together and connected to outrigger beams, outrigger pins. The column lateral supports should connect the outrigger columns to the concrete wall. There are four assembled lateral supports.



Step 9: Clean and paint wall white

We will sand off any significant bumps or marks. Also, we will paint with whitewash paint.

Step 10: Install specimen on shaking table

We will lift and bolt the specimen to the shaking table. Make sure the specimen are vertically aligned.



Step 11: Assemble the mass blocks

We will assemble the 32 mass blocks to the steel mass frame through screwed holes at all sides.



Step 12: Remove specimen from table, disassemble, and disposal.

After testing, remove specimen from table, disassemble and dispose specimen.